

TO: City of Portsmouth

03801

1 Junkins Avenue Portsmouth, NH

LETTER OF TRANSMITTAL

FROM: AMBIT ENGINEERING, INC.

Civil Engineers and Land Surveyors 200 Griffin Road, Unit 3 Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

DATE:	DATE: 11/18/2019 JOB NO. 3039					
ATTENTION: Planning Department						
RE: Site Plan Approval						
	Brick	Market - 60 P	enhallow Street LU - 19-TBD			
		COPY	OF LETTER PRINTS GE ORDER SPECIFICATIONS			
COPIES	DATE	REVISION	DESCRIPTION			
3	11-18-19		Site Plans			
7	11-18-19		11X17 of same			
1	11-18-19		Site Review Application / Site Cost Estimate / Fee Check			
1	11-18-19		Checklist			
10	11-18-19		Supplemental Information			
FOR Y FOR B	THESE ARE TRANSMITTED AS CHECKED BELOW FOR YOUR APPROVAL					
REMARK	S					
On Line Po	On Line Permit Application Submitted					
СОРУ ТО	(via email)	: Dagney Tagg	gart, JSA Architects, Woodburn & Company			

If enclosures are not as noted, kindly notify us at once.



City of Portsmouth, New Hampshire Site Plan Application Checklist

This site plan application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan review regulations for full details.

Applicant Responsibilities (Section 2.5.2): Applicable fees are due upon application submittal along with required attachments. The application shall be complete as submitted and provide adequate information for evaluation of the proposed site development. Waiver requests must be submitted in writing with appropriate justification.

Name	of Owner/Applicant: Dagny Taggart, LLC/McNabb Properties, LTD Date Sub	omitted:11/18/19	
Phone	Number: Applicant: 603-427-0725 E-mail: chris	stine@mcnabbgroup.com	
Site Ad	dress: 60 Penhallow Street	Map: <u>107</u> Lot:	27
Zoning	District: Character District 4 (CD4) Lot area: 23,279	9 sq. ft.	
	Application Requirements		
\square	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested
		riali Sileet/ Note #/	
	Fully executed and signed Application form. (2.5.2.3)	Fian Sheet/Note #)	N/A

	Site Plan Review Application Required Information				
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested		
	Statement that lists and describes "green" building components and systems. (2.5.3.1A)	See attached from JSA			
	Gross floor area and dimensions of all buildings and statement of uses and floor area for each floor. (2.5.3.1B)	See Sheet C3	N/A		
	Tax map and lot number, and current zoning of all parcels under Site Plan Review. (2.5.3.1C)	See Sheet C1	N/A		
	Owner's name, address, telephone number, and signature. Name, address, and telephone number of applicant if different from owner. (2.5.3.1D)	See Cover Sheet	N/A		

	Site Plan Review Application Required Information			
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	Names and addresses (including Tax Map and Lot number and zoning districts) of all direct abutting property owners (including properties located across abutting streets) and holders of existing conservation, preservation or agricultural preservation restrictions affecting the subject property. (2.5.3.1E)	See Standard Boundary Survey	N/A	
	Names, addresses and telephone numbers of all professionals involved in the site plan design. (2.5.3.1F)	See Cover Sheet	N/A	
	List of reference plans. (2.5.3.1G)	See Standard Boundary Survey	N/A	
	List of names and contact information of all public or private utilities servicing the site. (2.5.3.1H)	See Cover Sheet	N/A	

	Site Plan Specifications			
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	Full size plans shall not be larger than 22 inches by 34 inches with match lines as required, unless approved by the Planning Director. Submittals shall be a minimum of 11 inches by 17 inches as specified by Planning Dept. staff. (2.5.4.1A)	Required on all plan sheets	N/A	
	Scale: Not less than 1 inch = 60 feet and a graphic bar scale shall be included on all plans. (2.5.4.1B)	Required on all plan sheets	N/A	
	GIS data should be referenced to the coordinate system New Hampshire State Plane, NAD83 (1996), with units in feet. (2.5.4.1C)	N/A	N/A	
	Plans shall be drawn to scale. (2.5.4.1D)	Required on all plan sheets	N/A	
	Plans shall be prepared and stamped by a NH licensed civil engineer. (2.5.4.1D)	PE 07651	N/A	
	Wetlands shall be delineated by a NH certified wetlands scientist and so stamped. (2.5.4.1E)	N/A	N/A	
	Title (name of development project), north point, scale, legend. (2.5.4.2A)	See Cover Sheet	N/A	
	Date plans first submitted, date and explanation of revisions. (2.5.4.2B)	See Revision Block	N/A	
	Individual plan sheet title that clearly describes the information that is displayed. (2.5.4.2C)	Required on all plan sheets	N/A	
	Source and date of data displayed on the plan. (2.5.4.2D)	See Plan Notes	N/A	

	Site Plan Specifications		
\square	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	A note shall be provided on the Site Plan stating: "All conditions on this Plan shall remain in effect in perpetuity pursuant to the requirements of the Site Plan Review Regulations." (2.5.4.2E)	See Cover Sheet, C3 Site Plan	N/A
	Plan sheets submitted for recording shall include the following notes: a. "This Site Plan shall be recorded in the Rockingham County Registry of Deeds." b. "All improvements shown on this Site Plan shall be constructed and maintained in accordance with the Plan by the property owner and all future property owners. No changes shall be made to this Site Plan without the express approval of the Portsmouth Planning Director." (2.13.3)	See Sheet C3 Site Plan	N/A
	Plan sheets showing landscaping and screening shall also include the following additional notes: a. "The property owner and all future property owners shall be responsible for the maintenance, repair and replacement of all required screening and landscape materials." b. "All required plant materials shall be tended and maintained in a healthy growing condition, replaced when necessary, and kept free of refuse and debris. All required fences and walls shall be maintained in good repair." c. "The property owner shall be responsible to remove and replace dead or diseased plant materials immediately with the same type, size and quantity of plant materials as originally installed, unless alternative plantings are requested, justified and approved by the Planning Board or Planning Director." (2.13.4)	See Sheet L1 & L2	N/A

	Site Plan Specifications – Required Exhibits and Data				
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested		
	1. Existing Conditions: (2.5.4.3A)				
	a. Surveyed plan of site showing existing natural and bu	uilt features; C1			
	b. Zoning boundaries;	Cover Sheet			
	c. Dimensional Regulations;	C3 Zoning Development			
	d. Wetland delineation, wetland function and value ass	essment; N/A			
	e. SFHA, 100-year flood elevation line and BFE data.	Note 3, C1			
	2. Buildings and Structures: (2.5.4.3B)				
	 a. Plan view: Use, size, dimensions, footings, overhangs elevation; 	s, 1st fl. A0-1.0-1.2			
	 Elevations: Height, massing, placement, materials, lig façade treatments; 	ghting, A0-1.0-1.2			
	c. Total Floor Area;	A0-1.0-1.2			
	d. Number of Usable Floors;	A0-1.0-1.2			
	e. Gross floor area by floor and use.	A0-1.0-1.2			
	3. Access and Circulation: (2.5.4.3C)				
	a. Location/width of access ways within site;	C3			
	 b. Location of curbing, right of ways, edge of pavement sidewalks; 	and C3			
	 c. Location, type, size and design of traffic signing (pave markings); 	ement C3			
	d. Names/layout of existing abutting streets;	Cover Sheet			
	e. Driveway curb cuts for abutting prop. and public roa	ds; C3			
	f. If subdivision; Names of all roads, right of way lines a easements noted;	nd N/A			
	g. AASHTO truck turning templates, description of mini allowed being a WB-50 (unless otherwise approved by	I IN/A			
	4. Parking and Loading: (2.5.4.3D)				
	 a. Location of off street parking/loading areas, landscapareas/buffers; 	3, 1, 133, 1, 14, 15			
	b. Parking Calculations (# required and the # provided).	N/A			
	5. Water Infrastructure: (2.5.4.3E)				
	 Size, type and location of water mains, shut-offs, hyden Engineering data; 	04			
	b. Location of wells and monitoring wells (include prote	ective radii). N/A			
	6. Sewer Infrastructure: (2.5.4.3F)				
	 Size, type and location of sanitary sewage facilities & data. 	Engineering C4			
	7. Utilities: (2.5.4.3G)				
	a. The size, type and location of all above & below grou	nd utilities; C4			
	 Size type and location of generator pads, transforme fixtures. 	rs and other C4			

	Site Plan Specifications – Required Exhibits and Data			
\square	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	8. Solid Waste Facilities: (2.5.4.3H)			
	a. The size, type and location of solid waste facilities.	Architectural Plans		
	9. Storm water Management: (2.5.4.3I)			
	a. The location, elevation and layout of all storm-water drainage.	C5		
	10. Outdoor Lighting: (2.5.4.3J)			
	 a. Type and placement of all lighting (exterior of building, parking lot and any other areas of the site) and; b. photometric plan. 	Lighting Plans		
	11. Indicate where dark sky friendly lighting measures have been implemented. (10.1)	Lighting Plans		
	12. Landscaping: (2.5.4.3K)			
	 a. Identify all undisturbed area, existing vegetation and that which is to be retained; 	L1		
	b. Location of any irrigation system and water source.	L1		
	13. Contours and Elevation: (2.5.4.3L)			
	 a. Existing/Proposed contours (2 foot minimum) and finished grade elevations. 	C5		
	14. Open Space: (2.5.4.3M)			
	a. Type, extent and location of all existing/proposed open space.	C3 Community Space Plan		
	15. All easements, deed restrictions and non-public rights of ways. (2.5.4.3N)	Easement Plan		
	Location of snow storage areas and/or off-site snow removal. (2.5.4.30)	Remove Offsite/C3 Note		
	17. Character/Civic District (All following information shall be included): (2.5.4.3Q)			
	a. Applicable Building Height (10.5A21.20 & 10.5A43.30);	C3		
	b. Applicable Special Requirements (10.5A21.30);	C3		
	c. Proposed building form/type (10.5A43);	C3		
	d. Proposed community space (10.5A46).	Community Space Plan C3		

	Other Required Information		
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	Traffic Impact Study or Trip Generation Report, as required. (Four (4) hardcopies of the full study/report and Six (6) summaries to be submitted with the Site Plan Application) (3.2.1-2)	GPI Report	
	Indicate where Low Impact Development Design practices have been incorporated. (7.1)	C5	
	Indicate whether the proposed development is located in a wellhead protection or aquifer protection area. Such determination shall be approved by the Director of the Dept. of Public Works. (7.3.1)	N/A	
	Indicate where measures to minimize impervious surfaces have been implemented. (7.4.3)	N/A	
	Calculation of the maximum effective impervious surface as a percentage of the site. (7.4.3.2)	C3	
	Stormwater Management and Erosion Control Plan. (Four (4) hardcopies of the full plan/report and Six (6) summaries to be submitted with the Site Plan Application) (7.4.4.1)	Drainage Analysis	

	Final Site Plan Approval Required Information			
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	All local approvals, permits, easements and licenses required, including but not limited to: a. Waivers; b. Driveway permits; c. Special exceptions; d. Variances granted; e. Easements; f. Licenses. (2.5.3.2A)	Cover Sheet		
	Exhibits, data, reports or studies that may have been required as part of the approval process, including but not limited to: a. Calculations relating to stormwater runoff; b. Information on composition and quantity of water demand and wastewater generated; c. Information on air, water or land pollutants to be discharged, including standards, quantity, treatment and/or controls; d. Estimates of traffic generation and counts pre- and post-construction; e. Estimates of noise generation; f. A Stormwater Management and Erosion Control Plan; g. Endangered species and archaeological / historical studies; h. Wetland and water body (coastal and inland) delineations; i. Environmental impact studies. (2.5.3.2B)	Drainage Analysis C4 C5 GPI Report TBD Drainage Analysis N/A N/A N/A		

	Final Site Plan Approval Required Information				
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested		
	A document from each of the required private utility service providers indicating approval of the proposed site plan and indicating an ability to provide all required private utilities to the site. (2.5.3.2D)	Pending			
	A list of any required state and federal permit applications required for the project and the status of same. (2.5.3.2E)	See Cover Sheet			

Applicant's Signature:

Date: 11-18-19

Construction Cost Estimate

Ambit Engineering

Date: November 18, 2019

Project: McNabb Properties - 60 Penhallow Street Job No: 3039

Location: 60 Penhallow Street, Portsmouth, NH

Scope: Site Cost Estimate

ITEM NO	DESCRIPTION	UNIT	AMOUNT	UNIT COST	TOTAL
1	6" PVC Sewer	LF	30	\$120.00	\$3,600.00
2	8" - 24" HDPE Drainage Pipe	LF	350	\$100.00	\$35,000.00
3	Slot Drains & Covers	LF	150	\$65.00	\$9,750.00
4	4' DMH	EA	2	\$4,000.00	\$8,000.00
5	Grease Trap	EA	1	\$4,000.00	\$4,000.00
6	Common Excavation	CY	17860	\$25.00	\$446,500.00
7	Miscallaneous Paving	TON	60	\$100.00	\$6,000.00
8	Bluestone	SF	840	\$80.00	\$67,200.00
9	Red Granite Edging	SF	320	\$65.00	\$20,800.00
10	Deer Isle Granite	SF	2700	\$55.00	\$148,500.00
11	Crushed Gravel / Base Preparaation	CY	215	\$25.00	\$5,375.00
12	Fountain	LS	1	\$50,000.00	\$50,000.00
13	Brick Sidewalk	SY	150	\$96.00	\$14,400.00
14	Landscape Plantings	LS	1	\$75,000.00	\$75,000.00
15	Re-Set Curb	LF	300	\$60.00	\$18,000.00
16	Underground Electric / Conduit	LF	350	\$55.00	\$19,250.00
17	Transformers & Pole Relocation	EA	3	\$25,000.00	\$75,000.00
18	Seating Walls	LF	150	\$120.00	\$18,000.00
19	Mural Walls	LF	80	\$150.00	\$12,000.00
20	Water & Sprinkler Services	LF	120	\$180.00	\$21,600.00
21	Podium Bench	EA	1	\$12,000.00	\$12,000.00
22	Shoring	LF	560	\$35.00	\$19,600.00
23	Erosion Control	LS	1	\$2,500.00	\$2,500.00
	TOTAL				\$1,092,075

Note: This is an estimate of construction costs based upon various sources

APPLICATION FEE:

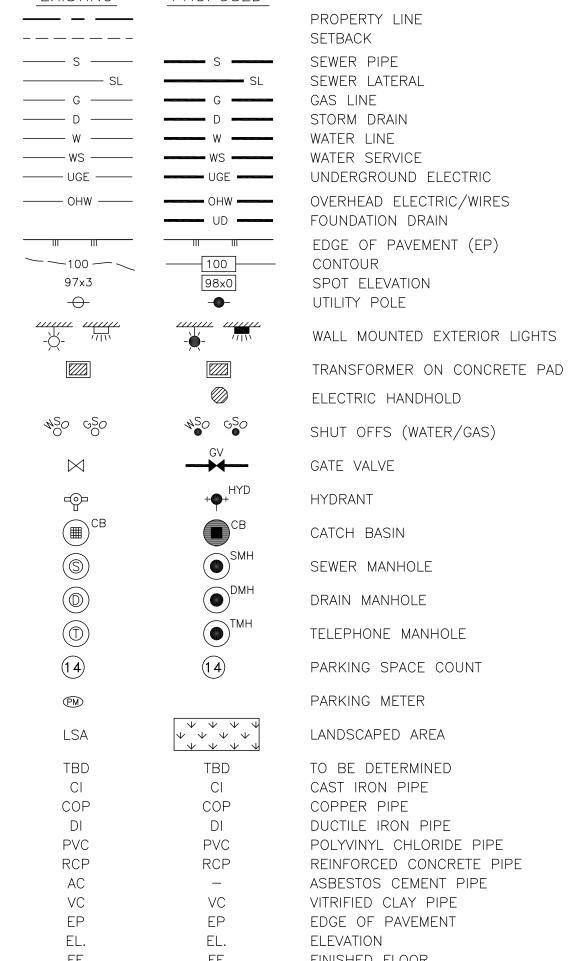
 $500 + (1,092,075/1000 \times 5) + (28,600/1,000 \times 10) = 6,246.38$

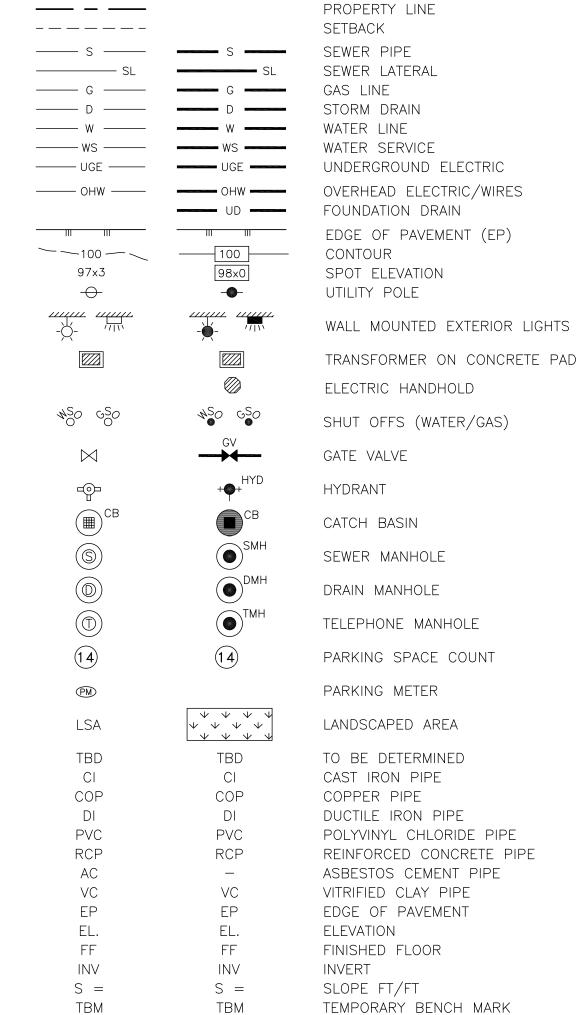
SITE REDEVELOPMENT

BRICK MARKET

60 PENHALLOW STREET

PORTSMOUTH HDC: APPROVED 11-13-19 PORTSMOUTH ZONING BOARD: PENDING PORTSMOUTH SITE REVIEW: PENDING PORTSMOUTH CONDITIONAL USE PERMIT: PENDING LEGEND: PROPOSED PROPERTY LINE

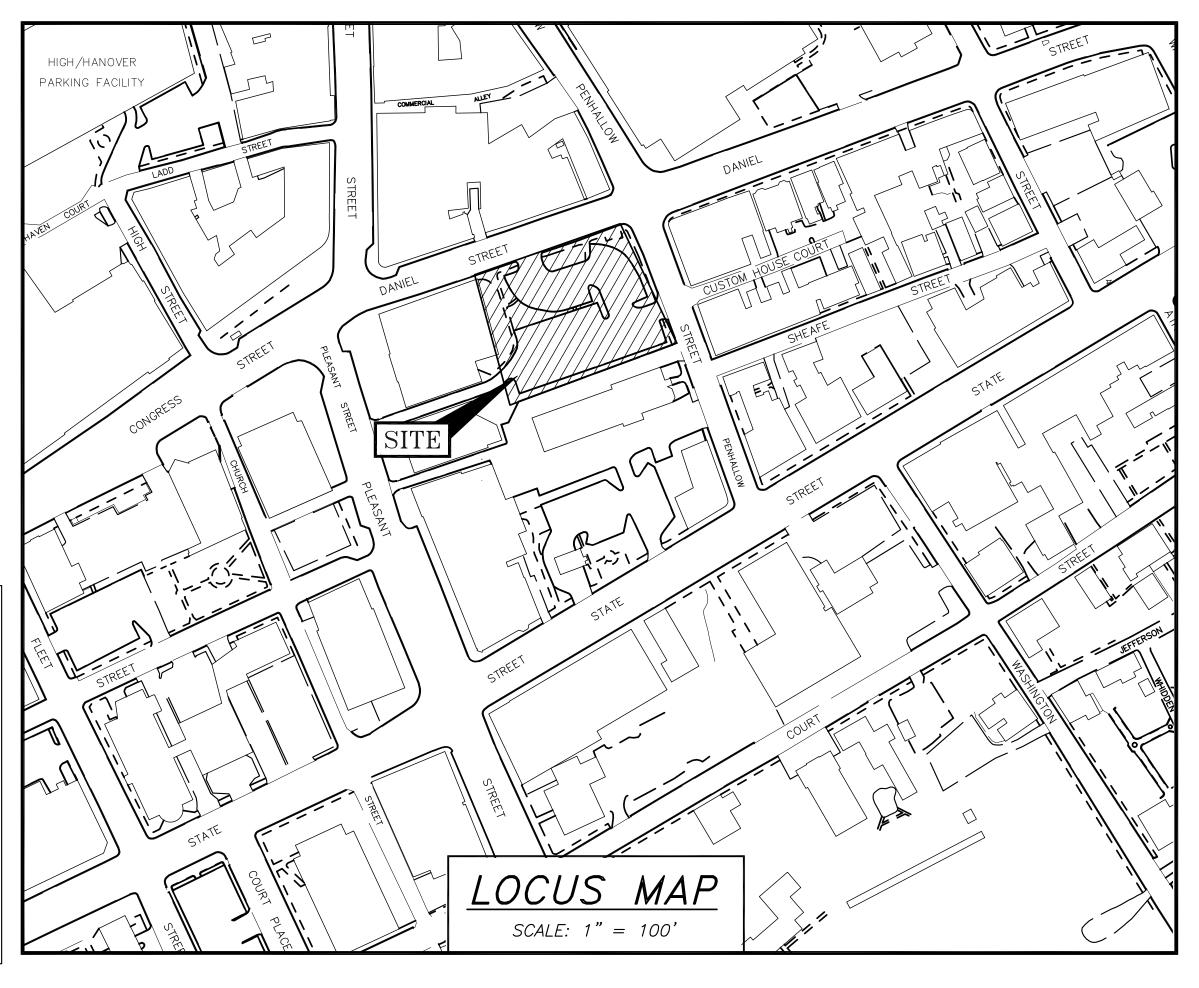


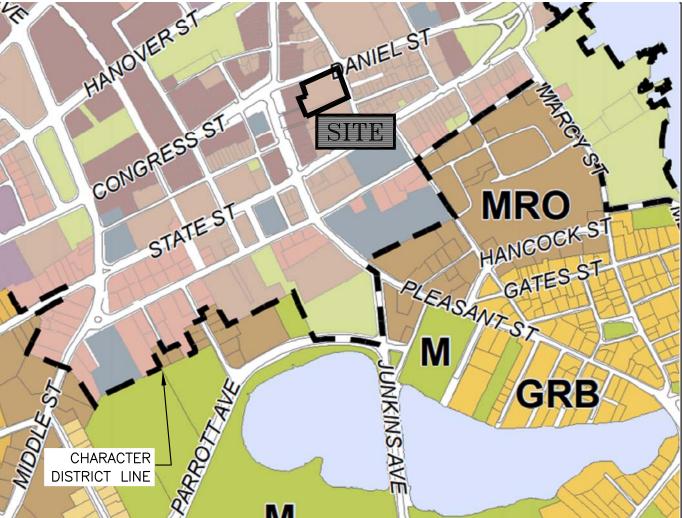


PERMIT LIST:

PORTSMOUTH, NEW HAMPSHIRE

SITE PERMIT PLANS





OWNER: DAGNY TAGGART, LLC

APPLICANT:

MCNABB PROPERTIES. LTD

30 PENHALLOW ST, STE 300 EAST

PORTSMOUTH, NH 03801

(603) 427-0725

CIVIL ENGINEER & LAND

SURVEYOR:

AMBIT ENGINEERING, INC.

200 GRIFFIN ROAD, UNIT 3 PORTSMOUTH, N.H. 03801

Tel. (603) 430-9282

Fax (603) 436-2315

STRUCTURAL ENGINEER:

JSN ASOCIATES, LLC

PORTSMOUTH NH, 03801

TEL.(603) 433-8639

MEP & FIRE PROTECTION:

PETERSEN ENGINEERING

127 PARROTT AVENUE

PORTSMOUTH NH, 03801

TEL.(603) 436-4233

1 AUTUMN STREET

Map 10.5A21A Character Districts and Civic Districts Downtown Overlay District Historic District **Character Districts** CD5 Character District 5 CD4 Character District 4 CD4-W Character District 4-W CD4-L1 Character District 4-L1 CD4-L2 Character District 4-L2 Civic District Civic District **Municipal District**

ATTORNEY:

FX BRUTON

BRUTON & BERUBE, PLLC

601 CENTRAL AVENUE

DOVER, NH 03820

(603) 749-4529

LANDSCAPE ARCHITECT:

WOODBURN & COMPANY

103 KENT PLACE

NEWMARKET, NH 03857

TEL. (603) 659-5949

FAX (603) 659-5939

ARCHITECT:

JSA ARCHITECTS

273 CORPORATE DRIVE SUITE 100

PORTSMOUTH NH 03801

TEL. (603) 436-2551

FAX (603) 436-6973

GEOTECHNICAL ENGINEER:

GSI

18 COTE AVENUE #11

GOFFSTOWN NH 03045

TEL. (603) 624-2722

INDEX OF SHEETS

DWG No.

Municipal District

STANDARD BOUNDARY SURVEY

EASEMENT PLAN

MASTER PLAN - EXISTING CONDITIONS

MASTER PLAN - COMMUNITY SPACE

EXISTING CONDITIONS PLAN

DEMOLITION PLAN

SITE LAYOUT PLAN LANDSCAPE PLANS

UTILITY PLAN

GRADING & DRAINAGE PLAN

OFFSITE IMPROVEMENTS

DETAILS D1-D4

ARCHITECTURAL PLANS AND ELEVATION

ARCHITECTURAL PARKING PLANS

UTILITY CONTACTS

ELECTRIC: **EVERSOURCE** 1700 LAFAYETTE ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 436-7708, Ext. 555.5678

ATTN: MICHAEL BUSBY, P.E. (MANAGER)

ATTN: JIM TOW

SEWER & WATER: PORTSMOUTH DEPARTMENT OF PUBLIC WORKS 680 PEVERLY HILL ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 427-1530

NATURAL GAS: UNITIL 325 WEST ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 294-5144

ATTN: DAVE BEAULIEU

155 COMMERCE WAY PORTSMOUTH, N.H. 03801 Tel. (603) 679-5695 (X1037) ATTN: MIKE COLLINS

CABLE:

COMCAST

DIG SAFE

COMMUNICATIONS: FAIRPOINT COMMUNICATIONS JOE CONSIDINE 1575 GREENLAND ROAD GREENLAND, N.H. 03840 Tel. (603) 427-5525

SITE PERMIT PLANS **BRICK MARKET 60 PENHALLOW STREET** PORTSMOUTH, N.H.

TYP



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282

TYPICAL

PLAN SET SUBMITTAL DATE: 18 NOVEMBER 2019

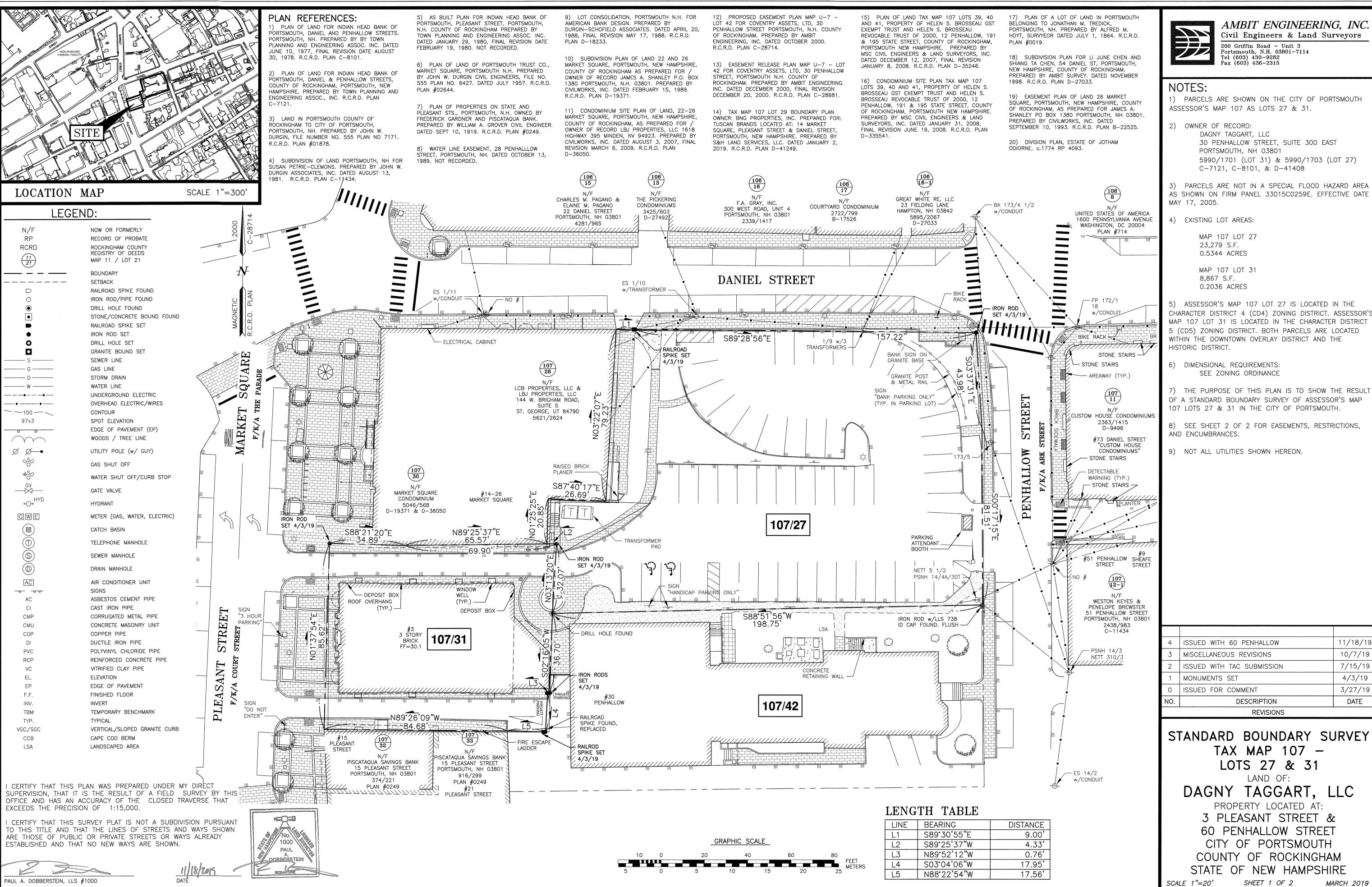
PORTSMOUTH APPROVAL CONDITIONS NOTE: ALL CONDITIONS ON THIS PLAN SET SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE CITY OF PORTSMOUTH SITE PLAN REVIEW REGULATIONS.

APPROVED BY THE PORTSMOUTH PLANNING BOARD

CHAIRMAN

DATE

3039



Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114

1) PARCELS ARE SHOWN ON THE CITY OF PORTSMOUTH

30 PENHALLOW STREET, SUITE 300 EAST 5990/1701 (LOT 31) & 5990/1703 (LOT 27)

3) PARCELS ARE NOT IN A SPECIAL FLOOD HAZARD AREA AS SHOWN ON FIRM PANEL 33015C0259E. EFFECTIVE DATE

5) ASSESSOR'S MAP 107 LOT 27 IS LOCATED IN THE CHARACTER DISTRICT 4 (CD4) ZONING DISTRICT. ASSESSOR'S MAP 107 LOT 31 IS LOCATED IN THE CHARACTER DISTRICT 5 (CD5) ZONING DISTRICT. BOTH PARCELS ARE LOCATED WITHIN THE DOWNTOWN OVERLAY DISTRICT AND THE

7) THE PURPOSE OF THIS PLAN IS TO SHOW THE RESULT OF A STANDARD BOUNDARY SURVEY OF ASSESSOR'S MAP 107 LOTS 27 & 31 IN THE CITY OF PORTSMOUTH.

8) SEE SHEET 2 OF 2 FOR EASEMENTS, RESTRICTIONS,

9) NOT ALL UTILITIES SHOWN HEREON.

		DEMICIONIC	
N	10.	DESCRIPTION	DATE
(0	ISSUED FOR COMMENT	3/27/19
	1	MONUMENTS SET	4/3/19
2	2	ISSUED WITH TAC SUBMISSION	7/15/19
	3	MISCELLANEOUS REVISIONS	10/7/19
4	4	ISSUED WITH 60 PENHALLOW	11/18/19

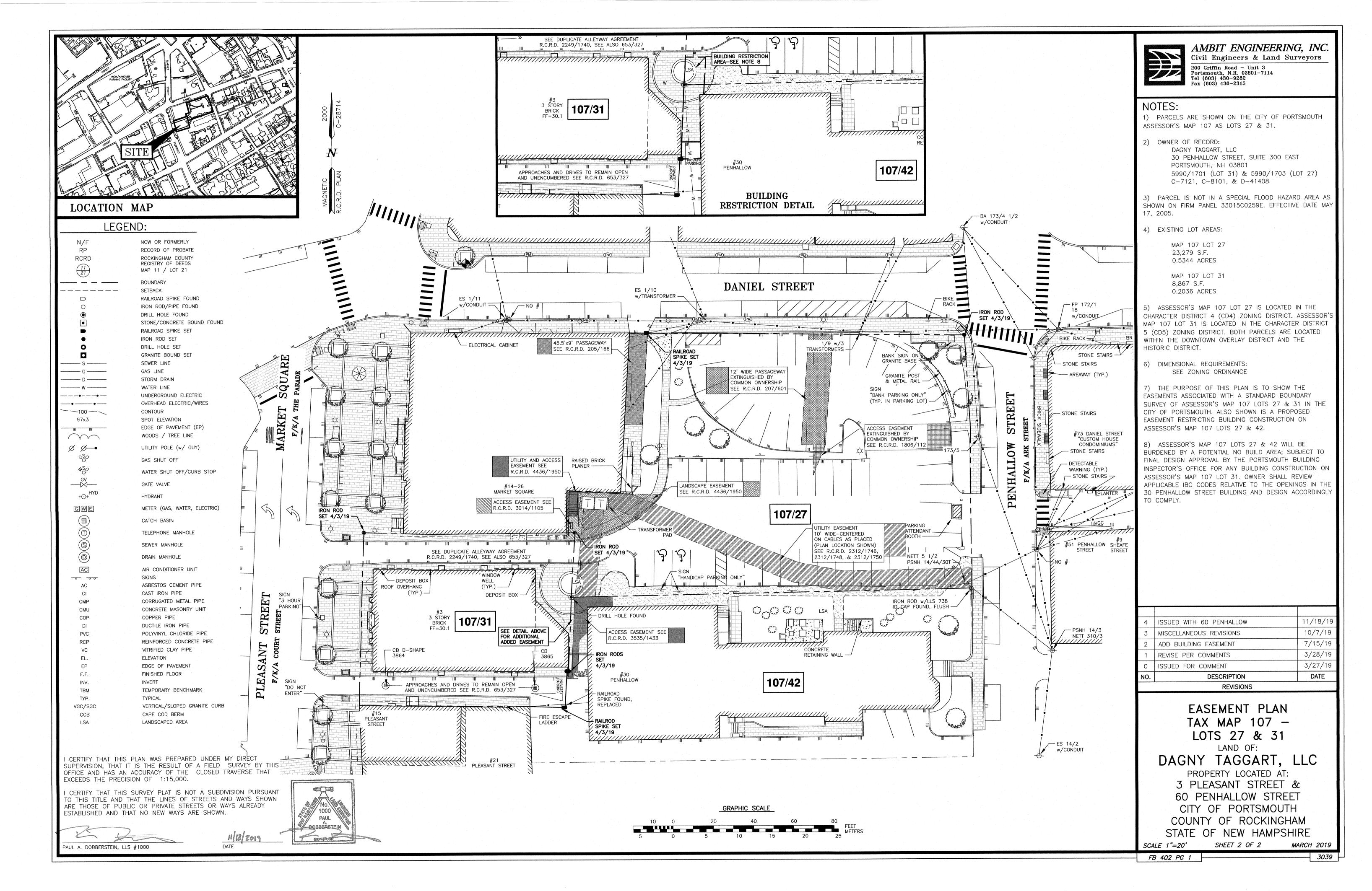
STANDARD BOUNDARY SURVEY TAX MAP 107 -LOTS 27 & 31

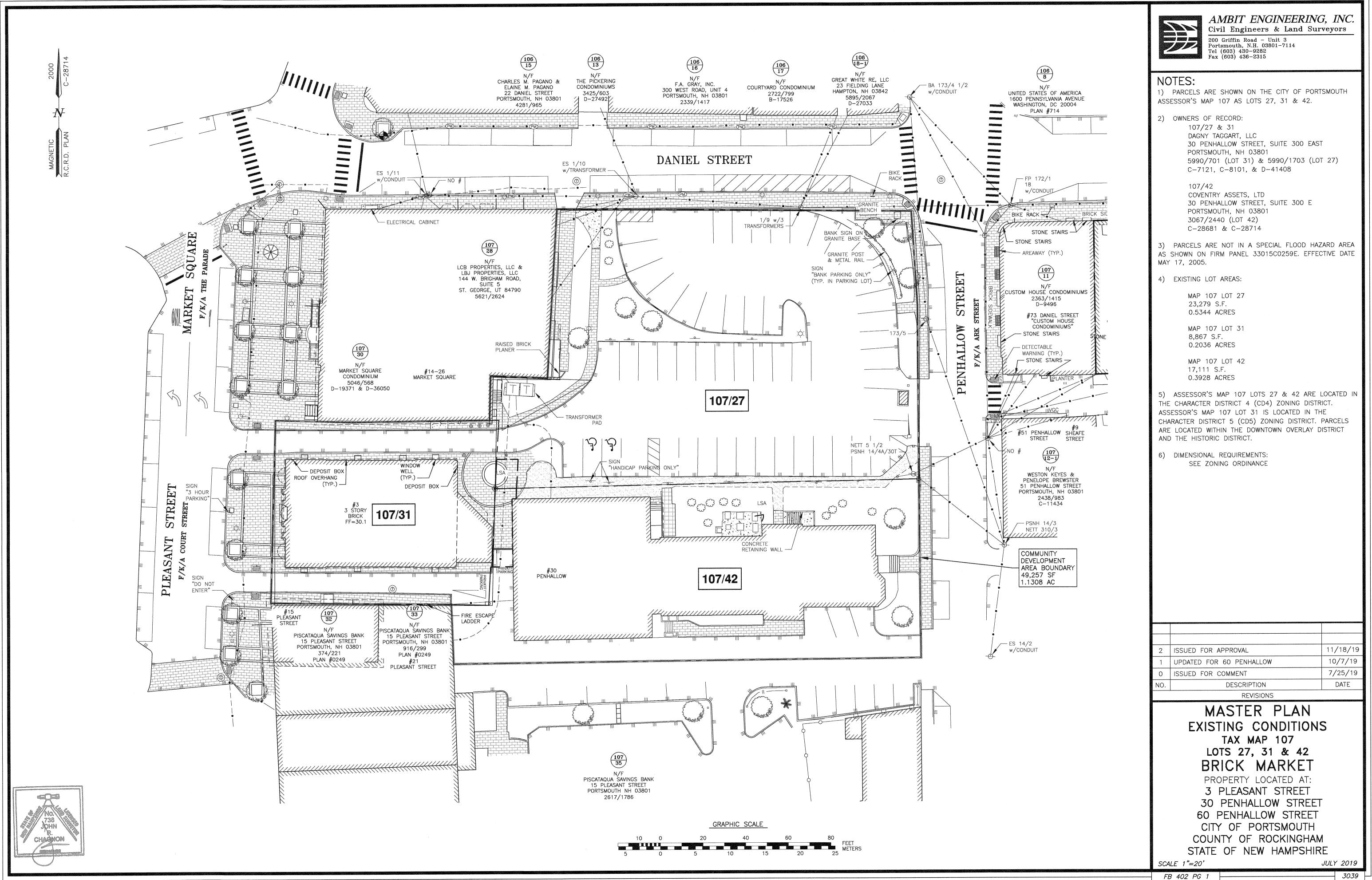
DAGNY TAGGART, LLC

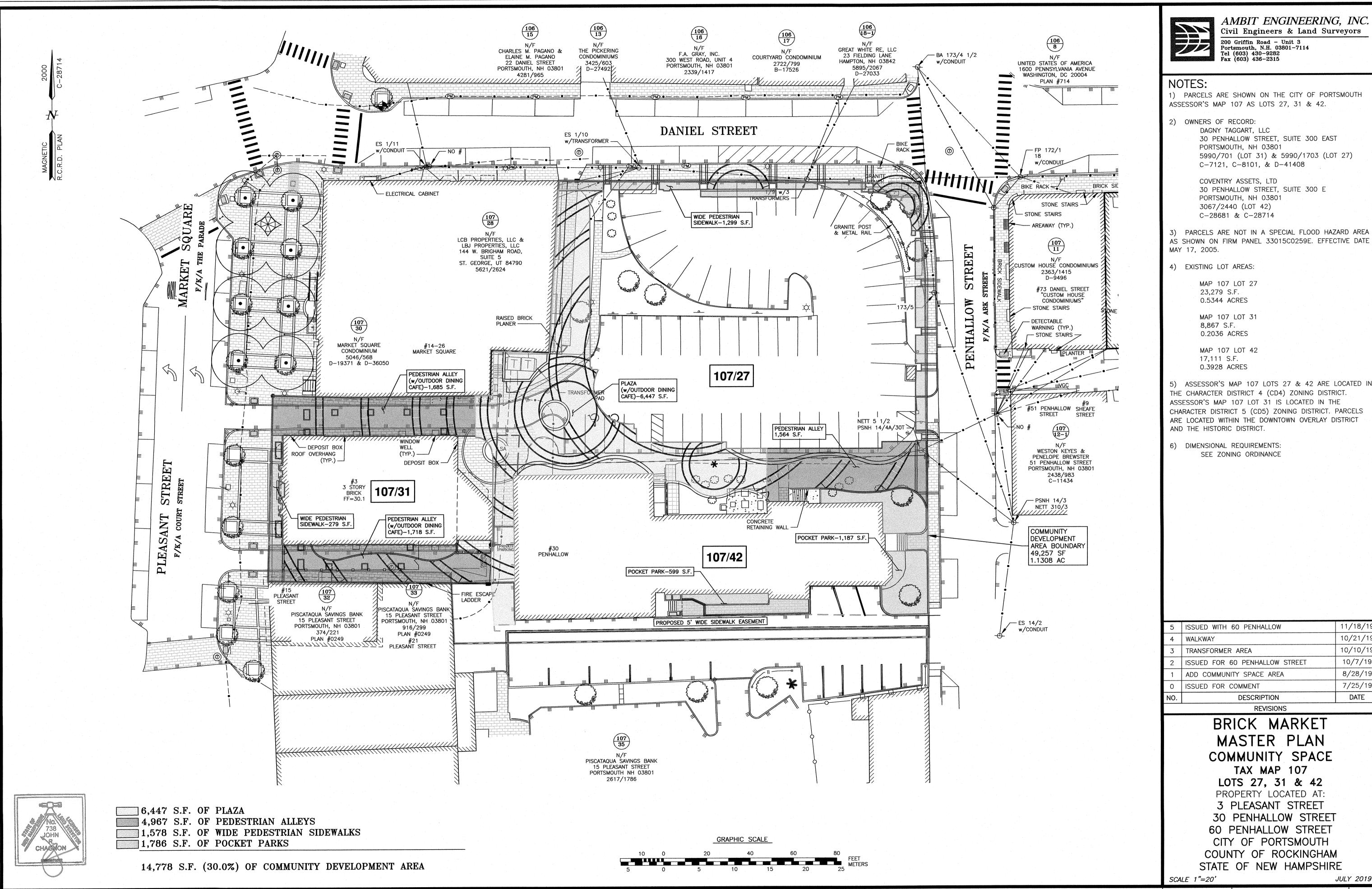
3 PLEASANT STREET & 60 PENHALLOW STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM STATE OF NEW HAMPSHIRE

MARCH 2019

FB 402 PG 1







1) PARCELS ARE SHOWN ON THE CITY OF PORTSMOUTH

30 PENHALLOW STREET, SUITE 300 EAST 5990/701 (LOT 31) & 5990/1703 (LOT 27)

30 PENHALLOW STREET, SUITE 300 E

AS SHOWN ON FIRM PANEL 33015C0259E. EFFECTIVE DATE

5) ASSESSOR'S MAP 107 LOTS 27 & 42 ARE LOCATED IN THE CHARACTER DISTRICT 4 (CD4) ZONING DISTRICT. ASSESSOR'S MAP 107 LOT 31 IS LOCATED IN THE CHARACTER DISTRICT 5 (CD5) ZONING DISTRICT. PARCELS ARE LOCATED WITHIN THE DOWNTOWN OVERLAY DISTRICT

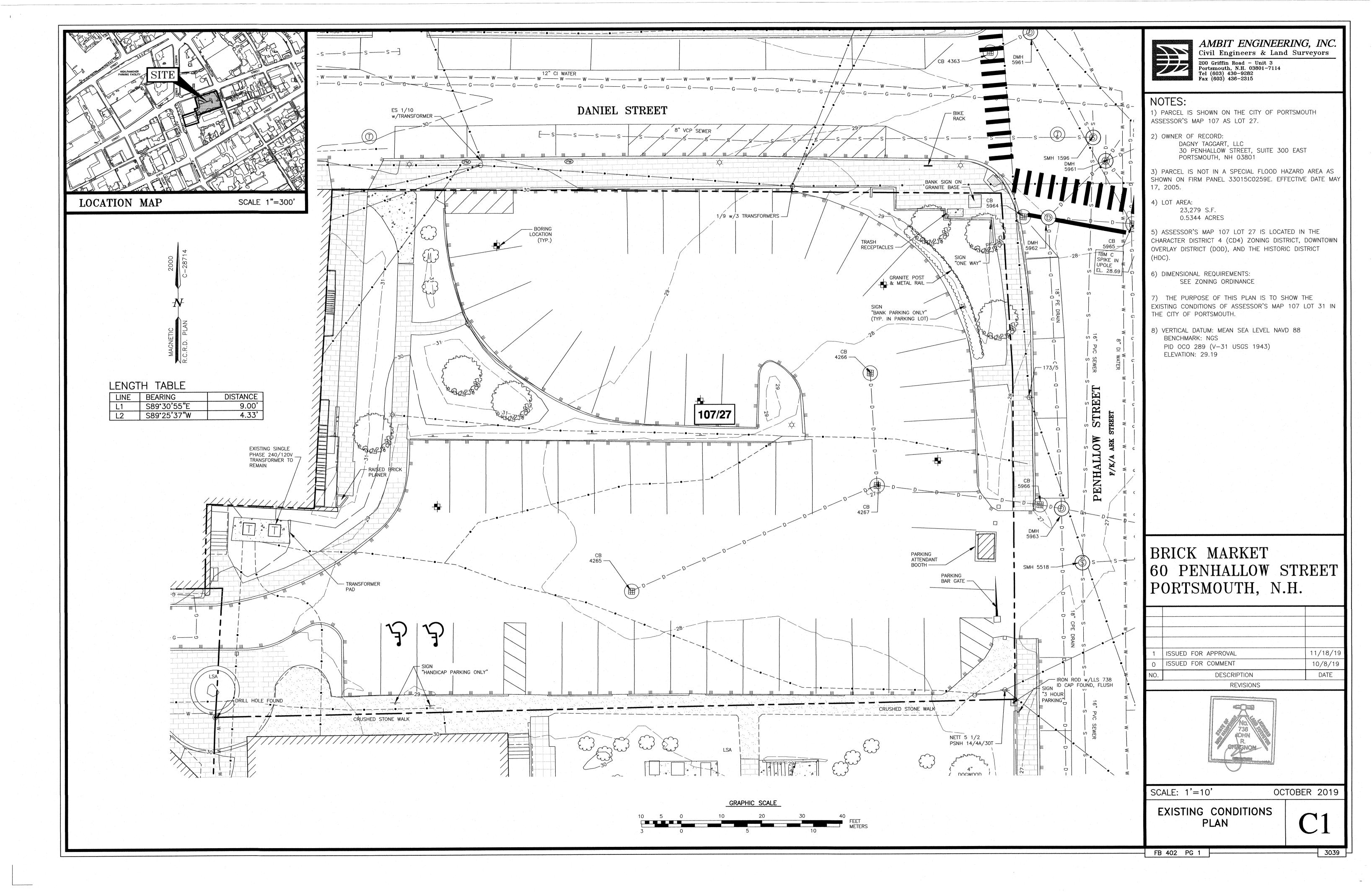
DEVISIONS						
NO.	DESCRIPTION	DATE				
0	ISSUED FOR COMMENT	7/25/19				
1	ADD COMMUNITY SPACE AREA	8/28/19				
2	ISSUED FOR 60 PENHALLOW STREET	10/7/19				
3	TRANSFORMER AREA	10/10/19				
4	WALKWAY	10/21/19				
5	ISSUED WITH 60 PENHALLOW	11/18/19				

MASTER PLAN COMMUNITY SPACE

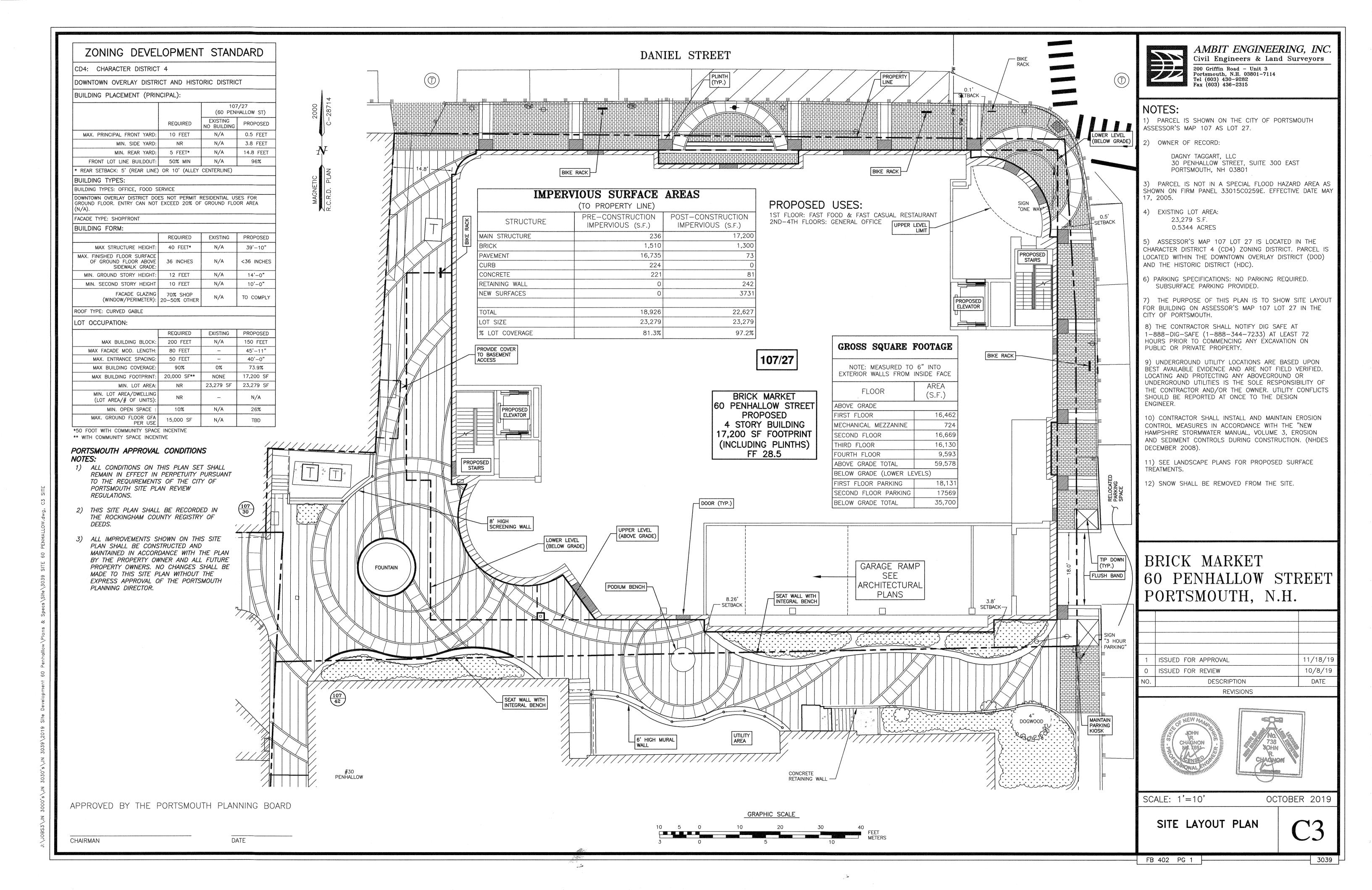
PROPERTY LOCATED AT: **3 PLEASANT STREET** 30 PENHALLOW STREET 60 PENHALLOW STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM

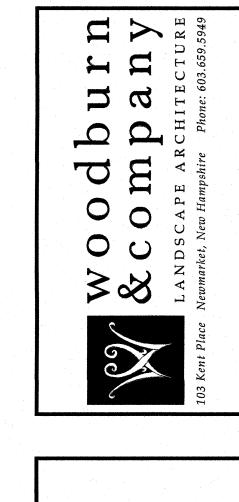
JULY 2019

FB 402 PG 1



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 **DEMOLITION NOTES** Tel (603) 430-9282 Fax (603) 436-2315 A) THE LOCATIONS OF UNDERGROUND UTILITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT GUARANTEED BY THE OWNER OR THE DESIGNER. IT IS THE NOTES: CONTRACTORS' RESPONSIBILITY TO LOCATE UTILITIES AND ANTICIPATE CONFLICTS. CONTRACTOR SHALL REPAIR DANIEL STREET EXISTING UTILITIES DAMAGED BY THEIR WORK AND ES 1/10 1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT RELOCATE EXISTING UTILITIES THAT ARE REQUIRED TO BE w/TRANSFORMER -RACK 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 RELOCATED PRIOR TO COMMENCING ANY WORK IN THE HOURS PRIOR TO COMMENCING ANY EXCAVATION ON IMPACTED AREA OF THE PROJECT. REMOVE PARKING METERS AS NEEDED PUBLIC OR PRIVATE PROPERTY. TIP DOWN DURING CONSTRUCTION. RETURN METERS RELOCATE B) ALL MATERIALS SCHEDULED TO BE REMOVED SHALL TO REMAIN TO DEPARTMENT OF PUBLIC WORKS, THEN BIKE RACK BECOME THE PROPERTY OF THE CONTRACTORS UNLESS 2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON RE-INSTALL UPON COMPLETION OF WORK OTHERWISE SPECIFIED. THE CONTRACTOR SHALL DISPOSE BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. OF ALL MATERIALS OFF-SITE IN ACCORDANCE WITH ALL LOCATING AND PROTECTING ANY ABOVEGROUND OR FEDERAL, STATE, AND LOCAL REGULATIONS, ORDINANCES UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF AND CODES. THE CONTRACTOR SHALL COORDINATE THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS REMOVAL, RELOCATION, DISPOSAL, OR SALVAGE OF ____ UTILITIES WITH THE OWNER AND APPROPRIATE UTILITY SHOULD BE REPORTED AT ONCE TO THE DESIGN GRANITE BASE COMPANY. C) ANY EXISTING WORK OR PROPERTY DAMAGED OR RECEPTACLES -3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION DISRUPTED BY CONSTRUCTION/ DEMOLITION ACTIVITIES CONTROL MEASURES IN ACCORDANCE WITH THE "NEW SHALL BE REPLACED OR REPAIRED TO THE ORIGINAL TREE TO 1/9 w/3 TRANSFORMERS — HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION EXISTING CONDITIONS BY THE CONTRACTOR AT NO CONCRETE AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES ADDITIONAL COST TO THE OWNER. REMOVED TREE TO REMOVED DECEMBER 2008). TRASH RECEPTACLE BRICK, CURB, D) THE CONTRACTOR SHALL VERIFY LOCATION OF ALL REMOVED TO BE RETURNED TO SIGN AND ONE WAY SIGN EXISTING UTILITIES AND CALL DIG SAFE AT LEAST 72 DEPARTMENT OF BENCHES TO BE TO BE RETURNED TREES TO BE HOURS PRIOR TO THE COMMENCEMENT OF ANY PUBLIC WORKS REMOVED REMOVED DEMOLITION/CONSTRUCTION ACTIVITIES. OF PUBLIC WORKS E) SAWCUT AND REMOVE PAVEMENT ONE FOOT OFF REMOVED -PROPOSED EDGE OF PAVEMENT OR EXISTING CURB LINE ALL PAVEMENT & METAL RAIL IN AREAS WHERE PAVEMENT TO BE REMOVED ABUTS IN LOT TO BE REMOVED EXISTING PAVEMENT OR CONCRETE TO REMAIN. TO BE REMOVED F) IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE CONDITIONS OF ALL THE PERMIT SIDEWALK TO "BANK PARKING ONLY" APPROVALS. BE REMOVED (TYP. IN PARKING LOT) -TREE TO BE LANDSCAPE ISLAND FIRE ESCAPES AND REMOVED INCLUDING CURB G) THE CONTRACTOR SHALL OBTAIN AND PAY FOR ADDITIONAL BASEMENT ACCESS TO BE REMOVED TO BE REMOVED CONSTRUCTION PERMITS, NOTICES AND FEES NECESSARY TO REMAIN TO COMPLETE THE WORK AND ARRANGE FOR AND PAY FOR ANY INSPECTIONS AND APPROVALS FROM THE AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL AND 4266 ---SIDEWALK TO OFF-SITE DISPOSAL OF MATERIALS REQUIRED TO REMOVED BE REMOVED COMPLETE THE WORK. TREE TO BE REMOVED CONCRETE TO H) THE CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL BE REMOVED EXISTING STRUCTURES, CONCRETE, UTILITIES, VEGETATION, POLE TO BE PAVEMENT, AND CONTAMINATED SOIL WITHIN THE WORK REMOVED BY LIMITS SHOWN UNLESS SPECIFICALLY IDENTIFIED TO LIGHT TO BE REMOVED FAIRPOINT REMOVED REMAIN. ANY EXISTING MONITORING WELLS IN THE LIGHT TO BE PROJECT AREA IDENTIFIED DURING THE CONSTRUCTION REMOVED AND NOT CALLED OUT ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER TO COORDINATE MONITORING WELL REMOVAL AND/OR RELOCATION WITH NHDES AND OTHER AUTHORITY WITH SYSTEM TO BE REMOVED JURISDICTION PRIOR TO CONSTRUCTION. FENCE TO BE I) ALL WORK WITHIN THE CITY OF PORTSMOUTH RIGHT OF REMOVED EXISTING SIGNS WAY SHALL BE COORDINATED WITH THE CITY OF TO BE REMOVED PORTSMOUTH DEPARTMENT OF PUBLIC WORKS (DPW). - RAISED BRICK PLANER FENCE POSTS TRANSFORMER TO J) CONTRACTOR SHALL PROTECT ALL PROPERTY TO BE REMOVED TO BE REMOVED BE REPLACED MONUMENTATION THROUGHOUT DEMOLITION AND CONSTRUCTION OPERATIONS. SHOULD ANY MONUMENTATION BE DISTURBED, THE CONTRACTOR SHALL EMPLOY A NH LICENSED LAND SURVEYOR TO REPLACE 4267 — REMOVED K) PROVIDE INLET PROTECTION BARRIERS AT ALL CATCH TO BE REMOVED BRICK WALL TO BASINS WITHIN CONSTRUCTION LIMITS AND IMMEDIATELY BE REMOVED BRICK MARKET BE REMOVED OFF-SITE TO BE MAINTAIN FOR THE DURATION OF THE PROJECT, INLET PROTECTION BARRIERS SHALL BE HIGH 107/27 FLOW SILT SACK BY ACF ENVIRONMENTAL OR APPROVED 60 PENHALLOW STREET ALL PAVEMENT EQUAL, INSPECT BARRIERS WEEKLY AND AFTER EACH ATTENDANT IN LOT TO BE REMOVED 4265 — RAIN OF 0.25 INCHES OR GREATER. CONTRACTOR SHALL REMOVED COMPLETE A MAINTENANCE INSPECTION REPORT AFTER PORTSMOUTH, N.H. EACH INSPECTION. SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT OR MORE OFTEN IF BAR GATE -TRANSFORMER WARRANTED OR FABRIC BECOMES CLOGGED. EROSION TO BE REMOVED PAD REMOVE LIGHT CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE START OF ANY CLEARING OR DEMOLITION ACTIVITIES. LANDSCAPE ISLAND INCLUDING CURB L) THE CONTRACTOR SHALL PAY ALL COSTS NECESSARY FOR TO BE REMOVED TEMPORARY PARTITIONING, BARRICADING, FENCING, SECURITY AND SAFELY DEVICES REQUIRED FOR THE MAINTENANCE OF A CLEAN AND SAFE CONSTRUCTION 11/18/19 ISSUED FOR APPROVAL M) ANY CONTAMINATED MATERIAL REMOVED DURING THE ISSUED FOR COMMENT 10/8/19 COURSE OF THE WORK WILL REQUIRE HANDLING IN ACCORDANCE WITH NHDES REGULATIONS. CONTRACTOR DATE DESCRIPTION SHALL HAVE A HEALTH AND SAFETY PLAN IN PLACE, AND REVISIONS "HANDICAP PARKING ONLY" COMPLY WITH ALL APPLICABLE PERMITS, APPROVALS, AUTHORIZATIONS, AND REGULATIONS CRUSHED STONE WALK CRUSHED STONE WALK NETT 5 1/2 PSNH 14/44/30T OCTOBER 2019 SCALE: 1'=10' APPROVED BY THE PORTSMOUTH PLANNING BOARD GRAPHIC SCALE **DEMOLITION PLAN** DATE CHAIRMAN FB 402 PG 1





STER Market Bri SC

Checked By: RW 1'' = 20' - 0''Scale: September 19, 2019 Date: Revisions: October 2, 2019

Drawn By:

October 7, 2019 October 8, 2019 October 14, 2019 November 18, 2019

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Sheet 1 of 4

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Landscape Notes

- 1. Design is based on drawings by Ambit Engineering dated November 18, 2019 and may require adjustment due to actual field
- 2. The contractor shall follow best management practices during construction and shall take all means necessary to stabilize and protect the site from erosion.
- Erosion Control shall be in place prior to construction.
- 4. The Contractor shall verify layout and grades and inform the Landscape Architect or Client's Representative of any
- discrepancies or changes in layout and/or grade relationships prior to construction.
- 5. It is the contractor's responsibility to verify drawings provided are to the correct scale prior to any bid, estimate or installation. A graphic scale bar has been provided on each sheet for this purpose. If it is determined that the scale of the drawing is incorrect, the landscape architect will provide a set of drawings at the correct scale, at the request of the contractor.
- 6. Trees to Remain within the construction zone shall be protected from damage for the duration of the project by snow fence or other suitable means of protection to be approved by Landscape Architect or Client's Representative. Snow fence shall be located at the drip line at a minimum and shall include any and all surface roots. Do not fill or mulch on the trunk flare. Do not disturb roots. In order to protect the integrity of the roots, branches, trunk and bark of the tree(s) no vehicles or construction equipment shall drive or park in or on the area within the drip line(s) of the tree(s). Do not store any refuse or construction materials or portalets within the tree protection area.
- This plan is for review purposes only, NOT for Construction. Construction Documents will be provided upon request.
- 8. Location, support, protection, and restoration of all existing utilities and appurtenances shall be the responsibility of the
- 9. The Contractor shall verify exact location and elevation of all utilities with the respective utility owners prior to construction. Call DIGSAFE at 1-888-344-7233
- 10. The Contractor shall procure any required permits prior to construction.
- 11. Prior to any landscape construction activities Contractor shall test all existing loam and loam from off-site intended to be used for lawns and plant beds using a thorough sampling throughout the supply. Soil testing shall indicate levels of pH, nitrates, macro and micro nutrients, texture, soluble salts, and organic matter. Contractor shall provide Landscape Architect with test results and recommendations from the testing facility along with soil amendment plans as necessary for the proposed plantings to thrive. All loam to be used on site shall be amended as approved by the Landscape Architect prior to placement.
- 12. Contractor shall notify landscape architect or owner's representative immediately if at any point during demolition or construction a site condition is discovered which may negatively impact the completed project. This includes, but is not limited to, unforeseen drainage problems, unknown subsurface conditions, and discrepancies between the plan and the site. If a contractor is aware of a potential issue, and does not bring it to the attention of the landscape architect or owner's representative immediately, they may be responsible for the labor and materials associated with correcting the problem.
- 13. The Contractor shall furnish and plant all plants shown on the drawings and listed thereon. All plants shall be nursery-grown under climatic conditions similar to those in the locality of the project. Plants shall conform to the botanical names and standards of size, culture, and quality for the highest grades and standards as adopted by the American Association of Nurserymen, Inc. in the American Standard of Nursery Stock, American Standards Institute, Inc. 230 Southern Building, Washington, D.C. 20005.
- 14. A complete list of plants, including a schedule of sizes, quantities, and other requirements is shown on the drawings. In the event that quantity discrepancies or material omissions occur in the plant materials list, the planting plans shall govern.
- 15. All plants shall be legibly tagged with proper botanical name.
- 16. The Contractor shall guarantee all plants for not less than one year from time of acceptance.
- 17. Owner or Owner's Representative will inspect plants upon delivery for conformity to Specification requirements. Such approval shall not affect the right of inspection and rejection during or after the progress of the work. The Owner reserves the right to inspect and/or select all trees at the place of growth and reserves the right to approve a representative sample of each type of shrub, herbaceous perennial, annual, and ground cover at the place of growth. Such sample will serve as a minimum standard for all plants of the same species used in this work.
- 18. No substitutions of plants may be made without prior approval of the Owner or the Owner's Representative for any reason.
- 19. All landscaping shall be provided with either of the following
- a. An underground sprinkling system b. An outside hose attachment within 150 feet
- 20. If an automatic irrigation system is installed, all irrigation valve boxes shall be located within planting bed areas. 21. The contractor is responsible for all plant material from the time their work commences until final acceptance. This includes but is not limited to maintaining all plants in good condition, the security of the plant material once delivered to the site, and watering of plants. Plants shall be appropriately watered prior to, during and after planting. It is the contractor's responsibility
- to provide water from off site, should it not be available on site. 22. All disturbed areas will be dressed with 6" of topsoil and planted as noted on the plans or seeded except plant beds. Plant
- beds shall be prepared to a depth of 12" with 75% loam and 25% compost. 23. Trees, ground cover, and shrub beds shall be mulched to a depth of 2" with one-year-old, well-composted, shredded native bark not longer than 4" in length and ½" in width, free of woodchips and sawdust. Mulch for ferns and herbaceous perennials shall be no longer than 1" in length. Trees in lawn areas shall be mulched in a 5' diameter min. saucer. Color of mulch shall be
- 24. In no case shall mulch touch the stem of a plant nor shall mulch ever be more than 3" thick total (including previously applied mulch) over the root ball of any plant.
- 25. Secondary lateral branches of deciduous trees overhanging vehicular and pedestrian travel ways shall be pruned up to a
- height of 6' to allow clear and safe passage of vehicles and pedestrians under tree canopy. 26. Snow shall be removed from the site.
- 27. Landscape Architect is not responsible for the means and methods of the contractor.

City of Portsmouth Notes

Nepeta faassenii x 'Six Hills Giant'

- 1. The property owner and all future property owners shall be responsible for the maintenance, repair and replacement of all required screening and landscape in 2. All required plant materials shall be tended and maintained in a healthy growing condition, replaced when necessary, and kept free of refuse and debris. All required
- fences and walls shall be maintained in good repair 3. The property owner shall be responsible to remove and replace dead or diseased plant materials immediately with the same type, size and quantity of plant materials as originally installed, unless alternative plantings are requested, justified and approved by the Planning Board or Planning Director.

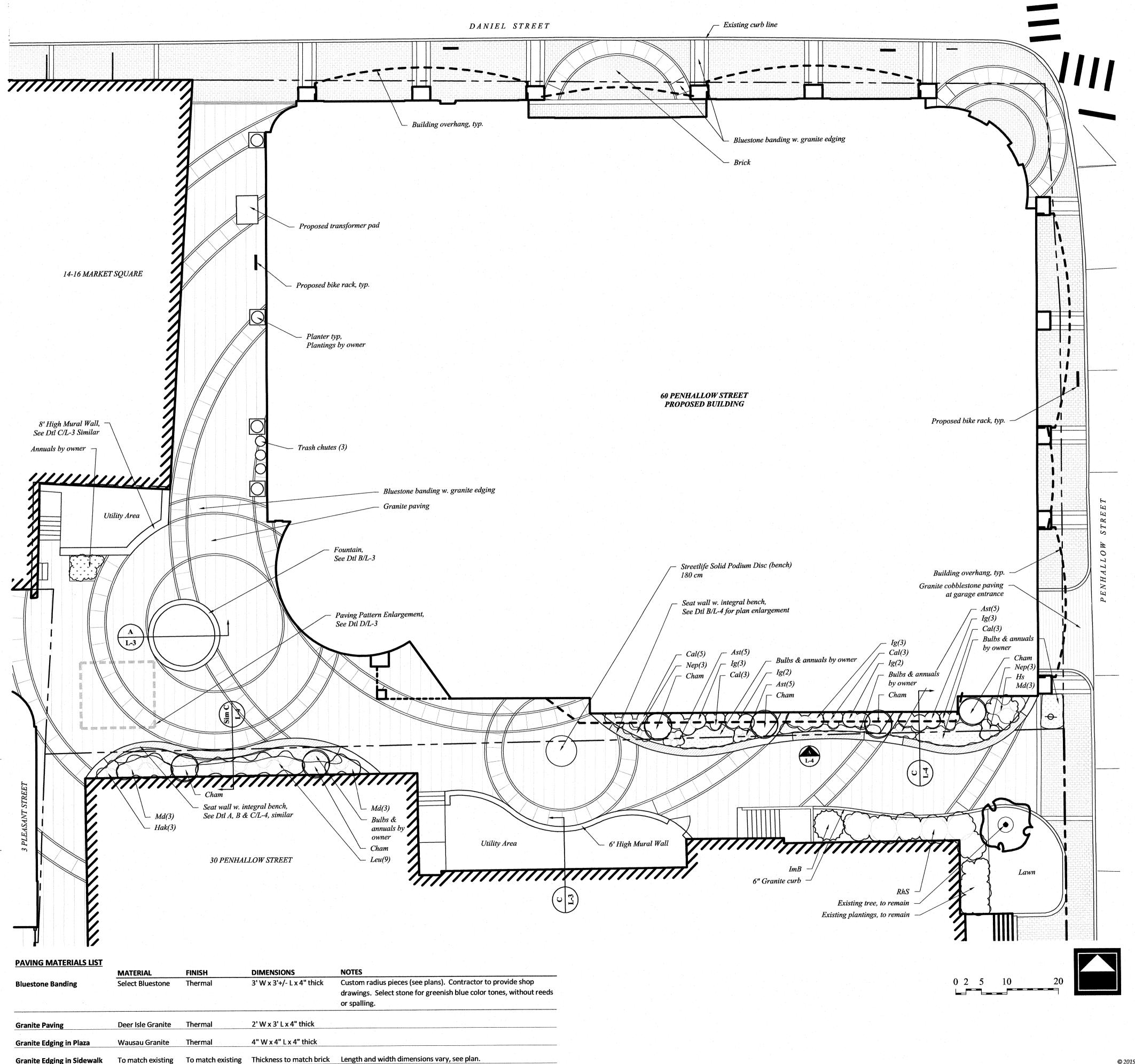
Plant List

Plant List

Symbol	Botanical Name	Common Name	Quantity	Size	Comments
Cham	Chamaecyparis obtusa 'Gracilis'	Gracilis Falsecypress	6	7-8' ht	B&B Matched Specimen
Hs	Hibiscus syriacus 'Ardens'	Ardens Rose-of -Sharon	1 .	6-7' ht	B&B Full
lg	llex glabra 'Shamrock'	Shamrock Inkberry	13	5 gal	Full
lmB	llex meserve 'Blue Maid'	Blue Maid Holly	1	6-7' ht	B&B Full
Leu	Leucothoe fontanesiana 'Silver Run'	Silver Run Leucothoe	9	3 gal	
Md	Microbiota decussata	Russian Cypress	9	5 gal	
RhS	Rhododendron 'Scintillation'	Scintillation Rhododendron	6	2.5-3' ht	B&B
PERENNIA	ALS, GROUNDCOVERS, VINES and AN	NUALS			
Symbol	Botanical Name	Common Name	Quantity	Size	Comments

SHRUBS Rubyred Astilbe Astilbe 'Fanal' Feather Reed Grass Calamagrostis acutifolia 'Karl Foerster' 3 gal Golden Japanese Forest Grass Hakonechioa macra 'Aureola' 1 gal

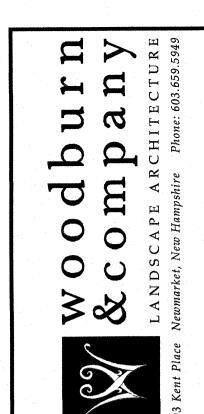
Lavender blue Catmint



Specification TBD

Clay paving brick

Brick

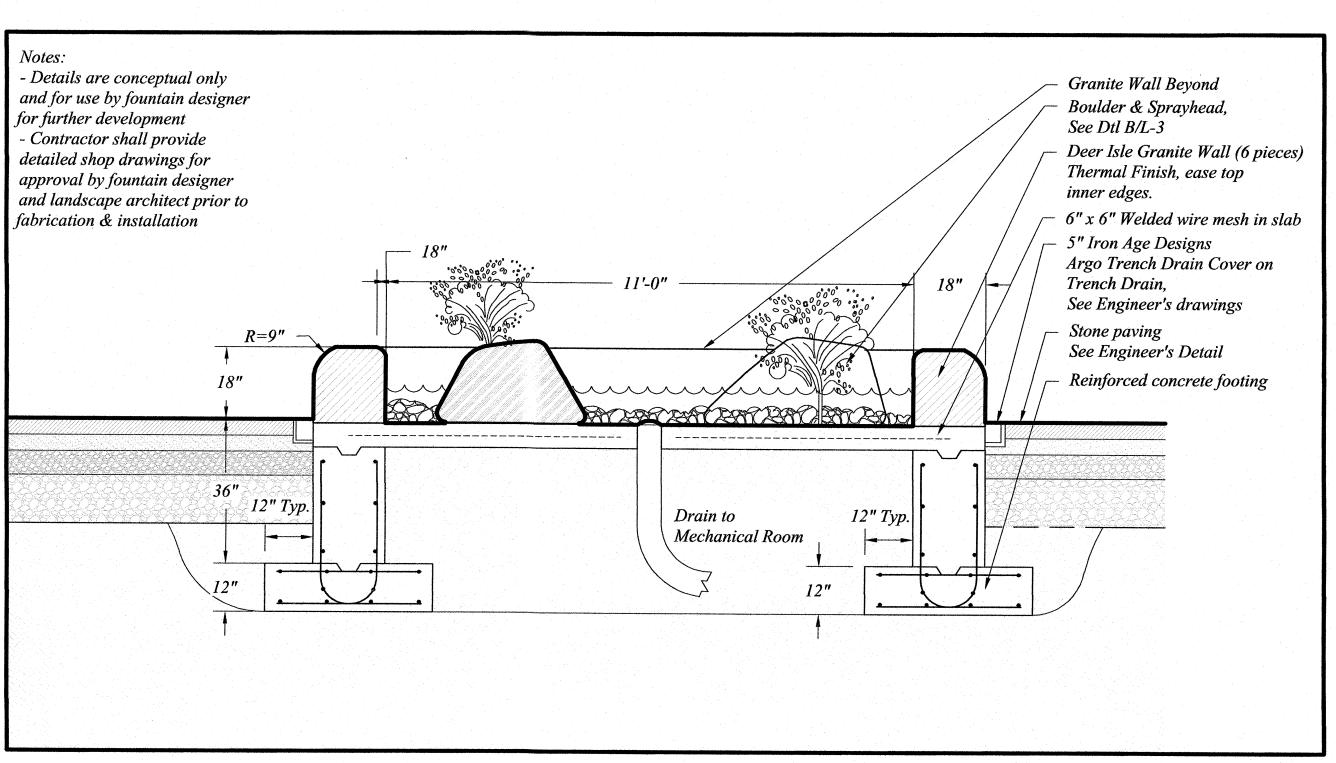


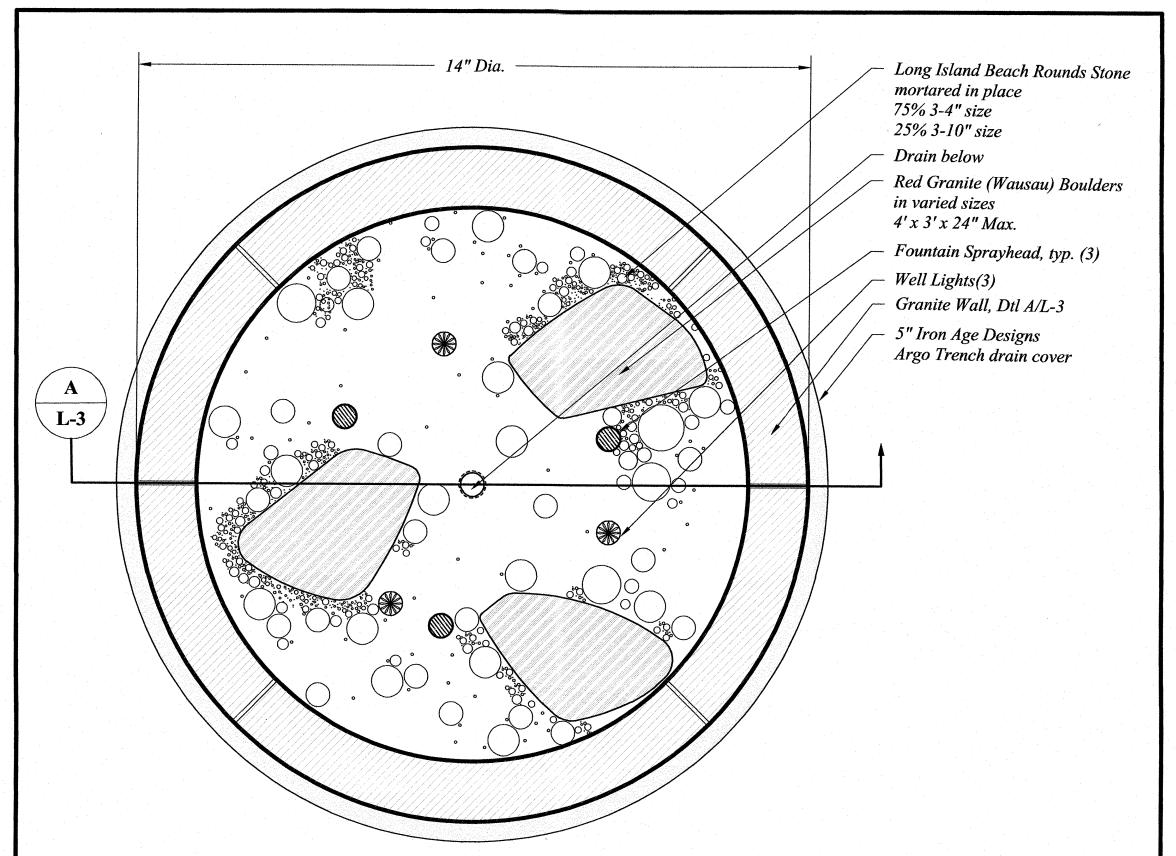
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Sheet 2 of 4

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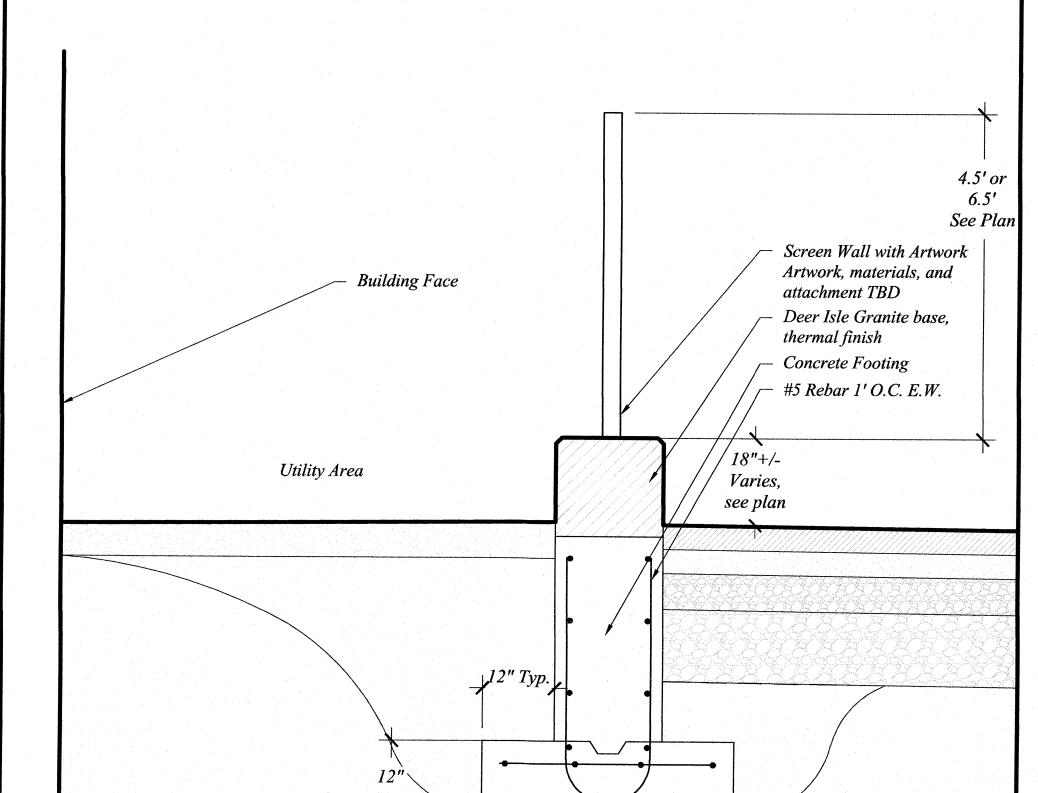




Fountain Section

Scale: 1/2"=1'-0"

0 1 2 4



Compacted Subgrade

		$\sqrt{2x3'}$	tone banding Granite Pavin pattern " Granite Edg	ng, running

Fountain Plan
Scale: 1/2"=1'-0"

MATERIAL	FINISH	DIMENSIONS	NOTES
Select Bluestone	Thermal	3' W x 3'+/- L x 4" thick	Custom radius pieces (see plans). Contractor to provide shop
			drawings. Select stone for greenish blue color tones, without reed or spalling.
Deer Isle Granite	Thermal	2' W x 3' L x 4" thick	
Wausau Granite	Thermal	4" W x 4" L x 4" thick	
To match existing	To match existing	Thickness to match brick	Length and width dimensions vary, see plan.
	Select Bluestone Deer Isle Granite Wausau Granite	Select Bluestone Thermal Deer Isle Granite Thermal Wausau Granite Thermal	Select Bluestone Thermal 3' W x 3'+/- L x 4" thick Deer Isle Granite Thermal 2' W x 3' L x 4" thick Wausau Granite Thermal 4" W x 4" L x 4" thick

C Mural Wall Section

Scale: \(\frac{3}{4}\''=1'-0''\)

Paving Enlargement

Scale: 1/2"=1'-0"

1 2 4

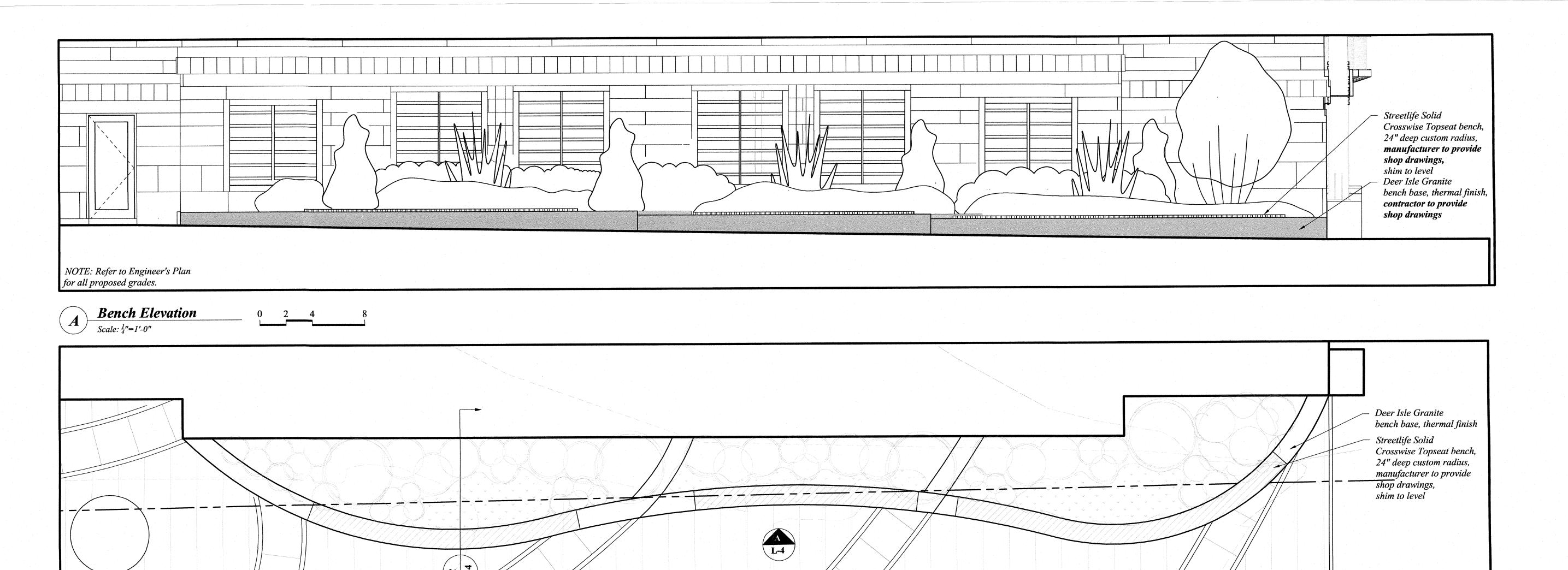
Brick Market
LANDSCAPE DETA

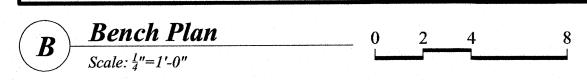
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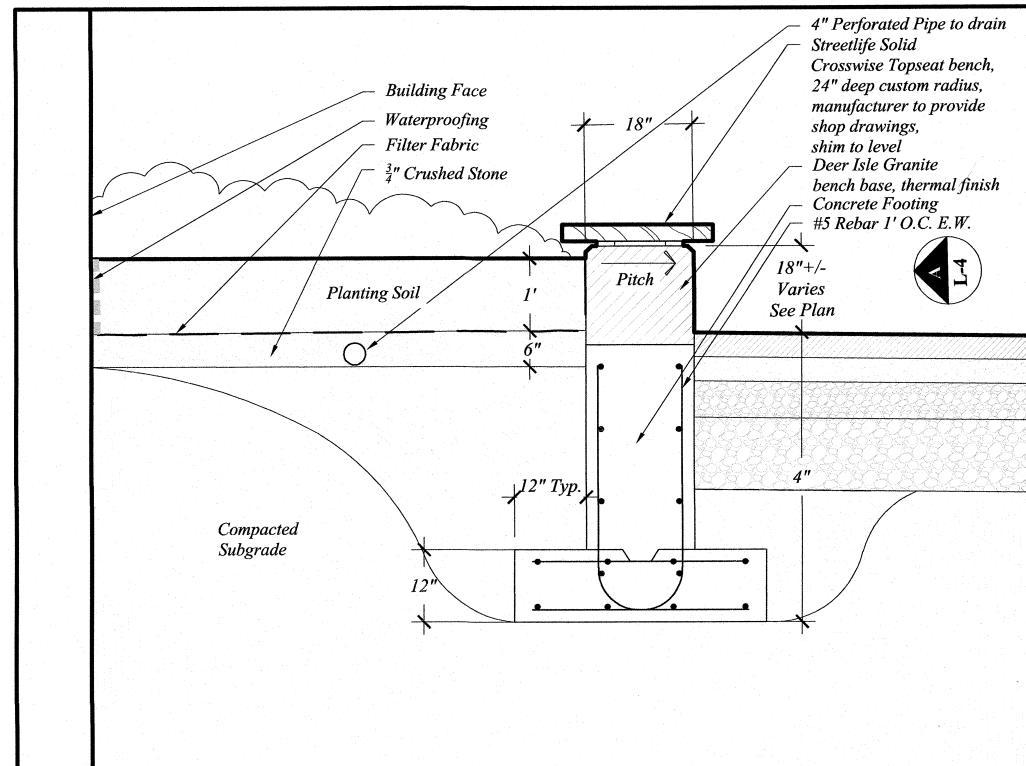
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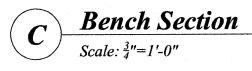
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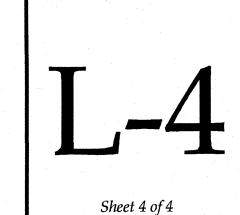


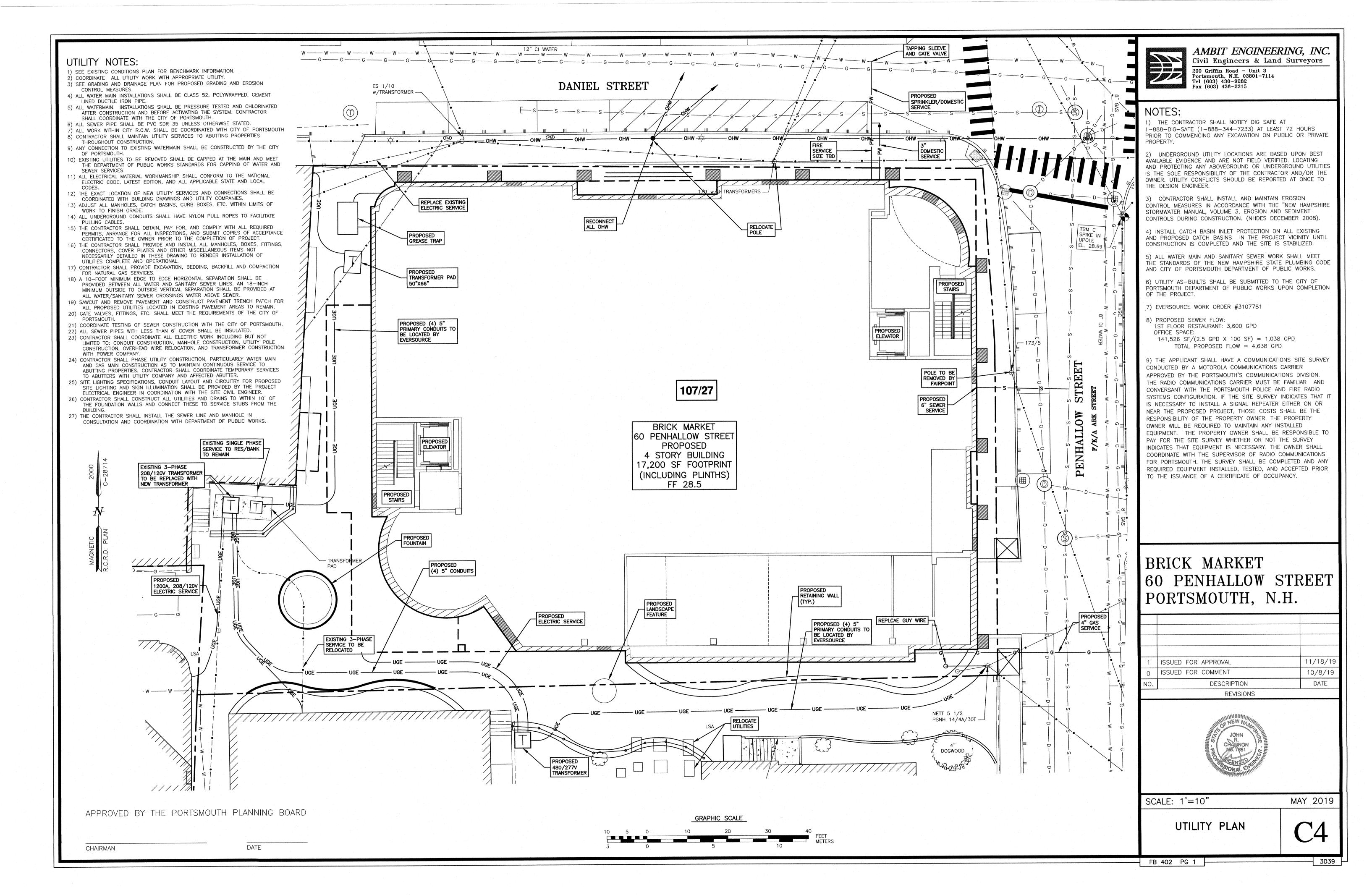
Brick Market LANDSCAPE DETAIL

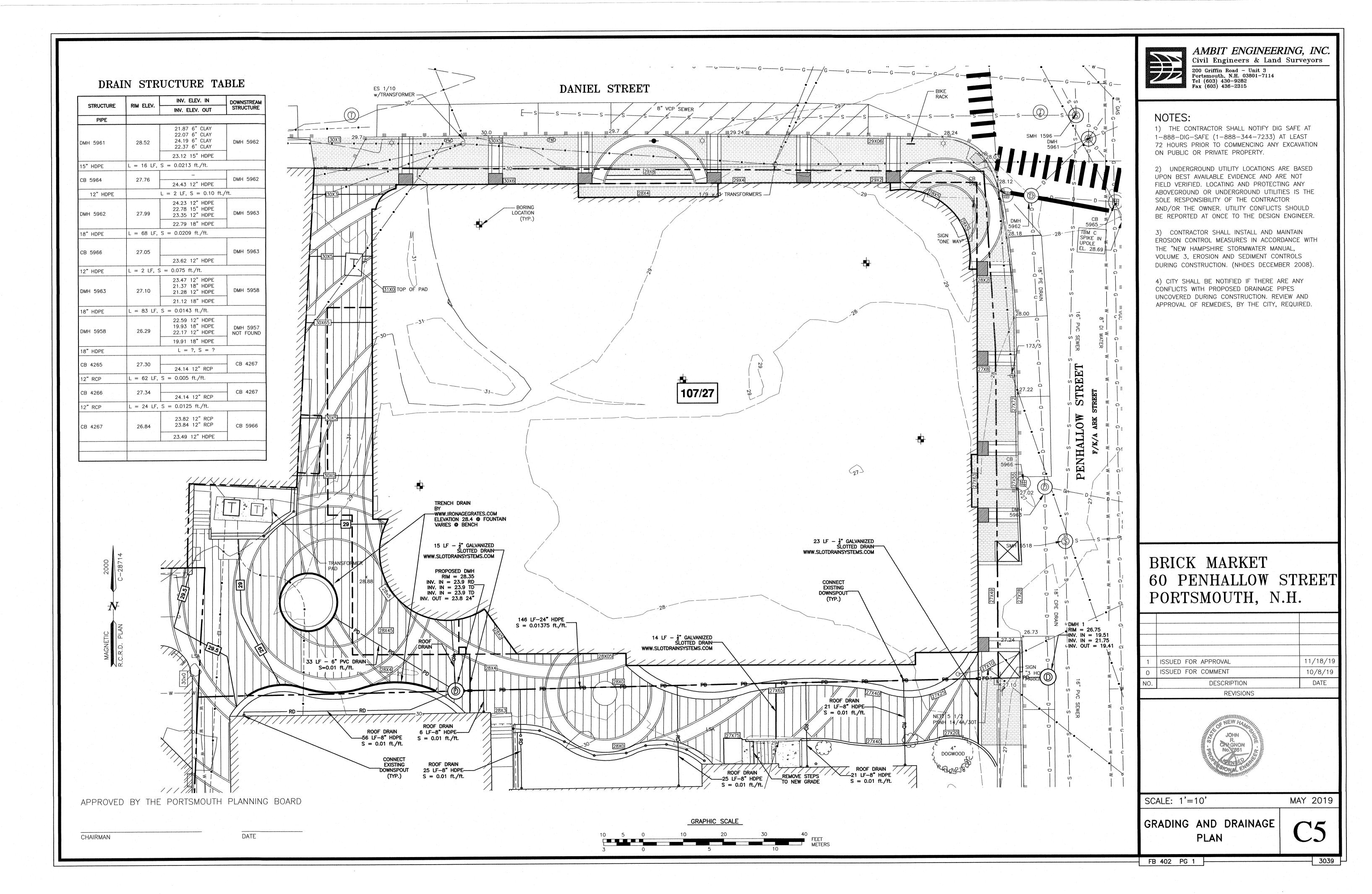
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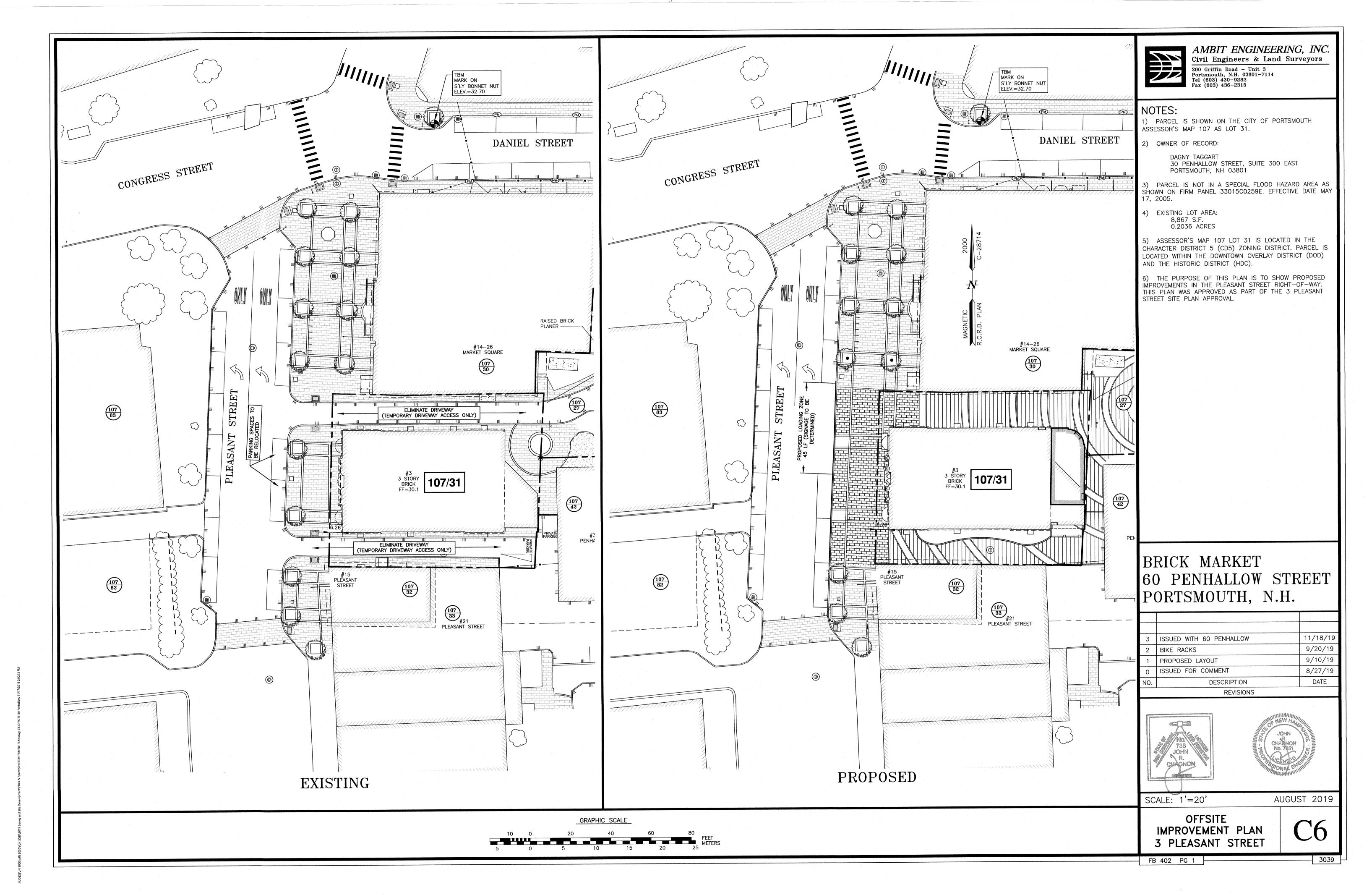
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Date: November 18, 2019
Revisions:









THE CONTRACTOR SHALL OBTAIN AN **NPDES** PHASE II STORMWATER PERMIT AND SUBMIT A NOTICE OF INTENT (N.O.I) BEFORE BEGINNING CONSTRUCTION AND SHALL HAVE ON SITE A STORMWATER POLLUTION PREVENTION PLAN (S.W.P.P.P.) AVAILABLE FOR INSPECTION BY THE PERMITTING AUTHORITY DURING THE CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CARRYING OUT THE S.W.P.P.P. AND INSPECTING AND MAINTAINING ALL BMP'S CALLED FOR BY THE PLAN. THE CONTRACTOR SHALL SUBMIT A NOTICE OF TERMINATION (N.O.T.) FORM TO THE REGIONAL EPA OFFICE WITHIN 30 DAYS OF FINAL STABILIZATION OF THE ENTIRE SITE OR TURNING OVER CONTROL OF THE SITE TO ANOTHER OPERATOR.

INSTALL PERIMETER CONTROLS, i.e., SILTSOXX, FODS AND CATCH BASIN PROTECTION AROUND THE LIMITS OF DISTURBANCE BEFORE ANY CONSTRUCTION. THE USE OF HAYBALES IS NOT ALLOWED.

REMOVE DEBRIS AND RUBBISH AS REQUIRED. DEMOLISH BUILDINGS AND OTHER IMPROVEMENTS AS SHOWN ON THE PLANS.

CUT AND CAP IMPACTED UTILITIES AS DIRECTED BY UTILITY PROVIDERS.

CONSTRUCT OFF SITE UTILITY IMPROVEMENTS NECESSARY TO CONSTRUCT BUILDING.

CONSTRUCT FOUNDATION

LAYOUT AND INSTALL ALL BURIED UTILITIES AND SERVICES UP TO THE PROPOSED BUILDING FOUNDATION. CAP AND MARK TERMINATIONS OR LOG SWING TIES.

BEGIN BUILDING CONSTRUCTION.

CONNECT UTILITIES AS NEEDED.

PLACE BASE MATERIALS IN WALKWAYS AND PROTECT

CONTINUE BUILDING CONSTRUCTION.

PLANT LANDSCAPING IN AREAS OUT OF WAY OF BUILDING CONSTRUCTION. CONSTRUCT OTHER SITE IMPROVEMENTS.

AFTER BUILDINGS ARE COMPLETED, FINISH ALL REMAINING WORK

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE SITE.

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE "STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS.

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT FROSION

DUST CONTROL: IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND MULCHING, DO NOT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILT FENCES AND SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM. ALL DAMAGED SILT FENCES AND SILTSOXX SHALL BE REPAIRED. SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

AVOID THE USE OF FUTURE OPEN SPACES (LOAM AND SEED AREAS) WHEREVER POSSIBLE DURING CONSTRUCTION. CONSTRUCTION TRAFFIC SHALL USE THE ROADBEDS OF FUTURE ACCESS DRIVES AND

ADDITIONAL TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN AMOUNTS NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS——CONSTRUCT SILT FENCE OR SILTSOXX AROUND TOPSOIL STOCKPILE.

AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL. STUMPS SHALL BE DISPOSED OF IN AN

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH, WOODY DEBRIS, LEAVES, BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED, ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL.

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:

- BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED
 A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED
- A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED
 A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS
- BEEN INSTALLED

 EROSION CONTROL BLANKETS HAVE BEEN INSTALLED

ACHIEVING FINISHED GRADE.

VEGETATIVE PRACTICE

FOR PERMANENT MEASURES AND PLANTINGS:

LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF 2 TONS

FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE.

FERTILIZER APPLICATION RATE SHALL BE 500 POUNDS PER ACRE OF 10-20-20 FERTILIZER.

SEED SHALL BE SOWN AT THE RATES SHOWN IN THE TABLE BELOW. IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND

THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AT A RATE OF 1.5 TO 2 TONS PER ACRE, AND SHALL BE HELD IN PLACE USING APPROPRIATE TECHNIQUES FROM THE EROSION AND SEDIMENT CONTROL HANDBOOK.

THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED. ANY AREAS WHICH ARE NOT SATISFACTORILY COVERED SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED.

100 LBS/ACRE

A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE:

GENERAL COVER PROPORTION SEEDING RATE

CREEPING RED FESCUE 50%
KENTUCKY BLUEGRASS 50%

SLOPE SEED (USED ON ALL SLOPES GREATER THAN OR EQUAL TO 3:1)

CREEPING RED FESCUE
TALL FESCUE

TALL FESCUE 42% 48 LBS/ACRE BIRDSFOOT TREFOIL 16%

IN NO CASE SHALL THE WEED CONTENT EXCEED ONE PERCENT BY WEIGHT. ALL SEED SHALL COMPLY WITH APPLICABLE STATE AND FEDERAL SEED LAWS.

FOR TEMPORARY PROTECTION OF DISTURBED AREAS:

MULCHING AND SEEDING SHALL BE APPLIED AT THE FOLLOWING RATES:

PERENNIAL RYE: 0.7 LBS/1,000 S.F.

MULCH: 1.5 TONS/ACRE

MAINTENANCE AND PROTECTION

THE CONTRACTOR SHALL MAINTAIN ALL LOAM & SEED AREAS UNTIL FINAL ACCEPTANCE AT THE COMPLETION OF THE CONTRACT. MAINTENANCE SHALL INCLUDE WATERING, WEEDING, REMOVAL OF STONES AND OTHER FOREIGN OBJECTS OVER 1/2 INCHES IN DIAMETER WHICH MAY APPEAR AND THE FIRST TWO (2) CUTTINGS OF GRASS NO CLOSER THEN TEN (10) DAYS APART. THE FIRST CUTTING SHALL BE ACCOMPLISHED WHEN THE GRASS IS FROM 2 1/2 TO 3 INCHES HIGH. ALL BARE AND DEAD SPOTS WHICH BECOME APPARENT SHALL BE PROPERLY PREPARED, LIMED AND FERTILIZED, AND RESEEDED BY THE CONTRACTOR AT HIS EXPENSE AS MANY TIMES AS NECESSARY TO SECURE GOOD GROWTH. THE ENTIRE AREA SHALL BE MAINTAINED, WATERED AND CUT UNTIL ACCEPTANCE OF THE LAWN BY THE OWNER'S REPRESENTATIVE.

THE CONTRACTOR SHALL TAKE WHATEVER MEASURES ARE NECESSARY TO PROTECT THE GRASS WHILE IT IS DEVELOPING.

TO BE ACCEPTABLE, SEEDED AREAS SHALL CONSIST OF A UNIFORM STAND OF AT LEAST 90 PERCENT ESTABLISHED PERMANENT GRASS SPECIES, WITH UNIFORM COUNT OF AT LEAST 100 PLANTS PER SQUARE FOOT.

SEEDED AREAS WILL BE FERTILIZED AND RESEEDED AS NECESSARY TO INSURE VEGETATIVE

THE SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATION IS ESTABLISHED.

THE SILT FENCE OR SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL.

SILT FENCING AND SILTSOXX SHALL BE REMOVED ONCE VEGETATION IS ESTABLISHED, AND DISTURBED AREAS RESULTING FROM SILT FENCE AND SILTSOXX REMOVAL SHALL BE PERMANENTLY

WINTER NOTES

ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.

AFTER NOVEMBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.

FILTREXX® -2" x 2" HARDWOOD COMPOST STAKES SPACED 10' SILTSOXXTM APART LINEALLY FLOW WOOD CHIPS FROM ON-SITE <u>PLAN</u> CHIPPING OPERATIONS MAY BE MOUNDED AT THE BASE OF THE SILTSOXX AND SPREAD AFTER REMOVAL OF THE SILTSOXX -FILTREXX® SILTSOXXTM (8" - 24" TYP) -SIZE PER INSTALLERS RECOMMENDATION WATER FLOW

NOTES:

1. ALL MATERIAL TO MEET FILTREXX SPECIFICATIONS.

FILLTREXX SYSTEM SHALL BE INSTALLED BY A CERTIFIED FILTREXX INSTALLER.
 THE CONTRACTOR SHALL MAINTAIN THE COMPOST FILTRATION SYSTEM IN A FUNCTIONAL CONDITION AT ALL TIMES. IT WILL BE DOLLTING Y INSPECTED AND REPAIRED WHEN PEOURIES.

HARDWOOD

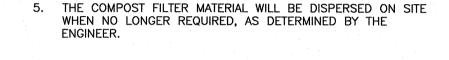
EXISTING

PAVEMENT

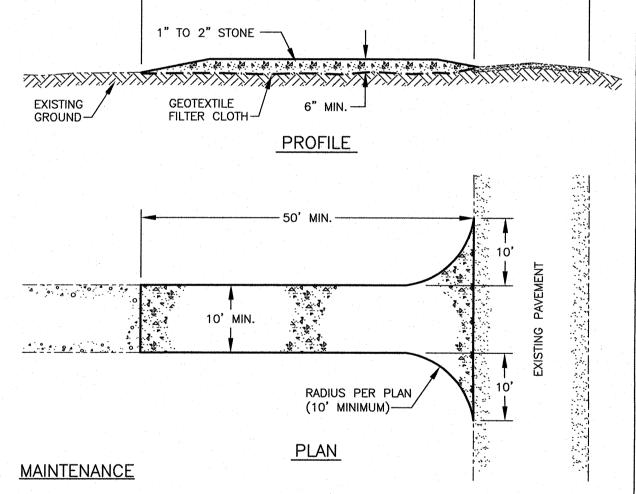
STAKE

ROUTINELY INSPECTED AND REPAIRED WHEN REQUIRED.

4. SILTSOXX DEPICTED IS FOR MINIMUM SLOPES, GREATER SLOPES MAY REQUIRE ADDITIONAL PLACEMENTS.







- 1) MUD AND SOIL PARTICLES WILL EVENTUALLY CLOG THE VOIDS IN THE GRAVEL AND THE EFFECTIVENESS OF THE GRAVEL PAD WILL NOT BE SATISFACTORY. WHEN THIS OCCURS, THE PAD SHOULD BE TOP DRESSED WITH NEW STONE. COMPLETE REPLACEMENT OF THE PAD MAY BE NECESSARY WHEN THE PAD BECOMES COMPLETELY CLOGGED.
- 2) IF WASHING FACILITIES ARE USED, THE SEDIMENT TRAPS SHOULD BE CLEANED OUT AS OFTEN AS NECESSARY TO ASSURE THAT ADEQUATE TRAPPING EFFICIENCY AND STORAGE VOLUME IS AVAILABLE. VEGETATIVE FILTER STRIPS SHOULD BE MAINTAINED TO INSURE A VIGOROUS STAND OF VEGETATION AT ALL TIMES.

CONSTRUCTION SPECIFICATIONS

- 1) STONE FOR A STABILIZED CONSTRUCTION ENTRANCE SHALL BE 1 TO 2 INCH STONE.

 2) THE LENGTH OF THE STABILIZED ENTRANCE SHALL NOT BE LESS THAN 50 FEET EXCEPT FOR
- THE LENGTH OF THE STABILIZED ENTRANCE SHALL NOT BE LESS THAN 50 FEET, EXCEPT FOR A SINGLE RESIDENTIAL LOT WHERE A 30 FOOT MINIMUM LENGTH WOULD APPLY.
 THE THICKNESS OF THE STONE FOR THE STABILIZED ENTRANCE SHALL NOT BE LESS THAN 6
- INCHES.

 4) THE WIDTH OF THE ENTRANCE SHALL NOT BE LESS THAN THE FULL WIDTH OF THE
- ENTRANCE WHERE INGRESS OR EGRESS OCCURS OR 10 FEET, WHICHEVER IS GREATER.

 5) GEOTEXTILE FILTER CLOTH SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING THE STONE. FILTER CLOTH IS NOT REQUIRED FOR A SINGLE FAMILY RESIDENCE LOT.

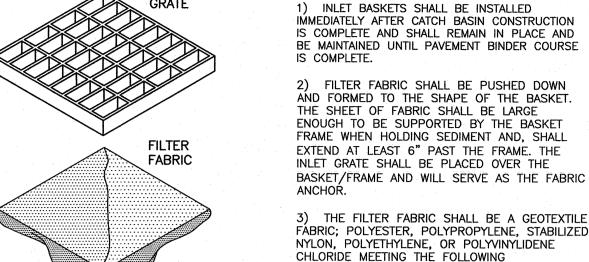
 6) ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE CONSTRUCTION
- ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE.

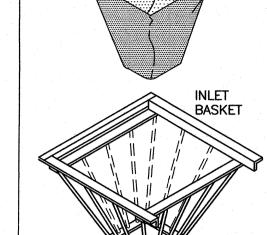
 7) THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS—OF—WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, WASHED, OR TRACKED
- ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED PROMPTLY.

 8) WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY, WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.

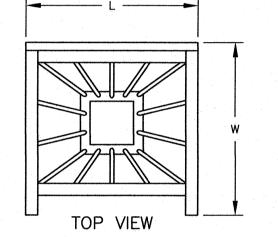
B STABILIZED CONSTRUCTION ENTRANCE

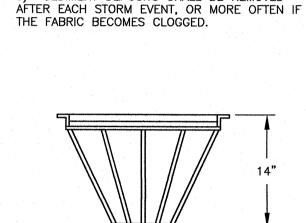
C5 FODS TRACKOUT CONTROL MAT SYSTEM MAY BE SUBSTITUTED











-RAB STRENGTH: 45 LB. MIN. IN ANY

PRINCIPAL DIRECTION (ASTM D1682)

-MULLEN BURST STRENGTH: MIN. 60

psi (ASTM D774)

4) THE FABRIC SHALL HAVE AN OPENING NO

GREATER THAN A NUMBER 20 U.S. STANDARD

SIEVE AND A MINIMUM PERMEABILITY OF 120 gpm/s.f. (MULTIPLY THE PERMITTIVITY IN SEC.-1 FROM ASTM 54491-85 CONSTANT HEAD TEST

5) THE INLET BASKET SHALL BE INSPECTED

WITHIN 24 HOURS AFTER EACH RAINFALL OR

SYSTEM AND/OR CAUSING SURFACE FLOODING.

6) SEDIMENT DEPOSITS SHALL BE REMOVED

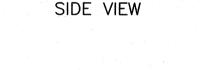
USING THE CONVERSION FACTOR OF 74.)

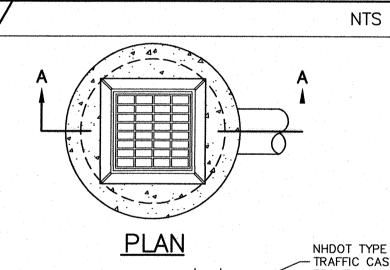
DAILY DURING EXTENDED PERIODS OF

PRECIPITATION. REPAIRS SHALL BE MADE

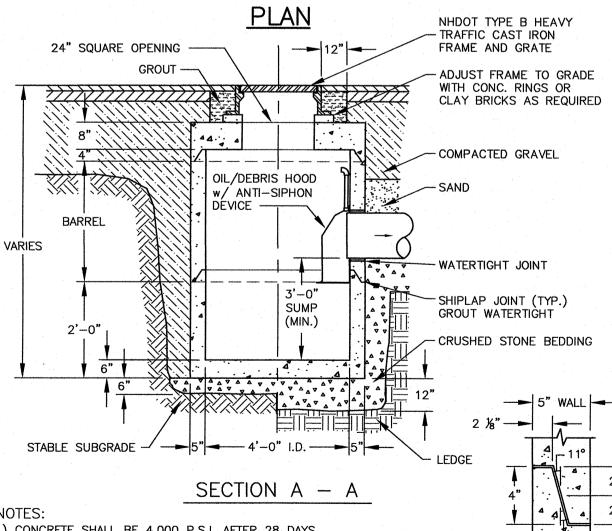
IMMEDIATELY, AS NECESSARY, TO PREVENT

PARTICLES FROM REACHING THE DRAINAGE





CATCH BASIN INLET BASKET



NOTES:

1) CONCRETE SHALL BE 4,000 P.S.I. AFTER 28 DAYS.

2) CIRCUMFERENTIAL REINFORCEMENT SHALL BE 0.12 SQ. IN. PER LINEAR FT. IN ALL SECTIONS & SHALL BE PLACED IN THE CENTER THIRD OF WALL.

CIRCUMFERENTIAL REINFORCEMENT EQUAL TO 0.12 SQ. IN. PER LINEAR FT.
4) EACH CASTING TO HAVE LIFTING HOLES CAST IN.
5) OUTLET HOOD SHALL BE A "SNOUT" BY BEST MANAGEMENT PRODUCTS, INC. OR APPROVED EQUAL. SIZING AND INSTALLATION PER MANUFACTURER'S RECOMMENDATIONS.

3) THE TONGUE OR THE GROOVE OF THE JOINT SHALL CONTAIN ONE LINE OF

D CATCH BASIN w/ OIL-DEBRIS HOOD

(IF NEEDED)

NTS



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3
Portsmouth, N.H. 03801-7114
Tel (603) 430-9282
Fax (603) 436-2315

NOTES

- 1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.
- 2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.
- 3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

BRICK MARKET 60 PENHALLOW STREET PORTSMOUTH, N.H.

1	ISSUED FOR APPROVAL	11/18/19
0	ISSUED FOR COMMENT	10/8/19
NO.	DESCRIPTION	DATE
	REVISIONS	



SCALE: AS SHOWN

DETAIL OF

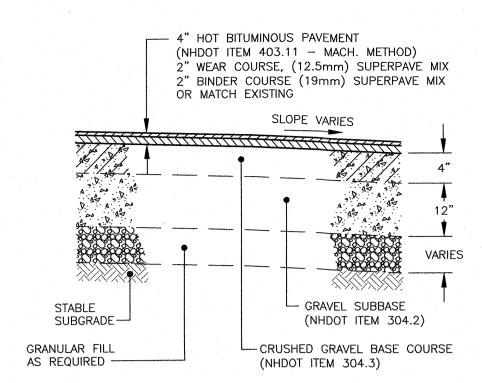
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OCTOBER 2019

DETAILS

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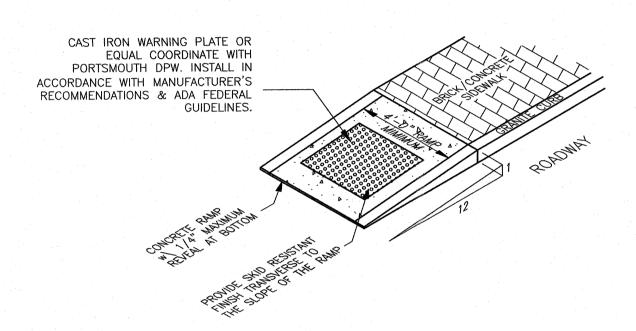
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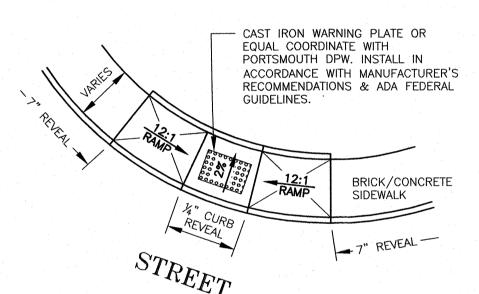


E TYPICAL PAVEMENT CROSS-SECTION

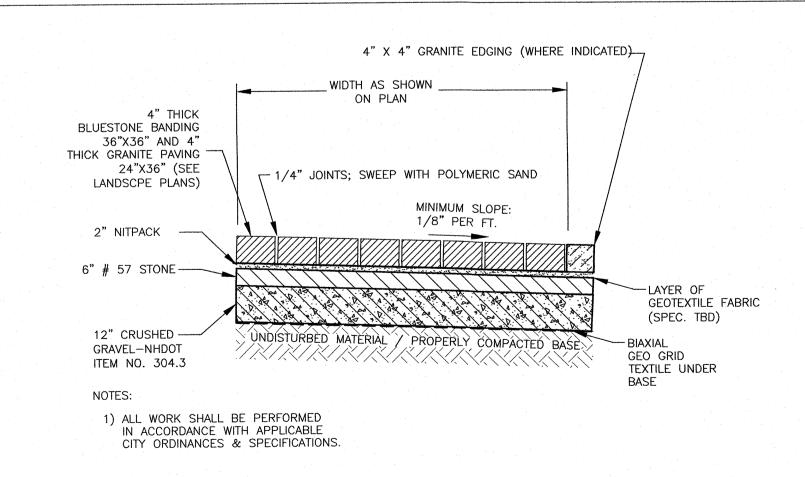
OFF SITE REPAIR AS NEEDED

NTS

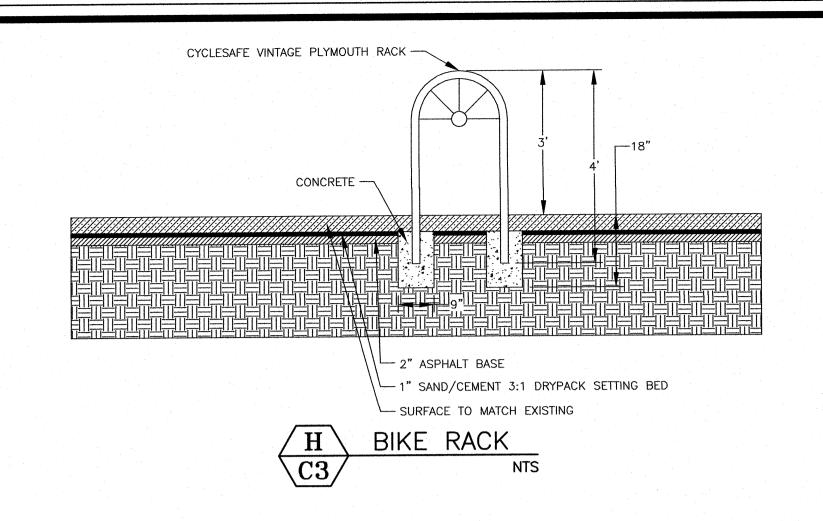












BRICK PAVEMENT NOTES

SCOPE OF WORK:

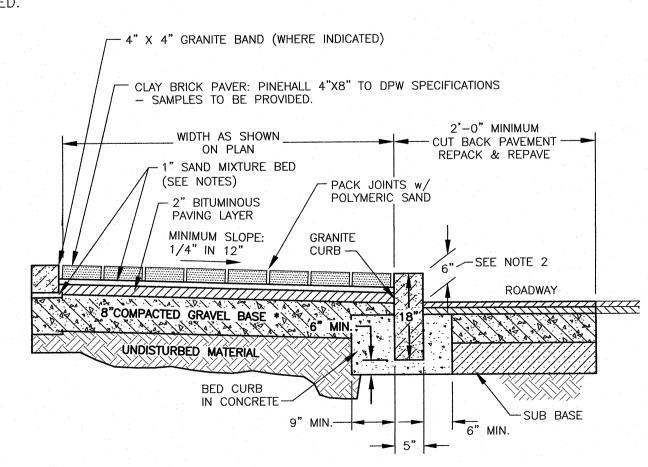
- 1) THE WORK SHALL CONSIST OF CONSTRUCTING/RECONSTRUCTING THE SUB-BASE AND CONSTRUCTING A NEW BRICK SIDEWALK AS DIRECTED IN THE FIELD BY THE ENGINEER AND COORDINATED WITH PORTSMOUTH DPW.
- 2) REVEAL SHALL BE COORDINATED WITH PORTSMOUTH DPW.

METHODS OF CONSTRUCTION:

- A) ALL LABOR AND MATERIALS SHALL CONFORM TO THE STATE OF NEW HAMPSHIRE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, SECTION 608, AND CITY OF PORTSMOUTH SPECIFICATIONS FOR NEW BRICK SIDEWALK, SECTION 6.
- B) ALL BRICKS SHALL CONFORM TO THE REQUIREMENTS OF ASTM STANDARD SPECIFICATIONS FOR BUILDING BRICKS: CLASS SX, TYPE 1, APPLICATION PX. THE BRICKS SHALL BE NO. 1, WIRE CUT TYPE FOR PAVING, WITH A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,000 POUNDS PER SQUARE INCH. THE BRICKS SHALL NOT BE CORED OR HAVE FROGS AND SHALL BE OF A STANDARD SIZE (2.25" X 4 X 8").
- C) EXCAVATION FOR SIDEWALKS SHALL BE AT A DEPTH OF 10 INCHES BELOW FINISH GRADE. IN AREAS NOT BUTTING CURBING OR BUILDINGS, THE EXCAVATION SHALL BE 6 INCHES WIDER THAN THE FINISHED SIDEWALK WIDTH. AT ALL DRIVE CROSSINGS, THE DEPTH OF EXCAVATION SHALL BE INCREASED ACCORDINGLY. THE CONTRACTOR SHALL PROVIDE NEAT AND SQUARE CUTTING OF EXISTING ASPHALT ROAD SURFACE AS NEEDED. ALL UNSUITABLE MATERIAL SHALL BE REMOVED AND DISPOSED OF OFF—SITE AT THE CONTRACTOR'S OWN EXPENSE.
- D) THE BASE MATERIAL SHALL CONSIST OF A MIXTURE OF STONES OR ROCK FRAGMENTS AND PARTICLES WITH 100% PASSING THE 3 INCH SIEVE, 95% TO 100% PASSING THE 2 INCH SIEVE, 55% TO 85% PASSING THE 1 INCH SIEVE, AND 27% TO 52% PASSING THE NO. 4 SIEVE. AT LEAST 50% OF THE MATERIALS RETAINED ON THE 1 INCH SIEVE SHALL HAVE A FRACTURED FACE. THE BASE MATERIAL SHALL BE THOROUGHLY COMPACTED TO THE DEPTH SPECIFIED OR DIRECTED. IN THE WAY OF ALL DRIVE CROSSINGS THE BASE WILL BE INCREASED TO A COMPACTED DEPTH OF 12 INCHES. GRAVEL REQUIREMENTS FOR RECONSTRUCTION WILL BE AS DIRECTED, BASED ON SITE CONDITIONS. THE WORK INCLUDES BACKING UP ANY AND ALL CURB BEING INSTALLED BY OTHERS ON BOTH SIDES.
- E) THE CLAY BRICK PAVERS SHALL BE LAID IN A 1 INCH BED OF A SAND MIXTURE COMPRISED OF: 3 PARTS SAND MIXED WITH 1 PART PORTLAND CEMENT.
- F) THE CONTRACTOR SHALL LAY THE BRICKS SO THAT APPROXIMATELY 4.5 BRICKS SHALL COVER ONE SQUARE FOOT.
- G) THE SIDEWALK SHALL PITCH TOWARDS THE STREET AS SHOWN ON THE GRADING PLAN.
- H) IN AREAS WHERE THE FRONT OF THE BRICK SIDEWALK IS NOT ADJACENT TO GRANITE CURBING, THE CONTRACTOR SHALL INSTALL EDGING TO HOLD THE BRICKS IN PLACE. SUCH EDGING SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
- I) THE CONTRACTOR SHALL SUBMIT A SAMPLE OF THE BRICKS FOR APPROVAL BY THE CITY BEFORE BRICKS ARE INSTALLED.

 CONSTRUCTION NOTE:

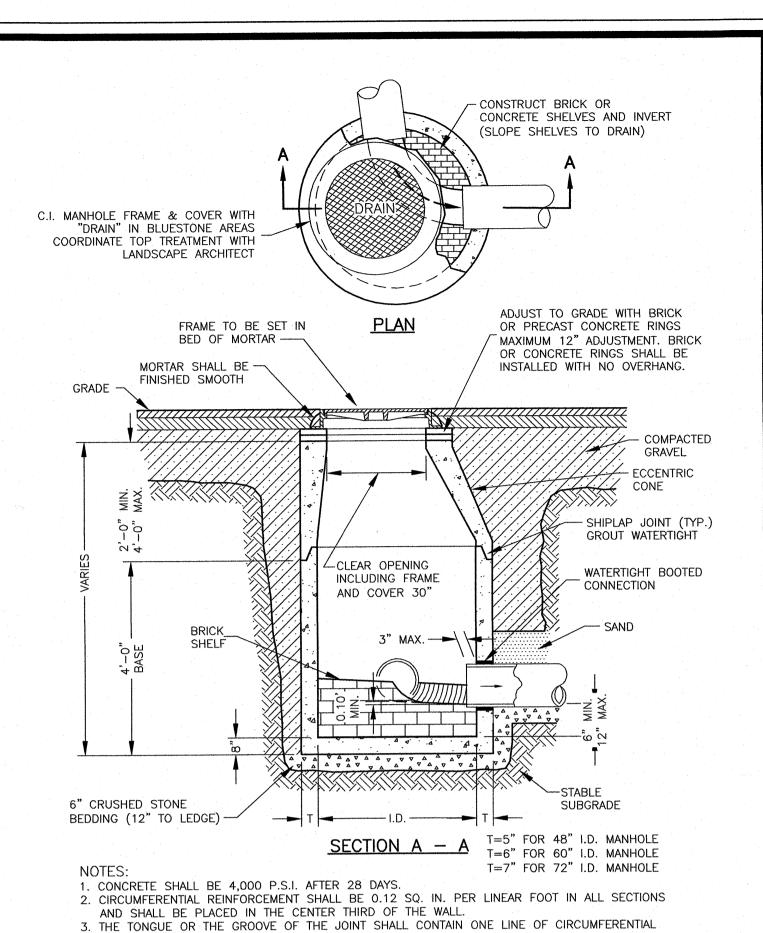
EXISTING GRANITE CURB DISTURBED BY CONSTRUCTION SHALL BE REUSED AND ANY MISSING CURB SHALL BE REPLACED WITH NEW CURB MATCHING EXISTING CURB SIZE. NO CURB LESS THAN 3' IN LENGTH WILL BE ALLOWED.



I BRICK SIDEWALK w/ VERTICAL GRANITE CURB

(STONE DUST BEDDING OVER BITUMINOUS PAVING)

NTS



MIN. LENGTH OF CURB STONES 3FT.
MAX. LENGTH OF CURB STONES 10FT.
MAX. LENGTH OF STRAIGHT CURB STONES

21'
3'
22' - 28'
4'

DRAIN MANHOLE WITH BOOT DETAIL

LAID ON CURVES SEE CHART

NOTE: ADJOINING STONES SHALL HAVE
THE SAME OR APPROXIMATE LENGTH.

REINFORCEMENT EQUAL TO 0.12 SQ. IN. PER LINEAR FOOT.

5. ALL MANHOLES SHALL BE 48" I.D. UNLESS SPECIFIED OTHERWISE ON THE PLANS.

6. MANHOLE SHALL BE DESIGNED AND CONSTRUCTED TO WITHSTAND H-20 LOADING.

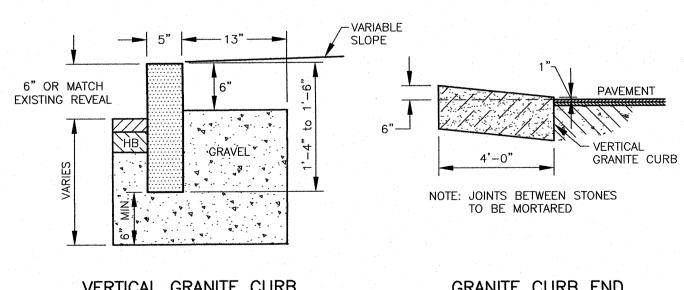
4. EACH CASTING TO HAVE LIFTING HOLES CAST IN.

Radius Max. length

21'
22' - 28'
29' - 35'
36' - 42'
43' - 49'
50' - 56'
57' - 60'
over 60'

Max. length

3'
4'
5'
6'
6'
8'
9'
10'



VERTICAL GRANITE CURB

GRANITE CURB END

NTS

K GRANITE

GRANITE CURBING DETAILS

#

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DETAILS

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FB 402 PG 1

3039

SEWER UTILITY GENERAL NOTES:

- 1) MINIMUM PIPE SIZE FOR COMMERCIAL SERVICE SHALL BE SIX INCHES.
- 2) PIPE AND JOINT MATERIALS: A. PLASTIC SEWER PIPE
 - 1. PIPE AND FITTINGS SHALL CONFORM TO THE FOLLOWING ASTM STANDARDS:

STANDARDS PIPE MATERIAL APPROVED 8" THROUGH 15" (SDR 35) D3034 *PVC (SOLID WALL) 18" THROUGH 27" (T-1 & T-2) F679 PVC (SOLID WALL) 4" THROUGH 18" (T-1 To T-3) F789 PVC (SOLID WALL) F794 PVC (RIBBED WALL) 8" THROUGH 36" AWWA C900 PVC (SOLID WALL) 8" THROUGH 18" *PVC: POLYVINYL CHLORIDE

- 2. JOINT SEALS FOR PVC PIPE SHALL BE OIL RESISTANT COMPRESSION RINGS OF ELASTOMERIC MATERIAL CONFORMING TO ASTM D-3212 AND SHALL BE PUSH-ON BELL AND SPIGOT TYPE.
- B. DUCTILE IRON PIPE, FITTINGS AND JOINTS.
- 1. DUCTILE IRON PIPE AND FITTINGS FOR SEWERS SHALL CONFORM TO THE FOLLOWING STANDARDS OF THE UNITED STATES OF AMERICA STANDARDS INSTITUTE:

A21.50 THICKNESS DESIGN OF DUCTILE IRON PIPE AND WITH ASTM A-536 DUCTILE IRON CASTINGS.

A21.51 DUCTILE IRON PIPE, CENTRIFUGALLY CAST IN METAL MOULDS OR SAND LINED MOULDS FOR SEWER APPLICATIONS.

2. JOINTS SHALL BE OF THE MECHANICAL OR PUSH ON TYPE. JOINTS AND GASKETS SHALL CONFORM TO:

A21.11 RUBBER GASKET JOINTS FOR CAST IRON PRESSURE PIPE & FITTINGS.

3) DAMAGED PIPE SHALL BE REJECTED AND REMOVED FROM THE JOB SITE.

4) JOINTS SHALL BE DEPENDENT UPON A NEOPRENE OR ELASTOMERIC GASKET FOR WATER TIGHTNESS. ALL JOINTS SHALL BE PROPERLY MATCHED WITH THE PIPE MATERIALS USED. WHERE DIFFERING MATERIALS ARE TO BE CONNECTED, AS AT THE STREET SEWER WYE OR AT THE FOUNDATION WALL, APPROPRIATE MANUFACTURED ADAPTERS SHALL BE USED.

5) TEES AND WYES: WHERE A TEE OR WYE IS NOT AVAILABLE IN THE EXISTING STREET SEWER, AN APPROPRIATE CONNECTION SHALL BE MADE DEPENDING ON THE PIPE ENCOUNTERED, FOR PVC PIPE, USE PVC SADDLES OR INSERT—A—TEE, OR CUT IN A SANITARY TEE. FOR CLAY PIPE, USE INSERT—A—TEE OR CUT IN A SANITARY TEE. ALL WORK TO BE APPROVED BY GOVERNING BODY.

WITH INSTALLATION GUIDES OF THE APPROPRIATE MANUFACTURER. IT SHALL BE CAREFULLY BEDDED ON A 4 INCH LAYER OF CRUSHED STONE AND/OR GRAVEL AS SPECIFIED IN NOTE 10. BEDDING AND REFILL FOR DEPTH OF 12 INCHES ABOVE THE TOP OF THE PIPE SHALL BE CAREFULLY AND THOROUGHLY TAMPED BY HAND OR WITH APPROPRIATE MECHANICAL DEVICES.

6) HOUSE SEWER INSTALLATION: THE PIPE SHALL BE HANDLED, PLACED AND JOINTED IN ACCORDANCE

THE PIPE SHALL BE LAID AT A CONTINUOUS AND CONSTANT GRADE FROM THE STREET SEWER CONNECTION TO THE FOUNDATION AT A GRADE OF NOT LESS THAN 1/4 INCH PER FOOT. PIPE JOINTS MUST BE MADE UNDER DRY CONDITIONS. IF WATER IS PRESENT, ALL NECESSARY STEPS SHALL BE TAKEN TO DEWATER THE TRENCH.

7) TESTING: WHEN REQUIRED BY THE GOVERNING AUTHORITY, TESTING SHALL CONFORM TO ENV-WQ

8) ILLEGAL CONNECTIONS: NOTHING BUT SANITARY WASTE FLOW FROM DWELLING TOILETS, SINKS, LAUNDRY ETC. SHALL BE PERMITTED. ROOF LEADERS, FOOTING DRAINS, SUMP PUMPS OR OTHER SIMILAR CONNECTIONS CARRYING RAIN WATER, DRAINAGE OR GROUND WATER SHALL NOT BE PERMITTED.

9) WATER SERVICE SHALL NOT BE LAID IN SAME TRENCH AS SEWER SERVICE, UNLESS IT IS ON A SHELF 12" HIGHER, AND 18" APART.

10) BEDDING: SCREENED GRAVEL AND/OR CRUSHED STONE, FREE FROM CLAY, LOAM, ORGANIC MATTER AND MEETING ASTM C33 STONE SIZE NO. 67.

100% PASSING 1 INCH SCREEN
90%-100% PASSING 3/4 INCH SCREEN
20%- 55% PASSING 3/8 INCH SCREEN
0%- 10% PASSING #4 SIEVE
0%- 5% PASSING #8 SIEVE

WHERE ORDERED BY THE ENGINEER TO STABILIZE THE TRENCH BASE, GRADED SCREENED GRAVEL OR CRUSHED STONE 1/2 INCH TO 1-1/2 INCH SHALL BE USED.

11) LOCATION: THE LOCATION OF THE TEE OR WYE SHALL BE RECORDED AND FILED IN THE MUNICIPAL RECORDS. IN ADDITION, A FERROUS METAL ROD OR PIPE SHALL BE PLACED OVER THE TEE OR WYE AS DESCRIBED IN THE TYPICAL "CHIMNEY" DETAIL, TO AID IN LOCATING THE BURIED PIPE WITH A DIP NEEDLE OR PIPE FINDER.

12) CAST—IN—PLACE CONCRETE: SHALL CONFORM TO THE REQUIREMENTS FOR CLASS A (3000 PSI) CONCRETE OF THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS AS FOLLOWS:

CEMENT: 6.0 BAGS PER CUBIC YARD WATER: 5.75 GALLONS PER BAG OF CEMENT MAXIMUM AGGREGATE SIZE: 3/4 INCH

13) CHIMNEYS: IF VERTICAL DROP INTO SEWER IS GREATER THAN 4 FEET, A CHIMNEY SHALL BE CONSTRUCTED FOR THE HOUSE CONNECTION. CHIMNEY INSTALLATION AS RECOMMENDED BY THE PIPE MANUFACTURER MAY BE USED IF APPROVED BY THE ENGINEER.

14) BACKFILL UP TO SUBBASE GRAVEL SHALL BE WITH EXCAVATED SOIL FROM TRENCHING OPERATIONS. COMPACT IN 8" LIFTS WITH VIBRATORY PLATE COMPACTORS TO 90% OF MODIFIED PROCTOR DENSITY. IF FINE—GRAINED, COMPACT WITH POGO STICKS OR SHEEPSFOOT ROLLERS. PLACE NO LARGE ROCKS WITHIN 24" OF PIPE. TRENCHES THAT ARE NOT ADEQUATELY COMPACTED SHALL BE RE—EXCAVATED AND BACKFILLED UNDER THE SUPERVISION OF THE DESIGN ENGINEER OR GOVERNING BODY. UNSUITABLE BACKFILL MATERIAL INCLUDES CHUNKS OF PAVEMENT, TOPSOIL, ROCKS OVER 6" IN SIZE, MUCK, PEAT OR PIECES OF PAVEMENT.

15) THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB—SITE SAFETY AND COMPLIANCE WITH GOVERNING REGULATIONS.

16) ORDERED EXCAVATION OF UNSUITABLE MATERIAL BELOW GRADE. REFILL WITH BEDDING MATERIAL. FOR TRENCH WIDTH SEE TRENCH DETAIL.

17) SAND BLANKET: CLEAN SAND, FREE FROM ORGANIC MATTER, SO GRADED THAT 90% — 100% PASSES A 1/2 INCH SIEVE AND NOT MORE THAN 15% WILL PASS A #200 SIEVE. BLANKET MAY BE OMITTED FOR DUCTILE IRON AND REINFORCED CONCRETE PIPE PROVIDED THAT NO STONE LARGER THAN 2 INCHES IS IN CONTACT WITH THE PIPE.

18) BASE COURSE GRAVEL, IF ORDERED BY THE ENGINEER, SHALL MEET THE REQUIREMENTS OF DIVISION 300 OF THE LATEST EDITION OF THE:
"STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION

OF THE STATE OF NEW HAMPSHIRE, DEPARTMENT OF TRANSPORTATION".

19) FOR CROSS COUNTRY CONSTRUCTION, BACKFILL OR FILL SHALL BE MOUNDED TO A HEIGHT OF 6 INCHES ABOVE THE ORIGINAL GROUND SURFACE.

20) IF FULL ENCASEMENT IS UTILIZED, DEPTH OF CONCRETE BELOW PIPE SHALL BE 1/4 I.D. (4" MIN.) BLOCK SUPPORT SHALL BE SOLID CONCRETE BLOCKS.

21) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

22) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION.

23) THE PURPOSE OF THIS PLAN IS TO SHOW STANDARDS FOR SEWER CONSTRUCTION.

24) ALL WORK SHALL BE IN COMPLIANCE WITH NHDES CODE OF ADMINISTRATIVE RULES PART ENV—WQ 704 DESIGN OF SEWERS.



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3
Portsmouth, N.H. 03801-7114

NOTES:

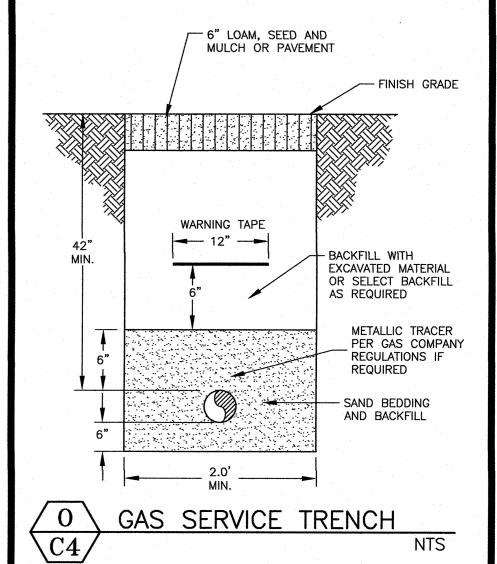
1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

Tel (603) 430-9282

Fax (603) 436-2315

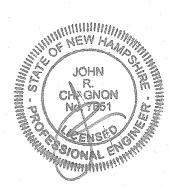
2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).



BRICK MARKET 60 PENHALLOW STREET PORTSMOUTH, N.H.

1	ISSUED FOR APPROVAL	11/18/19
- 0	ISSUED FOR COMMENT	10/8/19
NO.	DESCRIPTION	DATE
	REVISIONS	



SCALE: AS SHOWN

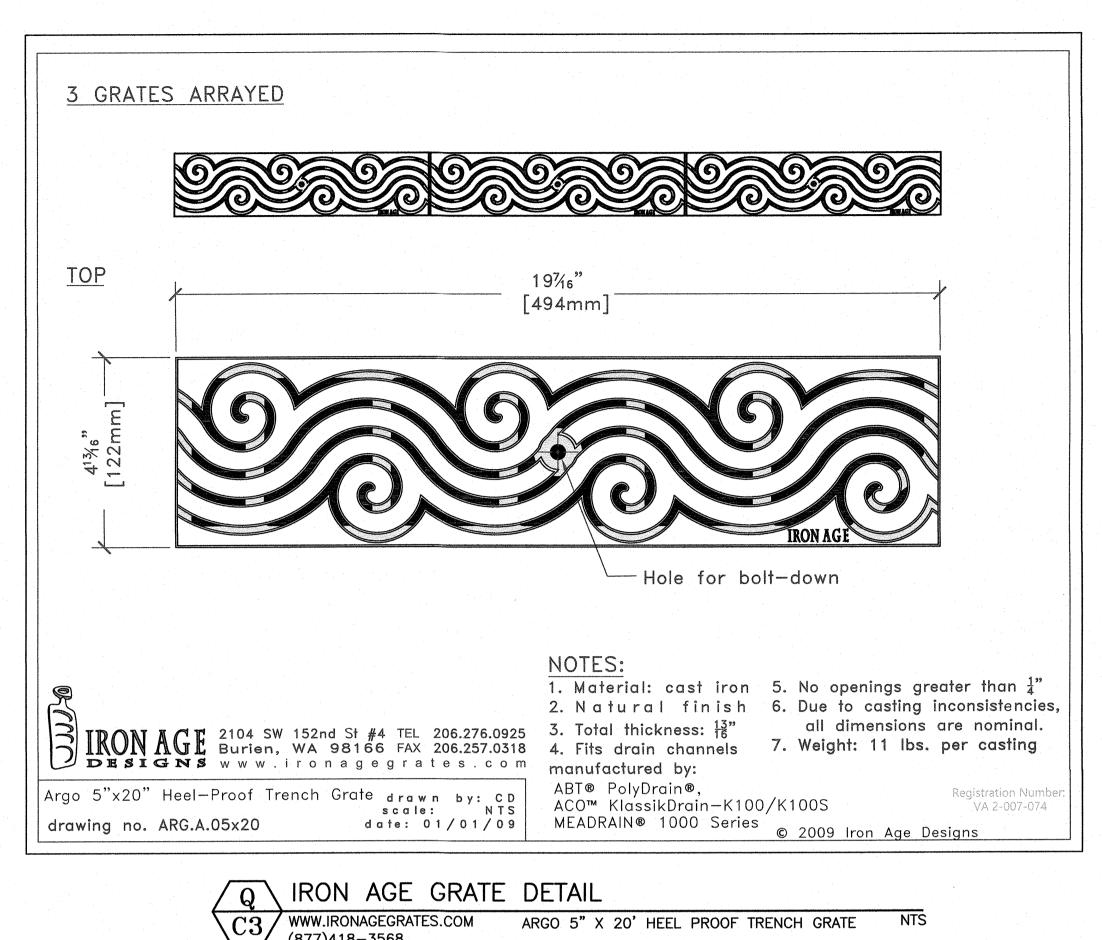
OCTOBER 2019

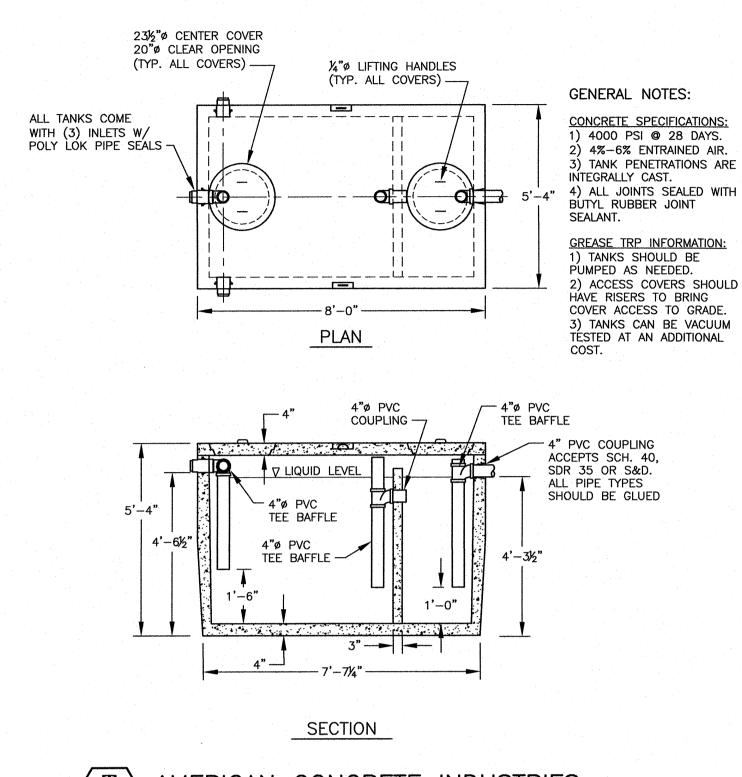
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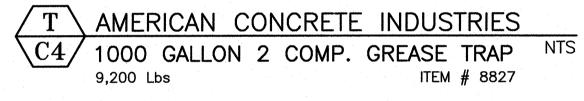
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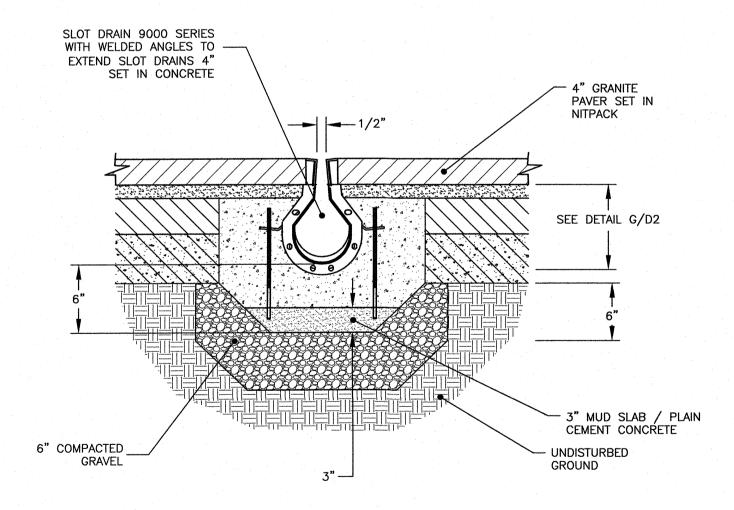
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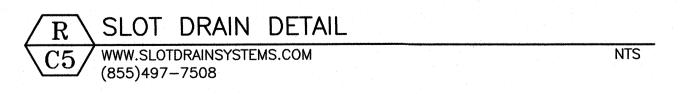


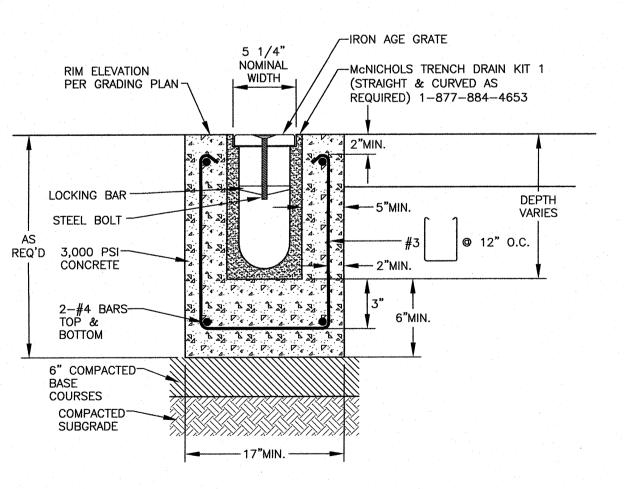






(877)418-3568





TRENCH DRAIN DETAIL AT FOUNTAIN AND BENCH

AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

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BRICK MARKET 60 PENHALLOW STREET PORTSMOUTH, N.H.

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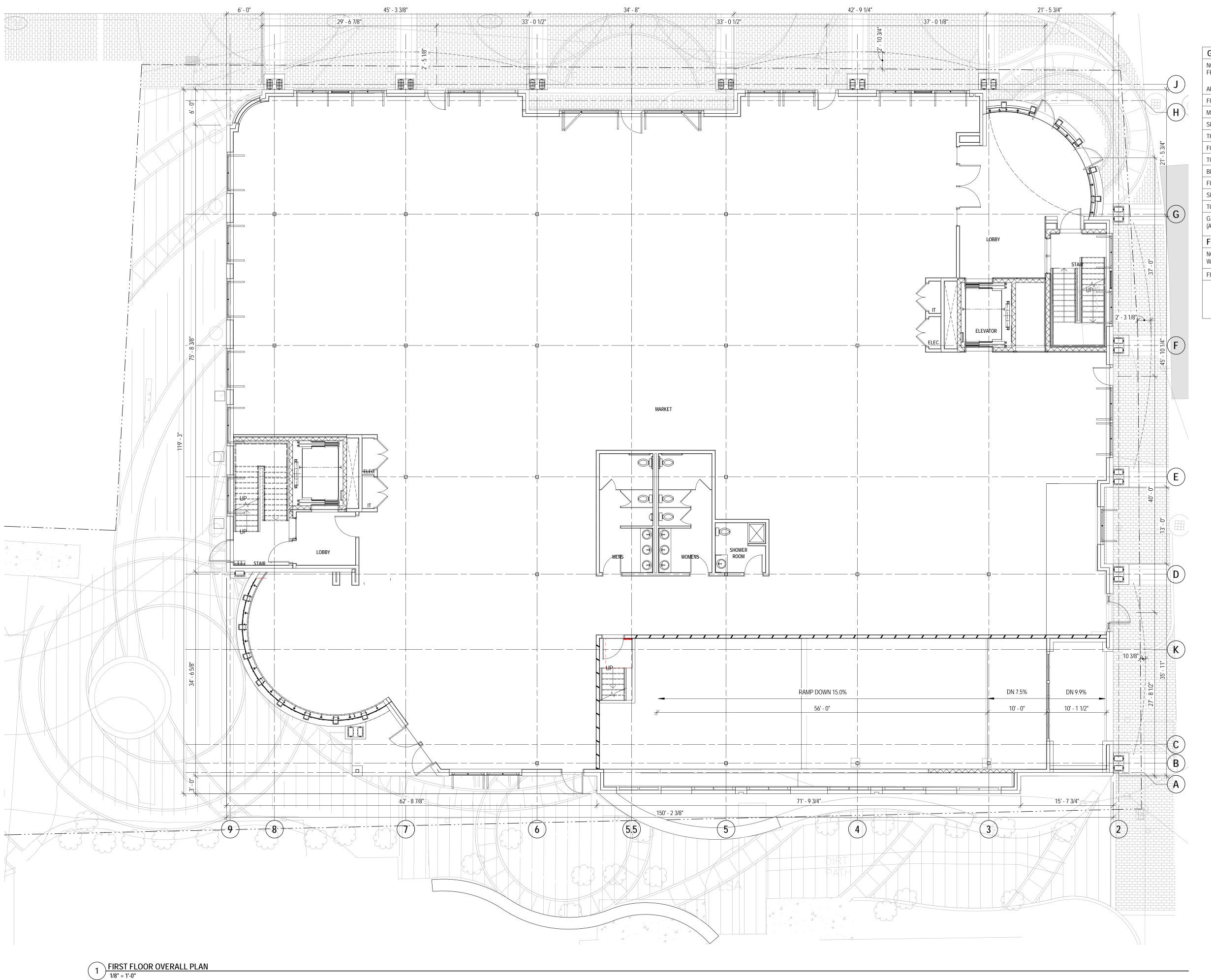


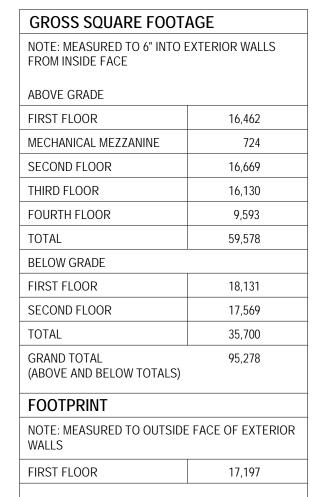
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OCTOBER 2019

DETAILS

FB 402 PG 1







273 CORPORATE DRIVE PORTSMOUTH, NH 03801 T 603.436.2551 F 603.436.6973 www.jsainc.com

60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

 Scale:
 1/8" = 1'-0"

 Date:
 11/18/2019

 Project Number:
 P19081.02

REVISIONS

NO. DESCRIPTION DATE

TAC SUBMISSION

FIRST FLOOR PLAN

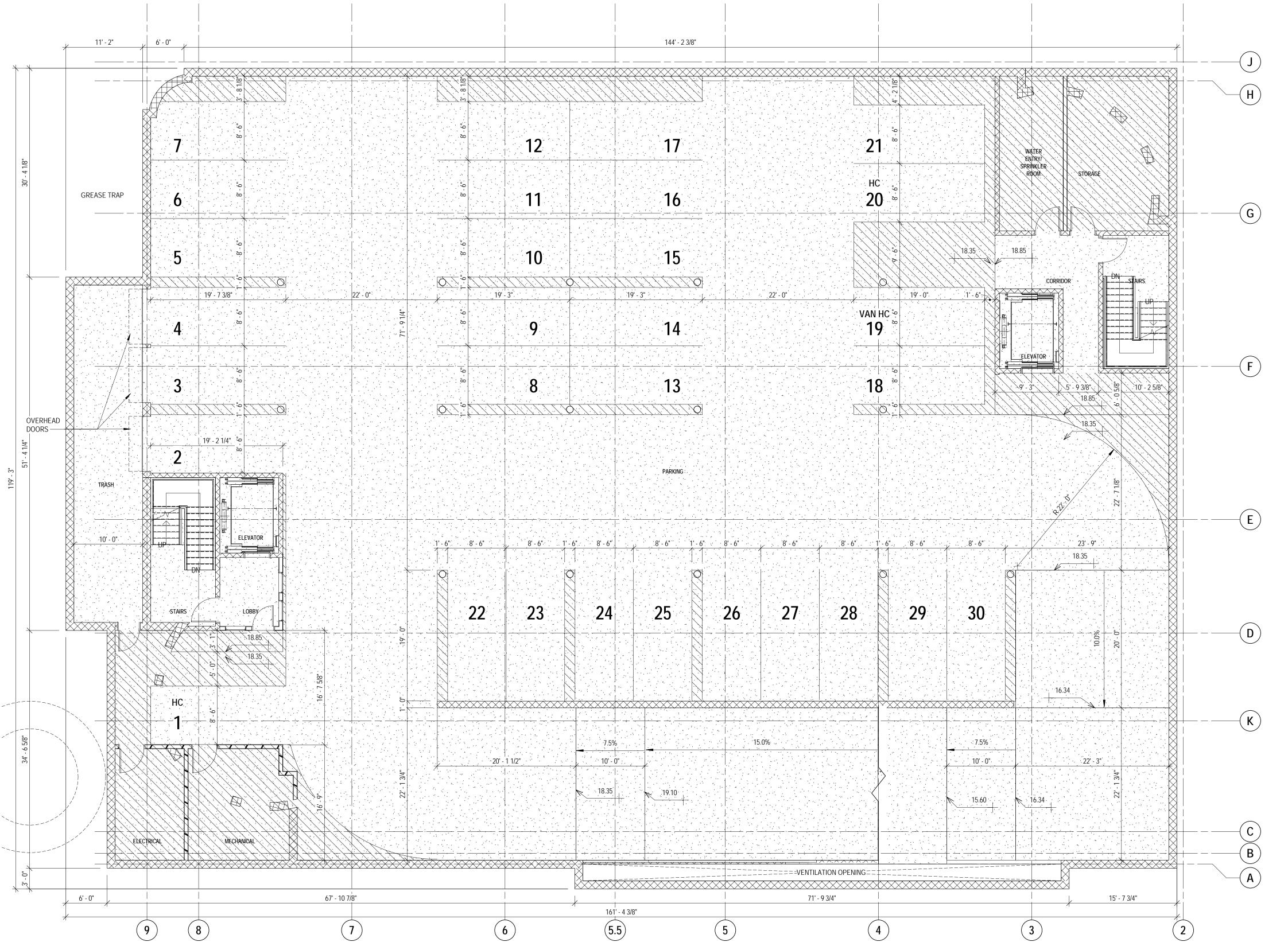
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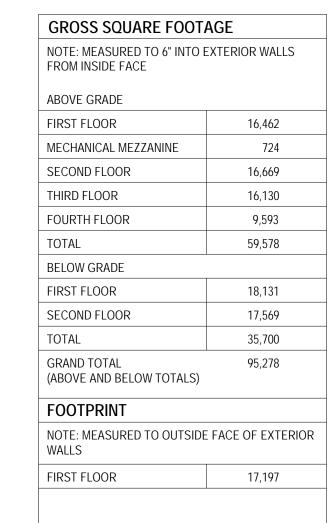
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GRAPHIC SCALE: 1/8" = 1'-0"

0' 4' 8'

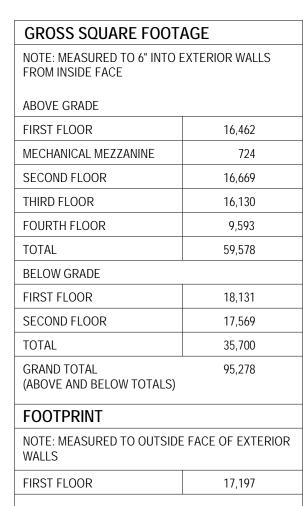
1 PARKING LEVEL 1 PLAN
1/8" = 1'-0"





GRAPHIC SCALE: 1/8" = 1'-0"

0' 4' 8'



60 PENHALLOW STREET at BRICK MARKET

ARCHITECTS I N T E R I O R S

PLANNERS

273 CORPORATE DRIVE

PORTSMOUTH, NH 03801

T 603.436.2551 F 603.436.6973

www.jsainc.com

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

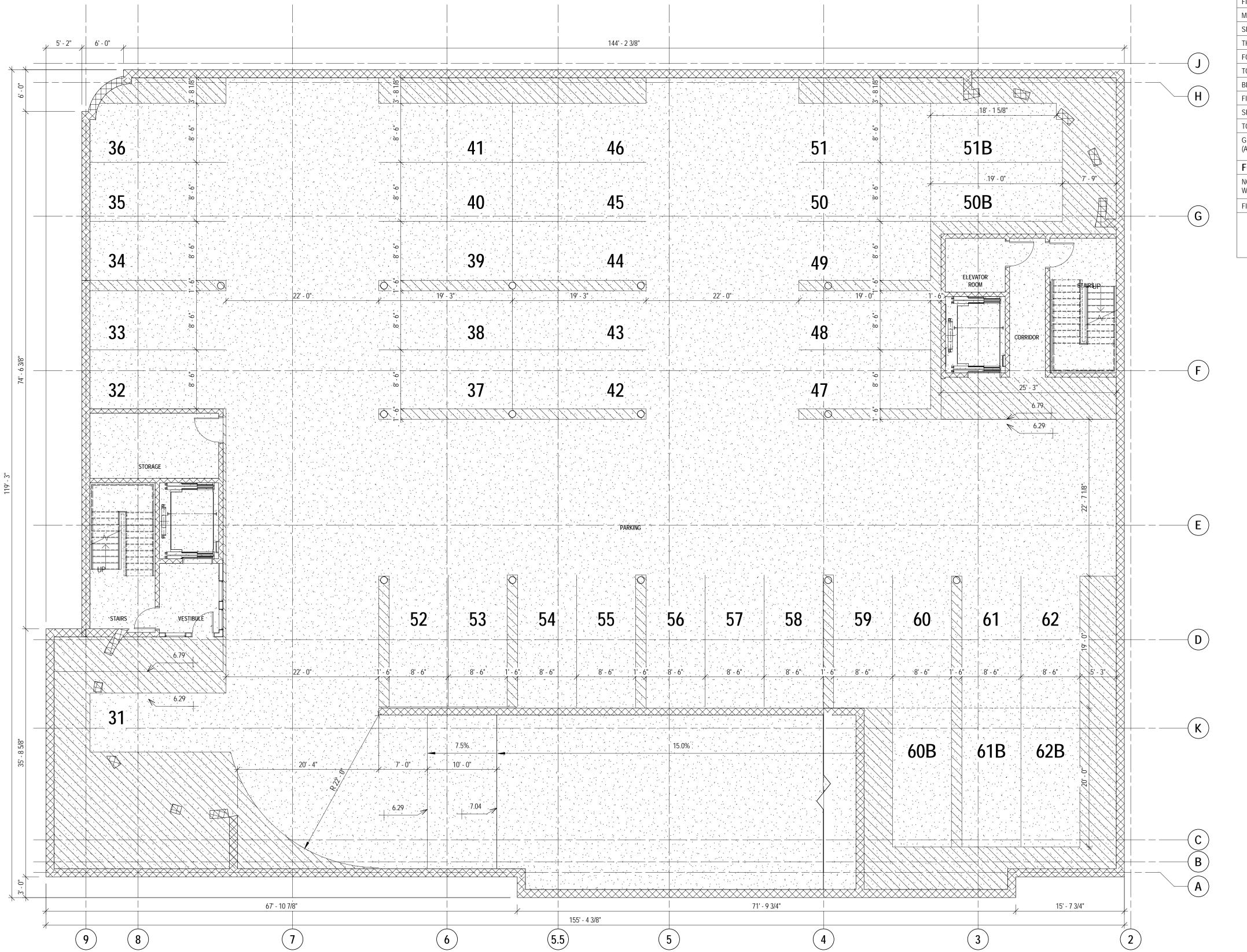
1/8" = 1'-0" Date: 11/18/2019 Project Number: P19081.02

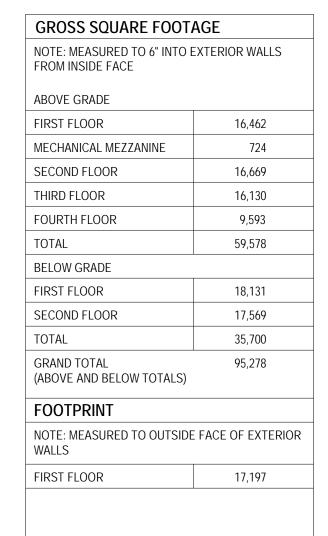
REVISIONS NO. DESCRIPTION DATE

TAC SUBMISSION

PARKING LEVEL 1 PLAN

1 PARKING LEVEL 2 PLAN
1/8" = 1'-0"





GRAPHIC SCALE: 1/8" = 1'-0"
0' 4' 8'

A R C H I T E C T S
I N T E R I O R S
P L A N N E R S

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PORTSMOUTH, NH 03801
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F 603.436.6973
www.jsainc.com

18,131
17,569

60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

 Scale:
 1/8" = 1'-0"

 Date:
 11/18/2019

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 P19081.02

REVISIONS

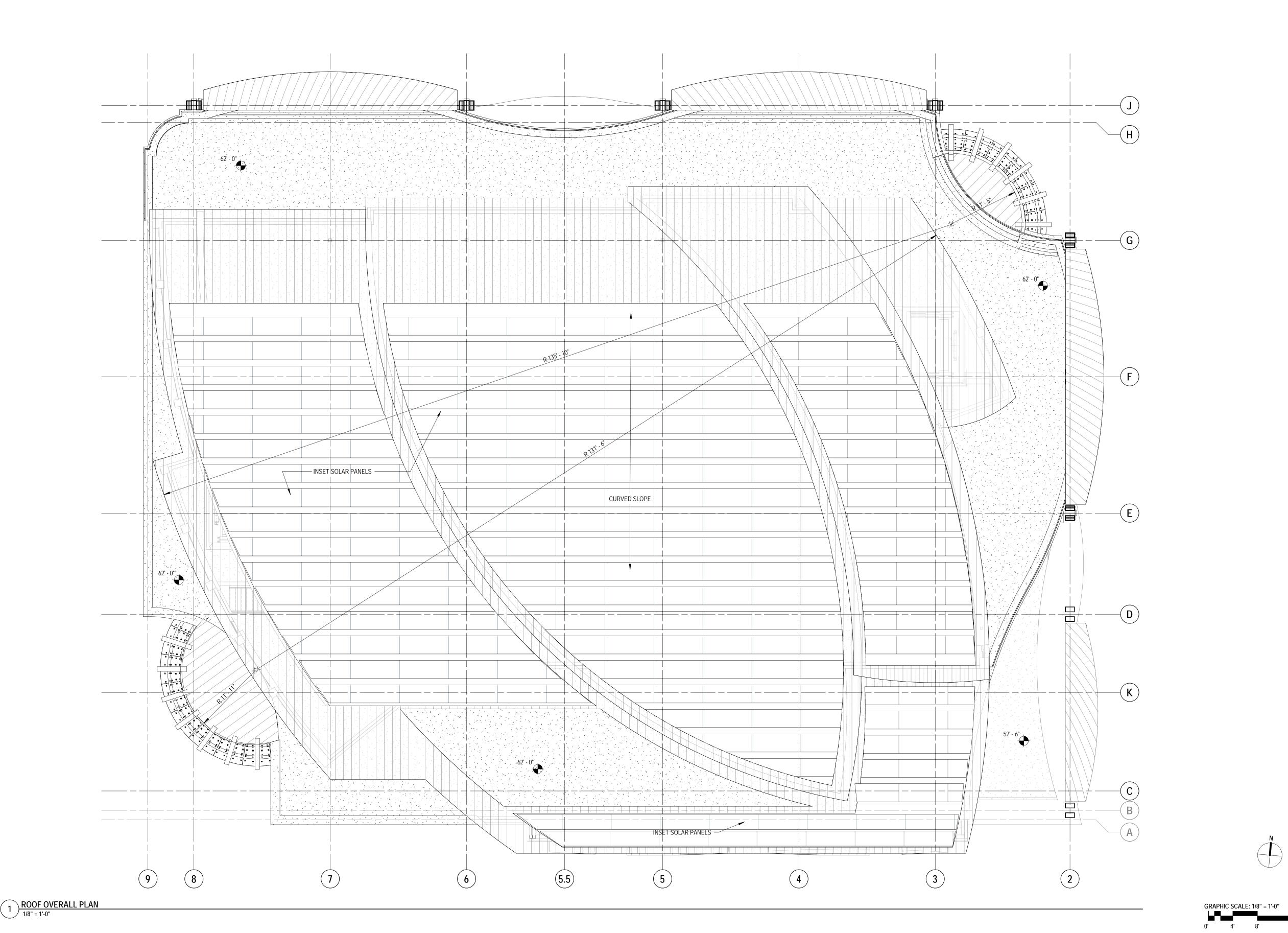
NO. DESCRIPTION DATE

TAC SUBMISSION

PARKING LEVEL 2 PLAN

A0.01B

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60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

 Scale:
 1/8" = 1'-0"

 Date:
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REVISIONS

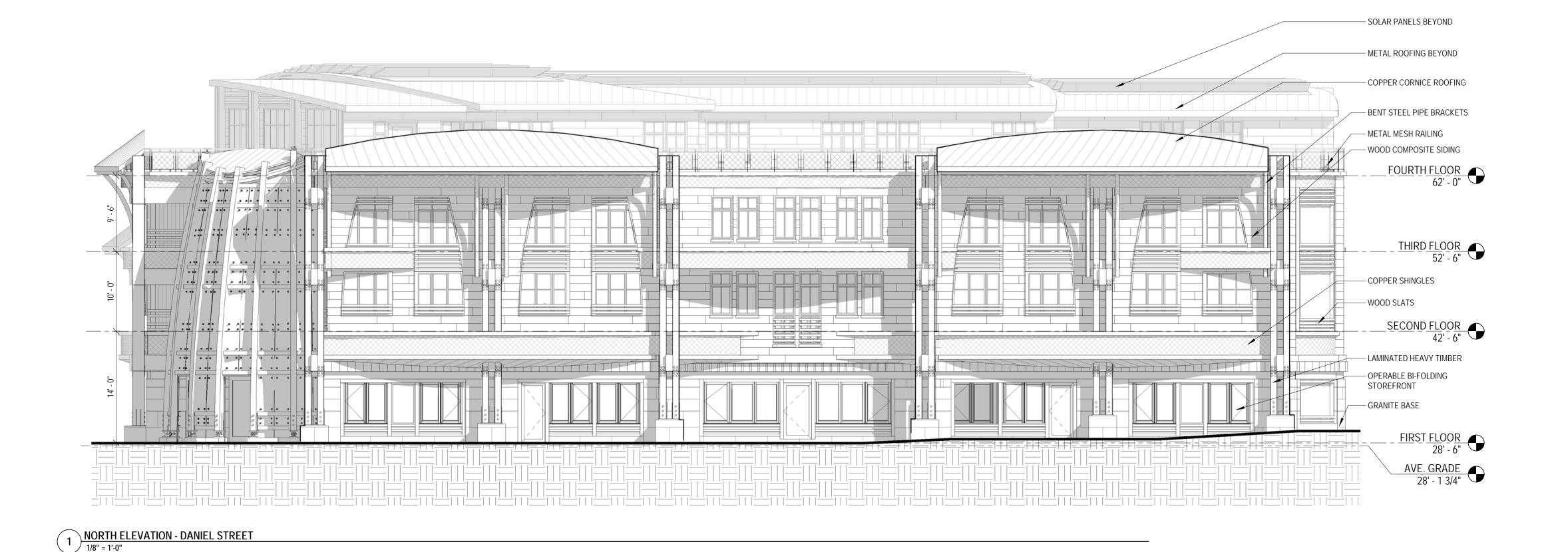
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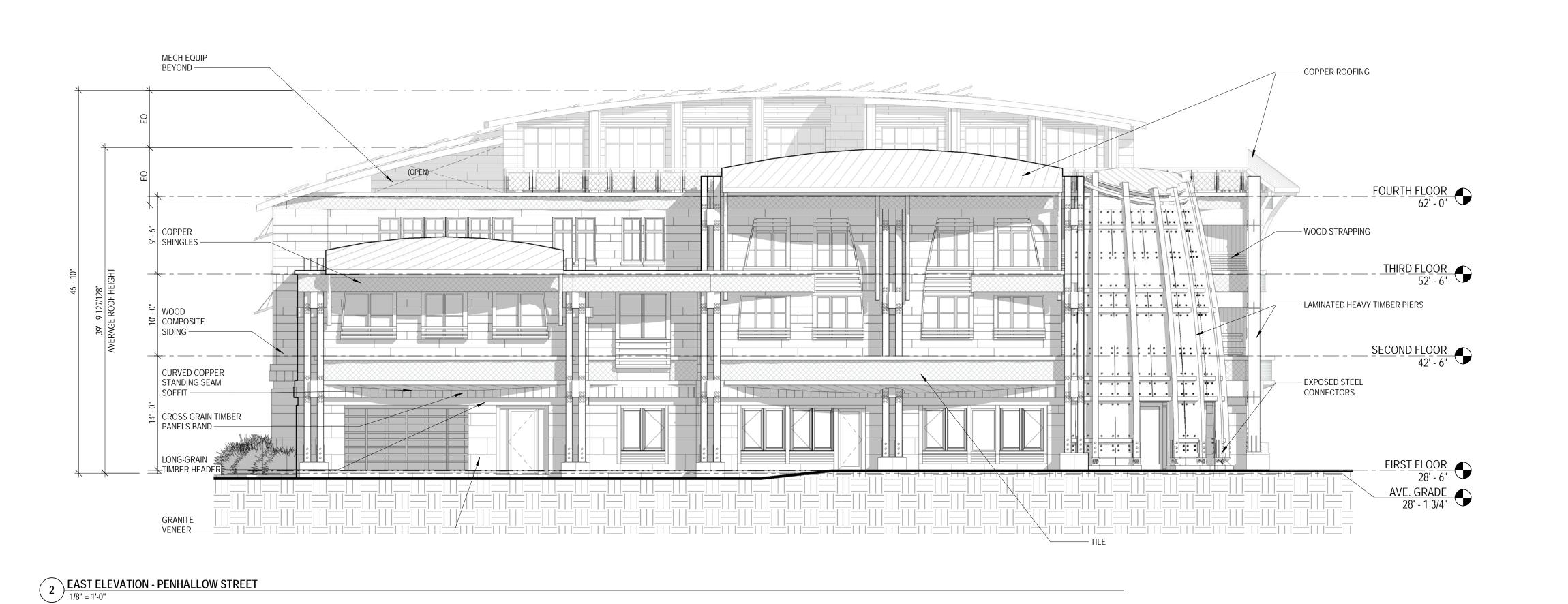
TAC SUBMISSION

ROOF PLAN

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JSA ARCHITECTS INTERIORS

P L A N N E R S

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www.jsainc.com

60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

 Scale:
 1/8" = 1'-0"

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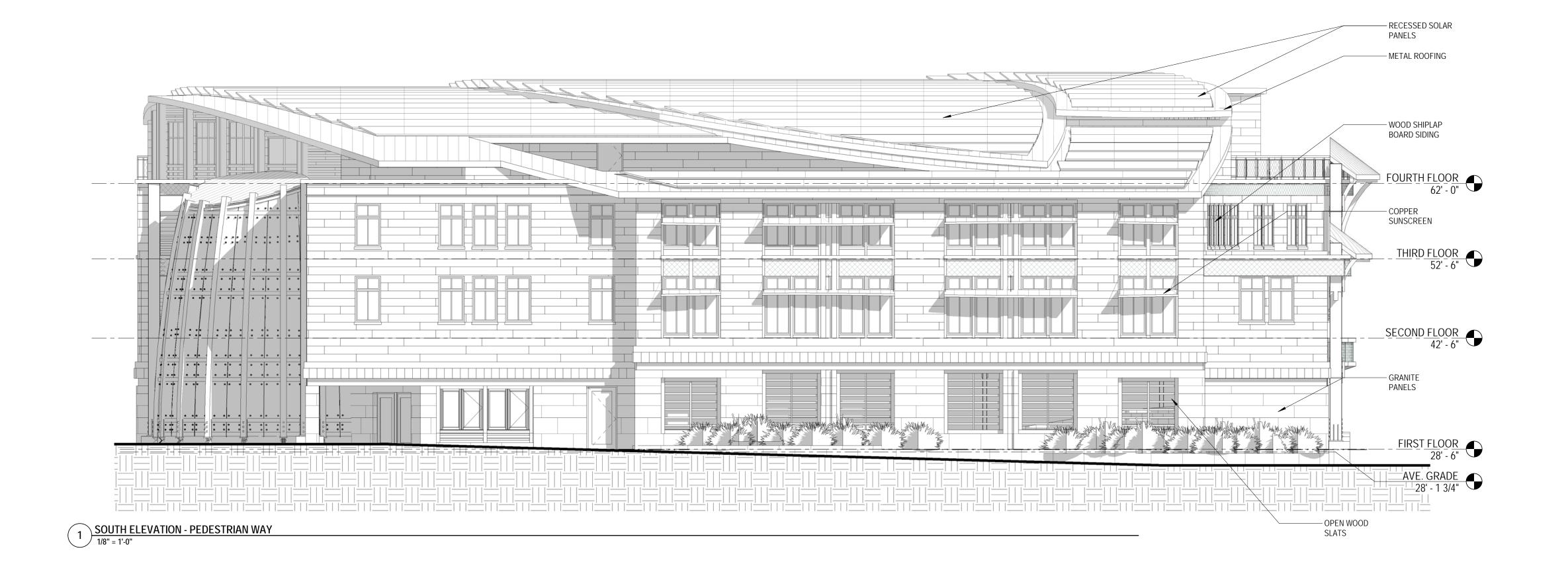
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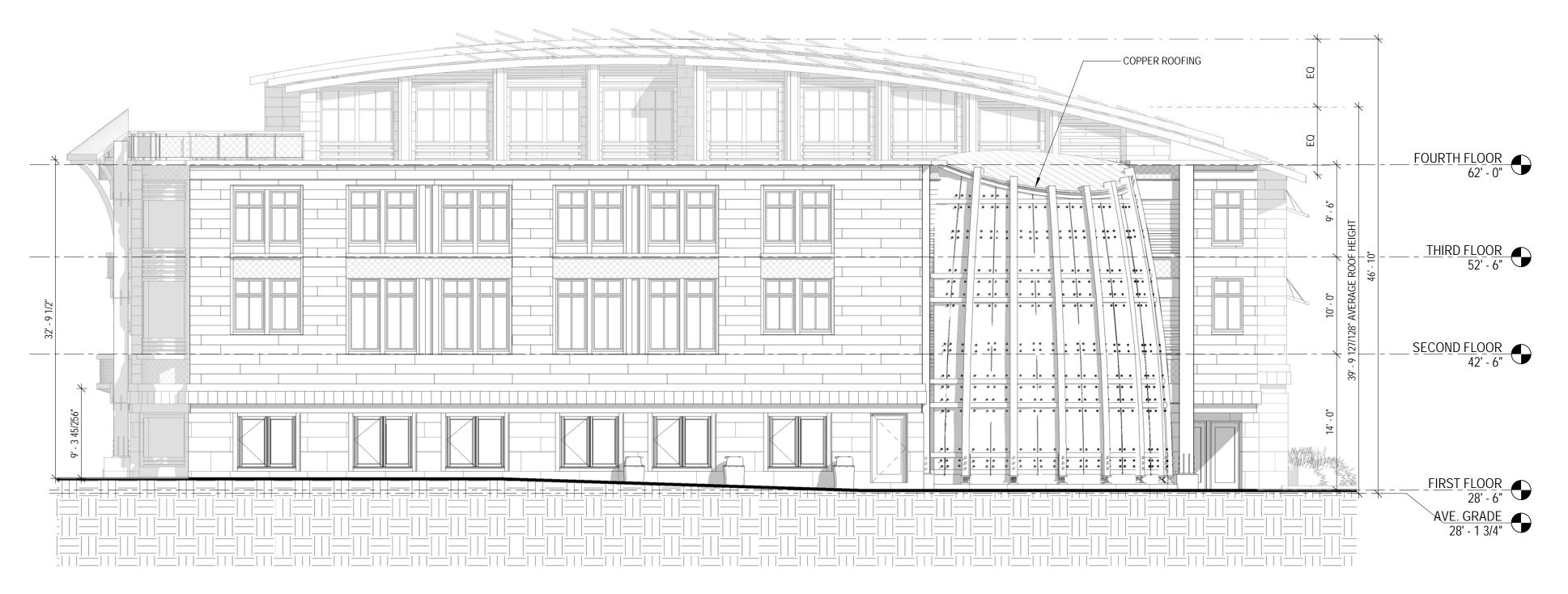
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TAC SUBMISSION

EXTERIOR ELEVATIONS

A0.03





2 WEST ELEVATION - PEDESTRIAN WAY
1/8" = 1'-0"



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60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

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 1/8" = 1'-0"

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 11/18/2019

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REVISIONS

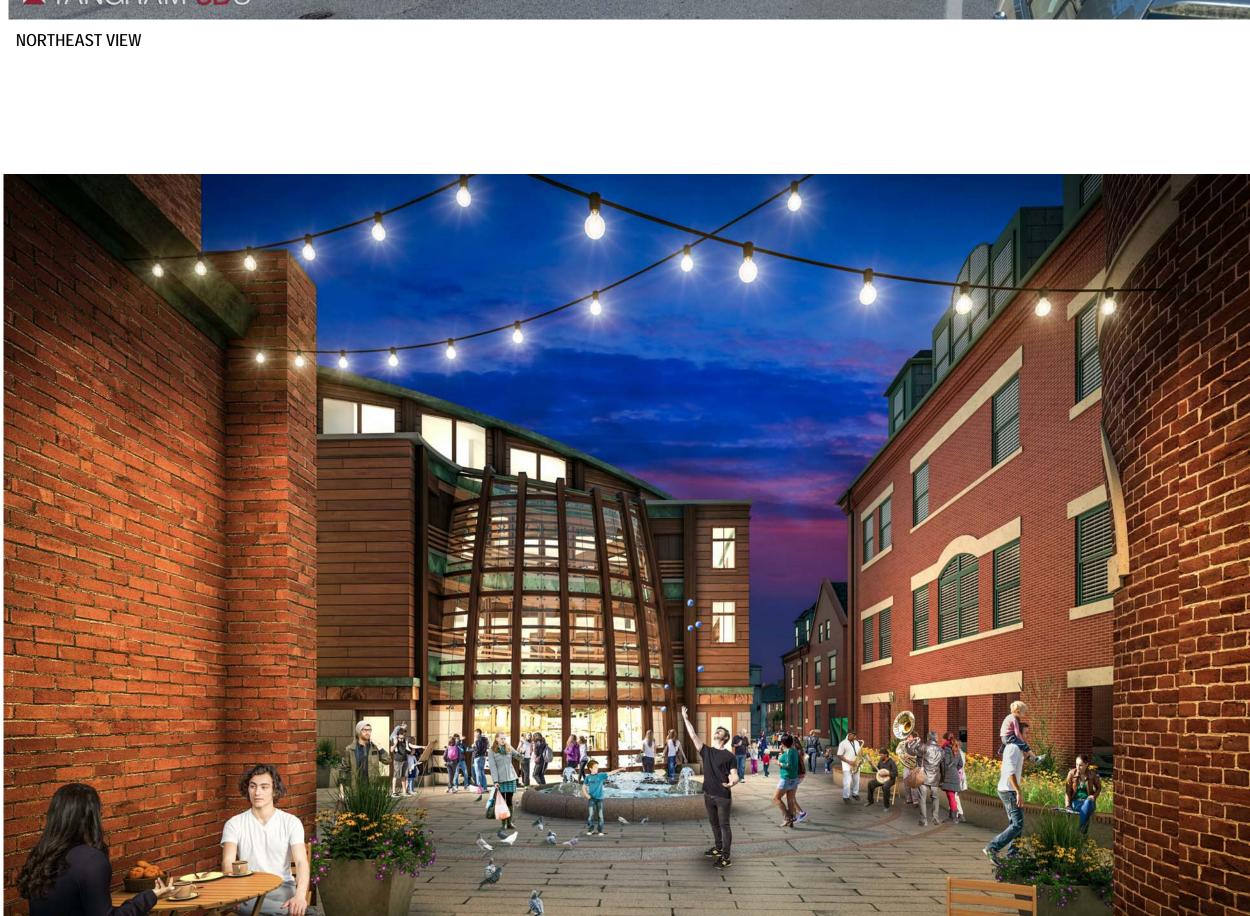
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TAC SUBMISSION

EXTERIOR ELEVATIONS

A0.04





SOUTHWEST VIEW



NORTHWEST VIEW



SOUTHEAST VIEW



ARCHITECT. INTERIOR. PIANNER.

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60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

Scale:

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REVISIONS

NO. DESCRIPTION DATE

TAC SUBMISSION

TANGRAM RENDERINGS

A0.05



0 Daniel Street (60 Penhallow Street) Site Plan Review 11-18-2019 Green Building Statement

WATER

- Protect water quality Eliminate surface parking lot.
- Conserve Water -- Target 30% reduction in fixtures water use over building code, meeting EPACT 2005.

ENERGY

- Conserve Energy -- Target 50% Energy Use Index (EUI) Reduction over code compliance (IECC2015) in each building. Use early energy modeling to analyze effective scenarios. Provide high performance thermal envelope. Achieve Energy Star certification and associated rebates. Use Heat Recovery for ventilation. Commission energy using systems. LED lighting throughout.
- Renewable Energy Rooftop Solar Photovoltaic system for portion of building's energy needs.
- Building Performance -- Use industry tools to annually monitor and benchmark buildings. Train staff on proper building operation with comprehensive Facilities Staff Training and Systems Manuals.
- Reduce Low level ozone (smog) -- Provide safe and secure bicycle storage. Use only low-VOC products for construction and operation.

MATERIALS & RESOURCES

- Minimize waste (during construction and operation)
- Use regional, renewable, low carbon footprint materials

INDOOR ENVIRONMENTAL QUALITY

- Thermal comfort -- Meet ASHRAE 55 Thermal Comfort Code. Address thermal envelope per above. Provide multiple zones of heating and cooling in each apartment.
- Indoor air quality (before and during occupancy) -- MEET ASHRAE 62 Ventilation Code in all occupied spaces. MEET LEED IEQ credit requirements.
- Views / connection to outdoors -- Provide views to outdoors for every regularly occupied space.
- Daylighting -- Achieve Daylight Factor of 2% minimum for every regularly occupied space.
- Individual controls (light, heat etc...) -- Provide individual controls for temperature and lighting.



November 18, 2019

MAX-2019184.00

Mr. Eric B. Eby, P.E. Department of Public Works City of Portsmouth 680 Peverly Hill Road Portsmouth, New Hampshire 03801

SUBJECT: Trip Generation Summary Letter

#60 Penhallow Street - Portsmouth, NH

Dear Mr. Eby:

Greenman-Pedersen, Inc. (GPI) is in the process of preparing a *Traffic Impact and Access Study (TIAS)* for a proposed mixed-use development, referred to as Brick Market, to located at #60 Penhallow Street in Portsmouth, New Hampshire. The existing site is currently a public 50-space parking lot with a single full-access / egress curb cut on Penhallow Street. The project consists of constructing a mixed-use development with a ±16,800 square foot (SF) of fast-food and fast-casual restaurant space on the first floor and ±41,600 SF of general office space on the second through fourth floors. The existing driveway will remain at Penhallow Street but be modified to provide garage access below street level. The site location in relation to the surrounding roadways is shown on the Project Location Map in Figure 1.

GPI and the Applicant met with representatives of the City of Portsmouth Planning Department on October 28, 2019 to review the scope of the TIAS. During this meeting, the Planning Department requested that the trip generation and distribution assumptions of the Project be provided to the City's Parking and Transportation Engineer for review prior to finalizing the TIAS. This letter is intended to provide a summary of the project-generated trips, mode split, and the corresponding trip distribution for review. A more comprehensive TIAS will be provided once the trip generation and distribution assumptions have been approved.

#60 Penhallow Street - Portsmouth, NH



Greenman-Pedersen, Inc. 181 Ballardvale Street Suite 202 Wilmington, MA 01887

Figure 1
Site Location Map

Mr. Eric B. Eby November 18, 2019 Page 3

TRIP GENERATION

The Project is proposed to consist of the construction of approximately 41,600 SF of office space and 16,800 SF of restaurant space. The restaurant space will be a food-court style restaurant, similar to Faneuil Hall in Boston, with a mix of fast-food and fast-casual dining options with shared seating. To estimate the volume of traffic to be generated by the proposed redevelopment, trip-generation rates published by the Institute of Transportation Engineers (ITE) *Trip Generation Manual* were utilized for Land Use Code (LUC) 710 (General Office), LUC 930 (Fast Casual Restaurant), and LUC 933 (Fast-Food Restaurant without Drive-Through Window). Approximately half of the restaurant space was assumed to be fast-food style and half was assumed to be fast-casual style. The detailed trip generation calculations are provided as an Attachment and are summarized in Table 1.

Multi-Use Trips

Internal Capture

Studies have shown that for developments of mixed-use or multi-use sites, it is realistic to assume that there will be some multi-use trips within the site itself. For example, someone working in the office spaces may dine at one of the restaurants on-site. Therefore, a reduction in the overall trips experienced at the site driveway can be anticipated as a result of multi-use trips that include stops at more than one use on the site. Based on information published in ITE's *Trip Generation Handbook* ², it is estimated that multi-use trips account for 2 to 10 percent of the trips generated by the site. The Multi-Use Development Trip Generation and Internal Capture Worksheets are provided in the Attachments.

External Capture

The proposed development is located within the downtown central business district of Portsmouth, in close proximity to numerous other retail, restaurant, office, residential, entertainment, hotel, and commercial uses. While many of the office trips generated by the project will be new to the area, a large portion of the restaurant trips will be shared with other retail, office, residential, and other uses within the downtown area. It is anticipated that patrons will park at a single location either within one of the public parking garages or within on-street parking spaces and visit multiple uses within the downtown, stopping at one of the restaurants as part of their trip. In addition, employees of surrounding area businesses, residents of downtown, or patrons of area hotels may choose to dine at one of the on-site restaurants. These patrons would likely walk to the site from other locations and would not be new to the area. GPI utilized the Multi-Use Development Trip Generation and Internal Capture worksheets contained within ITE's *Trip Generation Handbook* to estimate the potential number of trips that could be shared between the proposed restaurants and the surround area businesses, hotels, and residences. Based on this information, 45 to 75 percent of restaurant trips are anticipated to be shared with other downtown uses.

Pass-by Trips

Studies have shown that for restaurant developments, a substantial portion of the site-generated vehicle trips are already present in the adjacent passing stream of traffic or are diverted from another route to the proposed site. For example, some vehicles which are already on the roadways may decide to visit the site on their way to another destination. Based on information published in the ITE *Trip Generation Handbook*, the average *pass-by* trip percentage is 43 to 50 percent for fast-food and high-turnover sit-down restaurants. Due to the location of the proposed restaurants in the downtown business district, it is likely that pass-by trips will arrive to the site

¹ Trip Generation, 10th Edition. Institute of Transportation Engineers; Washington, DC; 2017.

² Trip Generation Handbook, 3rd Edition. Institute of Transportation Engineers; Washington, DC; September, 2017.

Mr. Eric B. Eby November 18, 2019 Page 4

in the form of walking trips from patrons parking at another location downtown and visiting multiple stops before returning to their vehicle. For example, an area employee may stop at the restaurants to purchase dinner before leaving the downtown in their vehicle, or a person shopping in the downtown area may stop at the restaurant to grab lunch while shopping. Therefore, any pass-by trips generated by the proposed restaurant have been accounted for within the *external capture multi-use trips* described above.

Walking and Bicycling Trips

As previously noted, the proposed development is located within downtown Portsmouth in close proximity to multiple retail, restaurant, office, and commercial developments. In addition, a strong sidewalk network on the surrounding area roadways provides pedestrian connections between the site and these establishments. The City of Portsmouth has also made concerted efforts to provide bicycle accommodations throughout the downtown including dedicated bicycle lanes, shared lanes, and bicycle parking. As described in the *Multi-Use Trips — External Capture* section of this letter, approximately 45 to 75 percent of the restaurant trips are anticipated to be shared with surrounding area businesses and residences, and will travel to the site via walking or bicycling. In addition, based on U.S. Census Bureau information on means of travel for residence of Portsmouth, approximately 7.7 percent of Portsmouth residents travel to work via walking or biking. To provide a conservative (worse case) analysis scenario, 5 percent of the office trips were assumed to travel to/from the site via walking or bicycle.

Transit Trips

The Cooperative Alliance for Seacoast Transportation (COAST) and the University of New Hampshire (UNH) Wildcat Transit provide bus service along Congress Street / Daniel Street, Hanover Street, and Maplewood Avenue in the vicinity of the site. Based on U.S. Census Bureau data for residence of Portsmouth, approximately 1.4 percent of Portsmouth residents utilize public transit services to travel to/from work. It should be noted that this percentage includes also Portsmouth residents traveling to all places of employment, most of which may be located outside of the City. The percentage of residents using public transit to travel to places of employment within the City is likely to be much higher. To provide a conservative (worse case) analysis condition, no credit was applied for trips traveling to the site via public transportation.

The detailed trip generation and mode split calculations are provided in the Attachments. Table 1 provides a summary of the resulting site-generated trips.

Table 1 TRIP-GENERATION SUMMARY

	Extern	al Trips	Walking /	Biking Trips	New Primary Trips			
Time Period / Direction	Office Trips ^a	Restaurant Trips ^b	Office Trips ^c	Restaurant Trips ^d	Office Trips ^e	Restaurant Trips ^f	Total Trips ^g	
Weekday Daily	377	5,479	19	3,534	358	1,945	2,303	
Weekday AM Peak Hour In Out Total	49 <u>3</u> 52	132 <u>82</u> 214	2 <u>0</u> 2	131 <u>40</u> 171	47 <u>3</u> 50	1 <u>42</u> 43	48 <u>45</u> 93	
Weekday PM Peak Hour In <u>Out</u> Total	6 <u>39</u> 45	182 <u>171</u> 353	0 <u>2</u> 2	96 <u>131</u> 227	6 <u>37</u> 43	86 <u>40</u> 126	92 <u>77</u> 169	
Saturday Daily	80	8,510	4	4,846	76	3,664	3,740	
Saturday Midday Peak Hour In <u>Out</u> Total	7 <u>8</u> 15	380 <u>358</u> 738	0 <u>0</u> 0	204 <u>130</u> 334	7 <u>8</u> 15	176 <u>228</u> 404	183 <u>236</u> 419	

^a Based on ITE LUC 710 (General Office) for ±41,600 SF.
^b Based on sum of ITE LUC 930 (Fast Casual Restaurant) for ±8,400 SF and ITE LUC 933 (Fast-Food Restaurant without Drive-Through) for ±8,400 SF.

^c Five percent of office trips based on U.S. Census data.
^d Trips based on mixed-use percentages to retail, residential, hotel, and entertainment uses.

e General office external trips minus walking / biking trips.

f Restaurant external trips minus walking / biking trips.

⁹ New Primary Office Trips plus New Primary Restaurant Trips.

TRIP DISTRIBUTION

Having estimated project-generated vehicle trips, the next step is to determine the distribution of project traffic and assign these trips to the local roadway network.

Office Trips

The distribution of site-generated office trips was based on a Journey-to-Work model using U.S. Census Data for the place of residency of employees of Portsmouth, which was prepared as part of the Traffic Impact Study for the proposed McIntyre Building Federal property redevelopment project by Tighe & Bond. As part of the #60 Penhallow Street project, 77 parking spaces will be provided on-site within an underground garage to accommodate the proposed office use. Therefore, the trip distribution prepared as part of the McIntyre Building study was slightly modified to account for vehicles traveling directly to/from the on-site parking garage. The resulting trip distribution is graphically depicted in Figure 2 and summarized in Table 2 below.

Restaurant Trips

Similarly, the McIntyre Building redevelopment includes a residential component and is located in close proximity to the #60 Penhallow Street site. Therefore, the trip distribution assumptions used for the McIntyre project were utilized to distribute restaurant trips for the #60 Penhallow Street project. As previously noted in the *Trip Generation* section of this letter, the majority of restaurant trips are likely to be shared with other uses in the downtown area and will travel to/from the site via walking. However, the portion of *new primary* trips traveling to the proposed restaurants will likely park in nearby parking garages and walk to the site. The Hanover Garage and Portwalk Garage provide a large amount of parking at relatively low costs and are in close proximity to the site. Therefore, all of the *new primary* restaurant trips were assumed to travel to one of these two garages on Hanover Street. The resulting trip distribution is graphically depicted in Figure 3 and summarized in Table 2 below.

Table 2
TRIP DISTRIBUTION SUMMARY

Direction	Office Distribution (%)	Restaurant Distribution (%)
Middle Street to/from South	5	25
Market Street to/from North	20	15
Maplewood Avenue to/from North	60	30
Memorial Bridge to/from East	10	20
Islington Street to/from West	<u>5</u>	<u>10</u>
Total	100	100

The site-generated trips were distributed to the study area intersections based on the percentages in Table 2 above. The resulting site-generated trips are illustrated in Figures 4A, 4B, and 4C for the weekday AM, weekday PM, and Saturday midday peak hours, respectfully.

Mr. Eric B. Eby November 18, 2019 Page 7

SUMMARY

GPI has estimated the site-generated vehicle trips associated with the proposed redevelopment of #60 Penhallow Street as described in this letter. This information will be utilized in the preparation of a comprehensive Traffic Impact and Access Study and will determine the extents of the study area. Therefore, we respectfully request your review and approval of the assumptions and resulting trip generation and distribution prior to moving forward with completion of the traffic study.

Should you have any questions, or require additional information, please contact me at (978) 570-2946.

Sincerely,

GREENMAN-PEDERSEN, INC.

Rebecca L. Brown, P.E., PTOE

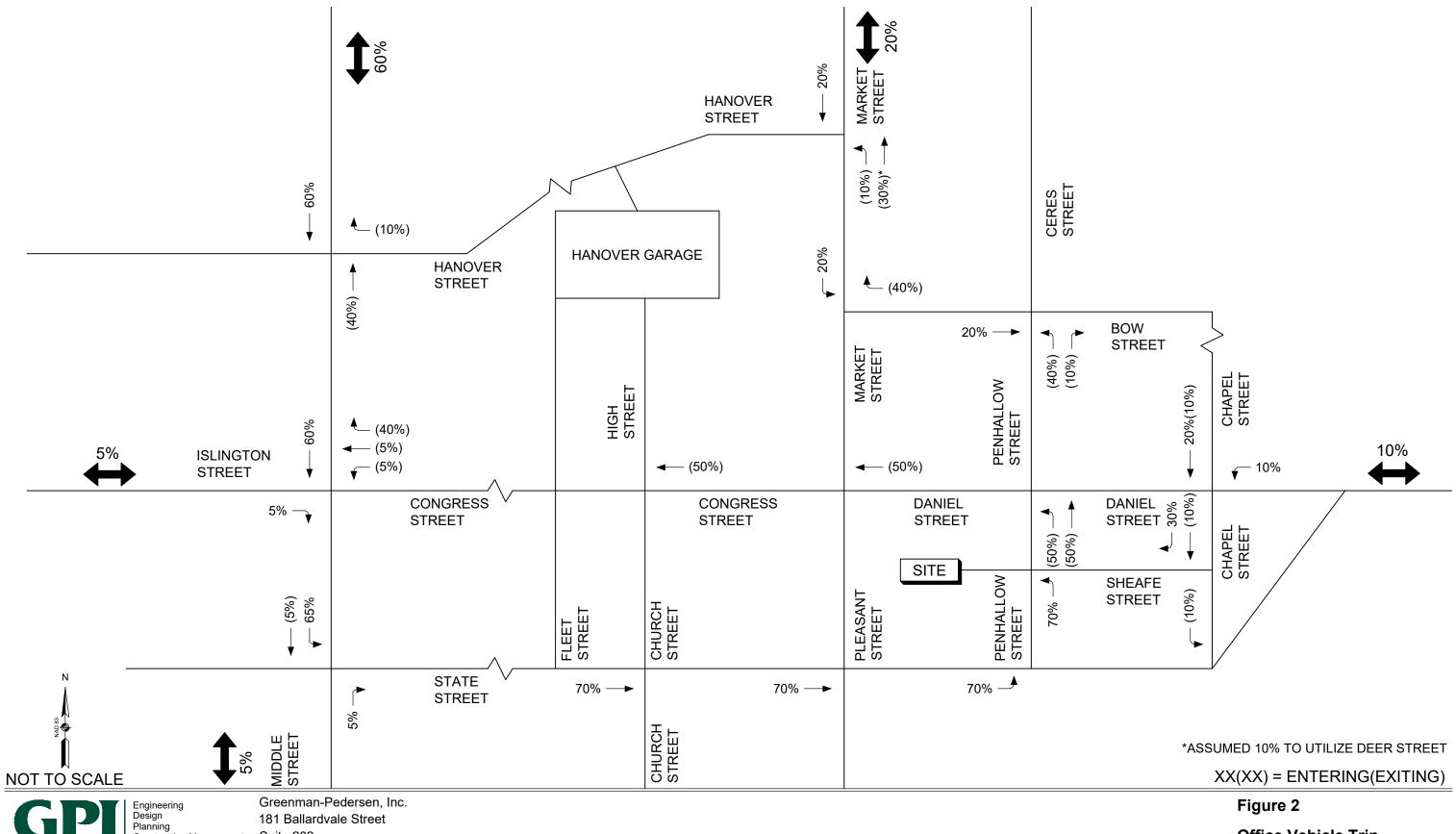
Senior Project Manager

Enclosures:

Site-Generated Vehicle Trip Traffic-Volume Networks Trip Generation Calculations Mode Split Calculations Trip Distribution Calculations

cc: Mark McNabb – Dagny Taggart, LLC (via email)
John Chagnon, P.E., LLS – Ambit Engineering (via email)

#60 Penhallow Street - Portsmouth, NH



181 Ballardvale Street

Wilmington, MA 01887

Suite 202

Construction Management

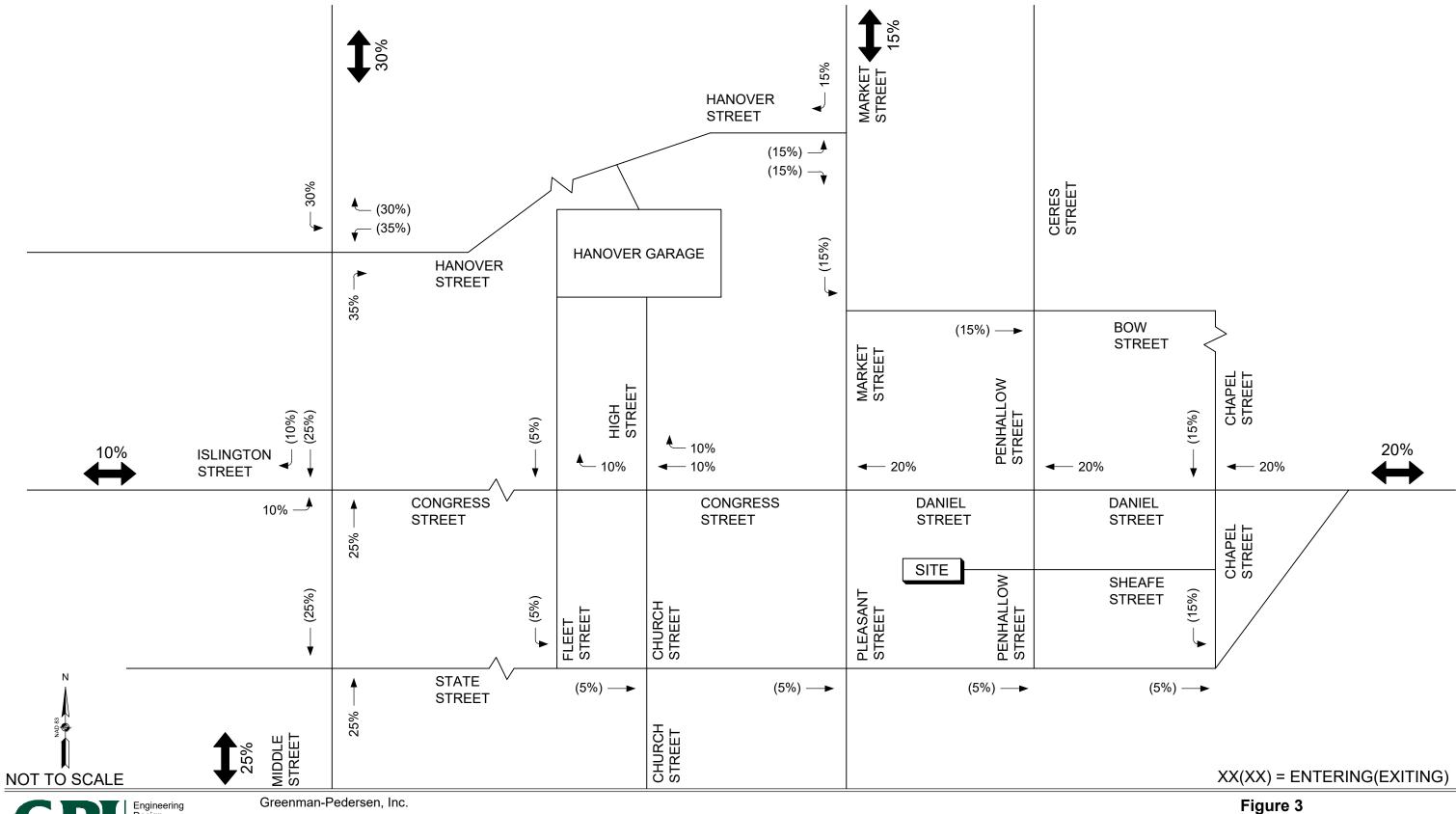
GPINET.COM

978.570.2999

Figure 2

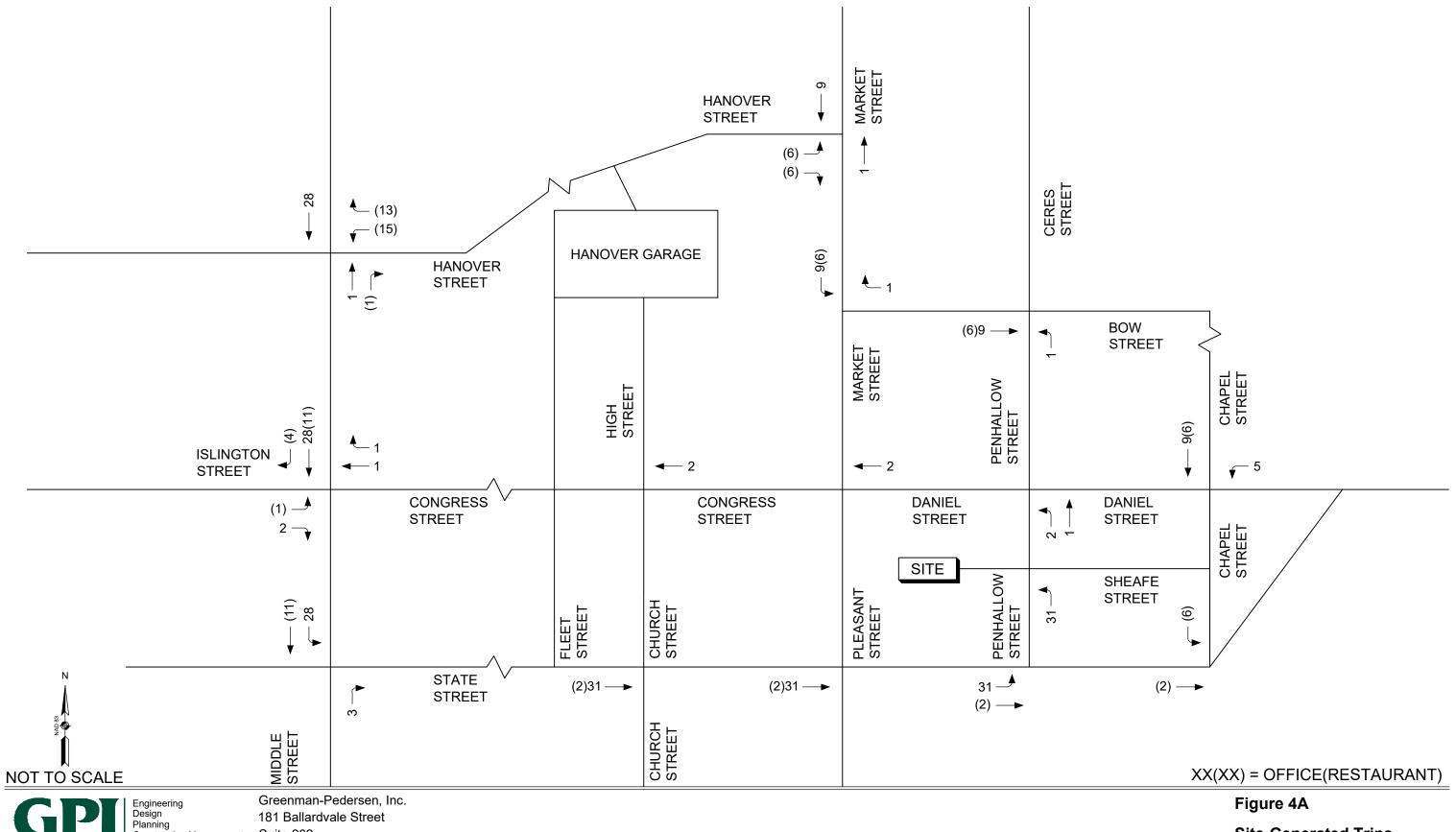
Office Vehicle Trip Distribution

#60 Penhallow Street - Portsmouth, NH



Restaurant Vehicle Trip Distribution

Mixed-Use Development - Portsmouth, NH



Greenman-Pedersen, Inc. 181 Ballardvale Street Suite 202 Construction Management Wilmington, MA 01887

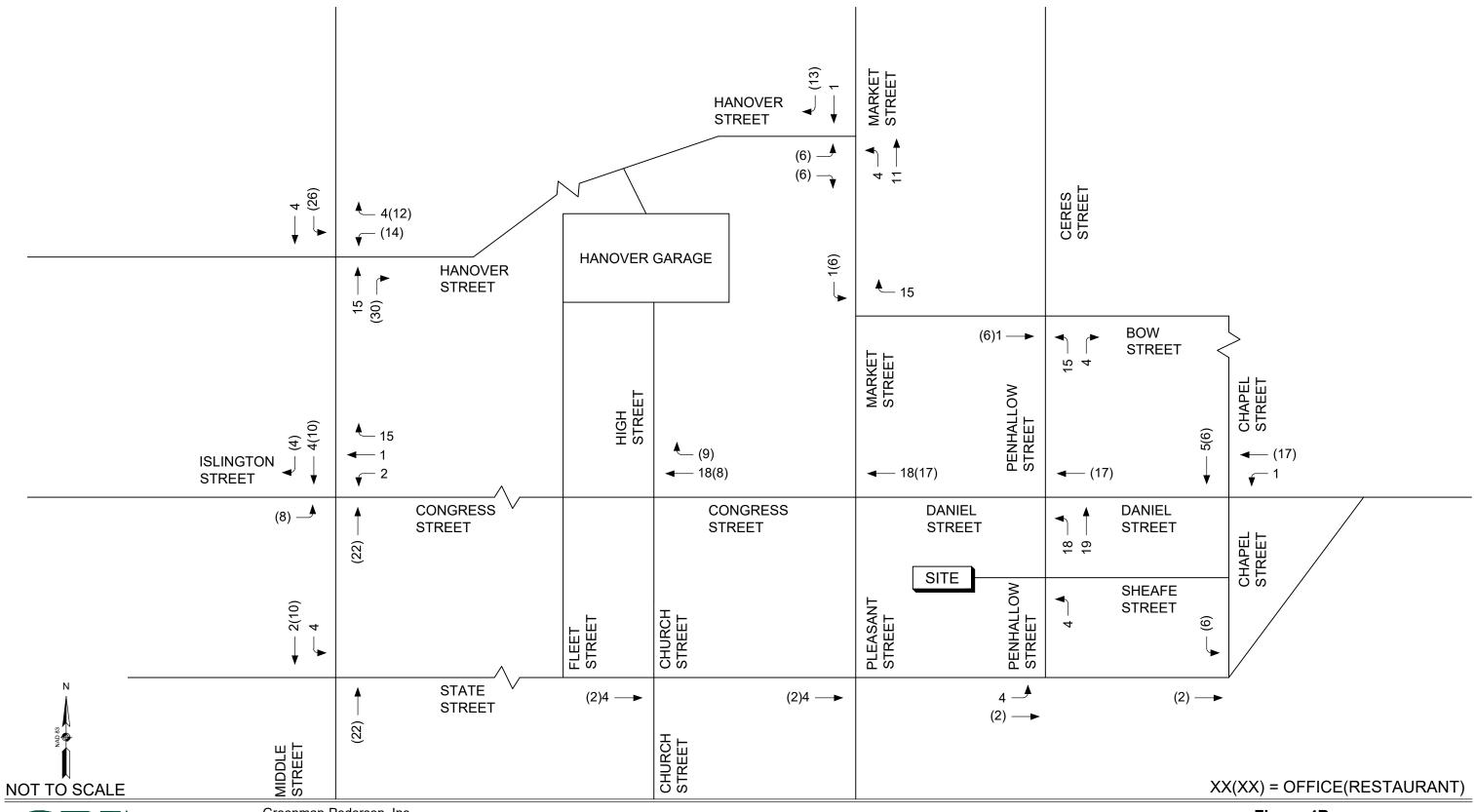
978.570.2999

GPINET.COM

Figure 4A

Site-Generated Trips Weekday AM

Mixed-Use Development - Portsmouth, NH



Engineering
Design
Planning
Construction Management

978.570.2999

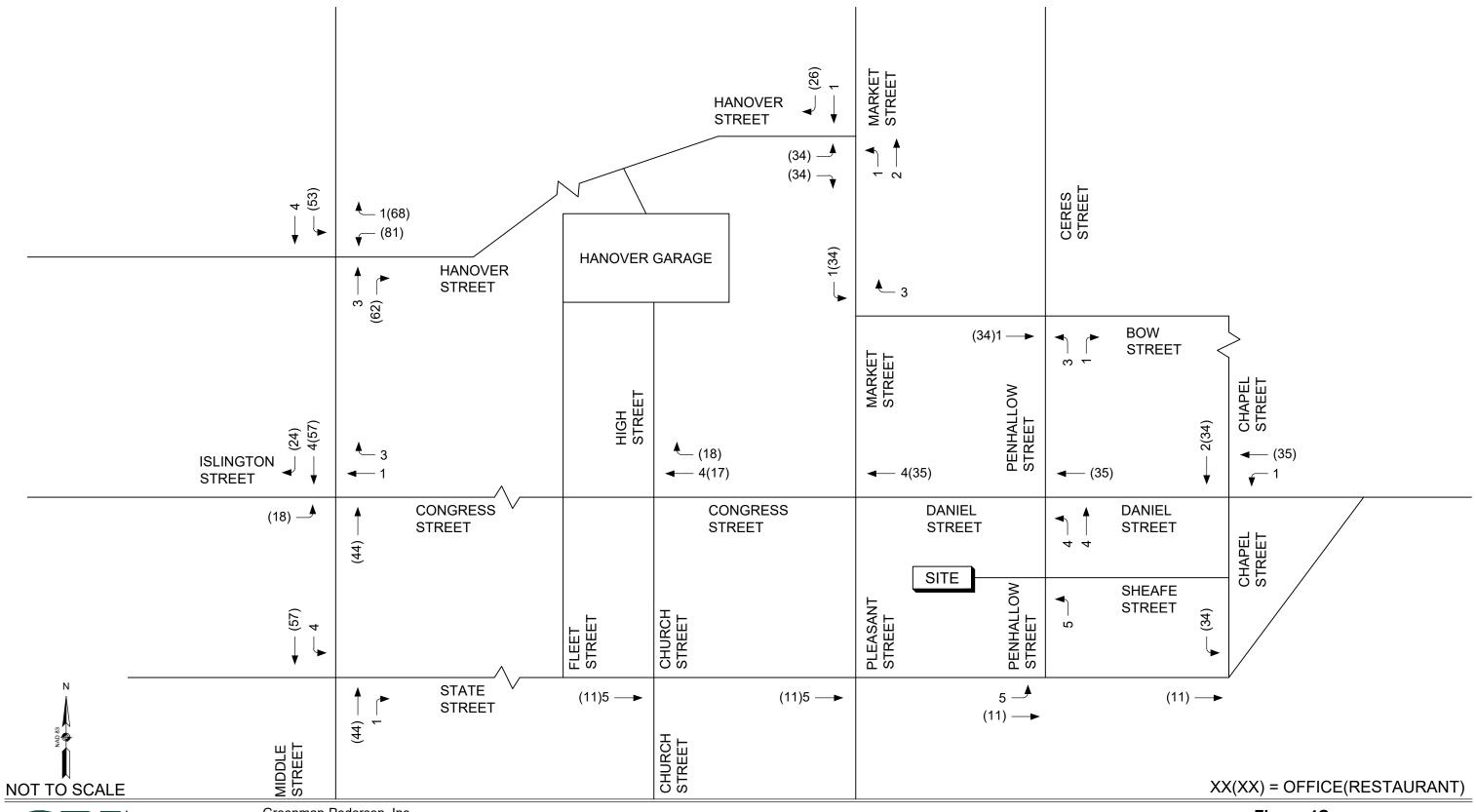
GPINET.COM

Greenman-Pedersen, Inc. 181 Ballardvale Street
Suite 202
Wilmington, MA 01887

Figure 4B

Site-Generated Trips Weekday PM

Mixed-Use Development - Portsmouth, NH



Engineering
Design
Planning
Construction Management
978.570.2999

GPINET.COM

Greenman-Pedersen, Inc. 181 Ballardvale Street Suite 202
Wilmington, MA 01887

Figure 4C

Site-Generated Trips Saturday Midday

Walk/Bike Credit:	5%	

Size	Units	Land Use
	41,600 SF	LUC 710
	8,400 SF	LUC 933
	8,400 SF	LUC 930

			Total	Trips		External Trips			Walking / Biking Trips			New Primary Trips		
		LUC 710	LUC 933	LUC 930	TOTAL	LUC 710	LUC 933 LUC 930	TOTAL	LUC 710	LUC 933 LUC 930	TOTAL	LUC 710	LUC 933 LUC 930	TOTAL
Weekday Daily	Entering	227	1,454	1,324	3,005	159	2,769	2,928	8	1,464	1,472	151	1,305	1,456
	Exiting	<u>227</u>	<u>1,454</u>	1,324	3,005	218	<u>2,710</u>	<u>2,928</u>	<u>11</u>	<u>2,070</u>	2,081	<u>207</u>	<u>640</u>	<u>847</u>
	Total	454	2,908	2,648	6,010	377	5,479	5,856	19	3,534	3,553	358	1,945	2,303
Weekday AM Peak Hour	Entering	57	127	11	195	49	132	181	2	131	133	47	1	48
	Exiting		<u>84</u> 211	<u>6</u>	<u>99</u>	<u>3</u>	<u>82</u> 214	<u>85</u>	<u>0</u>	<u>40</u> 171	<u>40</u>	<u>3</u>	<u>42</u> 43	<u>45</u> 93
	Total	66	211	17	294	52	214	266	2	171	173	50	43	93
Weekday PM Peak Hour	Entering	8	119	65	192	6	182	188	0	96	96	6	86	92
	Exiting	<u>41</u>	<u>119</u>	<u>54</u>	<u>214</u>	<u>39</u>	<u>171</u> 353	<u>210</u>	<u>2</u> 2	<u>131</u> 227	<u>133</u>	<u>37</u>	<u>40</u> 126	<u>77</u>
	Total	49	238	119	406	45	353	398	2	227	229	43	126	169
Saturday Daily	Entering	46	2,923	1,338	4,307	36	4,259	4,295	2	2,683	2,685	34	1,576	1,610
	Exiting	<u>46</u>	2,923	1,338	4,307	<u>44</u>	<u>4,251</u>	4,295	<u>2</u>	<u>2,163</u>	2,165	<u>42</u>	2,088	<u>2,130</u>
	Total	92	5,846	2,676	8,614	80	8,510	8,590	4	4,846	4,850	76	3,664	3,740
Saturday Midday Peak Hour	Entering	12	225	157	394	7	380	387	0	204	204	7	176	183
	Exiting	<u>10</u>	234	129	<u>373</u>	<u>8</u>	<u>358</u>	<u>366</u>	<u>0</u>	<u>130</u>	<u>130</u>	<u>8</u>	<u>228</u>	<u>236</u>
	Total	22	459	286	767	15	738	753	0	334	334	15	404	419

Institute of Transportation Engineers (ITE)

Land Use Code (LUC) 710 - General Office Building

General Urban/Suburban

Average Vehicle Trips Ends vs: 1000 Sq. Feet Gross Floor Area

Independent Variable (X): 41.600

AVERAGE WEEKDAY DAILY

$$Ln(T) = 0.97 Ln (X) + 2.50$$

$$Ln(T) = 0.97 Ln (41.600) + 2.50$$

$$Ln(T) = 6.12$$

$$T = 453.17$$

$$T = 454 vehicle trips$$
with 50% (227 vpd) entering and 50% (227 vpd) exiting.

WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
\begin{split} & Ln(T) = 0.95 \ Ln \ (X) + 0.36 \\ & Ln(T) = 0.95 \quad Ln \ ( \quad 41.600 \quad ) + 0.36 \\ & Ln(T) = 3.90 \\ & T = 49.49 \\ & T = 49 \qquad \text{vehicle trips} \\ & \text{with } 16\% \ ( \quad 8 \qquad \text{vph) entering and } 84\% \ ( \quad 41 \quad \text{vph) exiting.} \end{split}
```

SATURDAY DAILY

$$T = 2.21 * (X)$$

$$T = 2.21 * 41.600$$

$$T = 91.94$$

$$T = 92 vehicle trips$$

$$with 50% (46 vph) entering and 50% (46 vph) exiting.$$

SATURDAY PEAK HOUR OF GENERATOR

Institute of Transportation Engineers (ITE)

Land Use Code (LUC) 933 - Fast-Food Restaurant without Drive-Through Window General Urban/Suburban

Average Vehicle Trips Ends vs: 1,000 Sq. Ft. Gross Floor Area

Independent Variable (X): 8.400

AVERAGE WEEKDAY DAILY

WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

SATURDAY DAILY

SATURDAY PEAK HOUR OF GENERATOR

Institute of Transportation Engineers (ITE)

Land Use Code (LUC) 930 - Fast Casual Restaurant

General Urban/Suburban

Average Vehicle Trips Ends vs: 1,000 Sq. Ft. Gross Floor Area Independent Variable (X): 8.400

AVERAGE WEEKDAY DAILY

```
T = 315.17 * (X)

T = 315.17 * 8.400

T = 2647.43

T = 2,648 vehicle trips

with 50% ( 1,324 vpd) entering and 50% ( 1,324 vpd) exiting.
```

WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
T = 2.07 * (X)

T = 2.07  * 8.400

T = 17.39

T = 17  vehicle trips

with 67% ( 11 vph) entering and 33% ( 6 vph) exiting.
```

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
T = 14.13 * (X)

T = 14.13 * 8.400

T = 118.69

T = 119 vehicle trips

with 55% ( 65 vph) entering and 45% ( 54 vph) exiting.
```

SATURDAY DAILY

```
T = 318.62 * (X)

T = 318.62 * 8.400

T = 2676.41

T = 2,676 vehicle trips

with 50% ( 1,338 vpd) entering and 50% ( 1,338 vpd) exiting.
```

SATURDAY PEAK HOUR OF GENERATOR

```
T = 34.02 * (X)

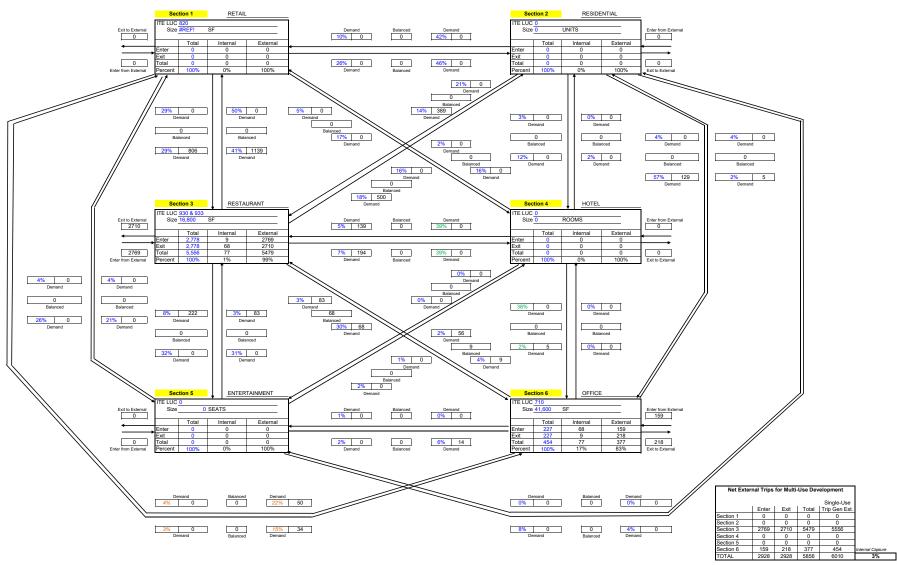
T = 34.02 * 8.400

T = 285.77

T = 286 vehicle trips

with 55% ( 157 vph) entering and 45% ( 129 vph) exiting.
```

MULTI-USE DEVELOPMENT TRIP GENERATION AND INTERNAL CAPTURE SUMMARY

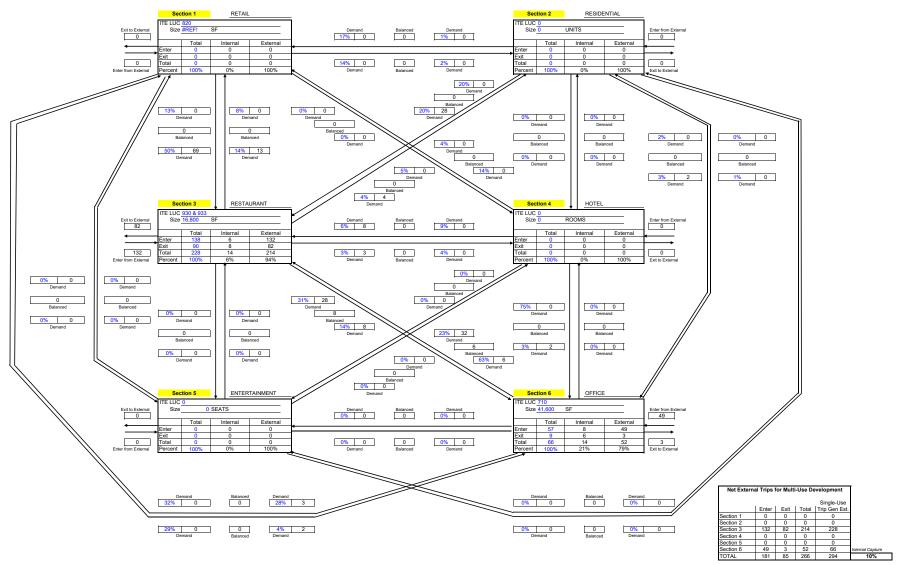


Based on Weekday PM from ITE Trip Generation Handbook, 3rd Edition, August 2014. Based on an average of Weekday AM or PM from ITE Trip Generation

Based on an average of Weekday AM or PM from ITE Trip Generation Handbook, 3rd Edition, August 2014.

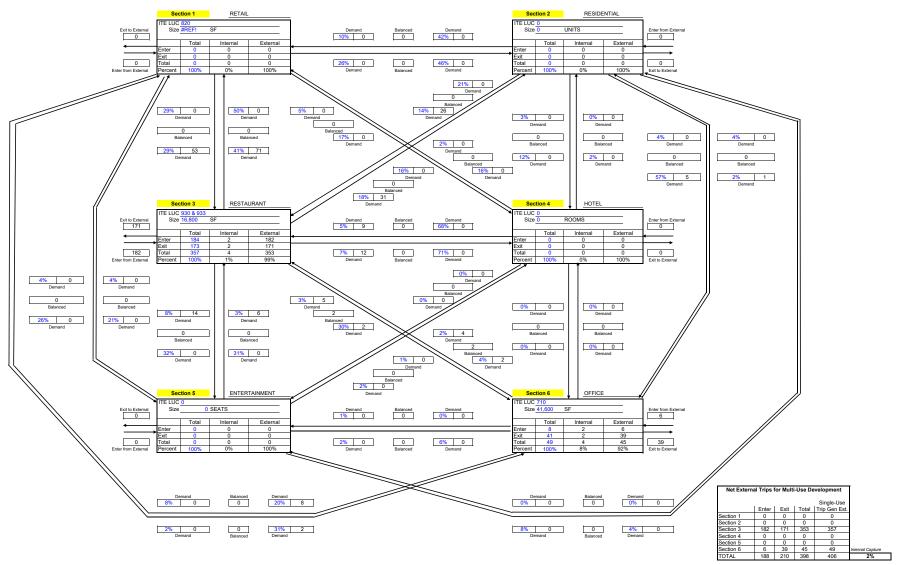
MULTI-USE DEVELOPMENT TRIP GENERATION AND INTERNAL CAPTURE SUMMARY

Name of Dvlpt: 60 Penhallow Street
Time Period: Weekday AM



MULTI-USE DEVELOPMENT TRIP GENERATION AND INTERNAL CAPTURE SUMMARY

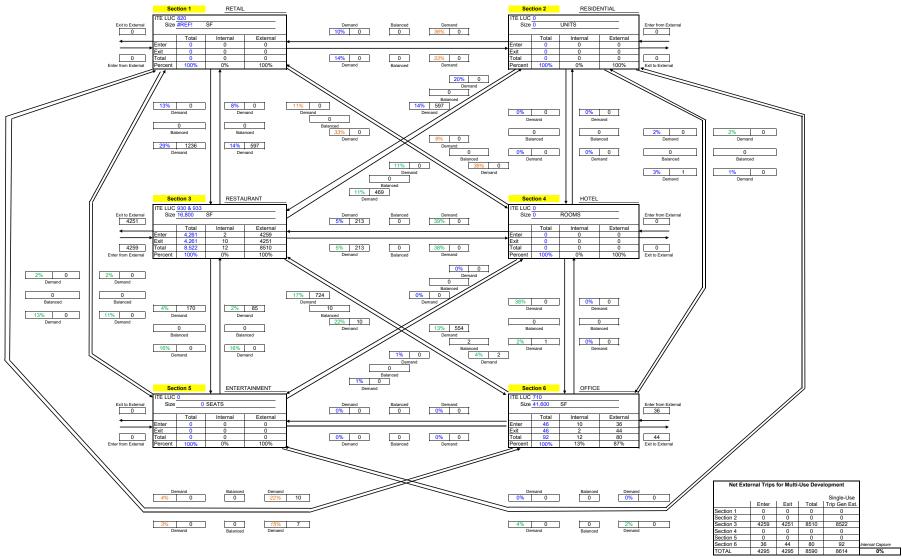
Name of Dvlpt: 60 Penhallow Street
Time Period: Weekday PM



Analyst: Douglas S. Halpert, P.E. Date: October 24, 2019

MULTI-USE DEVELOPMENT TRIP GENERATION AND INTERNAL CAPTURE SUMMARY

Name of Dvlpt: 60 Penhallow Street Time Period: Saturday Daily



Based on most conservative of Weekday AM or PM from ITE Trip Generation Handbook, 3rd Edition, August 2014.

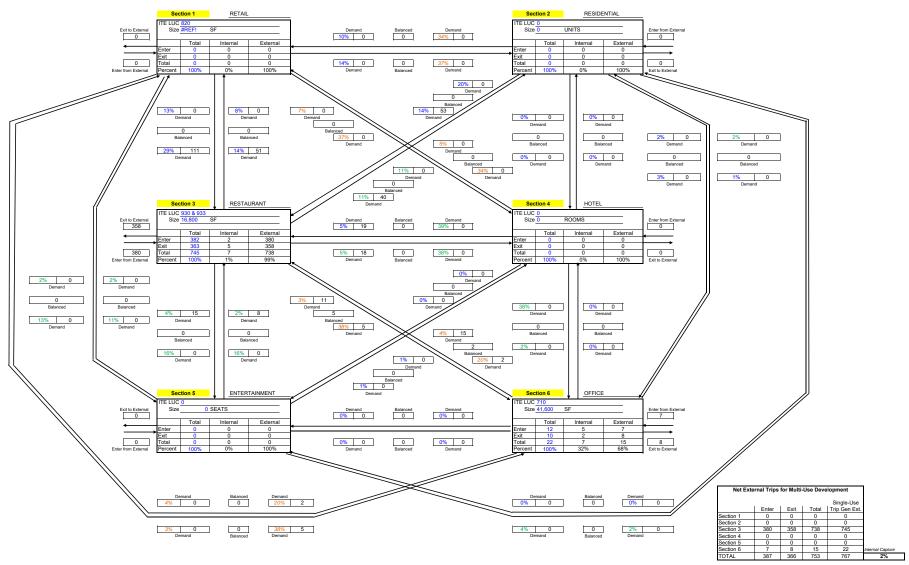
Based on an average of Weekday AM or PM from ITE Trip Generation Handbook, 3rd Edition, August 2014.

Based on ITE Trip Generation Handbook, 2nd Edition, June 2004.

Analyst: Douglas S. Halpert, P.E. Date: October 24, 2019

MULTI-USE DEVELOPMENT TRIP GENERATION AND INTERNAL CAPTURE SUMMARY

Name of Dvlpt: 60 Penhallow Street Time Period: Saturday Midday



Based on most conservative of Weekday AM or PM from ITE Trip Generatio Handbook, 3rd Edition, August 2014. Based on an average of Weekday AM or PM from ITE Trip Generation Handbook, 3rd Edition, August 2014.

Based on ITE Trip Generation Handbook, 2nd Edition, June 2004.



B08101

MEANS OF TRANSPORTATION TO WORK BY AGE

Universe: Workers 16 years and over 2013-2017 American Community Survey 5-Year Estimates

Supporting documentation on code lists, subject definitions, data accuracy, and statistical testing can be found on the American Community Survey website in the Technical Documentation section.

Sample size and data quality measures (including coverage rates, allocation rates, and response rates) can be found on the American Community Survey website in the Methodology section.

Although the American Community Survey (ACS) produces population, demographic and housing unit estimates, it is the Census Bureau's Population Estimates Program that produces and disseminates the official estimates of the population for the nation, states, counties, cities, and towns and estimates of housing units for states and counties.

	Portsmouth city, I	New Hampshire
	Estimate	Margin of Error
Total:	12,584	+/-398
16 to 19 years	381	+/-161
20 to 24 years	1,265	+/-251
25 to 44 years	6,084	+/-431
45 to 54 years	2,327	+/-249
55 to 59 years	1,096	+/-198
60 to 64 years	714	+/-168
65 years and over	717	+/-141
Car, truck, or van - drove alone:	9,564	+/-465
16 to 19 years	145	+/-68
20 to 24 years	981	+/-267
25 to 44 years	4,631	+/-414
45 to 54 years	1,859	+/-245
55 to 59 years	838	+/-187
60 to 64 years	512	+/-120
65 years and over	598	+/-131
Car, truck, or van - carpooled:	896	+/-235
16 to 19 years	132	+/-106
20 to 24 years	54	+/-37
25 to 44 years	475	+/-173
45 to 54 years	97	+/-63
55 to 59 years	84	+/-58
60 to 64 years	54	+/-44
65 years and over	0	+/-21
Public transportation (excluding taxicab):	180	+/-79
16 to 19 years	0	+/-21
20 to 24 years	57	+/-47
25 to 44 years	71	+/-58
45 to 54 years	21	+/-24
55 to 59 years	15	+/-15
60 to 64 years	16	+/-17
65 years and over	0	+/-21
Walked:	761	+/-240
16 to 19 years	79	+/-86

1 of 2 10/22/2019

	Portsmouth city, New Hampshire				
	Estimate	Margin of Error			
20 to 24 years	131	+/-88			
25 to 44 years	376	+/-154			
45 to 54 years	70	+/-60			
55 to 59 years	27	+/-22			
60 to 64 years	57	+/-58			
65 years and over	21	+/-24			
Taxicab, motorcycle, bicycle, or other means:	210	+/-90			
16 to 19 years	6	+/-11			
20 to 24 years	0	+/-21			
25 to 44 years	134	+/-74			
45 to 54 years	55	+/-39			
55 to 59 years	0	+/-21			
60 to 64 years	15	+/-16			
65 years and over	0	+/-21			
Worked at home:	973	+/-225			
16 to 19 years	19	+/-45			
20 to 24 years	42	+/-58			
25 to 44 years	397	+/-115			
45 to 54 years	225	+/-70			
55 to 59 years	132	+/-63			
60 to 64 years	60	+/-46			
65 years and over	98	+/-64			

Data are based on a sample and are subject to sampling variability. The degree of uncertainty for an estimate arising from sampling variability is represented through the use of a margin of error. The value shown here is the 90 percent margin of error. The margin of error can be interpreted roughly as providing a 90 percent probability that the interval defined by the estimate minus the margin of error and the estimate plus the margin of error (the lower and upper confidence bounds) contains the true value. In addition to sampling variability, the ACS estimates are subject to nonsampling error (for a discussion of nonsampling variability, see Accuracy of the Data). The effect of nonsampling error is not represented in these tables

Workers include members of the Armed Forces and civilians who were at work last week.

While the 2013-2017 American Community Survey (ACS) data generally reflect the February 2013 Office of Management and Budget (OMB) definitions of metropolitan and micropolitan statistical areas; in certain instances the names, codes, and boundaries of the principal cities shown in ACS tables may differ from the OMB definitions due to differences in the effective dates of the geographic entities.

Estimates of urban and rural populations, housing units, and characteristics reflect boundaries of urban areas defined based on Census 2010 data. As a result, data for urban and rural areas from the ACS do not necessarily reflect the results of ongoing urbanization.

Source: U.S. Census Bureau, 2013-2017 American Community Survey 5-Year Estimates

Explanation of Symbols:

- 1. An '**' entry in the margin of error column indicates that either no sample observations or too few sample observations were available to compute a standard error and thus the margin of error. A statistical test is not appropriate.
- 2. An '-' entry in the estimate column indicates that either no sample observations or too few sample observations were available to compute an estimate, or a ratio of medians cannot be calculated because one or both of the median estimates falls in the lowest interval or upper interval of an open-ended distribution.
 - 3. An '-' following a median estimate means the median falls in the lowest interval of an open-ended distribution.
 - 4. An '+' following a median estimate means the median falls in the upper interval of an open-ended distribution.
- 5. An '***' entry in the margin of error column indicates that the median falls in the lowest interval or upper interval of an open-ended distribution. A statistical test is not appropriate.
 - 6. An '*****' entry in the margin of error column indicates that the estimate is controlled. A statistical test for sampling variability is not appropriate.
- 7. An 'N' entry in the estimate and margin of error columns indicates that data for this geographic area cannot be displayed because the number of sample cases is too small.

8. An '(X)' means that the estimate is not applicable or not available.

2 of 2 10/22/2019

FIGURE 3

www.tighebond.com

Oct 18, 2019-8:22am Plotted By: YMayboroda Tighe & Bond, Inc. C:\Users\YMayboroda\appdata\local\temp\AcPublish_8288\Figures1.dwg

FIGURE 5

www.tighebond.com

Oct 18, 2019-8:22am Plotted By: YMayboroda Tighe & Bond, Inc. C:\Users\YMayboroda\appdata\local\temp\AcPublish_8288\Figures1.dwg MENU





(/en)

Swing o Mat

The underground system with a foldable walkway platform

The walkway platform of our Swing o Mat underground container system is 90° foldable. Thus, no emptying hook is visible, and the waste can be collected with either a steel container or big bag

Product Specifications

- Volume: 3.0 5.0 m³
- Modular System
- Steel Container or Big Bag
- Throw-in column: different models
- Emptying Hooks: Mushroom, 1 Hook

Suitable to collect

- Mixed Waste
- Paper + Cardboard
- Plastic + Plastic Bottles
- Aluminium
- Recycables

Options



Fill Level Sensor

Electronic Access Control







[/en/products/options/verwiegungsdyate/ardrvices/service-

Service + Maintenance

TRASH CHUTES

Get in touch with us

BAMBOO

Design by Antoni Arola & Enric Rodríguez.



Reference

4812.

Application

Outdoor

Installation type

Recessed

Description

Bamboo is a recessed outdoor lamp with three arms. Available in two finishes – khaki, and oxide – that make it possible to camouflage and integrate it into the landscape, with the added surprise that it produces light. Designed by Antoni Arola & Enric Rodríguez.

Diffuser

Acrylic diffuser

Materials

Body: Resin + fiberglass Diffuser: Acrylic

Finishes

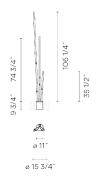


4812-07 Khaki (RAL 7006)



4812-54 Oxide (RAL 8017)

Sketch



Electrical characteristics

9 x LED 2.1W 700mA

Total 18.9 W







Light source: 2700K CRI >80 1794 lm 95 lm/W

Fixture: 420 lm 22 lm/W

Driver included: CC - Constant Current 700 mA 120-277V 50/60Hz

Electronic dimming: NO DIMMING

Installation and assembly

Please see the installation manual

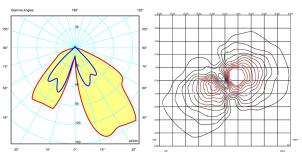
Light distribution

Ground lighting

Outdoor lighting



Photometric data



Certificates

[IP68] № (Wet Location) (LED)



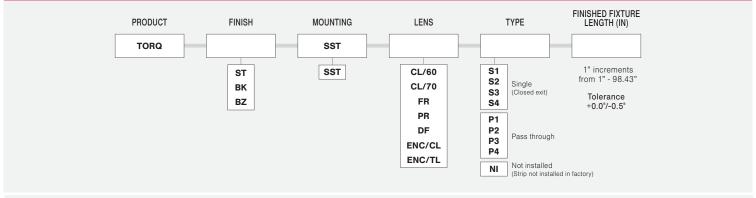
TORQ EXTRUSIONS - ALUMINUM









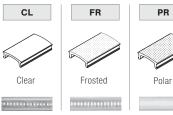


NOTES: • UL Listed when assembled with STRIP LEDs at Q-Tran

- NRTL Listed for install in Storage Areas with Clothing, NEC Field 410.2 and 410.16 when assembled as a fixture, with 4.0 w/ft or less, at Q-Tran facility (Not applicable for encapsulation)
 Field modifications must comply with Q-Tran's installation methods otherwise warranty is null and void













DIMENSIONS



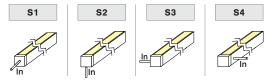
Profile



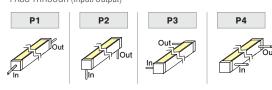
Profile with Stainless Steel Mounting Clip

TYPE

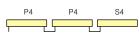












END CAPS





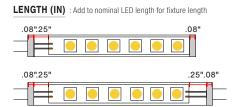


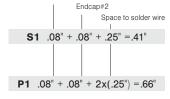
CUT OFF





CL/70





Fndcap#1

PROJECT NAME	DATE	COMPANY	TYPE	NOTE

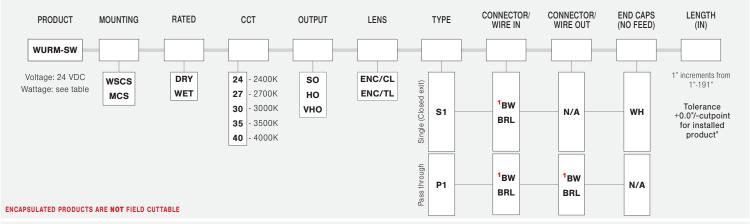
WURM-SW

FIXTURES - FLEXIBLE (Q-CAP)









NOTES: • Field modifications are not covered under Q-Tran warranty

- · Data subject to change, all data has +/- 5% tolerance
- 1 BW comes in standard 24"- request custom length (Max 120") by writing it in inches next to "BW" in the order code box (ex. BW48) •Connector/Wire In or Out not needed to specify product. Standard configuration is Type S1, Connector/Wire In: BW & Connector/Wire Out: N/A with White Endcap (WH)

OUTPUT Tested for WURM-SW-WSCS-DRY [L70 = 40000 HRS]

	SO				НО			VHO				
	Standard Output				High Output			Very High Output				
		1.5	W/ft			3.0	W/ft			5.0	W/ft	
	ENC	C/CL	ENC	C/TL	ENC	C/CL	ENC	C/TL	ENC	C/CL	ENC	C/TL
CCT	LM	CRI	LM	CRI	LM	CRI	LM	CRI	LM	CRI	LM	CRI
2400K	127	94	107	94	231	93	189	93	329	94	274	94
2700K	127	97	103	97	227	98	197	98	329	98	273	98
3000K	135	99	110	98	242	98	202	98	337	98	267	97
3500K	144	96	115	96	242	96	201	97	359	97	304	97
4000K	151	96	126	96	262	96	218	96	370	97	316	97

MOUNTING NOTE: 2 clips provided per first 12", 1 clip provided per additional 12"





White Snug Clip Small



MCS



Magnetic Clip Small (Cove use only)



FLEXIBILITY

Light Source



Un/Down & Helical*

DIMENSIONS



Profile (Standard)

LENS with LED visibility



ENC/TL Encapsulated

in Translucent

CONNECTOR/WIRE IN

BRL



Male Barrel 6" Bare Wire 24'

CONNECTOR/WIRE OUT





BRL

Bare Wire 24"

Female Barrel 6"

TYPE

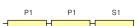
SINGLE (Input only)



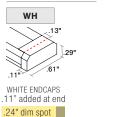
PASS THROUGH (Input/Output)



ORDER EXAMPLE



END CAPS (NO FEED)



END CAPS (WITH FEED)



.0" added at end

.38" dim spot

PROJECT NAME	DATE	COMPANY	TYPE	NOTE

^{*}Max recommended rotation of 1 rotation per 16"

MUE.5.18.15

The Brightest Idea is Emergency Lighting with LEDS

GENERAL DESCRIPTION

Operating in emergency mode or optional normal-on, this fixture is designed to mount directly on structural mullion beams used in typical glass-fronted entrances, with vertical surface as small as 2". This fixture has full 90° cut-off and will provide efficient emergency lighting in front of egress doorways, or along extended pathways.

CONSTRUCTION

- Rugged extruded aluminum housing with stainless hardware is corrosion proof.
- Wet location listed UL 924. Certified IP66.
- Uniform, high brightness lighting over the path of egress.
- Full 90° cut- off.
- Three versions are available:
 - **RE**= Central Battery System Series CBS or other qualified source 12V- 24 VDC.
 - **BB**= Battery backup from Remote Battery Supply Series RPS.
 - AC= 120/ 277 VAC supply.

ELECTRONICS

- Dual operation from either a battery or optional normally on power source.
- Lamps are connected in parallel-series strings, as required to meet requirements of NEC and Life Safety Codes. Lighting continues even after failure of One lamp or circuit.
- LED color temperature standard 5300K; available color temperatures from 2900K, 3200K, to 3800K.



ENERGY EFFICIENT OPERATION

- Dual function operation for optional normally on night or security lighting as well as emergency lighting.
- Very low power consumption in optional night/ security mode. The security lighting circuit is independent of emergency lighting and may be switched manually, by an exterior photocell, or other automatic means.
- Over 50,000 hour lamp life in normal use.
- IES photometric data available for all models.

CODES

 Manufactured and tested to UL Standard 924 and NFPA Life Safety Code 101.

WARRANTY

 5 year total customer satisfaction warranty. For Details see product catalog technical data section.

FIXTURE SCHEDULE

MODEL	CATALOG NO
APPROVAL	JOB INFORMATION





Moonlite LED®

Mullion Mount Emergency Light LED Outdoor Egress Emergency with Night Lighting Option Series MUE

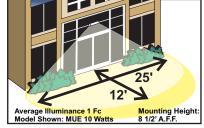
SPACING GUIDE

SUGGESTED SPECIFICATIONS:

Supply and install the MOONLITE LED Series MUE Mullion Mount emergency lighting fixture manufactured by Signtex Lighting Inc. The MUE assembly shall be listed for installation in wet locations in compliance with UL 924 and IP66 standards and shall be capable of operating from Signtex remote power supply Series RPS, the Signtex central battery system Series CBS, or from other remote power sources supplying 12-24 VDC or VAC. Upon loss of AC building power, emergency models shall operate for a minimum of 90 minutes in compliance with UL Standard 924 and NFPALSC 101.

8 1/2"

MOUNTING DATA & DIMENSIONS:



MUE.5.18.15

NOTE: FOR REFERENCE ONLY, STANDARD REFLECTANCES

INPUT 7 3/8" CONTACT SIGNTEX FOR LAYOUT ASSISTANCE 120/277 VAC **BACK BOX WALL MOUNT** STATUS INDICATOR GLASS **PUSH TO TEST** PANEL 5" **ACCESS** COVER 2 1/2" 5 3 3/4 **Typical Wiring Specification:** UL AWM Style: Control Cable 2 1/4" #20 AWG 300V 0UTPUT 5 V - 24 V **MULLION** BB and AC Models: 4 Conductors **REMOTE POWER SUPPLY (RPS)** (BB & AC Models Only) **TOP MOUNT** 3 1/2"

TABLE 1 **MAXIMUM WIRING LENGTH**

WIRING	LENGTH (FT)
SIZE AWG	MUEBB
#18	25
#16	50
#14	75
#12	125

FROM RPS TO FIXTURE

LENGTH TABLE

(SUPPLIED BY OTHERS)

RACO # 698

TYPICAL STOCK MODELS: **CROUSE HINDS #TP693**

STANDARD MASONRY BOX 3 3/4" X 3 1/2" X 7 3/8"

POWER	L
10 Watts	10"
20 Watts*	19"

*RE & AC Models Only

SECURITY LIGHTING CONTROL

Requires SEC Option 'S' with CBL RE Models: Requires Option '-SB120' for connection BB Models: to 120 VAC

Requires Option '-SD277' for connection

2 3/4" MOUNT

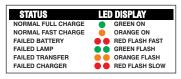
SCREW LOCATIONS

to 277 VAC

BB-DG Models: Requires Option '-SD' for connection to 120/277 VAC

RPS SELF-TEST DIAGNOSTIC FUNCTIONS

BB MODELS WITH DG FUNCTION



FIXTURE ORDERING INFORMATION: EXAMPLE: MUEBB10AW-DG

— —					
MODEL SERIES	OPERATION	OPERATION POWER H		MOUNT -	•
MUE	RE= Central Battery or other 12- 24 VDC Remote Source BB= Battery Backup (Includes RPS) AC= No Battery (Includes RPS)	10= 10 Watts Emergency & Normal On Power 20= 20 Watts Emergency & Normal On power (RE & AC Models Only)	W= Satin White A= Aluminum B= Dark Bronze X= Custom		DG= Self- Tes (BB Mod SB120= Secu Switch fo (120V) SD277= Secu Switch fo (277V)
l			SUITABLE FOR WE		SD= Security for BB O (120/ 27)

220 VFWAvenue, Grasonville, MD21638 TEL:(410)827-8300 Fax:(410)827-8866 sales@signtexinc.com www.signtexinc.com

-40° C to +50° C

-DG **OPTIONS**

est Diagnostics dels Only)

curity Lighting with Control for Standard BB Operation

curity Lighting with Control for Standard BB Operation

ty Lighting with Control Switch Operation with DG option 277V)

CW1= Custom Window Filter- 3800K CW2=Custom Window Filter- 3200K CW3=Custom Window Filter- 2900K DAC= Dual AC Input

2HT= 2" Canopy Height 5HT= 5" Canopy Height

DISTRIBUTOR:

Specifications and Dimensions subject to change without notice.



Tumbler

Specification Sheet

Project Name:					Job Locati	on:				
Fixture Type:					Fixture Qu	antity:				_
	ninaire is aluminum extr	dz s (luminaire only) usion with a natura		coat finish offered by						
Pole & Wall Mou	oidermis with a clear or o	opar/ uniuseu tem	pereu giass.							
Total wall will	Product	TML								
	LED Configuration	16	24	C (CoB) *						
	Drive Current	A (350 mA)	B (500 mA)							
	Color Temperature	1 (3000K)	2 (4000K)							
	Distribution	WF (Wide Flood)	F (Flood)	M (Medium)	S (Spot Flood)	TII (Type II)	TIII (Type III)	TIV (Type IV)		
	Lens	Null (Clear)	O (Opal / Diffused)							
	Color	AS (Aluminum Silver)	BK (Black)							
* CoB provides a W	CoB provides a Wide Flood distribution only.									

EXAMPLE: TML - 16 - B - 1 - WF - O - AS

Catenary Lumir	Catenary Luminaire						
	Product	TML					
	LED Configuration	16	24	C(CoB)*			
	Drive Current	A (350 mA)	B (500 mA)				
	Color Temperature	1 (3000K)	2 (4000K)				
	Distribution	WF (Wide Flood)	TII+II (Quad Oval)	TV (Type V)			
	Lens	Null (Clear)	O (Opal / Diffused)				
	Bracket	C (Catenary)					
	Color	AS (Aluminum Silver)	BK (Black)				

EXAMPLE: TML - 16 - B - 1 - TV - O - C - BK



Catenary

^{*} CoB provides a Wide Flood distribution only.

Mounting Acce	Mounting Accessories				
	Product	TML			
	Bracket	01 (Single Column)	02 (Wall)	04 (Double Column)	
	Color	AS (Aluminum Silver)	BK (Black)		

EXAMPLE: TML - 01 - DK







Single Column

Wall

Double Column

Tumbler Aluminu	Tumbler Aluminum Pole										
Height	Pole #	Mounting	Diameter	Wall	Weight						
2.4 (44.2 ft.)	TNAFAAD	(A) Circle Column Brooket	Ch:- - 4 5"	0.42"	ad II						
3.4m (11.2 ft)	TMF11P	(1) Single Column Bracket	Straight 4.5"	0.13"	41 lbs						
	TMF11P	(1) Double Column Bracket	Straight 4.5"	0.13"	41 lbs						
4.2m (13.8 ft)	TMF21P	(1) Single Column Bracket	Straight 4.5"	0.13"	45 lbs						
	TMF22P	(2) Single Column Brackets, Staggered Heights, 180° Orientation	Straight 4.5"	0.13"	45 lbs						
	TMF21P	(1) Double Column Bracket	Straight 4.5"	0.13"	45 lbs						
5.0m (16.4 ft)	TMF31P	(1) Single Column Bracket	Straight 4.5"	0.13"	51 lbs						
	TMF32P	(2) Single Column Brackets, Staggered Heights, 180° Orientation	Straight 4.5"	0.13"	51 lbs						
	TMF31P	(1) Double Column Bracket	Straight 4.5"	0.13"	51 lbs						
6.6m (21.7 ft)	TMF41P	(3) Single Column Brackets, Spiral Configuration, 120° Orientation	Stepped 6"/4.5"	0.19"/0.13"	97 lbs						

^{*} Landscape Forms can provide poles for your catenary project. Contact the factory for more information.

Į	Pole Options								
		Twist Lock	Null (None)	T (Twist Lock Receptacle)					
		Color	BLK (Black)	DSK (Dusk)	MBK (Matte Black)	MER (Mercury) *	SIL (Silver)	SCL (Storm Cloud)	TNM (Titanium)

^{*} Mercury from Landscape Forms is our recommended color match for aluminum silver from Santa & Cole.

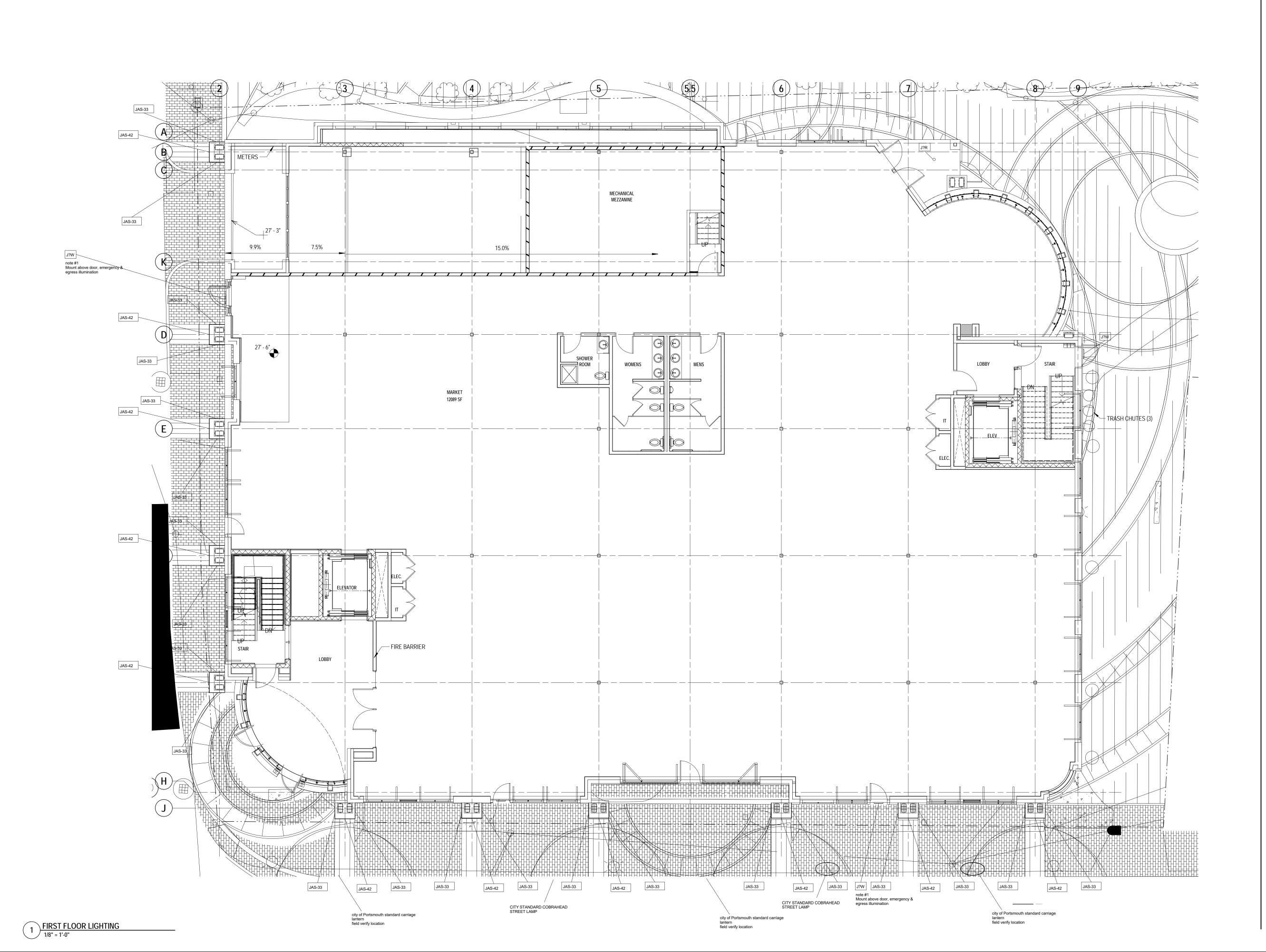
EXAMPLE: TMF41P - T - MER

Modifications

Don't see what you are looking for? Our goal is to partner with you as the designer to manufacture solutions needed for the space you are creating. We offer the option to modify our standard product to meet certain design specifications or needs. Common modifications can include GCFI outlets, custom RAL colors, banner arm(s) mounting, and custom pole heights. Contact your local Landscape Forms representative to learn more about these offerings.

Notes			

Please send completed forms to your Landscape Forms representative or contact us at (800) 430-6209 with any questions.





P L A N N E R S

273 CORPORATE DRIVE
PORTSMOUTH, NH 03801
T 603.436.2551
F 603.436.6973
www.jsainc.com

60 PENHALLOW STREET at BRICK MARKET

Penhallow Street Portsmouth, NH

DAGNY TAGGART LLC McNABB PROPERTIES

 Scale:
 1/8" = 1'-0"

 Date:
 11/18/2019

 Project Number:
 P19081.02

REVISIONS

NO. DESCRIPTION DATE

TAC SUBMISSION

LIGHTING PLAN

A0.06

COPYRIGHT 2019

EASEMENT FOR PUBLIC ACCESS AND USE OF COMMUNITY SPACE

THIS EASEMENT HEREIN IS GRANTED this _____ day of _______, 2019 by Dagny Taggart, LLC, a New Hampshire limited liability company having an address of 30 Penhallow Street, Suite 300 East, City of Portsmouth, County of Rockingham, State of New Hampshire 03801, ("Grantor") unto the City of Portsmouth, New Hampshire ("Grantee") for public access to and use of certain pocket parks, use of community space and sidewalks as set forth herein.

WITNESSETH

WHEREAS, Grantor acquired a tract of land located at 3 Pleasant Street, City of Portsmouth, County of Rockingham, State of New Hampshire described in Exhibit A attached hereto and made a part hereof (the "Property"); and

WHEREAS, reference is made to a plan entitled "Brick Market, Master Plan, Community Space, Tax Map 107, Lots 27, 31 & 42, Owners: Dagny Taggart, LLC & Coventry Assets, Ltd. Property Located At: 3 Pleasant Street, 30 & 60 Penhallow Street, City of Portsmouth, County of Rockingham, State of New Hampshire," prepared by Ambit Engineering, Inc., dated July 25, 2019, as revised and recorded herewith at the Rockingham County Registry of Deeds (the "Community Space Plan"); and

WHEREAS, the Grantor, as provided herein, wishes to dedicate a certain portion of the Property as Community Space, as defined by the Portsmouth Zoning Ordinance further, to convey an easement for public access to, and use of community space and sidewalks, all as shown on the Community Space Plan;

NOW THEREFORE, in consideration of the sum of One Dollar (\$1.00), to be paid by the City, and other good and valuable consideration, the receipt of which is hereby acknowledged by the Grantor, Grantor conveys the easement as follows:

- 1. Grant of Easements. Grantor hereby grants, transfers and conveys to Grantee, for the benefit of the public, with only pedestrian access thereto, a nonexclusive permanent right to use and enjoy those portions of the Property depicted on the Community Space Plan consisting of an area of 3,952 square feet on the Property, to be used concurrently with similar Community Space on Tax Map 107, Lot 31 and Tax Map 107 Lot 42, with an aggregate Community Space with all three lots of 11,962 square feet, all as shown on the Community Space Plan (the "Community Space Easement").
- **Restrictions.** The Community Space Easement shall be used by the public pursuant to this instrument only during the hours of 8:00 a.m. through 10:00 p.m. Notwithstanding any provision of this instrument to the contrary, the Grantor reserves the right, in its sole discretion, to change the hours during which the Community Space Easement is available for use by the public and to impose reasonable restrictions on the use of the Community Space Easement to enable Grantor to maintain and repair the Property and improvements thereon, provided that such restrictions do not substantially and permanently impair or diminish the rights of the public provided herein. Subject to the terms of this instrument, the public use of the Community Space Easement shall be governed by the City Ordinances of the City of Portsmouth, including without limitation, Chapter 3, Public Health, Article IV Noise Control, Section 3.401 et seq. ("City Ordinance"), so long as the City Ordinance does not conflict with any existing easements affecting the Property or conflict with the existing use of the Property by the Grantor.
- **Reserved Rights.** Grantor reserves the rights to conduct all legally permitted activities within the Community Space Easement, and to alter and improve the Community Space Easement, provided that such activities, alterations and/or improvements do not substantially interfere with the rights granted hereby. Not by way of limitation of the foregoing, Grantor shall have the right to use the Community Space Easement as collateral for subsequent borrowings, provided that any mortgage or lien arising from such borrowing shall be subordinated to this Community Space Easement. Grantor may, from time to time, relocate one or more portions of the Community Space Easement to another location on the Property, subject to approval by the Grantee, which approval shall not be unreasonably withheld, conditioned or delayed.
- 4. <u>Nonexclusive Easement.</u> The Community Space Easement is nonexclusive. Grantor retains the right to make any use of the Community Space Easement, including, but not limited to, the right to utilize the Community Space Easement for outside activities including, the placement of tables, umbrellas and chairs for customer dinning from restaurants during seasonal weather when patrons desire to sit outside, the creation of staging and audience areas for artists, musical performances and other entertainment purposes provided such uses do not unreasonably interfere with the Grantee's and the public's use and enjoyment of the Community Space Easement. Grantor also retains the right to grant concurrent and additional easements on, over or under the Community Space Easement to third parties for such uses as the location of underground improvements, the location of utilities and drainage or otherwise, provided such use or uses to not unreasonably interfere with Grantee's and the public's use and enjoyment of the Community Space Easement. Nothing contained in this Community Space Easement shall be construed an exclusive right to the Grantee, or the general public, and/or as affording the

public a right of access to any portion of the Property other than access which is consistent with this Community Space Easement.

- 5. <u>Maintenance and Repair.</u> The maintenance of the Community Space Easement shall be the sole responsibility of the Grantor, and its successors and assigns.
- 6. <u>Encroachments</u>. The Community Space Easement is subject to all existing encroachments of utilities and improvements on, over and under the Community Space Easement, and to all future encroachments of utilities and improvements constructed or installed on or around the Community Space Easement (subject, however, to the terms of the preceding paragraphs).
- Costs and Liabilities. Grantor agrees to bear all costs and liabilities of any kind related to the operation, upkeep, and maintenance of the Property, and to defend, indemnify, hold harmless, and release the City of Portsmouth, from and against any and all actions, claims, damages, liabilities, or expenses that may be asserted by any person or entity, including Grantor, relating thereto. Without limiting the foregoing, the City of Portsmouth shall not be liable to Grantor or any other person or entity in connection with any entry upon the Property pursuant to this Community Space Easement, or on account of any claim, liability, damage, or expense suffered or incurred by or threatened against Grantor or any other person or entity, except as such claim, liability, damage, or expense is the result of the City of Portsmouth's, its agents or employee's negligence or willful misconduct.
- 8. <u>Acts Beyond Grantor's Control</u>. Nothing contained in this Community Space Easement shall be construed to entitle Grantee to bring any action against Grantor for any injury to or change in the Community Space Easement resulting from causes beyond Grantor's control, including, without limitation, natural processes, by force majeure, without limitation, fire, flood, storm, and earth movement, or from any prudent action taken by Grantor under emergency conditions to prevent, abate, or mitigate significant injury to the Community Space Easement or the remainder of the Property resulting from such causes.
- 9. <u>Covenants Run with the Land.</u> The Community Space Easement granted herein shall be perpetual in nature, shall run with the land and shall benefit and be binding upon the Grantor, its successors and assigns. This Community Space Easement shall be recorded in the Rockingham County Registry of Deeds.
- 10. <u>City Ordinance Application.</u> The use, public or private, of the Community Space Easement shall be subject to and comply with the City Ordinances of the City of Portsmouth.
- 11. <u>Notices.</u> Any notice, demand, request, or other communication that either party desires or is required to give to the other under this Community Space Easement shall be in writing and either served personally or sent by United States mail, postage prepaid, certified, return receipt requested, and shall be mailed to the parties at the following addresses:

To Grantor:

Dagny Taggart, LLC, 30 Penhallow Street, Suite 300 East Portsmouth, NH 03801

To City:

City of Portsmouth, New Hampshire 1 Junkins Avenue Portsmouth, NH 03801

- 12. <u>Amendment.</u> Grantor, or its successors and/or assigns, and City may mutually agree to amend or modify this Community Space Easement, provided that any such amendment or modification is in writing and signed by both parties, and is consistent with the purpose of this Community Space Easement. No amendment or modification of this Community Space Easement shall take effect unless and until it is recorded in the Rockingham County Registry of Deeds.
- 13. <u>Applicable Law.</u> This Community Space Easement shall be construed and interpreted according to the substantive law of the State of New Hampshire.

Meaning and intending to convey an easement over a portion of the Property conveyed to the Grantor by Warranty Deed of Jarbel Realty, LLC, dated April 5, 2019 and recorded at the Rockingham County Registry of Deeds at Book 5990, Page 1701.

This is an exempt transfer pursuant to RSA 78-B:2(I).

IN WITNESS WHEREOF, Grantor and City have executed this Community Space Easement as set forth, below.

Grantor: Dagny Taggart, LLC,
By:
Mark A. McNabb, Manager
Grantee: City of Portsmouth, New Hampshire
By:
John P. Bohenko, City Manager

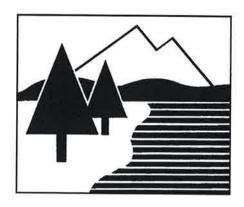
ACKNOWLEDGEMENTS

STATE OF NEW HAMPSHIRE	
COUNTY OF ROCKINGHAM	
personally appeared Mark A. McNabb, Managlimited liability company, proved to me through	9, before me, the undersigned notary public, ger of Dagny Taggart, LLC, a New Hampshire gh satisfactory evidence of identification, which nose name is signed on the preceding or attached e signed it voluntarily for its stated purpose.
	otary Public:
My	y Commission Expires:
STATE OF NEW HAMPSHIRE COUNTY OF ROCKINGHAM	
On this day of, 2019, 10 personally appeared John P. Bohenko, Manager proved to me through satisfactory evidence of license, to be the person whose name is signed acknowledged to me that he/she signed it in his its stated purpose.	r of the City of Portsmouth New Hampshire, f identification, which was a valid driver's on the preceding or attached document, and
No	otary Public:
	y Commission Expires:

DRAINAGE ANALYSIS

SITE REDEVELOPMENT

60 PENHALLOW STREET PORTSMOUTH, NH



October 8, 2019

Revised: November 18, 2019





Amon Engineering, Inc.

Civil Engineers and Land Surveyors 200 Griffin Road, Unit 3 Portsmouth, NH 03801

Phone: 603.430.9282; Fax: 603.436.2315 E-mail: jlm@ambitengineering.com

(Ambit Job Number 3039)

TABLE OF CONTENTS

REPORT

Executive Summary	1
Introduction / Project Description	2
Methodology	3
Site Specific Information	3
Pre-Development Drainage	4
Post-Development Drainage	4
Erosion and Sediment Control Practices	5
Conclusion	6
References	6

APPENDIX

- A. Vicinity (Tax) Map
- B. Tables, Charts, Etc.
- C. HydroCAD Drainage Analysis Calculations
- D. Soil Survey Information
- E. Inspection & Maintenance Plan

ATTACHMENTS

Existing Drainage Plan - W1

Proposed Drainage Plan - W2

EXECUTIVE SUMMARY

This drainage analysis examines the pre-development (existing) and post-development (proposed) stormwater drainage patterns for the proposed development which includes a constructing a multi-story mixed use building at 60 Penhallow Street in Portsmouth, NH. The site is shown on the City of Portsmouth Assessor's Tax Map 107 as Lot 27. The lot size is 23,279 square-feet (0.53 acres).

The new building will be serviced by public water and public sewer. The development has the potential to increase stormwater runoff to adjacent properties, and therefore must be designed in a manner to prevent that occurrence. This will be done primarily by capturing stormwater runoff and routing it through appropriate stormwater facilities, designed to ensure that there will be no increase in peak runoff from the site as a result of this project.

The hydrologic modeling uses the "Extreme Precipitation" values from The Northeast Regional Climate Center (Cornell University) for modeling purposes. Because Portsmouth is in the Seacoast area, we have increased these values by 15% and incorporated these values in this report.

SITE REDEVELOPMENT

60 Penhallow Street

PORTSMOUTH, NH

INTRODUCTION / PROJECT DESCRIPTION

This drainage report is designed to assist the owner, planning board, contractor, regulatory reviewer, and others in understanding the impact of the proposed development project on local surface water runoff and quality. The project site is shown on the City of Portsmouth, NH Assessor's Tax Map 107 Lot 27.

Bounding the site to the north is Daniel Street. Bounding the site to the West are commercial buildings with frontage along Daniel Street and Market Square / Pleasant Street. Bounding the site to the south is a commercial building with frontage on Penhallow Street. Bounding the Site to the east is Penhallow Street. The subject property is situated in the Character District 4 (CD4), Downtown Overlay District (DOD) and the Historic District (HDC). A vicinity map is included in the Appendix to this report.

The proposed development plan is to construct a new commercial building with a below grade, two level garage and other associated improvements such as utilities and landscaping. The project is anticipated to begin construction in the spring of 2020 and be substantially completed by the summer of 2021.

This report includes information about the existing site and the proposed development necessary to analyze stormwater runoff and to design any required mitigation. The report includes maps of pre-development and post-development watersheds, sub-catchment areas and calculations of runoff. The report will provide a narrative of the stormwater runoff and describe numerically and graphically the surface water runoff patterns for this site. Proposed stormwater management and treatment structures and methods will also be described, as well as erosion and sediment control practices. To fully understand the proposed site development the reader should also review a complete site plan set in addition to this report.

METHODOLOGY

This report uses the US Soil Conservation Service (SCS) Method for estimating stormwater runoff. The SCS method is published in The National Engineering Handbook (NEH), Section 4 "Hydrology" and includes the Technical Release No. 20, (TR-20) "Computer Program for Project Formulation Hydrology", and Technical Release No. 55 (TR-55) "Urban Hydrology for Small Watersheds" methods. This report uses the HydroCAD version 10.0 program, written by HydroCAD Software Solutions LLC, Chocorua, N.H., to apply these methods for the calculation of runoff and for pond modeling. Hydrologic modeling employs the "Extreme Precipitation" values from The Northeast Regional Climate Center (Cornell University) increased by 15%. These values have been used and are included in this report.

Time of Concentration (Tc) is calculated by entering measured flow path data such as flow path type, length, slope and surface characteristics into the HydroCAD program. For the purposes of this report, and as directed by TR55, a minimum time of concentration of 5 minutes is used.

The storm events used for the calculations in this report are the 2-year, 10-year and 50-year (24-hour) storms. Watershed basin boundaries have been delineated and subsequently revised using topographic maps prepared and updated by Ambit Engineering survey data, record plans and field observations to confirm.

SITE SPECIFIC INFORMATION

Based on the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS), Soil Survey of Rockingham County, New Hampshire, the site is made up of one soil type:

699 – Urban land. This soil has been assigned a Hydrologic Soil Group (HSG) classification of B, with a Low runoff class.

The physical characteristics of the site consist of (3-15%) grades that generally slope downward into the center of the site. At least three catch basins located on site provide adequate drainage in the existing conditions. Elevations on the site range from 30 to 27 feet above sea level. Currently the site is a private commercial parking lot. The existing vegetation around the lot consists of established grasses, shrubs and trees.

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 33015C0259E (effective date May 17, 2005), the project site is not located in a floodplain. A copy of the FIRM map is included in the Appendix.

PRE-DEVELOPMENT DRAINAGE

The existing site drains via overland flow from the outer bounds of the property towards the center of the site to three catch basins located within the parking lot. These three catch basins combine and discharge to a 12" HDPE through CB 5966 located along the curb line in Penhallow Street. We have placed the design point at the end of the existing 12" HDPE, entering CB 5966 and then into an 18" HDPE main trunkline at DMH 5963. There is no existing stormwater detention or treatment on the site.

In the pre-development condition, the site has been analyzed as four watershed basins (ES1, ES2, ES3 and ES4) based on localized topography and discharge location. As described above, ES1 represents the majority of on site runoff while ES2, ES3 and ES4 are the offsite runoff from adjacent streets. The runoff curve number (CN) for Subcatchment ES1 is calculated to be 91 with impervious coverage of 76.9%. The runoff curve numbers for ES2, ES3 and ES4 is 98 since they are entirely impervious surface consisting of asphalt and brick sidewalk.

Table 1: Pre-Development Watershed Basin Summary

Watershed Basin ID	Basin Area (SF)	Tc (MIN)	CN	2-Year Runoff (CFS)	10-Year Runoff (CFS)	50-Year Runoff (CFS)	Design Point
ES1	30,432	5.0	91	2.25	3.67	5.80	DP1
ES2	4,330	5.0	98	0.37	0.56	0.86	DP1
ES3	1,701	5.0	98	0.14	0.22	0.34	DP1
ES4	803	5.0	98	0.07	0.10	0.16	DP1

POST-DEVELOPMENT DRAINAGE

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. In the post-development condition, the site has been analyzed as four (4) separate subcatchments (PS1, PS2, PS3 and PS4) based on localized topography and discharge locations. In general, the proposed subcatchments are similar area as the existing subcatchemnts. Basin PS1 is the rooftop runoff from the new building. PS2 is the runoff from Daniel Street. PS3 is the runoff from Penhallow Street. PS4 is runoff from the alley way that flows out to Penhallow Street.

The runoff curve number (CN), Time of Concentration (TC), % Impervious, and Peak Flow Rate (CFS) for the Post Development Watersheds are shown in Table 2: Post Development Water Shed Summary below.

Table 2: Post-Development Watershed Basin Summary

Watershed Basin ID	Basin Area (SF)	Tc (MIN)	CN	2-Year Runoff (CFS)	10-Year Runoff (CFS)	50-Year Runoff (CFS)	Design Point
PS1	17,104	5.0	98	1.46	2.23	3.39	DP1
PS2	5,601	5.0	98	0.48	0.73	1.11	DP1
PS3	1,995	5.0	98	0.17	0.26	0.40	DP1
PS4	12,558	5.0	94	1.00	1.58	2.45	DP1

The overall impervious coverage of the area analyzed in this report for all basins **increases** from 30,251 square feet (81.1%) in the pre-development condition to 35,773 square feet (95.9%) in the post-development condition. In the existing condition, parking is on the surface and surfaces treated with asphalt and used for vehicles are known to be high pollutant load areas. In the proposed condition this parking is located underground and since runoff from the site in the proposed condition is largely roof top and brick type paver walkways, there is no real need for treatment of stormwater runoff as the runoff will be relatively clean.

Table 3 shows a summary of the comparison between pre-developed flows and post-developed flows for the design point.

Table 3: Pre-Development to Post-Development Comparison

	Q2 (CFS)	Q10	(CFS)	Q25 ((CFS)	Q50	(CFS)
Design Point	Pre	Post	Pre	Post	Pre	Post	Pre	Post
DP1	2.83	3.11	4.56	4.80	5.90	6.11	7.16	7.34

EROSION AND SEDIMENT CONTROL PRACTICES

The erosion potential for this site as it exists is low due to the existing pavement at the site. During construction, the major potential for erosion is wind and stormwater runoff. The

contractor will be required to inspect and maintain all necessary erosion control measures, as well as installing any additional measures as required. All erosion control practices shall conform to "The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire." Some examples of erosion and sediment control measures to be utilized for this project during construction may include:

- Silt Soxx (or approved alternative) located at the toe of disturbed slopes
- Stabilized construction entrance at access point to the site
- Temporary mulching and seeding for disturbed areas
- Spraying water over disturbed areas to minimize wind erosion

After construction, permanent stabilization will be accomplished by permanent seeding, landscaping and surfacing the access drives and parking areas with asphalt paving

CONCLUSION

The existing site is largely impervious surface. The proposed development will add a nominal amount of impervious surface to the overall area. This results in marginal increases between 0.28 cfs and 0.18 cfs in stormwater runoff for the range of storms analyzed. Considering that there is a closed drainage system located within Penhallow Street, in our opinion these increases can be absorbed with no concern for negative impacts.

REFERENCES

- 1. City of Portsmouth, NH. Site Plan Review Regulations amended September 15, 2016.
- 2. Comprehensive Environmental Inc. and New Hampshire Department of Environmental Services. *New Hampshire Stormwater Manual (Volumes 1, 2 and 3)*, December 2008 (Revision 1.0).
- 3. Minnick, E.L. and H.T. Marshall. Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing Areas in New Hampshire, prepared by Rockingham County Conservation District, prepared for New Hampshire Department of Environmental Services, in cooperation with USDA Soil Conservation Service, August 1992.
- 4. HydroCAD Software Solution, LLC. *HydroCAD Stormwater Modeling System Version* 10.0 copyright 2013. HydroCAD Software Solution, LLC. *HydroCAD Stormwater Modeling System Version* 10.0 copyright 2013.
- 5. University of New Hampshire Stormwater Center 2009 Biannual Report, Pages 14-21 for references to Lag time (TC) for Porous Pavement and Filtration Basins.

APPENDIX A VICINITY (TAX) MAP

60 Penhallow Street



Property Information

Property ID 0107-0027-0000 Location DANIEL ST Owner





MAP FOR REFERENCE ONLY NOT A LEGAL DOCUMENT

City of Portsmouth, NH makes no claims and no warranties, expressed or implied, concerning the validity or accuracy of the GIS data presented on this map.

Geometry updated 4/1/2019 Data updated 7/17/2019

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	581	

APPENDIX B TABLES, CHARTS, ETC.

×		

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing Yes	Yes	
State	New Hampshire	Q2 = 3.20 X 1.15 = 3.68
Location		010 4 010 110
Longitude	70.756 degrees West	$Q10 = 4.80 \text{ A} \cdot 1.15 = 5.59$
Latitude	43.077 degrees North	$O25 = 6.16 \times 1.15 = 7.08$
Elevation	0 feet	
Date/Time	Mon, 30 Sep 2019 15:36:40 -0400	$Q50 = 7.37 \times 1.15 = 8.48$

Extreme Precipitation Estimates

Q100 = 8.83 X 1.15 = 10.15

	lyr	2yr	5yr	10yr	25yr	50yr	100yr	200yr	500yr
10day	4.54	5.32	69.9	7.96	10.03	11.95	14.24	16.97	21.43
7day	3.94	4.67	5.93	7.09	9.00	10.79	12.93	10.58 12.52 200yr 9.36 12.04 13.72 15.50	500yr 3.00 4.38 5.76 7.70 10.20 13.44 16.10 500yr 11.90 15.48 17.62 19.72 21.43
4day	3.22	3.93	5.03	6.07	7.79	9.40	9.96 11.35 12.93	13.72	17.62
2day	2.81	3.43	4.40	5.31	6.81	8.24	-	12.04	15.48
1day	2.35	2.84	3.59	4.30	5.45	6.52	7.81	9.36	11.90
	1yr	2yr	5yr	10yr	25yr	50yr	100yr	200yr	500yr
48hr	2.92	3.57	4.57	5.52	7.09	8.57	10.36	12.52	16.10
12hr 24hr	2.65	3.20	4.06	4.86	6.16	7.37	8.83	10.58	13.44
	2.03	2.48	3.14	3.74	4.73	59:5	92.9	8.07	10.20
3hr 6hr	1.21 1.56	1.94	1.89 2.43	1.73 2.23 2.89	1.53 2.14 2.78 3.63	1.79 2.53 3.29 4.32	100yr 2.09 2.98 3.90 5.15	200yr 2.44 3.51 4.61 6.12	7.70
3hr	1.21	1.18 1.52	1.89	2.23	2.78	3.29	3.90	4.61	5.76
. 2hr	86.0	8 1.18	3 1.47	5 1.73	3 2.14	2.53) 2.98	13.51	14.38
1hr	0.70	0.88	1.08	1.25	_		- 2.09	- 2.4	. 3.00
	1yr	2yr	5yr	10yr	25yr	50yr	$ 100y_1 $	200yı	500yı
5min 10min 15min 30min 60min 120min	1.04	1.30	1.61	1.89	2.34	2.76	3.26	3.83	4.76
60min	0.81	1.02	1.25	1.45	1.77	2.07	2.42	2.82	3.48
30min	9.65	0.81	0.97	1.11	1.34	1.54	1.77	2.05	2.48
15min	0.50	0.62	0.73	0.82	0.97	1.10	1.25	1.43	1.71
10min	0.40	0.50	0.58	0.65	25yr 0.48 0.76 0.97	50yr 0.54 0.86	100yr 0.60 0.97	1.10	500yr 0.80 1.31
5min	0.26	0.32	0.37	0.41	0.48	0.54	09.0	0.67	0.80
	lyr	2yr	5yr	10yr	25yr	50yr	100yr	200yr 0.67	500yr

Lower Confidence Limits

Г	lyr	F	5yr	10yr
Ļ		2yr	⊢	\vdash
10day	3.89	5.08	6.22	7.17
7day	3.19	4.54	5.52	6:39
1day 2day 4day 7day 10day	1.98 2.38 2.86 3.19	2.70 3.31 3.82 4.54	3.34 4.01 4.71 5.52	5.42
2day	2.38	3.31	4.01	4.65
1day	1.98	2.70	3.34	3.86
	lyr	2yr	5yr	10yr 3.86 4.65 5.42 6.39
1hr 2hr 3hr 6hr 12hr 24hr 48hr	0.63 0.86 0.93 1.33 1.68 2.23 2.47	0.86 1.16 1.37 1.82 2.34 3.05 3.44	1.01 1.37 1.61 2.12 2.73 3.78 4.17	1.14 1.56 1.80 2.39 3.06 4.36 4.84
24hr	2.23	3.05	3.78	4.36
12hr	1.68	2.34	2.73	3.06
6hr	1.33	1.82	2.12	2.39
3µr	0.93	1.37	1.61	1.80
2hr	0.86	1.16	1.37	1.56
1hr	0.63	0.86	1.01	1.14
	1yr	2yr	5yr	10yr
120min	0.88	1.19	1.40	1.60
60min	0.73	1.00	1.17	1.32
30min	0.59	0.81	0.92	1.02
15min	0.44	0.60	0.67	0.73
5min 10min 15min 30min 60min 120mi	0.23 0.36 0.44 0.59 0.73	0.49 0.60 0.81	0.35 0.54 0.67 0.92	10yr 0.38 0.59 0.73
5min	0.23	0.31	0.35	0.38
	1yr	2yr	5yr	10yr

Extreme Precipitation Tables: 43.077°N, 70.756°W

5min10min15min30min60min120min14h3hr6hr12h24hr48hr										I						Ì						
0.83 1.18 1.56 1.90 0.91 1.31 1.76 2.16 1.01 1.46 2.00 2.47 1.12 1.63 2.27 2.81 1.31 1.90 2.70 3.36		5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
0.91 1.31 1.76 2.16 1.01 1.46 2.00 2.47 1.12 1.63 2.27 2.81 1.31 1.90 2.70 3.36	25yr	0.44	0.67	0.83	1.18	1.56	1.90	25yr	1.35	1.86	2.10	2.75	3.53	4.71	5.86	25yr	4.17	5.63	6.61	7.75	8.64	25yr
1.01 1.46 2.00 2.47 1.12 1.63 2.27 2.81 1.31 1.90 2.70 3.36	50yr	0.48	0.73	0.91	1.31	1.76	2.16	_	1.52	2.12	2.34	3.07	3.92	5.32		50yr	4.71	6.50	7.67	8.99	6.97	50yr
1.12 1.63 2.27 2.81 1.31 1.90 2.70 3.36	100yr	0.53	0.81	1.01	-	2.00	2.47	100yr	1.73	2.41	2.62	3.41	4.34	5.98	7.79	$100 \mathrm{yr}$	5.30	7.49	8.89	10.43	11.50	100yr
3.36	200yr	0.59	68.0	1.12	1.63	2.27		200yr	1.96	2.75	2.93	3.78	4.78	6.71	8.97	200yr	5.93	8.63	10.30	12.13	13.29	200yr
	500yr	89.0	1.01	1.31	1.90	2.70	i	500yr	2.33	3.28	3.41	4.31	5.43	7.80	10.82	$500 \mathrm{yr}$	06.9	10.41	12.52	14.82	16.09	500yr

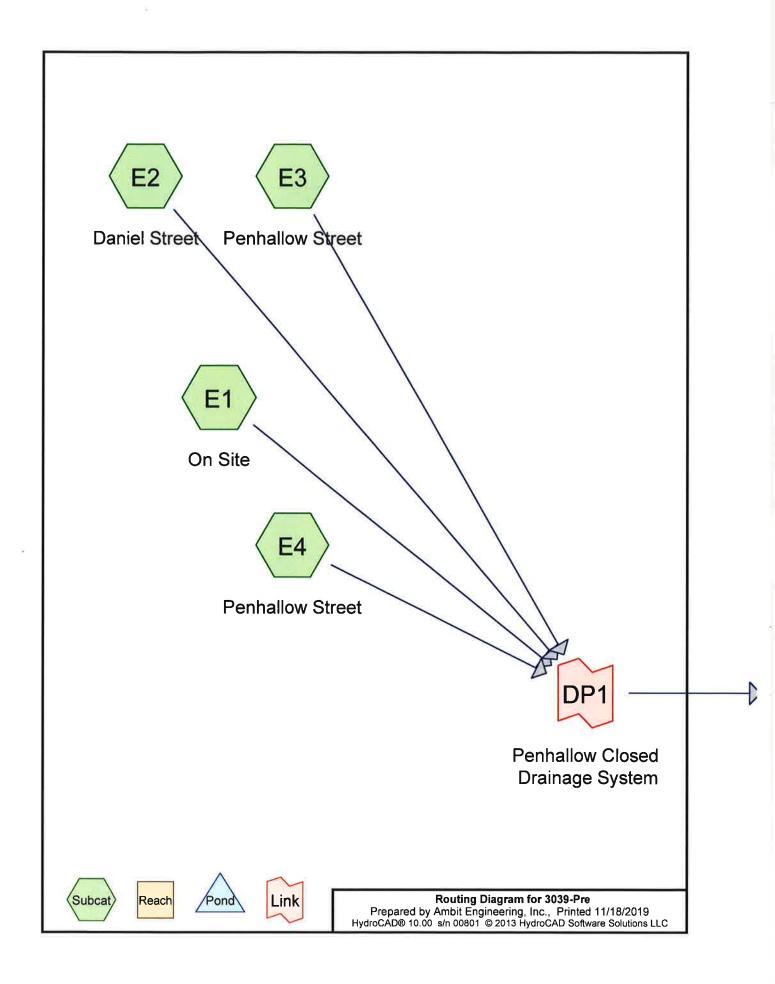
Upper Confidence Limits

1µr 2µr 3µr 4µr 4µr <th></th> <th>. 01</th> <th></th> <th>ΙF</th> <th>. 06</th> <th></th> <th>. 00</th> <th></th> <th></th> <th>7</th> <th>21.5</th> <th>1.7</th> <th>15,</th> <th>2415.2</th> <th>401</th> <th></th> <th>1 3 0 2.</th> <th>2 down</th> <th>Adox</th> <th>74087</th> <th>10dox</th> <th></th>		. 01		ΙF	. 06		. 00			7	21.5	1.7	15,	2415.2	401		1 3 0 2.	2 down	Adox	74087	10dox	
1.08 1yr 0.77 1.06 1.26 1.74 2.20 2.98 3.16 1yr 2.63 3.04 3.57 4.37 5.03 1.27 2yr 0.92 1.24 1.48 1.96 2.52 3.42 3.70 2yr 3.56 4.09 4.84 5.62 1.62 5yr 1.15 1.58 1.88 2.54 3.25 4.96 5yr 3.84 4.77 5.37 6.37 7.15 1.62 5yr 1.15 1.58 1.88 2.54 4.35 6.87 3.84 4.77 5.37 6.37 7.15 1.98 1.09 4.37 5.36 5.37 6.81 7.76 8.35 25yr 6.87 8.03 7.14 1.41 1.41 2.57 2.5yr 6.87 8.60 8.05 6.33 9.71 10.48 50yr 8.60 1.08 11.41 17.10 3.81 100yr 2.56 3.73 <	5min 10min 15min 30min 60min 120min	10min 15min 30min	15min 30min	30min		60min	120min		Ihr	2hr	3hr	ohr	12hr	24hr	48hr		Iday	2day	4day	/day	10day	
1.27 2yr 0.92 1.24 1.96 2.52 3.42 3.70 2yr 3.56 4.09 4.84 5.62 1.62 5yr 1.15 1.28 1.28 3.25 4.33 4.96 5yr 3.84 4.77 5.37 6.37 7.15 1.198 1.09r 1.39 1.93 2.28 3.11 3.96 5.33 6.21 10yr 4.72 5.97 6.83 7.84 8.75 7.15 2.57 25yr 1.77 2.51 2.96 4.07 5.16 7.76 8.35 25yr 6.87 8.03 9.17 10.34 11.41 2.57 25yr 1.77 2.51 2.96 4.07 5.16 7.76 8.35 25yr 6.87 8.03 9.17 10.34 11.41 3.13 3.05 3.06 3.06 6.33 9.71 10.48 50yr 10.75 12.64 14.37 15.71 17.10 4.65	0.28 0.44 0.54 0.72	0.54		0.72		0.89	1.08		0.77	1.06	1.26	1.74	2.20	2.98	3.16	1yr	2.63	3.04	3.57	4.37	5.03	1yr
1.62 5yr 1.15 1.88 2.54 3.25 4.33 4.96 5yr 3.84 4.77 5.37 6.37 7.15 1.98 10yr 1.98 1.0yr 4.72 5.97 6.83 7.84 8.75 2.57 2.5yr 1.77 2.51 2.96 4.07 5.16 7.76 8.35 25yr 6.87 8.03 9.17 10.34 11.41 3.13 50yr 2.12 3.06 3.06 5.06 6.33 9.71 10.48 50yr 8.60 10.08 11.48 12.73 13.97 3.81 100yr 2.56 3.73 4.38 6.16 7.78 12.15 13.14 100yr 10.75 12.64 14.37 15.71 17.10 4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 18.23 21.44 24.31 25.55 27.36	0.34 0.52 0.64 0.86	0.64		98.0		1.07	1.27	$\overline{}$	0.92	1.24	_	1.96	2.52	3.42	3.70	2yr		3.56	4.09	4.84	5.62	2yr
1.98 10yr 1.39 1.93 2.28 3.11 3.96 5.33 6.21 10yr 4.72 5.97 6.83 7.84 8.75 2.57 2.5yr 1.77 2.51 2.96 4.07 5.16 7.76 8.35 25yr 6.87 8.03 9.17 10.34 11.41 3.13 50yr 2.12 3.06 3.60 6.33 9.71 10.48 50yr 8.60 10.08 11.48 12.73 13.97 3.81 100yr 2.56 3.73 4.38 6.16 7.78 12.15 13.14 100yr 10.75 12.64 14.37 15.71 17.10 4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 18.23 21.44 24.31 25.55 27.36	0.40 0.62 0.76 1.05	92.0	$\overline{}$	1.05		1.34	1.62		1.15	1.58	1.88	2.54	3.25	4.33	4.96	5yr	3.84	4.77	5.37	6.37	7.15	5yr
2.57 25yr 1.77 2.51 2.96 4.07 5.16 7.76 8.35 25yr 6.87 8.03 9.17 10.34 11.41 3.13 50yr 2.12 3.06 3.60 5.00 6.33 9.71 10.48 50yr 8.60 10.08 11.48 12.73 13.97 3.81 100yr 2.56 3.73 4.38 6.16 7.78 12.15 13.14 10.075 12.64 14.37 15.71 17.10 4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 13.49 15.86 18.02 19.37 20.93 6.04 500yr 3.92 5.90 6.94 10.03 12.60 20.59 20.29 500yr 18.23 21.44 24.31 25.55 27.36	10yr 0.47 0.72 0.89 1.24	0.89		1.24		1.61	1.98		1.39	1.93	2.28	3.11	3.96	5.33	6.21	10yr	4.72	5.97	6.83	7.84	8.75	10yr
3.13 50yr 2.12 3.06 3.60 6.33 9.71 10.48 50yr 8.60 10.08 11.48 12.73 13.97 3.81 100yr 2.56 3.73 4.38 6.16 7.78 12.15 13.14 100yr 10.75 12.64 14.37 15.71 17.10 4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 13.49 15.86 18.02 19.37 20.93 6.04 500yr 3.92 5.90 6.94 10.03 12.60 20.59 20.229 500yr 18.23 21.44 24.31 25.55 27.36	25yr 0.58 0.88 1.09 1.56	1.09		1.56		2.05		25yr	1.77	2.51	2.96	4.07	5.16	1.76	8.35	25yr		8.03	9.17	10.34		25yr
3.81 100yr 2.56 3.73 4.38 6.16 7.78 12.15 13.14 100yr 10.75 12.64 14.37 15.71 17.10 17.10 4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 13.49 15.86 18.02 19.37 20.93 6.04 500yr 3.92 5.90 6.94 10.03 12.60 20.59 22.29 500yr 18.23 21.44 24.31 25.55 27.36	50yr 0.67 1.02 1.27 1.83	1.27	_	1.83		2.46			2.12	3.06	3.60	5.00	6.33	9.71	10.48	50yr	8.60	10.08	11.48	12.73	13.97	50yr
4.65 200yr 3.07 4.55 5.34 7.59 9.56 15.24 16.50 200yr 13.49 15.86 18.02 19.37 20.93 6.04 500yr 3.92 5.90 6.94 10.03 12.60 20.59 22.29 500yr 18.23 21.44 24.31 25.55 27.36	100yr 0.79 1.19 1.50 2.16 2.96	_	_	2.16		2.96		100yr	2.56	3.73	4.38	6.16	7.78	12.15	13.14	100yr	10.75	12.64	14.37	15.71	17.10	100yr
6.04 500yr 3.92 5.90 6.94 10.03 12.60 20.59 22.29 500yr 18.23 21.44 24.31 25.55 27.36	200yr 0.92 1.39 1.76 2.55	1.76		2.55		3.56		200yr	3.07	4.55	5.34	7.59	9:26	15.24	16.50	200yr	13.49	15.86	18.02	19.37	20.93	200yr
	500yr 1.15 1.71 2.20 3.19 4.54	1.71 2.20 3.19	2.20 3.19	3.19	_	4.54		500yr	3.92	5.90	6.94	10.03	12.60	20.59	22.29	$500 \mathrm{yr}$	18.23	21.44	24.31	25.55	27.36	500yr



APPENDIX C HYDROCAD DRAINAGE ANALYSIS CALCULATIONS

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Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.129	61	>75% Grass cover, Good, HSG B (E1)
0.032	85	Gravel roads, HSG B (E1)
0.459	98	Paved parking, HSG B (E1)
0.157	98	Paved roads w/curbs & sewers, HSG B (E2, E3, E4)
0.078	98	Unconnected roofs, HSG B (E1)
0.856	92	TOTAL AREA

Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.856	HSG B	E1, E2, E3, E4
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.856		TOTAL AREA

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Ground Covers (selected nodes)

 HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.129	0.000	0.000	0.000	0.129	>75% Grass cover, Good	E1
0.000	0.032	0.000	0.000	0.000	0.032	Gravel roads	E1
0.000	0.459	0.000	0.000	0.000	0.459	Paved parking	E1
0.000	0.157	0.000	0.000	0.000	0.157	Paved roads w/curbs & sewers	E2,
							E3,
							E4
0.000	0.078	0.000	0.000	0.000	0.078	Unconnected roofs	E1
0.000	0.856	0.000	0.000	0.000	0.856	TOTAL AREA	

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Pipe Listing (selected nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	E1	0.00	0.00	62.0	0.0050	0.013	12.0	0.0	0.0
2	E1	0.00	0.00	42.0	0.0031	0.013	12.0	0.0	0.0

Type III 24-hr 2YR-Extreme Rainfall=3.68"

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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method

Subcatchment E1: On Site Runoff Area=30,432 sf 76.89% Impervious Runoff Depth=2.71"

Flow Length=227' Tc=5.0 min CN=91 Runoff=2.25 cfs 0.158 af

Subcatchment E2: Daniel Street Runoff Area=4,330 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.37 cfs 0.029 af

Subcatchment E3: Penhallow Street Runoff Area=1,701 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.14 cfs 0.011 af

Subcatchment E4: Penhallow Street Runoff Area=803 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.07 cfs 0.005 af

Link DP1: Penhallow Closed Drainage System Inflow=2.83 cfs 0.203 af

Primary=2.83 cfs 0.203 af

Total Runoff Area = 0.856 ac Runoff Volume = 0.203 af Average Runoff Depth = 2.85" 18.87% Pervious = 0.161 ac 81.13% Impervious = 0.694 ac

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Summary for Subcatchment E1: On Site

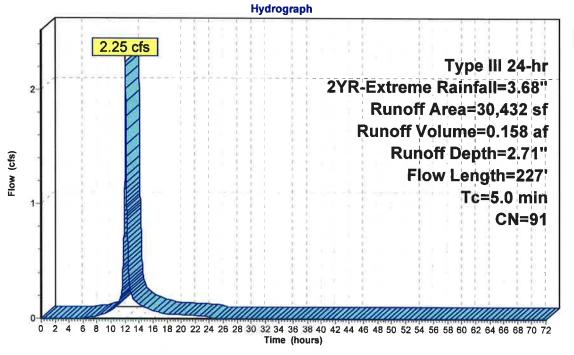
Runoff 2.25 cfs @ 12.07 hrs, Volume= 0.158 af, Depth= 2.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

A	rea (sf)	CN E	escription		
	3,398	98 L	Inconnecte	ed roofs, HS	SG B
	20,002	98 F	aved park	ing, HSG B	
	5,633				ood, HSG B
	1,399		ravel road		,
2	30,432	91 V	Veighted A	verage	
	7,032		•	vious Area	
	23,400			pervious Ar	
	3,398		4.52% Un		
	,				
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
1.0	80	0.0179	1.27		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.26"
0.2	43	0.0306	3.55		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.3	62	0.0050	3.21	2.52	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.3	42	0.0031	2.53	1.98	·
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
1.8	227	Total, I	ncreased t	o minimum	Tc = 5.0 min

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Subcatchment E1: On Site





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Summary for Subcatchment E2: Daniel Street

Runoff =

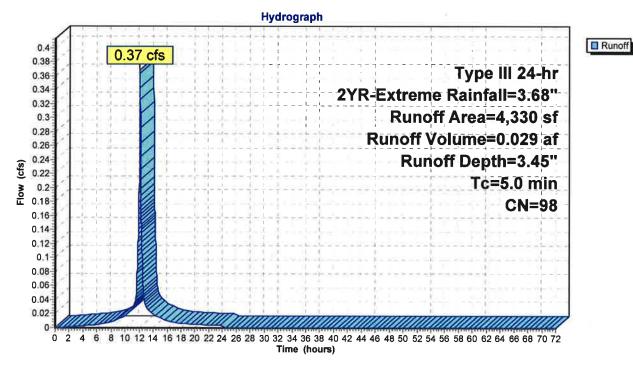
0.37 cfs @ 12.07 hrs, Volume=

0.029 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

Α	rea (sf)	CN [Description		
	4,330	98 F	Paved road	s w/curbs 8	k sewers, HSG B
	4,330	1	00.00% Im	npervious A	rea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry

Subcatchment E2: Daniel Street



Summary for Subcatchment E3: Penhallow Street

Runoff

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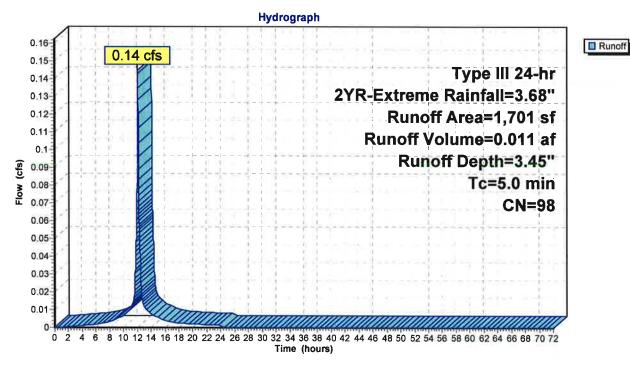
0.14 cfs @ 12.07 hrs, Volume=

0.011 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

A	rea (sf)	CN [Description		
	1,701	98 F	Paved road	s w/curbs 8	& sewers, HSG B
	1,701	1	00.00% In	npervious A	леа
_					
Tc	Length	Slope	•	Capacity	Description
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Subcatchment E3: Penhallow Street



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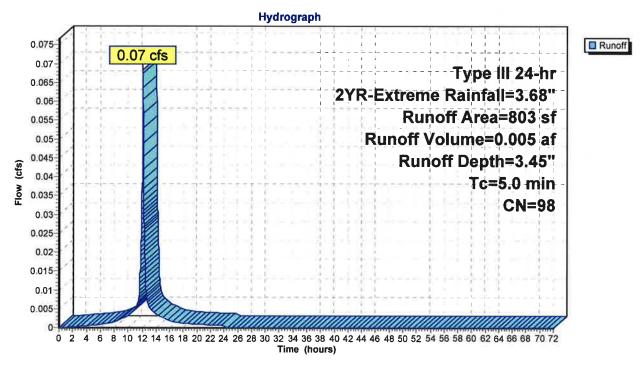
Summary for Subcatchment E4: Penhallow Street

Runoff = 0.07 cfs @ 12.07 hrs, Volume= 0.005 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

_	Area (sf)		CN [Description			
803 98 Paved roads w/curbs & sewers, HSG B					& sewers, HSG B		
803 100.00% Impervious A						rea	
	Tc	Lenath	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description	
	5.0		7. 2.			Direct Entry,	

Subcatchment E4: Penhallow Street



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Summary for Link DP1: Penhallow Closed Drainage System

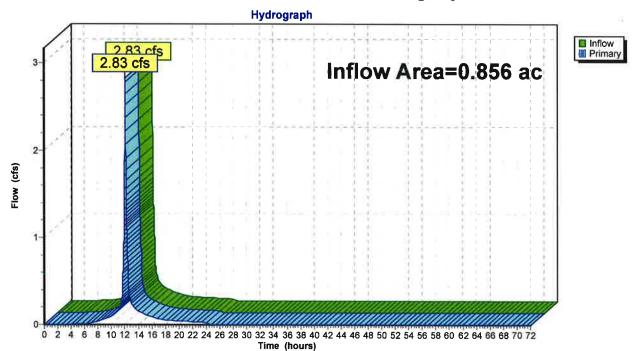
Inflow Area = 0.856 ac, 81.13% Impervious, Inflow Depth = 2.85" for 2YR-Extreme event

Inflow = 2.83 cfs @ 12.07 hrs, Volume= 0.203 af

Primary = 2.83 cfs @ 12.07 hrs, Volume= 0.203 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment E1: On Site

Runoff Area=30,432 sf 76.89% Impervious Runoff Depth=4.56" Flow Length=227' Tc=5.0 min CN=91 Runoff=3.67 cfs 0.265 af

Subcatchment E2: Daniel Street

Runoff Area=4,330 sf 100.00% Impervious Runoff Depth=5.35" Tc=5.0 min CN=98 Runoff=0.56 cfs 0.044 af

Subcatchment E3: Penhallow Street

Runoff Area=1,701 sf 100.00% Impervious Runoff Depth=5.35" Tc=5.0 min CN=98 Runoff=0.22 cfs 0.017 af

Subcatchment E4: Penhallow Street

Runoff Area=803 sf 100.00% Impervious Runoff Depth=5.35" Tc=5.0 min CN=98 Runoff=0.10 cfs 0.008 af

Link DP1: Penhallow Closed Drainage System

Inflow=4.56 cfs 0.335 af Primary=4.56 cfs 0.335 af

Total Runoff Area = 0.856 ac Runoff Volume = 0.335 af Average Runoff Depth = 4.70" 18.87% Pervious = 0.161 ac 81.13% Impervious = 0.694 ac Prepared by Ambit Engineering, Inc.

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Summary for Subcatchment E1: On Site

Runoff 3.67 cfs @ 12.07 hrs, Volume= 0.265 af, Depth= 4.56"

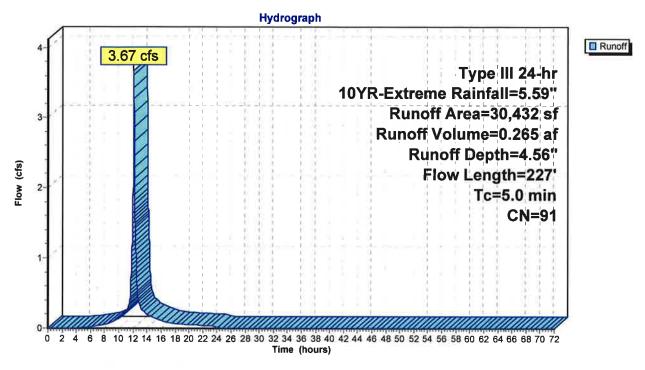
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

A	rea (sf)	CN D	escription					
	3,398	98 U	98 Unconnected roofs, HSG B					
	20,002	98 P	aved park	ing, HSG B				
	5,633	61 >	75% Grass	s cover, Go	ood, HSG B			
	1,399	85 G	ravel road	s, HSG B				
	30,432	91 V	Veighted A	verage				
	7,032	2	3.11% Per	vious Area				
	23,400	7	6.89% Imp	ervious Are	ea			
	3,398	1	4.52% Und	connected				
Tc		Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
1.0	80	0.0179	1.27		Sheet Flow,			
					Smooth surfaces n= 0.011 P2= 3.26"			
0.2	43	0.0306	3.55		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
0.3	62	0.0050	3.21	2.52				
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'			
					n= 0.013 Corrugated PE, smooth interior			
0.3	42	0.0031	2.53	1.98	Pipe Channel,			
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'			
					n= 0.013 Corrugated PE, smooth interior			
1.8	227	Total, 1	ncreased t	o minimum	Tc = 5.0 min			

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Subcatchment E1: On Site



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Summary for Subcatchment E2: Daniel Street

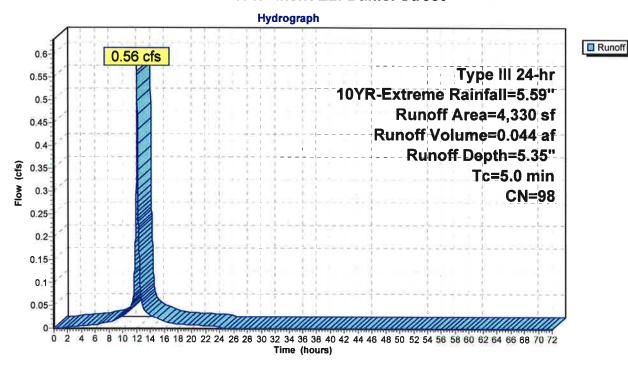
Runoff = 0.56 cfs @ 12.07 hrs, Volume=

0.044 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	A	rea (sf)	CN E	Description							
		4,330	98 F	aved roads w/curbs & sewers, HSG B							
		4,330	100.00% Impervious Area								
	То	Longth	Clana	\/alaaih.	Conseile	Description					
	(min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	(cfs)	Description					
٠.	5.0					Direct Entry	=				

Subcatchment E2: Daniel Street



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Summary for Subcatchment E3: Penhallow Street

Runoff =

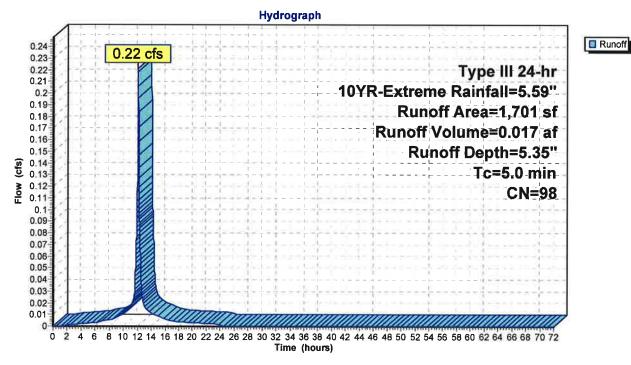
= 0.22 cfs @ 12.07 hrs, Volume=

0.017 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	A	rea (sf)	CN [Description							
22		1,701	98 F	Paved roads w/curbs & sewers, HSG B							
-		1,701	1	100.00% Impervious Area							
2	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)						
	5.0			Direct Entry							

Subcatchment E3: Penhallow Street



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■ Runoff

Summary for Subcatchment E4: Penhallow Street

Runoff

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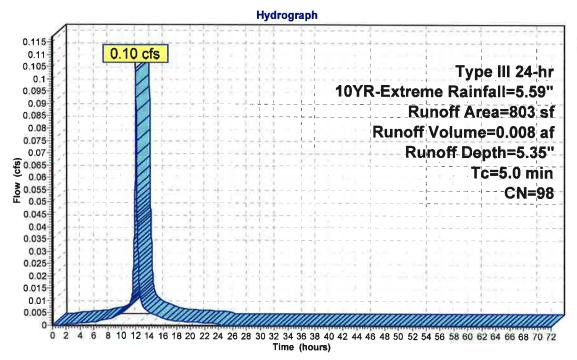
0.10 cfs @ 12.07 hrs, Volume=

0.008 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

V-	A	rea (sf)	CN E	Description								
		803	98 F	98 Paved roads w/curbs & sewers, HSG B								
		803	803 100.00% Impervious Area									
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	5.0					Direct Entry						

Subcatchment E4: Penhallow Street



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Summary for Link DP1: Penhallow Closed Drainage System

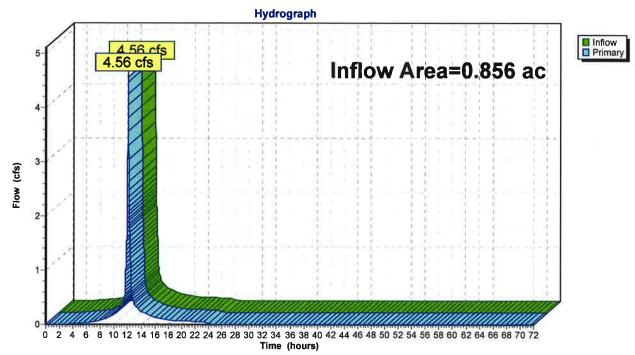
Inflow Area = 0.856 ac, 81.13% Impervious, Inflow Depth = 4.70" for 10YR-Extreme event

Inflow = 4.56 cfs @ 12.07 hrs, Volume= 0.335 af

Primary = 4.56 cfs @ 12.07 hrs, Volume= 0.335 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



Type III 24-hr 25YR-Extreme Rainfall=7.08"

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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method

Subcatchment E1: On Site

Runoff Area=30,432 sf 76.89% Impervious Runoff Depth=6.02"

Flow Length=227' Tc=5.0 min CN=91 Runoff=4.78 cfs 0.350 af

Subcatchment E2: Daniel Street

Runoff Area=4,330 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.72 cfs 0.057 af

Subcatchment E3: Penhallow Street

Runoff Area=1,701 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.28 cfs 0.022 af

Subcatchment E4: Penhallow Street

Runoff Area=803 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.13 cfs 0.011 af

Link DP1: Penhallow Closed Drainage System

Inflow=5.90 cfs 0.440 af

Primary=5.90 cfs 0.440 af

Total Runoff Area = 0.856 ac Runoff Volume = 0.440 af Average Runoff Depth = 6.17" 18.87% Pervious = 0.161 ac 81.13% Impervious = 0.694 ac Prepared by Ambit Engineering, Inc.

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Summary for Subcatchment E1: On Site

4.78 cfs @ 12.07 hrs, Volume= Runoff

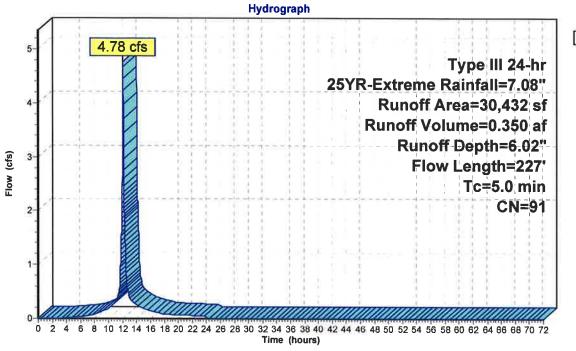
0.350 af, Depth= 6.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

A	rea (sf)	CN E	escription				
	3,398	98 L	Inconnected roofs, HSG B				
	20,002	98 F	aved park	ing, HSG B			
	5,633	61 >	75% Gras	s cover. Go	ood, HSG B		
	1,399		Gravel road		,		
Ú.	30,432	91 V	Veighted A	verage			
	7,032			vious Area			
	23,400			pervious Are			
	3,398		4.52% Und				
	-,						
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
1.0	80	0.0179	1.27		Sheet Flow,		
5					Smooth surfaces n= 0.011 P2= 3.26"		
0.2	43	0.0306	3.55		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
0.3	62	0.0050	3.21	2.52	· · · · · · · · · · · · · · · · · · ·		
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
0.3	42	0.0031	2.53	1.98	· · · · · · · · · · · · · · · · · · ·		
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
1.8	227	Total, I	ncreased t	o minimum	Tc = 5.0 min		

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Subcatchment E1: On Site





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Summary for Subcatchment E2: Daniel Street

Runoff =

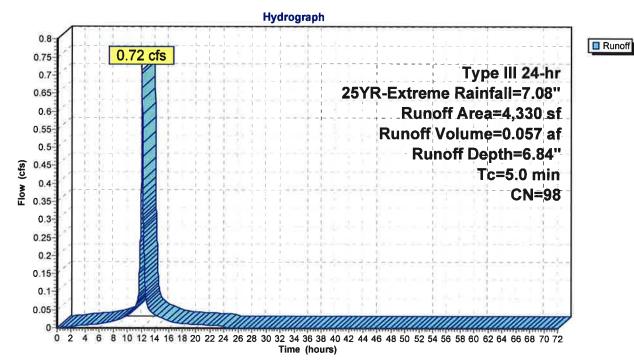
= 0.72 cfs @ 12.07 hrs, Volume=

0.057 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

2	A	rea (sf)	CN [Description							
		4,330	98 F	Paved roads w/curbs & sewers, HSG B							
-		4,330	1	100.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	5.0			Direct Entry.							

Subcatchment E2: Daniel Street



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Summary for Subcatchment E3: Penhallow Street

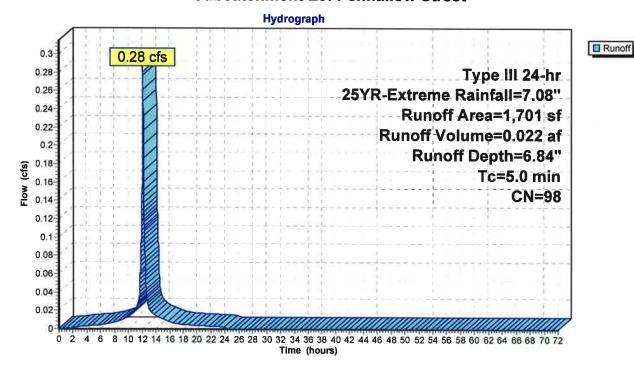
Runoff = 0.28 cfs @ 12.07 hrs, Volume=

0.022 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

	A	rea (sf)	CN [Description							
		1,701	98 F	Paved roads w/curbs & sewers, HSG B							
		1,701	1	100.00% Impervious Area							
	_		•								
	Tc		Slope			Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry.					

Subcatchment E3: Penhallow Street



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Summary for Subcatchment E4: Penhallow Street

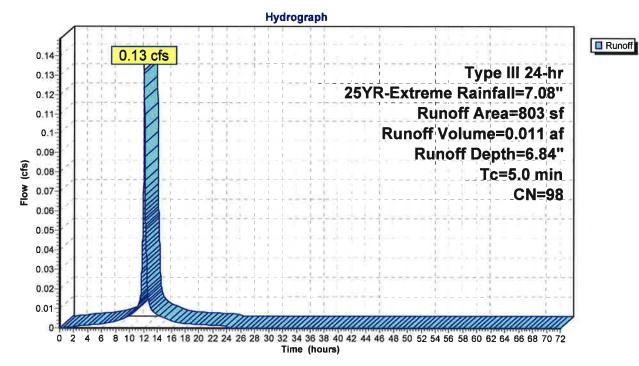
Runoff = 0.13 cfs @ 12.07 hrs, Volume=

0.011 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

	A	rea (sf)	CN [Description								
0.5		803	98 F	Paved road	aved roads w/curbs & sewers, HSG B							
		803 100.00% Impervious Area										
	_				_							
	T¢	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
0.5	5.0					Direct Entry						

Subcatchment E4: Penhallow Street



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Inflow Area = 0.856 ac, 81.13% Impervious, Inflow Depth = 6.17" for 25YR-Extreme event

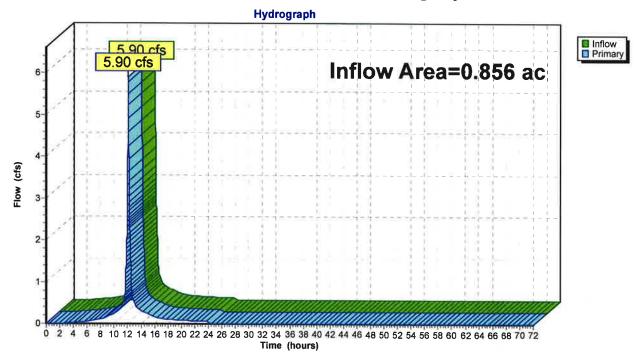
Summary for Link DP1: Penhallow Closed Drainage System

Inflow = 5.90 cfs @ 12.07 hrs, Volume= 0.440 af

Primary = 5.90 cfs @ 12.07 hrs, Volume= 0.440 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



Type III 24-hr 50YR-Extreme Rainfall=8.48"

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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment E1: On Site

Runoff Area=30,432 sf 76.89% Impervious Runoff Depth=7.40" Flow Length=227' Tc=5.0 min CN=91 Runoff=5.80 cfs 0.431 af

Subcatchment E2: Daniel Street

Runoff Area=4,330 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=0.86 cfs 0.068 af

Subcatchment E3: Penhallow Street

Runoff Area=1,701 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=0.34 cfs 0.027 af

Subcatchment E4: Penhallow Street

Runoff Area=803 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=0.16 cfs 0.013 af

Link DP1: Penhallow Closed Drainage System

Inflow=7.16 cfs 0.538 af Primary=7.16 cfs 0.538 af

Total Runoff Area = 0.856 ac Runoff Volume = 0.538 af Average Runoff Depth = 7.55" 18.87% Pervious = 0.161 ac 81.13% Impervious = 0.694 ac

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Summary for Subcatchment E1: On Site

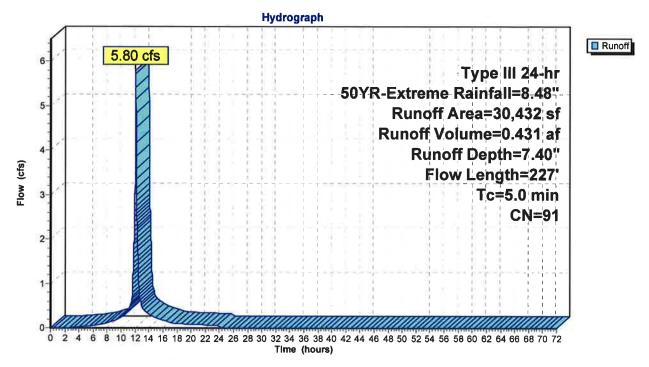
Runoff 5.80 cfs @ 12.07 hrs, Volume= 0.431 af, Depth= 7.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

A	rea (sf)	CN D	escription				
	3,398	98 L	98 Unconnected roofs, HSG B				
	20,002			ing, HSG B			
	5,633	61 >	75% Gras	s cover. Go	ood, HSG B		
	1,399		Fravel road				
	30,432	91 V	Veighted A	verage			
	7,032			vious Area			
	23,400			pervious Ar			
	3,398			connected	-		
	-,						
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
1.0	80	0.0179	1.27		Sheet Flow,		
					Smooth surfaces n= 0.011 P2= 3.26"		
0.2	43	0.0306	3.55		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
0.3	62	0.0050	3.21	2.52	• • • • • • • • • • • • • • • • • • •		
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
0.3	42	0.0031	2.53	1.98			
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
1.8	227	Total, I	ncreased t	o minimum	Tc = 5.0 min		

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Subcatchment E1: On Site



Runoff

Summary for Subcatchment E2: Daniel Street

Runoff

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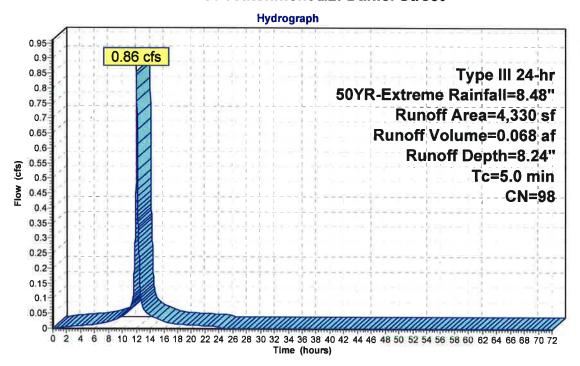
0.86 cfs @ 12.07 hrs, Volume=

0.068 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

A	rea (sf)	CN [Description							
	4,330	98 F	Paved road	aved roads w/curbs & sewers, HSG B						
	4,330	1	100.00% Impervious Area							
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.0	72-1-1-1-1				Direct Entry					

Subcatchment E2: Daniel Street



Summary for Subcatchment E3: Penhallow Street

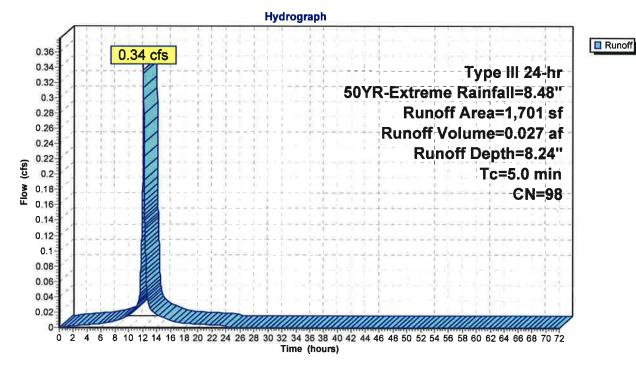
Runoff = 0.34 cfs @ 12.07 hrs, Volume=

0.027 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

	Α	rea (sf)	CN [Description						
		1,701	98 F	aved roads w/curbs & sewers, HSG B						
-		1,701	1	100.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
1	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0			Direct Entry						

Subcatchment E3: Penhallow Street



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Summary for Subcatchment E4: Penhallow Street

Runoff

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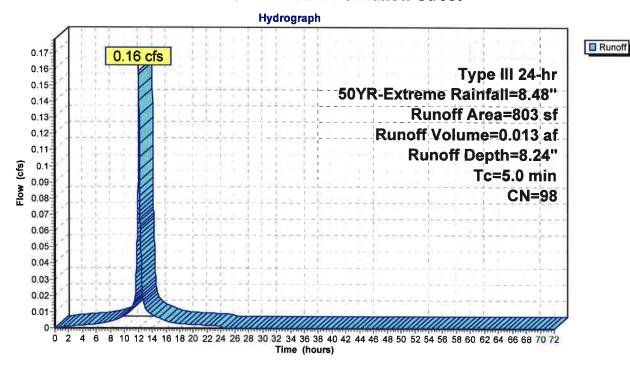
0.16 cfs @ 12.07 hrs, Volume=

0.013 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

_	A	rea (sf)	CN [Description								
-		803	98 F	aved roads w/curbs & sewers, HSG B								
		803 100.00% Impervious Area										
	т.	1	Class	\	0	Description						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
-	5.0			, , , , ,	(0.0)	Direct Entry.						

Subcatchment E4: Penhallow Street



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Summary for Link DP1: Penhallow Closed Drainage System

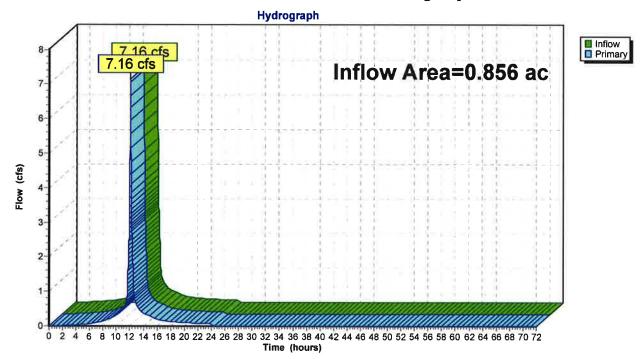
Inflow Area = 0.856 ac, 81.13% Impervious, Inflow Depth = 7.55" for 50YR-Extreme event

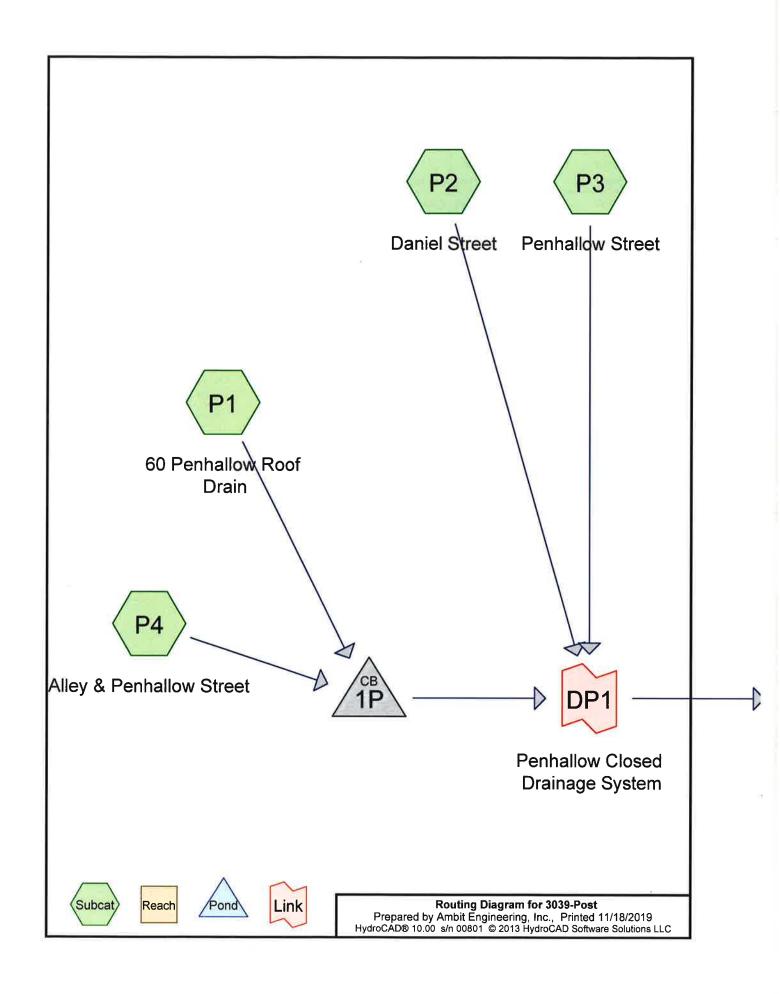
Inflow = 7.16 cfs @ 12.07 hrs, Volume= 0.538 af

Primary = 7.16 cfs @ 12.07 hrs, Volume= 0.538 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System





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Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.035	61	>75% Grass cover, Good, HSG B (P4)
0.350	98	Paved roads w/curbs & sewers, HSG B (P2, P3, P4)
0.471	98	Roofs, HSG B (P1, P4)
0.855	96	TOTAL AREA

Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.855	HSG B	P1, P2, P3, P4
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.855		TOTAL AREA

3039-Post

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Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.035	0.000	0.000	0.000	0.035	>75% Grass cover, Good	P4
0.000	0.350	0.000	0.000	0.000	0.350	Paved roads w/curbs & sewers	P2,
							P3,
							P4
0.000	0.471	0.000	0.000	0.000	0.471	Roofs	P1,
							P4
0.000	0.855	0.000	0.000	0.000	0.855	TOTAL AREA	

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Pipe Listing (selected nodes)

Line#	Node Number	in-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1P	23.80	21.75	150.0	0.0137	0.013	24.0	0.0	0.0

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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method

Subcatchment P1: 60 Penhallow Roof

Runoff Area=17,104 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=1.46 cfs 0.113 af

Subcatchment P2: Daniel Street

Runoff Area=5,601 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.48 cfs 0.037 af

Subcatchment P3: Penhallow Street

Runoff Area=1,995 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.17 cfs 0.013 af

Subcatchment P4: Alley & Penhallow StreetRunoff Area=12,558 sf 87.97% Impervious Runoff Depth=3.01"

Tc=5.0 min CN=94 Runoff=1.00 cfs 0.072 af

Pond 1P:

Peak Elev=24.54' Inflow=2.46 cfs 0.185 af

24.0" Round Culvert n=0.013 L=150.0' S=0.0137 '/' Outflow=2.46 cfs 0.185 af

Link DP1: Penhallow Closed Drainage System

Inflow=3.11 cfs 0.235 af

Primary=3.11 cfs 0.235 af

Total Runoff Area = 0.855 ac Runoff Volume = 0.235 af Average Runoff Depth = 3.30" 4.06% Pervious = 0.035 ac 95.94% Impervious = 0.821 ac

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Summary for Subcatchment P1: 60 Penhallow Roof Drain

Runoff

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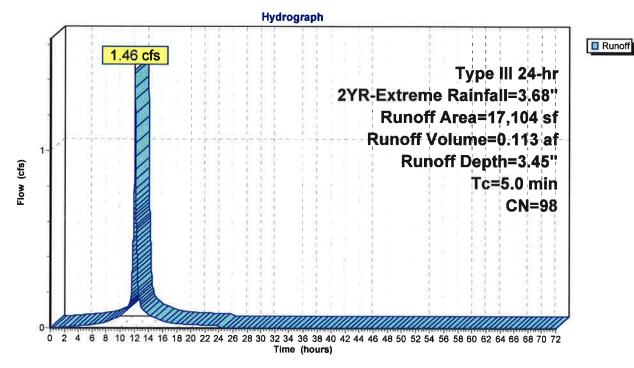
1.46 cfs @ 12.07 hrs, Volume=

0.113 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

	Area (sf)	CN I	Description					
	17,104	98 I	Roofs, HSG B					
	17,104		100.00% Impervious Area					
To (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	100		
5.0					Direct Entry,			

Subcatchment P1: 60 Penhallow Roof Drain



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Summary for Subcatchment P2: Daniel Street

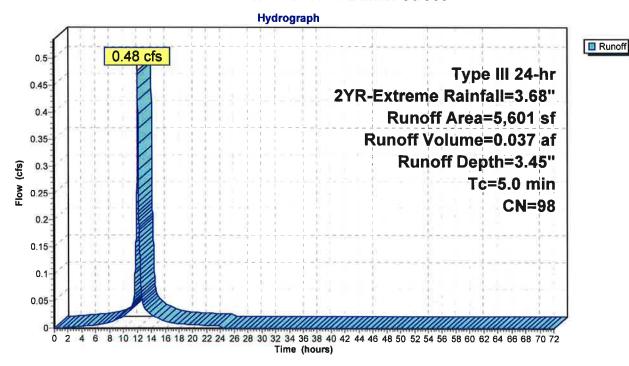
Runoff = 0.48 cfs @ 12.07 hrs, Volume=

0.037 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

Area (sf) CN Description	Description						
5,601 98 Paved roads w/curbs & sewers, HSG B	Paved roads w/curbs & sewers, HSG B						
5,601 100.00% Impervious Area	100.00% Impervious Area						
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry.							

Subcatchment P2: Daniel Street



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Summary for Subcatchment P3: Penhallow Street

Runoff

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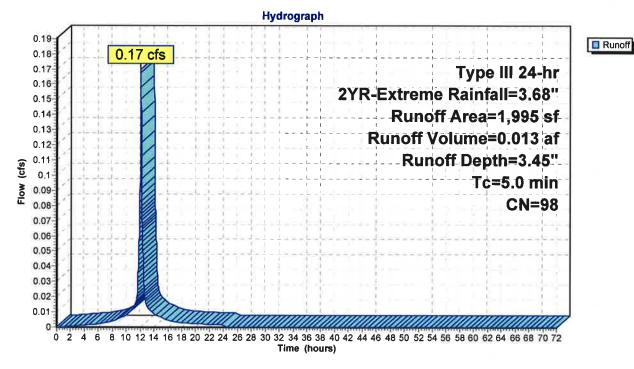
0.17 cfs @ 12.07 hrs, Volume=

0.013 af, Depth= 3.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

A	rea (sf)	CN [Description					
	1,995	98 F	Paved roads w/curbs & sewers, HSG B					
	1,995	1	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Subcatchment P3: Penhallow Street



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Summary for Subcatchment P4: Alley & Penhallow Street

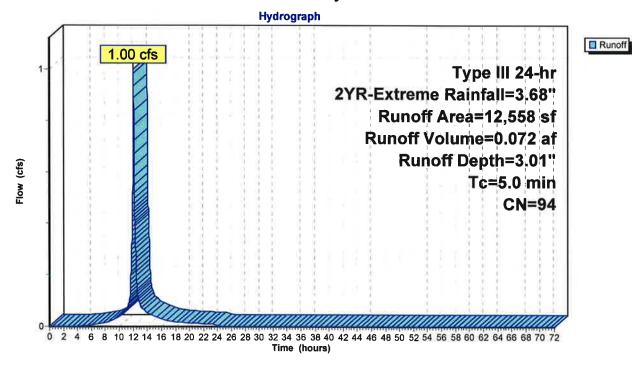
Runoff = 1.00 cfs @ 12.07 hrs, Volume=

0.072 af, Depth= 3.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR-Extreme Rainfall=3.68"

A	rea (sf)	CN	Description						
	7,649	98	Paved roads w/curbs & sewers, HSG B						
	1,511	61	>75% Grass cover, Good, HSG B						
	3,398	98	Roofs, HSG B						
	12,558	94	Weighted Average						
	1,511		12.03% Pervious Area						
	11,047		87.97% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
5.0					Direct Entry.				

Subcatchment P4: Alley & Penhallow Street



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Inflow Primary

Summary for Pond 1P:

Inflow Area = 0.681 ac, 94.91% Impervious, Inflow Depth = 3.26" for 2YR-Extreme event

Inflow = 0.185 af

2.46 cfs @ 12.07 hrs, Volume= 2.46 cfs @ 12.07 hrs, Volume= 2.46 cfs @ 12.07 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min Outflow

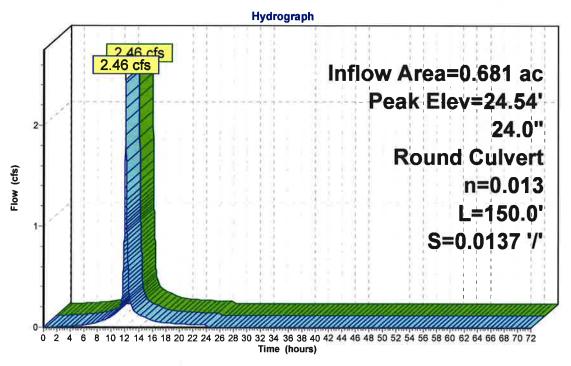
Primary 0.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 24.54' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	23.80'	24.0" Round Culvert
			L= 150.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 23.80' / 21.75' S= 0.0137 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.46 cfs @ 12.07 hrs HW=24.54' TW=0.00' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.46 cfs @ 2.32 fps)

Pond 1P:



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Summary for Link DP1: Penhallow Closed Drainage System

Inflow Area =

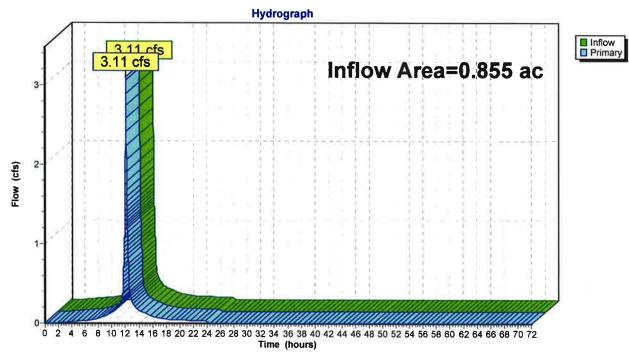
0.855 ac, 95.94% Impervious, Inflow Depth = 3.30" for 2YR-Extreme event

Inflow Primary 3.11 cfs @ 12.07 hrs, Volume= 3.11 cfs @ 12.07 hrs, Volume=

0.235 af 0.235 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment P1: 60 Penhallow Roof

Runoff Area=17,104 sf 100.00% Impervious Runoff Depth=5,35"

Tc=5.0 min CN=98 Runoff=2.23 cfs 0.175 af

Subcatchment P2: Daniel Street

Runoff Area=5,601 sf 100.00% Impervious Runoff Depth=5.35"

Tc=5.0 min CN=98 Runoff=0.73 cfs 0.057 af

Subcatchment P3: Penhallow Street

Runoff Area=1,995 sf 100.00% Impervious Runoff Depth=5.35"

Tc=5.0 min CN=98 Runoff=0.26 cfs 0.020 af

Subcatchment P4: Alley & Penhallow StreetRunoff Area=12,558 sf 87.97% Impervious Runoff Depth=4.89"

Tc=5.0 min CN=94 Runoff=1.58 cfs 0.117 af

Pond 1P:

Peak Elev=24.74' Inflow=3.81 cfs 0.293 af

24.0" Round Culvert n=0.013 L=150.0' S=0.0137 '/' Outflow=3.81 cfs 0.293 af

Link DP1: Penhallow Closed Drainage System

Inflow=4.80 cfs 0.370 af

Primary=4.80 cfs 0.370 af

Total Runoff Area = 0.855 ac Runoff Volume = 0.370 af Average Runoff Depth = 5.20" 4.06% Pervious = 0.035 ac 95.94% Impervious = 0.821 ac HydroCAD® 10.00 s/n 00801 © 2013 HydroCAD Software Solutions LLC

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Summary for Subcatchment P1: 60 Penhallow Roof Drain

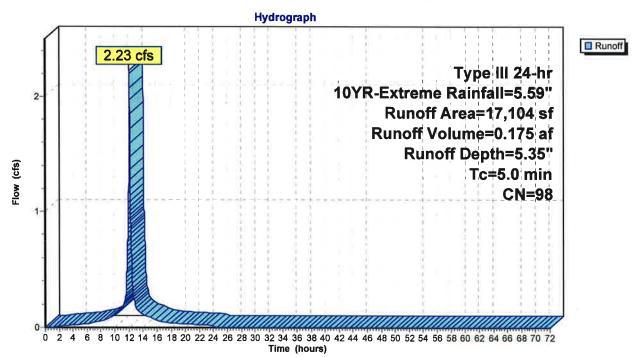
Runoff = 2.23 cfs @ 12.07 hrs, Volume=

0.175 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	Are	ea (sf)	CN	Description		
	1	7,104	98	Roofs, HSC	B B	
	1	7,104		100.00% In	npervious A	\rea
[(mi		Length (feet)	Slope (ft/ft	•	Capacity (cfs)	Description
5	.0					Direct Entry,

Subcatchment P1: 60 Penhallow Roof Drain



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Summary for Subcatchment P2: Daniel Street

Runoff

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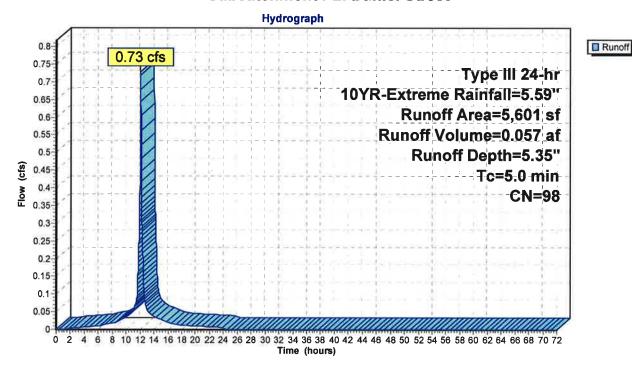
0.73 cfs @ 12.07 hrs, Volume=

0.057 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	A	rea (sf)	CN [Description						
		5,601	1 98 Paved roads w/curbs & sewers, HSG B							
		5,601	100.00% Impervious Area							
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry				

Subcatchment P2: Daniel Street



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Summary for Subcatchment P3: Penhallow Street

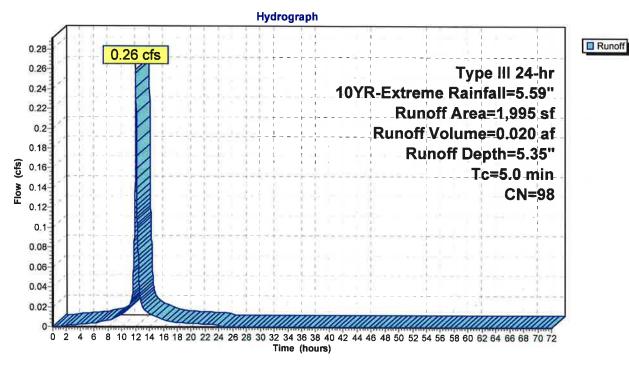
Runoff = 0.26 cfs @ 12.07 hrs, Volume=

0.020 af, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	Ar	ea (sf)	CN I	Description					
	1,995 98 Paved roads w/curbs & sewers, HSG B								
		1,995 100.00% Impervious Area							
		Length	Slope	•		Description			
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0					Direct Entry.			

Subcatchment P3: Penhallow Street



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Summary for Subcatchment P4: Alley & Penhallow Street

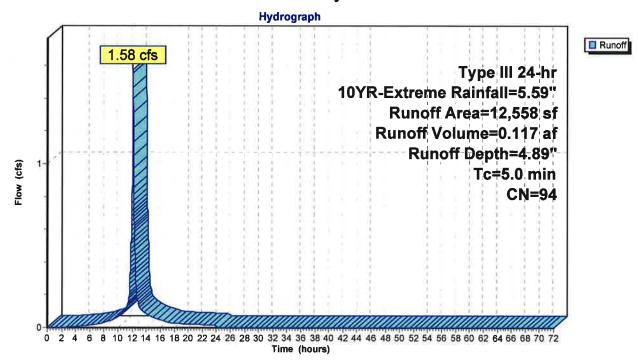
Runoff = 1.58 cfs @ 12.07 hrs, Volume=

0.117 af, Depth= 4.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR-Extreme Rainfall=5.59"

	Area (sf)	CN Description								
	7,649	98	98 Paved roads w/curbs & sewers, HSG B							
	1,511	61 :	>75% Grass cover, Good, HSG B							
	3,398	98 Roofs, HSG B								
	12,558	558 94 Weighted Average								
	1,511	•	a							
	11,047	8	37.97% Imp	pervious Ar	rea					
To		Slope	•	Capacity	Description					
<u>(min</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
5.0)				Direct Entry					

Subcatchment P4: Alley & Penhallow Street



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Summary for Pond 1P:

Inflow Area = 0.681 ac, 94.91% Impervious, Inflow Depth = 5.16" for 10YR-Extreme event

Inflow = 0.293 af

3.81 cfs @ 12.07 hrs, Volume= 3.81 cfs @ 12.07 hrs, Volume= 3.81 cfs @ 12.07 hrs, Volume= Outflow 0.293 af, Atten= 0%, Lag= 0.0 min

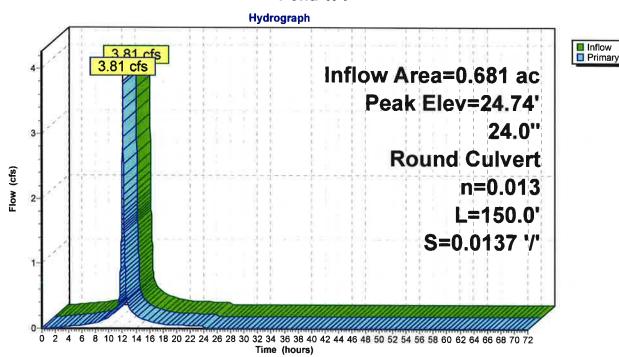
Primary 0.293 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 24.74' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	23.80'	24.0" Round Culvert
			L= 150.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 23.80' / 21.75' S= 0.0137 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior Flow Area= 3.14 sf

Primary OutFlow Max=3.81 cfs @ 12.07 hrs HW=24.74' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.81 cfs @ 2.61 fps)

Pond 1P:



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Summary for Link DP1: Penhallow Closed Drainage System

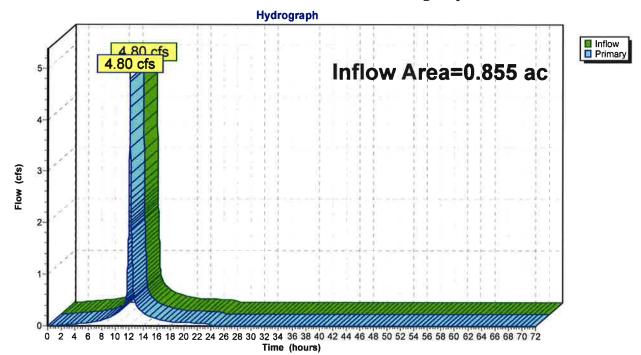
Inflow Area = 0.855 ac, 95.94% Impervious, Inflow Depth = 5.20" for 10YR-Extreme event

Inflow = 4.80 cfs @ 12.07 hrs, Volume= 0.370 af

Primary = 4.80 cfs @ 12.07 hrs, Volume= 0.370 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment P1: 60 Penhallow Roof

Runoff Area=17,104 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=2.83 cfs 0.224 af

Subcatchment P2: Daniel Street

Runoff Area=5,601 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.93 cfs 0.073 af

Subcatchment P3: Penhallow Street

Runoff Area=1,995 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.33 cfs 0.026 af

Subcatchment P4: Alley & Penhallow StreetRunoff Area=12,558 sf 87.97% Impervious Runoff Depth=6.37"

Tc=5.0 min CN=94 Runoff=2.03 cfs 0.153 af

Pond 1P:

Peak Elev=24.88' Inflow=4.85 cfs 0.377 af

24.0" Round Culvert n=0.013 L=150.0' S=0.0137 '/' Outflow=4.85 cfs 0.377 af

Link DP1: Penhallow Closed Drainage System

Inflow=6.11 cfs 0.476 af

Primary=6.11 cfs 0.476 af

Total Runoff Area = 0.855 ac Runoff Volume = 0.476 af Average Runoff Depth = 6.68" 4.06% Pervious = 0.035 ac 95.94% Impervious = 0.821 ac

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Summary for Subcatchment P1: 60 Penhallow Roof Drain

Runoff

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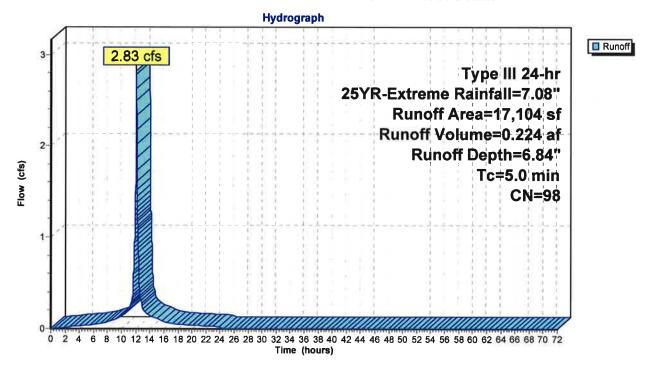
2.83 cfs @ 12.07 hrs, Volume=

0.224 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

	Area (sf)	CN	Description			
	17,104	98	Roofs, HSC	3 B		
	17,104		100.00% In	Area		
T (mir	c Length		,	Capacity (cfs)	• • • • • • • • • • • • • • • • • • •	
5.	0		· · · · · · · · · · · · · · · · · · ·		Direct Entry.	

Subcatchment P1: 60 Penhallow Roof Drain



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Summary for Subcatchment P2: Daniel Street

Runoff

=

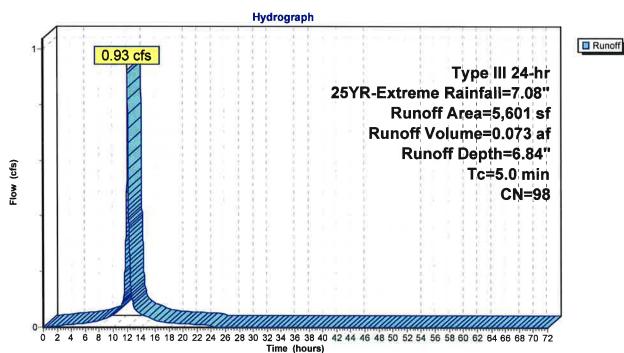
0.93 cfs @ 12.07 hrs, Volume=

0.073 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

	A	rea (sf)	CN E	Description								
		5,601	98 F	Paved roads w/curbs & sewers, HSG B								
		5,601	100.00% Impervious Area									
-	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)							
- 3	5.0					Direct Entry.						

Subcatchment P2: Daniel Street



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Summary for Subcatchment P3: Penhallow Street

Runoff

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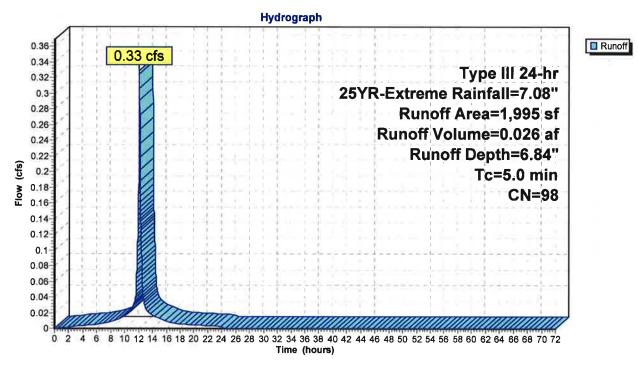
0.33 cfs @ 12.07 hrs, Volume=

0.026 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

	A	rea (sf)	CN E	Description						
-		1,995								
-		1,995								
	т.	1	Olana	\/-I: /-	0	Description				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	- 4			
-	5.0	(:/	(1010)	(12000)	(0.0)	Direct Entry.				

Subcatchment P3: Penhallow Street



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Summary for Subcatchment P4: Alley & Penhallow Street

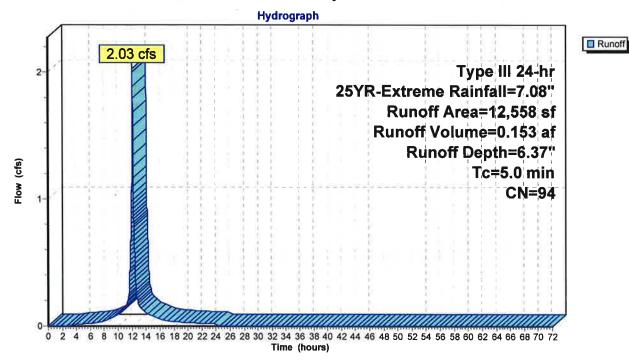
Runoff = 2.03 cfs @ 12.07 hrs, Volume=

0.153 af, Depth= 6.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR-Extreme Rainfall=7.08"

_	Α	rea (sf)	a (sf) CN Description								
		7,649	98								
		1,511	61	>75% Grass cover, Good, HSG B							
_		3,398 98 Roofs, HSG B									
		12,558	2,558 94 Weighted Average								
		1,511		12.03% Pei	vious Area	a					
		11,047		37.97% lmp	pervious Are	rea					
	Тс	Length	Slope	,	Capacity	•					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry.					

Subcatchment P4: Alley & Penhallow Street



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Summary for Pond 1P:

Inflow Area = 0.681 ac, 94.91% Impervious, Inflow Depth = 6.64" for 25YR-Extreme event

Inflow 4.85 cfs @ 12.07 hrs, Volume= 0.377 af

4.85 cfs @ 12.07 hrs, Volume= 4.85 cfs @ 12.07 hrs, Volume= Outflow 0.377 af, Atten= 0%, Lag= 0.0 min

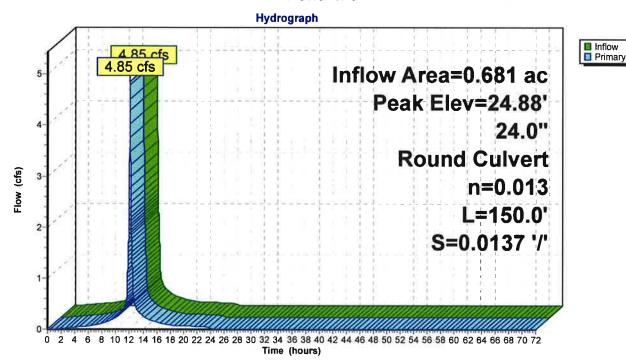
Primary 0.377 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 24.88' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	23.80'	24.0" Round Culvert
			L= 150.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 23.80' / 21.75' S= 0.0137 '/' Cc= 0.900
			n= 0.013 Corrugated PE_smooth interior_Flow Area= 3.14 sf

Primary OutFlow Max=4.85 cfs @ 12.07 hrs HW=24.88' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.85 cfs @ 2.80 fps)

Pond 1P:



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Summary for Link DP1: Penhallow Closed Drainage System

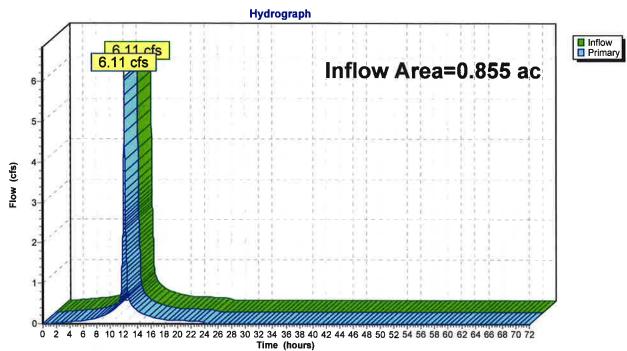
Inflow Area = 0.855 ac, 95.94% Impervious, Inflow Depth = 6.68" for 25YR-Extreme event

Inflow = 6.11 cfs @ 12.07 hrs, Volume= 0.476 af

Primary = 6.11 cfs @ 12.07 hrs, Volume= 0.476 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



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Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment P1: 60 Penhallow Roof

Runoff Area=17,104 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=3.39 cfs 0.270 af

Subcatchment P2: Daniel Street

Runoff Area=5,601 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=1.11 cfs 0.088 af

Subcatchment P3: Penhallow Street

Runoff Area=1,995 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=0.40 cfs 0.031 af

Subcatchment P4: Alley & Penhallow Street Runoff Area=12,558 sf 87.97% Impervious Runoff Depth=7.76"

Tc=5.0 min CN=94 Runoff=2.45 cfs 0.186 af

Pond 1P:

Peak Elev=25.00' Inflow=5.83 cfs 0.456 af

24.0" Round Culvert n=0.013 L=150.0' S=0.0137 '/' Outflow=5.83 cfs 0.456 af

Link DP1: Penhallow Closed Drainage System

Inflow=7.34 cfs 0.576 af

Primary=7.34 cfs 0.576 af

Total Runoff Area = 0.855 ac Runoff Volume = 0.576 af Average Runoff Depth = 8.08" 4.06% Pervious = 0.035 ac 95.94% Impervious = 0.821 ac

Summary for Subcatchment P1: 60 Penhallow Roof Drain

Runoff

=

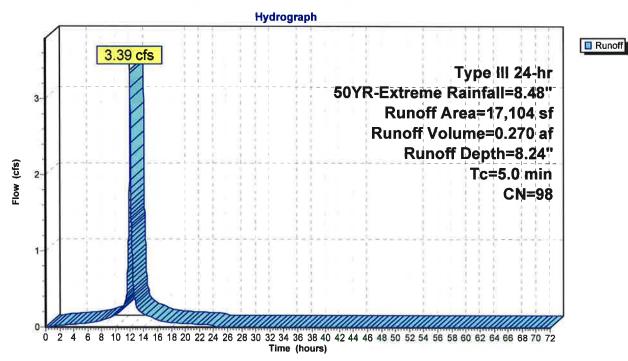
3.39 cfs @ 12.07 hrs, Volume=

0.270 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

0	Area (sf)	CN	Description		
2	17,104	98	Roofs, HSC	₿B	
9,	17,104		100.00% In	npervious A	Area
T (mir	c Length	•	,	Capacity (cfs)	Description
5.	0				Direct Entry.

Subcatchment P1: 60 Penhallow Roof Drain



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Summary for Subcatchment P2: Daniel Street

Runoff

=

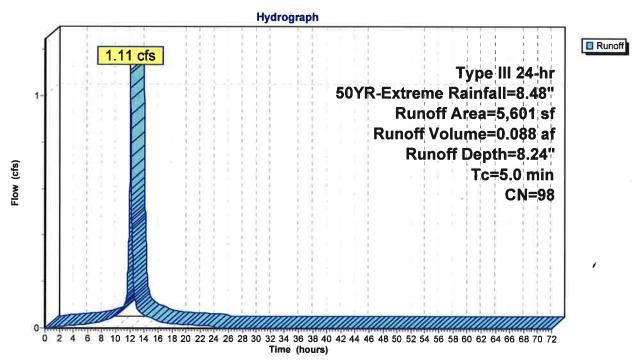
1.11 cfs @ 12.07 hrs, Volume=

0.088 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

<u></u>	rea (sf)	CN I	Description							
	5,601	98 I	98 Paved roads w/curbs & sewers, HSG B							
	5,601	100.00% Impervious Area								
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	1					
5.0					Direct Entry,					

Subcatchment P2: Daniel Street



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Summary for Subcatchment P3: Penhallow Street

Runoff

=

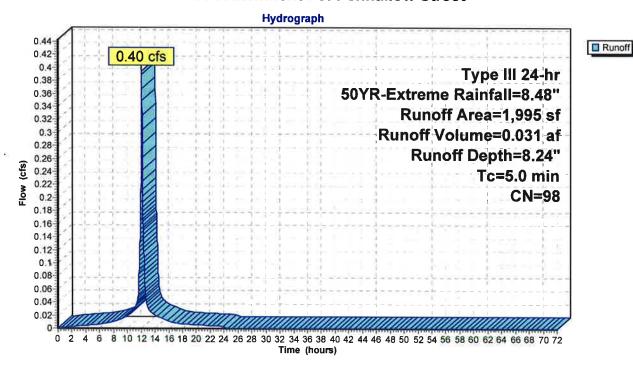
0.40 cfs @ 12.07 hrs, Volume=

0.031 af, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

	A	rea (sf)	CN [Description						
_		1,995 98 Paved roads w/curbs & sewers, HSG B								
		1,995	995 100.00% Impervious Area							
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0				7.	Direct Entry	•			

Subcatchment P3: Penhallow Street



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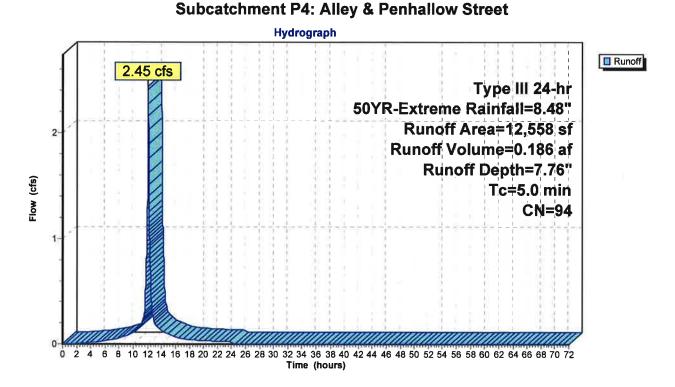
Summary for Subcatchment P4: Alley & Penhallow Street

Runoff = 2.45 cfs @ 12.07 hrs, Volume=

0.186 af, Depth= 7.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 50YR-Extreme Rainfall=8.48"

A	rea (sf)	CN I	Description			
	7,649	98	Paved road	s w/curbs &	& sewers, HSG B	
	1,511	61 :	>75% Gras	s cover, Go	ood, HSG B	
	3,398	98	Roofs, HSC	B		
	12,558	94 \	Neighted A	verage		
	1,511	•	12.03% Per	vious Area		
	11,047	1	37.97% Imp	pervious Ar	ea	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
5.0			4-		Direct Entry,	



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Summary for Pond 1P:

Inflow Area = 0.681 ac, 94.91% Impervious, Inflow Depth = 8.04" for 50YR-Extreme event

Inflow = 5.83 cfs @ 12.07 hrs, Volume= 0.456 af

Outflow = 5.83 cfs @ 12.07 hrs, Volume= 0.456 af, Atten= 0%, Lag= 0.0 min

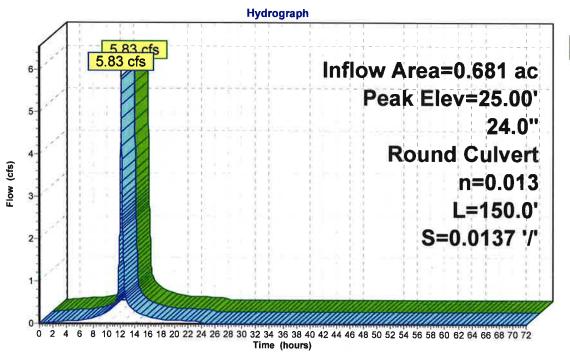
Primary = 5.83 cfs @ 12.07 hrs, Volume= 0.456 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 25.00' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	23.80'	24.0" Round Culvert
			L= 150.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 23.80' / 21.75' S= 0.0137 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 3.14 sf

Primary OutFlow Max=5.83 cfs @ 12.07 hrs HW=25.00' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.83 cfs @ 2.95 fps)

Pond 1P:





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Summary for Link DP1: Penhallow Closed Drainage System

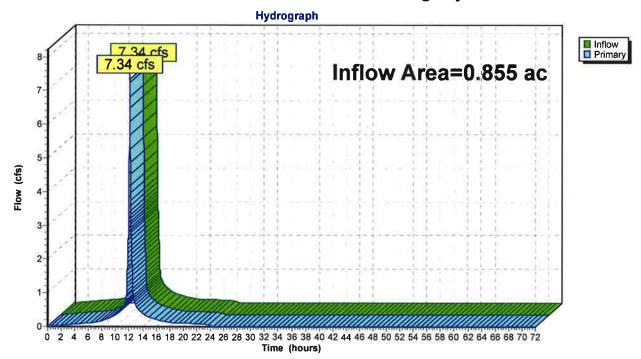
Inflow Area = 0.855 ac, 95.94% Impervious, Inflow Depth = 8.08" for 50YR-Extreme event

Inflow = 7.34 cfs @ 12.07 hrs, Volume= 0.576 af

Primary = 7.34 cfs @ 12.07 hrs, Volume= 0.576 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link DP1: Penhallow Closed Drainage System



×

APPENDIX D SOIL SURVEY INFORMATION



Conservation Service Natural Resources

USDA

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000

Warning: Soil Map may not be valid at this scale.

contrasting soils that could have been shown at a more detailed misunderstanding of the detail of mapping and accuracy of soil Enlargement of maps beyond the scale of mapping can cause line placement. The maps do not show the small areas of

Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rockingham County, New Hampshire Survey Area Data: Version 21, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Sep

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Severely Eroded Spot

Slide or Slip Sodic Spot

Sinkhole

Sandy Spot Saline Spot

MAP LEGEND

Special Line Features Very Stony Spot Stony Spot Spoil Area Wet Spot Other W 8 ◁ Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Special Point Features Area of Interest (AOI)

Soils

Water Features

Streams and Canals Transportation

Borrow Pit

X

Blowout

Clay Spot

Interstate Highways Rails ŧ

Closed Depression



Major Roads

Gravelly Spot

Gravel Pit

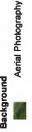












Marsh or swamp

Lava Flow

Landfill

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop



Map Unit Legend

Totals for Area of Interest		1.3	100.0%	
699 Urban land		1.3	100.0%	
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	

	w		

APPENDIX E INSPECTION & MAINTENANCE PLAN

INSPECTION & MAINTENANCE PLAN FOR

60 Penhallow Street

Portsmouth, NH

Introduction

The intent of this plan is to provide 60 Penhallow Street (herein referred to as "owner") with a list of procedures that document the inspection and maintenance requirements of the stormwater management system for this development. Specifically, the filtration system and associated structures on the project site (collectively referred to as the "Stormwater Management System").

The following inspection and maintenance program is necessary to keep the stormwater management system functioning properly. These measures will also help minimize potential environmental impacts. By following the enclosed procedures, the owner will be able to maintain the functional design of the stormwater management system and maximize its ability to remove sediment and other contaminants from site generated stormwater runoff.

Annual Report

The owner shall prepare an annual Inspection & Maintenance Report. The report shall include a summary of the system's maintenance and repair by transmission of the Inspection & Maintenance Log and other information as required. A copy of the report shall be delivered annually to the City of Portsmouth Code Enforcement Officer.

Inspection & Maintenance Checklist/Log

The following pages contain a Stormwater Management System Inspection & Maintenance Checklist and a blank copy of the Stormwater Management System Inspection & Maintenance Log. These forms are provided to the owner as a guideline for performing the inspection and maintenance of the Stormwater Management System. This is a guideline and should be periodically reviewed for conformance with current practice and standards.

STORMWATER MANAGEMENT SYSTEM COMPONENTS

The Stormwater Management System is designed to mitigate both the quantity and quality of site-generated stormwater runoff. As a result, the design includes the following elements:

Non-Structural BMP's

Non-Structural best management practices (BMP's) include temporary and permanent measures that typically require less labor and capital inputs and are intended to provide protection against erosion of soils. Examples of non-structural BMP's on this project include but are not limited to: temporary and permanent mulching, temporary and permanent grass cover, trees, shrubs and ground covers, miscellaneous landscape plantings, dust control, tree protection, topsoiling, sediment barriers, and a stabilized construction entrance.

Structural BMP's

Structural BMP's are more labor and capital-intensive structures or installations that require more specialized personnel to install. Examples on this project include but are not limited to: storm drain catch basins, slot drains and pipes.

Inspection and Maintenance Requirements

The following summarizes the inspection and maintenance requirements for the various BMP's that may be found on this project.

- 1. Grassed areas: After each rain event of 0.5" or more during a 24-hour period, inspect grassed areas for signs of disturbance, such as erosion. If damaged areas are discovered, immediately repair the damage. Repairs may include adding new topsoil, lime, seed, fertilizer and mulch.
- 2. Plantings: Planting and landscaping (trees, shrubs) shall be monitored bi-monthly during the first year to insure viability and vigorous growth. Replace dead or dying vegetation with new stock and adjust the conditions that caused the dead or dying vegetation. During dryer times of the year, provide weekly watering or irrigation during the establishment period of the first year. Make the necessary adjustments to ensure long-term health of the vegetated covers, i.e. provide more permanent mulch or compost or other means of protection.
- 3. Storm Drain Catch Basins, Slot Drains and Pipes: Monitor drain inlets and outlets where visible. Monitor slot drains for clogging and follow manufacturers recommendations for maintenance. Monitor sediment levels in catch basin sumps and remove as necessary.

Invasive Species

Monitor Stormwater Management System for signs of invasive species growth. If caught earlier enough, their eradication is much easier. The most likely places where invasions start is in wetter, disturbed soils or detention ponds. Species such as phragmites and purple loose-strife are common invaders in these wetter areas. If they are found, then the owner shall contact a wetlands scientist with

experience in invasive species control to implement a plan of action to eradicate the invaders. Measures that do not require the application of chemical herbicides should be the first line of defense.

Stormwater Management System Inspection & Maintenance Checklist for Post Construction Condition—for 60 Penhallow Street, Portsmouth, NH

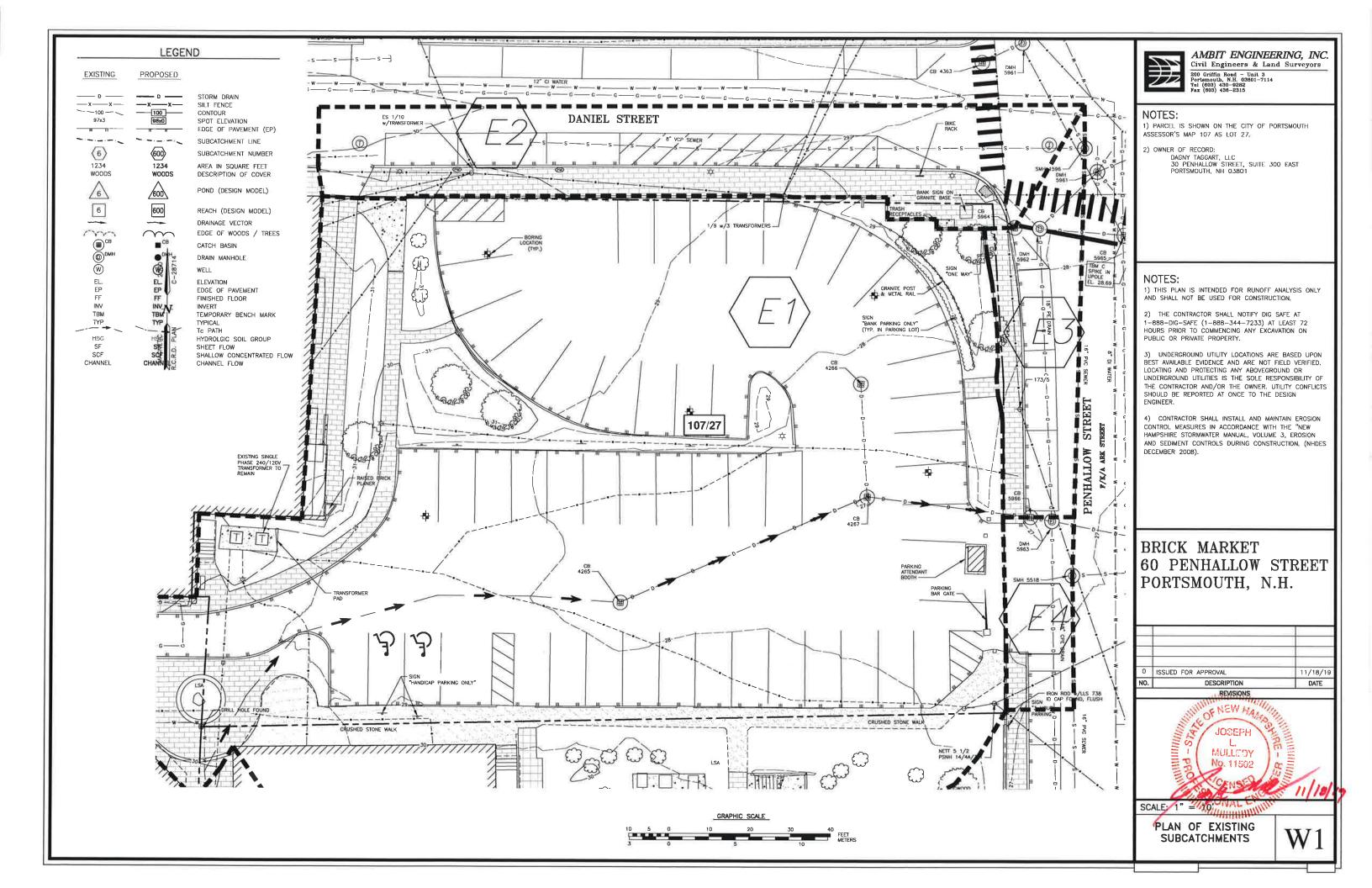
BMP/System Component	Minimum Inspection Frequency	BMP/System Minimum Minimum Inspection Requirements Component Inspection Frequency	Maintenance/Cleanout Threshold
Closed Drainage System			
Drainage Pipes	Yearly	Check for sediment clogging, or soiled runoff.	Clean entire drainage system and remove all sediments if discovered in piping.
Slot Drains	Bi-Annually	Check for sediment clogging, or soiled runoff.	Clean per manufacturers recommendations
Catch Basins	Bi-Annually	Check for excessive accumulation of sediment in sump	Remove sediment as necessary
Annual Report	Yearly	Prepare Annual Report, including all Inspection & Maintenance Logs. Provide to City (if required).	N/A

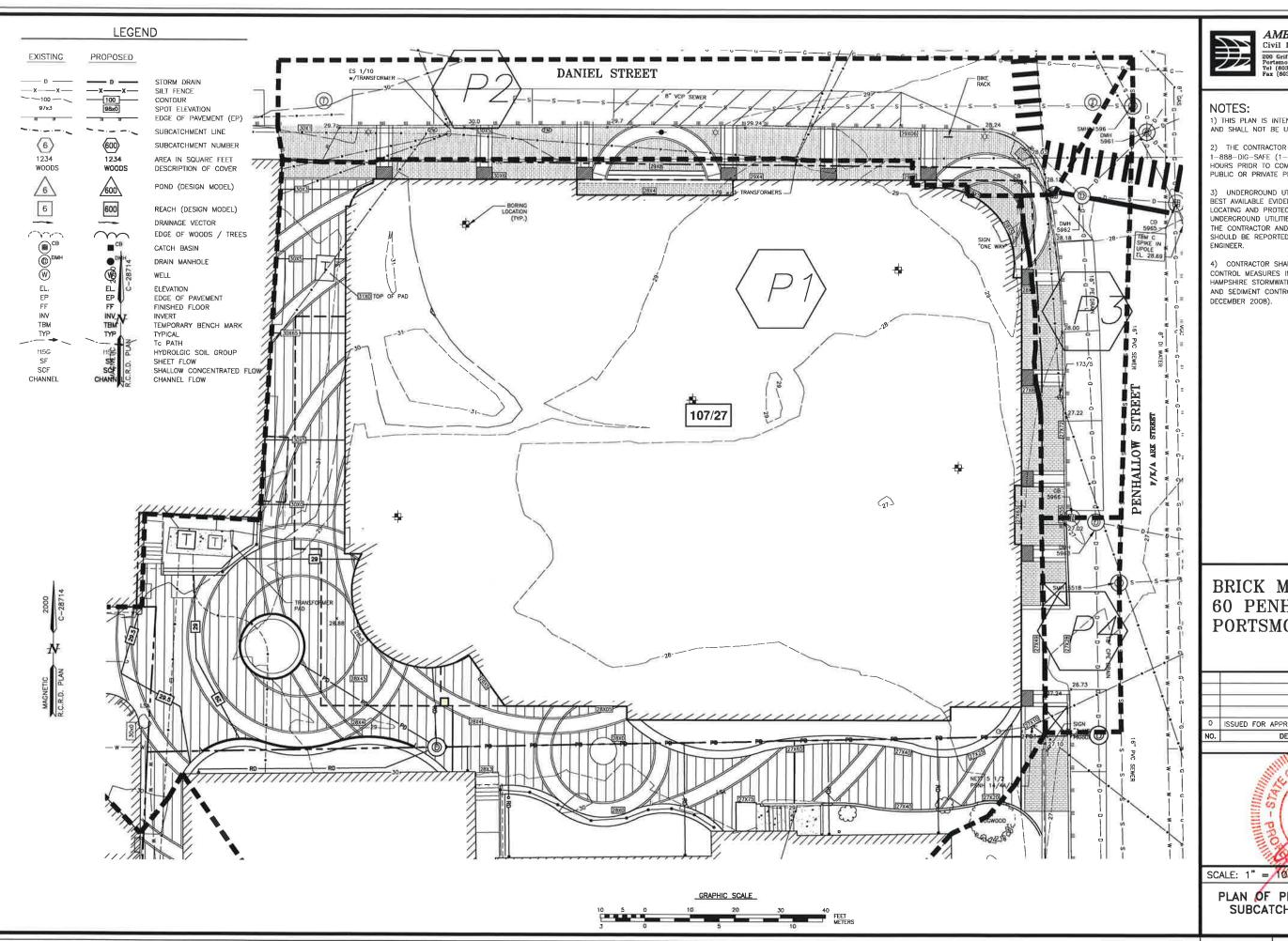
Stormwater Management System Maintenance Summary

Inspection & Maintenance Log—for 60 Penhallow Street, Portsmouth, NH

Performed By							
Date of	Maintenance						
Problems Noted, Required Maintenance	(List Items/Comments)						
Inspector							
Date	Inspected						
BMP/System Date Inspector Problems	Component		2				

Data Sheets





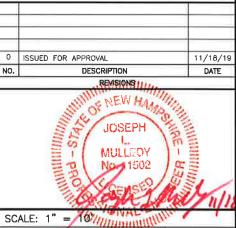
AMBIT ENGINEERING, INC.

Civil Engineers & Land Surveyors

1) THIS PLAN IS INTENDED FOR RUNOFF ANALYSIS ONLY AND SHALL NOT BE USED FOR CONSTRUCTION.

- 2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-88B-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY,
- 3) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER, UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN
- 4) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION, (NHDES

BRICK MARKET 60 PENHALLOW STREET PORTSMOUTH, N.H.



PLAN OF PROPOSED SUBCATCHMENTS