

C0960-011 May 19, 2021

Ms. Juliet Walker Planning Director City of Portsmouth Planning Department 1 Junkins Avenue Portsmouth, New Hampshire 03801

Re: Site Review Permit & Lot Line Revision Applications Proposed Mixed Use Development 53 Green Street, Portsmouth, NH

Dear Juliet:

On behalf of Stone Creek Realty, LLC (owner), and CPI Management, LLC (applicant), we are pleased to submit the following information to support a request for a Site Review Permit and Lot Line Revision Permit for the above referenced project:

- One (1) full size & one (1) half size copy of the Site Plan Set, last revised May 19, 2021;
- One (1) copy of the TAC Comment Response, dated May 19, 2021;
- One (1) copy of the Site Review Checklist, dated March 22, 2021;
- One (1) copy of the Subdivision Checklist, last revised May 19, 2021;
- One (1) copy of the Boundary Line Adjustment Plan, dated May 17, 2021;
- One (1) copy of the Drainage Analysis, last revised May 19, 2021;
- One (1) copy of the Operations and Maintenance Plan, dated May 19, 2021;
- One (1) copy of the Aerial Site Plan, last revised May 19, 2021;
- One (1) copy of the Grade Plane Exhibit, last revised May 19, 2021;
- One (1) copy of the Wetland and Buffer Report, dated January 6, 2020;
- One (1) copy of the Existing Buffer Photograph Log, dated January 27, 2021;
- One (1) copy of the Wetland Buffer Impervious Surface Exhibit, last revised May 19, 2021;
- One (1) copy of the Community Space Exhibit, last revised May 19, 2021;
- One (1) copy of the Fire Truck Turning Exhibit, last revised May 19, 2021;
- One (1) copy of the Site Traffic Exhibit, dated May 19, 2021;
- One (1) copy of the Trip Generation Analysis, last revised May 19, 2021;
- One (1) copy of the Unitil Will Service Letter, dated February 22, 2021;
- One (1) copy of the Green Building Statement, dated March 22, 2021;
- One (1) copy of the Site Lighting Fixture Cut Sheets

The proposed project is located at 53 Green Street on property identified as Map 119 Lot 2 on the City of Portsmouth Tax Maps. The existing 1.66-acre parcel is bound by Green Street to south, the AC Hotel to the west, North Mill Pond to the north and the railroad to the east.

The project will include a lot line revision between Map 119 Lot 2 and Map 119 Lot 3 as shown in the enclosed Boundary Line Adjustment Plan prepared by Doucet Survey Inc. The proposed lot line revision will relocate the lot line between the project parcel and the adjacent railroad. This will increase the development lot area by 0.12 acres for a total lot area of 1.78 acres. The project is also currently under review by the Historic District Commission (HDC).



The proposed project will include the construction of a 5-story mixed-use residential building that includes basement level parking, first floor residential lobby, commercial space and parking, and 48 upper floor residential units. The project will include associated site improvements such as paving, utilities, lighting, landscaping and community space. The proposed project is providing 22,100 SF of community spaces (28.5% of the total lot area) which meets the 20% of total lot area required to receive the incentive bonus for one additional story (10 ft) above the maximum height requirement on the building. The community space calculation is depicted in the enclosed Community Space Exhibit. A Conditional Use Permit for Wetland Buffer Impact will be required for the project for work within the 100 ft wetland buffer. The project received a unanimous recommendation for approval from the Conservation Commission at their April 14, 2021 meeting.

To date the applicant has attending the following meetings with the local land-use boards related to the Site Plan:

- January 21, 2021 Planning Board Conceptual Consultation
- February 9, 2021 Technical Advisory Committee Work Session
- February 10, 2021 Conservation Commission Work Session
- April 6, 2021 Technical Advisory Committee Meeting
- April 14, 2021 Conservation Commission Regular Meeting
- May 4, 2021 Technical Advisory Committee Meeting

The enclosed revised plans and supplemental materials have been provided to address comments received from the Technical Advisory Committee (TAC) in correspondence dated May 4, 2021 and at their meeting held on May 4, 2021.

We respectfully request to be placed on the TAC meeting agenda for June 1, 2021. If you have any questions or need any additional information, please contact Patrick Crimmins by phone at (603) 433-8818 or by email at pmcrimmins@tighebond.com.

Sincerely,

TIGHE & BOND, INC.

Patrick M. Crimmins, PE Senior Project Manager

Cc: Stone Creek Realty, LLC (via e-mail)
CPI Management, LLC (via e-mail)

Neil A. Hansen, PE Project Engineer



City of Portsmouth, New Hampshire Site Plan Application Checklist

This site plan application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. The checklist is required to be completed and uploaded to the Site Plan application in the City's online permitting system. A preapplication conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan review regulations for full details.

Applicant Responsibilities (Section 2.5.2): Applicable fees are due upon application submittal along with required attachments. The application shall be complete as submitted and provide adequate information for evaluation of the proposed site development. <u>Waiver requests must be submitted in writing with appropriate justification</u>.

Name of Applicant:	CPI Management, LLC	Date Submitted: March 22, 2021	
Application # (in City	s online permitting): <u>LU 21-XX</u>		
Site Address: 53 Gr	een Street	Map: 119	Lot: 2

	Application Requirements			
V	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested	
Ø	Complete <u>application</u> form submitted via the City's web-based permitting program (2.5.2.1 (2.5.2.3A)	Enclosed	N/A	
V	All application documents, plans, supporting documentation and other materials uploaded to the application form in viewpoint in digital Portable Document Format (PDF). One hard copy of all plans and materials shall be submitted to the Planning Department by the published deadline. (2.5.2.8)	Enclosed	N/A	

	Site Plan Review Application Required Information			
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
V	Statement that lists and describes "green" building components and systems. (2.5.3.1B)	Enclosed		
M	Existing and proposed gross floor area and dimensions of all buildings and statement of uses and floor area for each floor. (2.5.3.1C)	Site Plan Sheet C-102.1	N/A	
V	Tax map and lot number, and current zoning of all parcels under Site Plan Review. (2.5.3.1D)	Site Plan Sheet C-102.1	N/A	

	Site Plan Review Application Required Info	ormation	
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	Owner's name, address, telephone number, and signature. Name, address, and telephone number of applicant if different from owner. (2.5.3.1E)	Enclosed Existing Conditions Plan	N/A
V	Names and addresses (including Tax Map and Lot number and zoning districts) of all direct abutting property owners (including properties located across abutting streets) and holders of existing conservation, preservation or agricultural preservation restrictions affecting the subject property. (2.5.3.1F)	Existing Conditions Plan	N/A
V	Names, addresses and telephone numbers of all professionals involved in the site plan design. (2.5.3.1G)	Cover Sheet	N/A
V	List of reference plans. (2.5.3.1H)	Existing Conditions Plan	N/A
V	List of names and contact information of all public or private utilities servicing the site. (2.5.3.1I)	Utilities Plan Sheet C-104	N/A

	Site Plan Specifications			
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
V	Full size plans shall not be larger than 22 inches by 34 inches with match lines as required, unless approved by the Planning Director (2.5.4.1A)	Required on all plan sheets	N/A	
A	Scale: Not less than 1 inch = 60 feet and a graphic bar scale shall be included on all plans. (2.5.4.1B)	Required on all plan sheets	N/A	
V	GIS data should be referenced to the coordinate system New Hampshire State Plane, NAD83 (1996), with units in feet. (2.5.4.1C)	Existing Conditions Plan	N/A	
Ø	Plans shall be drawn to scale and stamped by a NH licensed civil engineer. (2.5.4.1D)	Required on all plan sheets	N/A	
Ø	Wetlands shall be delineated by a NH certified wetlands scientist and so stamped. (2.5.4.1E)	Existing Conditions Plan	N/A	
V	Title (name of development project), north point, scale, legend. (2.5.4.2A)	Required on all plan sheets	N/A	
Ø	Date plans first submitted, date and explanation of revisions. (2.5.4.2B)	Required on all plan sheets	N/A	
V	Individual plan sheet title that clearly describes the information that is displayed. (2.5.4.2C)	Required on all plan sheets	N/A	
Ø	Source and date of data displayed on the plan. (2.5.4.2D)	Required on all plan sheets	N/A	

	Site Plan Specifications – Required Exhibits and Data				
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested		
	 Existing Conditions: (2.5.4.3A) Surveyed plan of site showing existing natural and built features; Existing building footprints and gross floor area; Existing parking areas and number of parking spaces provided; Zoning district boundaries; Existing, required, and proposed dimensional zoning requirements including building and open space coverage, yards and/or setbacks, and dwelling units per acre; Existing impervious and disturbed areas; Limits and type of existing vegetation; Wetland delineation, wetland function and value assessment (including vernal pools); SFHA, 100-year flood elevation line and BFE data, as required. 	Existing Conditions Plan			
	 2. Buildings and Structures: (2.5.4.3B) Plan view: Use, size, dimensions, footings, overhangs, 1st fl. elevation; Elevations: Height, massing, placement, materials, lighting, façade treatments; Total Floor Area; Number of Usable Floors; Gross floor area by floor and use. 	Site Plan Sheets C.102.1 & C.102.2			
	 3. Access and Circulation: (2.5.4.3C) Location/width of access ways within site; Location of curbing, right of ways, edge of pavement and sidewalks; Location, type, size and design of traffic signing (pavement markings); Names/layout of existing abutting streets; Driveway curb cuts for abutting prop. and public roads; If subdivision; Names of all roads, right of way lines and easements noted; AASHTO truck turning templates, description of minimum vehicle allowed being a WB-50 (unless otherwise approved by TAC). 	Site Plan Sheet C-102.1			
V	 4. Parking and Loading: (2.5.4.3D) Location of off street parking/loading areas, landscaped areas/buffers; Parking Calculations (# required and the # provided). 	Site Plan Sheet C-102.1			
V	 Water Infrastructure: (2.5.4.3E) Size, type and location of water mains, shut-offs, hydrants & Engineering data; Location of wells and monitoring wells (include protective radii). 	Utilities Plan Sheet C-104			
Ø	 Sewer Infrastructure: (2.5.4.3F) Size, type and location of sanitary sewage facilities & Engineering data, including any onsite temporary facilities during construction period. 	Utilities Plan Sheet C-104			

\square	/.	Utilities: (2.5.4.3G)	Utilities Plan Sheet	
	•	The size, type and location of all above & below ground utilities;	C-104	
	•	Size type and location of generator pads, transformers and other	0-104	
		fixtures.		
\square	8.	Solid Waste Facilities: (2.5.4.3H)	Site Plan Sheet C-102.1	
	•	The size, type and location of solid waste facilities.	Site Plan Sheet C-102.1	
$\overline{\mathbf{A}}$	9.	Storm water Management: (2.5.4.3I)		
	•	The location, elevation and layout of all storm-water drainage.		
	•	The location of onsite snow storage areas and/or proposed off-	Grading and Drainage	
		site snow removal provisions.	Plan Sheet C-103	
	•	Location and containment measures for any salt storage facilities		
	•	Location of proposed temporary and permanent material storage		
		locations and distance from wetlands, water bodies, and		
	10	stormwater structures.	Disatementalies Disa	
	10.	Outdoor Lighting: (2.5.4.3J)	Photometrics Plan	
	•	Type and placement of all lighting (exterior of building, parking lot and any other areas of the site) and photometric plan.	Sheet	
Ø	11	Indicate where dark sky friendly lighting measures have	Photometrics Plan Sheets	
	11.	been implemented. (10.1)	Filotometrics Flam Sheets	
$\overline{\mathbf{Q}}$	12	Landscaping: (2.5.4.3K)		
	12.	Identify all undisturbed area, existing vegetation and that	Landscaping Plan	
		which is to be retained;	Sheets L-1 & L-2	
		 Location of any irrigation system and water source. 		
$\overline{\mathbf{Q}}$	13	Contours and Elevation: (2.5.4.3L)	Grading and Drainage	
	13.	Existing/Proposed contours (2 foot minimum) and finished	Plan Sheet C-103	
		grade elevations.	I lair officer o 100	
$\overline{\mathbf{Q}}$	14.	Open Space: (2.5.4.3M)	Site Dien Sheet C 102	
		 Type, extent and location of all existing/proposed open space. 	Site Plan Sheet C-102 & Open Space Exhibit	
			& Open Space Exhibit	
\square	15.	All easements, deed restrictions and non-public rights of	Existing Conditions Plan	
		ways. (2.5.4.3N)	-	
$\overline{\mathbf{V}}$	16.	Character/Civic District (All following information shall be		
		included): (2.5.4.3P)		
		• Applicable Building Height (10.5A21.20 & 10.5A43.30);	Site Plan Sheet C-102.1	
		Applicable Special Requirements (10.5A21.30); Applicable Special Requirements (10.5A21.30);	One i lan oneet o-102.1	
		Proposed building form/type (10.5A43); Proposed building form/type (10.5A46); Proposed building		
		• Proposed community space (10.5A46).		
V	17.	Special Flood Hazard Areas (2.5.4.3Q)		
		The proposed development is consistent with the need to		
		minimize flood damage;	Grading and Drainage	
		All public utilities and facilities are located and construction to	Plan Sheet and Utility	
		minimize or eliminate flood damage;	Plan Sheet C.103 &	
		Adequate drainage is provided so as to reduce exposure to	C.104	
		flood hazards.		

	Other Required Information			
V	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	Traffic Impact Study or Trip Generation Report, as required. (3.2.1-2)	Enclosed		
Ø	, , , , , , , , , , , , , , , , , , , ,	Grading and Drainage Pla Sheet C-103	n	
V	Indicate whether the proposed development is located in a wellhead protection or aquifer protection area. Such determination shall be approved by the Director of the Dept. of Public Works. (7.3.1)	N/A		
Ø	Stormwater Management and Erosion Control Plan. (7.4)	Enclosed		
V	Inspection and Maintenance Plan (7.6.5)	Enclosed		

	Final Site Plan Approval Required Information			
M	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	All local approvals, permits, easements and licenses required, including but not limited to: • Waivers; • Driveway permits; • Special exceptions; • Variances granted; • Easements; • Licenses. (2.5.3.2A)	Cover Sheet		
	 Exhibits, data, reports or studies that may have been required as part of the approval process, including but not limited to: Calculations relating to stormwater runoff; Information on composition and quantity of water demand and wastewater generated; Information on air, water or land pollutants to be discharged, including standards, quantity, treatment and/or controls; Estimates of traffic generation and counts pre- and post-construction; Estimates of noise generation; A Stormwater Management and Erosion Control Plan; Endangered species and archaeological / historical studies; Wetland and water body (coastal and inland) delineations; Environmental impact studies. 	Enclosed		
V	A document from each of the required private utility service providers indicating approval of the proposed site plan and indicating an ability to provide all required private utilities to the site. (2.5.3.2D)	Enclosed		

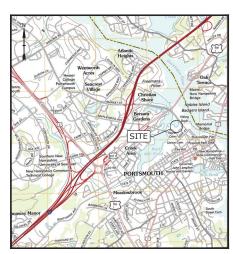
	Final Site Plan Approval Required Info	rmation	
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
Ø	A list of any required state and federal permit applications required for the project and the status of same. (2.5.3.2E)	Cover Sheet	
V	A note shall be provided on the Site Plan stating: "All conditions on this Plan shall remain in effect in perpetuity pursuant to the requirements of the Site Plan Review Regulations." (2.5.4.2E)	Site Plan Sheets C-102	N/A
	For site plans that involve land designated as "Special Flood Hazard Areas" (SFHA) by the National Flood Insurance Program (NFIP) confirmation that all necessary permits have been received from those governmental agencies from which approval is required by Federal or State law, including Section 404 of the Federal Water Pollution Control Act Amendments of 1972, 33 U.S.C. 1334. (2.5.4.2F)	N/A	
V	Plan sheets submitted for recording shall include the following notes: a. "This Site Plan shall be recorded in the Rockingham County Registry of Deeds." b. "All improvements shown on this Site Plan shall be constructed and maintained in accordance with the Plan by the property owner and all future property owners. No changes shall be made to this Site Plan without the express approval of the Portsmouth Planning Director."	Site Plan Sheets C-102.1	N/A
	(2.13.3)		

Applicant's Signature: _	AMC.	Date: 3/22/21	
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PROPOSED MIXED USE DEVELOPMENT

53 GREEN STREET
PORTSMOUTH, NEW HAMPSHIRE
JANUARY 27, 2021
LAST REVISED: MAY 19, 2021

LIST OF DRAWINGS			
SHEET NO.	SHEET TITLE	LAST REVISED	
	COVER SHEET	5/19/2021	
1 OF 2	EXISTING CONDITIONS PLAN	5/17/2021	
2 OF 2	EXISTING CONDITIONS PLAN	5/17/2021	
C-101	DEMOLITION PLAN	5/19/2021	
C-102.1	SITE PLAN	5/19/2021	
C-102.2	BASEMENT & UPPER FLOOR PLAN	5/19/2021	
C-103	GRADING, DRAINAGE AND EROSION CONTROL PLAN	5/19/2021	
C-104	UTILITIES PLAN	5/19/2021	
C-201	WATER MAIN REPLACEMENT PLAN	5/19/2021	
C-301	EASEMENT PLAN	5/19/2021	
C-501	EROSION CONTROL NOTES AND DETAILS SHEET	5/19/2021	
C-502	DETAILS SHEET	5/19/2021	
C-503	DETAILS SHEET	5/19/2021	
C-504	DETAILS SHEET	5/19/2021	
C-505	DETAILS SHEET	5/19/2021	
C-506	DETAILS SHEET	5/19/2021	
C-507	DETAILS SHEET	5/19/2021	
C-508	DETAILS SHEET	5/19/2021	
L-1	LANDSCAPE PLAN	5/19/2021	
1 OF 1	PHOTOMETRIC PLAN	3/22/2021	
1	BUILDING ELEVATION	3/22/2021	



LOCATION MAP

PREPARED BY:

177 CORPORATE DRIVE PORTSMOUTH, NEW HAMPSHIRE 03801 603-433-8818

OWNER: TAX MAP 119, LOT 12 STONE CREEK REALTY, LLC C/O DOUGLAS PINCIARO PO BOX 121

NEW CASTLE, NEW HAMPSHIRE 03854

APPLICANT:

CPI MANAGEMENT, LLC 100 SUMMER STREET, SUITE 1600 BOSTON. MASSACHUSETTS 02110

SURVEYOR:
DOUCET SURVEY, LLC

192 KENT PLACE

NEWMARKET, NEW HAMPSHIRE 30857

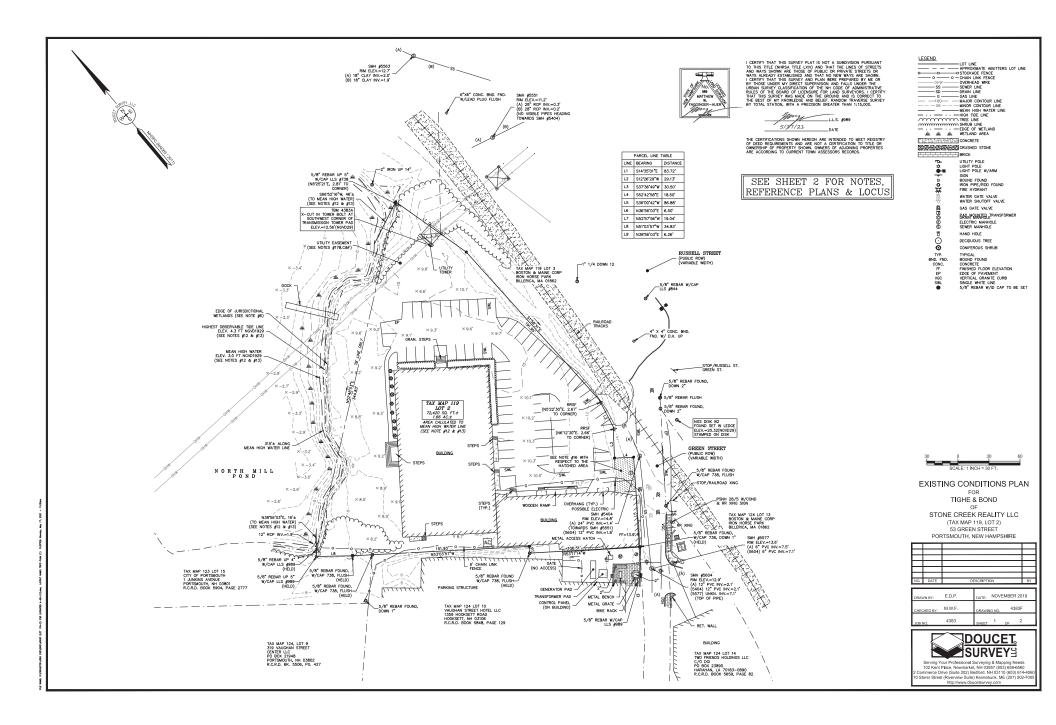
LIST OF PERMIT	ΓS	
LOCAL	STATUS	DATE
SITE PLAN REVIEW PERMIT	PENDING	
LOT LINE REVISION PERMIT	PENDING	
CONDITIONAL USE PERMIT - WETLAND BUFFER	PENDING	
STATE		
NHDES - SEWER CONNECTION PERMIT	PENDING	
NHDES - ALTERATION OF TERRAIN PERMIT	PENDING	
NHDES - WETLAND PERMIT	PENDING	





TAC RESUBMISSION SET COMPLETE SET 21 SHEETS

T & B PROJECT NO: C-0960-011



TAX MAP 119, LOT 2 53 GREEN STREET D.S.L. PROJECT NO. 4383

2. TOTAL PARCEL AREA:

72,420 SO. FT.± OR 1.66 AC.± (AREA CALCULATED TO MEAN HIGH WATER) (SEE NOTE #12)

C/O DOUGLAS PINGARGO PO BOX 121 NEW CASTLE, NH 03854 R.C.R.D. BOOK 3300, PAGE 329

OVERLAY DISTRICTS
-DOWNTOWN OVERLAY DISTRICT
-HISTORIC DISTRICT 4. ZONE: CD5

ZONNIC DISTRICTS BASSI ON THE CITY OF PORTSHOUTH ZURING MAP DATES 11/12/16 AS ANALABLE ON THE CITY INSERT ON 11/16/AS SEE CITY OF PORTSHOUTH ZONNIC ORDINACE ARTICLE SA SECTION 15-AM FOR MOMENSIONAL REQUAINONS. THE LAND COMMENT IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE MUNICIPAL, STATE AND FEDERAL REQULATIONS.

THE SITE IS SUBJECT TO THE STATE OF NH SHORELAND WATER QUALITY PROTECTION ACT. SEE NHDES WEBSITE FOR SPECIFIC DIMENSIONAL REQUIREMENT.

- FIELD SURVEY PERFORMED BY D.C.B. & K.J.L. DURING NOVEMBER 2019 USING A TRIMBLE S7 TOTAL STATION AND A TRIMBLE RB SURVEY GRADE OPS WITH A TRIMBLE TSCS DATA COLLECTOR AND A TRIMBLE DINI DIGITAL LEVEL. TRAVERSE ADJUSTMENT BASED ON LEAST SOUARE ANALYSIS.
- 6. JURISDICTIONAL WETLANDS DELINEATED BY TIGHE & BOND, DURING OCTOBER 2019 IN ACCORDANCE WITH 1987 CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, TECHNICAL REPORT Y-87-1 AND THE INTERIM REGIONAL SUPPLEMENT TO THE CORPS OF ENSINEERS WETLAND DELINEATION MANUAL: NORTH CENTRAL AND NORTHEAST REGION (OCTOBER, 2009).
- 7. VERTICAL DATUM IS BASED ON NGVD29 PER DISK B2 1923.
- HORIZONTAL DATUM BASED ON NEW HAMPSHIRE STATE PLANE(2800) NAD83(2011) DERIVED FROM REDUNDANT GPS OBSERVATIONS UTILIZING THE KEYNET GPS VRS NETWORK.
- PROPER FIELD PROCEDURES WERE FOLLOWED IN ORDER TO GENERATE CONTOURS AT 2' INTERVALS, ANY MODIFICATION OF THIS INTERVAL WILL DIMINIST THE INTEGRITY OF THE DATA, AND DOUCET SURVEY, INC. WILL NOT BE RESPONSIBLE FOR ANY SUCH ALTERATION PERFORMED BY THE USER.
- 10. UNDERGROUND UTILITIES SHOWN HEREON ARE BASED ON OBSERVABLE PHYSICAL EVIDENCE AND PAINT MARKS FOUND ON-SITE.
- THE ACCURACY OF MEASURED UTILITY INVERTS AND PIPE SIZES/TYPES IS SUBJECT TO NUMEROUS FIELD CONDITIONS, INCLUDING: THE ABILITY TO MAKE VISUAL OBSERVATIONS, DIRECT ACCESS TO THE VARIOUS ELEMENTS, MANHOLE CONFIGURATION, EVALUATION
- WATER BOUNDARIES ARE DYNAMIC IN NATURE AND ARE SUBJECT TO CHANGE DUE TO NATURAL CAUSES SUCH AS EROSION OR ACCRETION.
- 13. MEAN HIGH WATER (EL. 3.0" NGV01929) AND HIGHEST OBSERVABLE TIDE (EL. 4.3" NGV01929) ELEVATIONS FER "MAPERWOOD AVENUE CULVERT REPLACEMENT AND NORTH MILL POND RESTORATION, WATERFRONT ISTUCULTURAL BASIS OF DESION, BY WATERFRONT ENGINEERS, LLC, DATED DECEMBER 30, 2009", PROVIDED BY TIGHE & BOND ON 11-30-15.

- 16. THE GEOMETRY SHOWN ON REFERENCE PLANS 12 & 13 INDICATE THE HATCHED AREA MAY BE SUBJECT TO THE GREEN STREET RIGHT-OF-WAY, R.C.R.D. BOOK 589, PAGE 206 INDICATES FEE OWNERSHIP EXTENDS TO THE CENTERLINE OF GREEN STREET IN THIS AREA.
- 17. TAX MAP 119 LOT 2 SHOWN HEREON IS SUBJECT TO AND/OR IN BENEFIT OF THE FOLLOWING EASEMENTS & COMPINANTS
- EASBERTS & COVENANTS.

 A) SIGNAL FALLIES EXCEPTIONS AND RESERVATIONS, SEE R.C.R.D. BOOK 1339, PAGE 298,

 B) EASBERT IN FAVOR OF WESTERN UNION TELEGRAPH COMPANY, SEE R.C.R.D. BOOK 1339,

 PAGE 298 (DO DENESSION SIGNA).

 1030, PAGE 298 (DO DENESSION SIGNA).

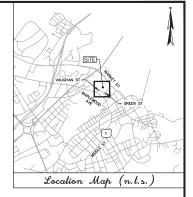
 2050, PAGE 298 (DO DENESSION SIGNA).

 2050, PAGE 298 (DO DENESSION SIGNA).

- 18. ALL UNDERGROUND UTILITIES (ELECTRIC, GAS, TEL. WATER, SEMER DRAIN SERVICES) ARE SHOWN IN SOHEMATIC FASHON, THEIR LOCATIONS ARE NOT PRECISE OR NECESSARILY ACCURATE. NO WORK WHATSDEVER SHALL BE UNDERTAKEN USON THIS PLAN TO LOCATE THE ABOVE SERVICES. CONSULT WITH THE PROPER AUTHORITIES CONCERNED WITH THE SUBJECT SERVICE LOCATIONS FOR NYFORMATION REGARDING SUCH CALL DIG-SAFE AT 1-588-DIG-SAFE.

REFERENCE PLANS

- "STANDARD BOUNDARY SURVEY, TAX MAP 119 LOT 2, LAND OF STONE CREEK REALTY", DATED MARCH 2016, BY AMBIT ENGINEERING, INC., NOT RECORDED.
- "PLAN OF LAND, VAUGHAN AND GREEN STREETS, PORTSMOUTH, NH" DATED JULY 1955 BY JOHN W. DURGIN R.C.R.D. PLAN #02541.
- "STANDARD BOUNDARY SURVEY, TAX MAP 123 LOT 15 & TAX MAP 124 LOT 10" DATED JULY 2008, REVISED 4/25/13 BY AMBIT ENGINEERING, INC. R.C.R.D. PLAN #D-37722.
- "EASEMENT PLAN, EGRESS EASEMENT TO 319 VAUGHAN STREET CENTER, LLC, TAX MAP 124, LOT 9 & TAX MAP 123, LOT 15, PROPERTY OF 299 VAUGHAN STREET, LLC C/O CATHARTES PRIVATE INVESTMENTS", BY AMBIT FORMICERING, INC., DATED MARCH 2014, R.C.R.D. PLAN #0-3830.
- "CONDOMINUM SITE PLAN TAX MAP 124 LOT 14, 233 VAUGHAN STREET, A CONDOMINIUM FOR 233 VAUGHAN STREET, LLC", BY AMBIT ENGINEERING, INC., DATED NOVEMBER 2013, R.C.R.D. PLAN #D-39078.
- "LOT LINE RELOCATION PLAN PROPERTY OF HARBORCORP, LLC & BOSTON & MAINE CORPORATION", BY AMES MSC, DATED MARCH 15, 2005, R.C.R.D. PLAN #D-32675.
- "LAND AT 233 VAUGHAN STREET PORTSMOUTH, NH BOSTON & MAINE CORPORATION TO BLUE STAR PROPERTIES, LLC", BY JAMES VERRA & ASSOCIATES, INC., DATED 6/3/01, R.C.R.D. PLAN #0-29702.
- "VAUGHAN STREET URBAN RENEWAL PROJECT N.H. R-10 PORTSMOUTH, NH, DISPOSITION MAP", BY ANDERSON-NICHOLS & CO., INC., DATED NOVEMBER 1969, R.C.R.D. PLAN D-2408
- "PLAN OF LAND FOR SOLIMON NEGM", BY TOWN PLANNING & ENGINEERING ASSOCIATES, INC., DATED 3/28/79, R.C.R.D. PLAN #C-8575.
- "VAUGHAN STREET URBAN RENEWAL PROJECT N.H. R-10 PORTSMOUTH, NH, DISPOSITION PLAN PARCEL 2", BY ANDERSON-MICHOLS & CO., NC., DATED OCTOBER 1973, R.C.R.D. PLAN D-4115.
- 11. "PLAN OF PROPERTY CORNER VAUGHAN AND GREEN STREETS". DATED FEBRUARY 1907, R.C.R.D. PLAN #306
- "LAND SHOWING LAND WHARFAGE OWNED BY SILAS PEIRCE AND CO. LTD.", BY A.C. HOYT SURVEYOR, DATED AUGUST 8, 1902, R.C.R.D. PLAN #266.
- *PLAN OF LAND PORTSMOUTH, NH FOR GEORGE D. EMERSON CO., BY JOHN W. DURGIN, DATED APRIL 1952, ON FILE AT JAMES VERRA AND ASSOCIATES.
- 14. *PLAN OF LAND VAUGHAN AND GREEN STREETS PORTSMOUTH, NH FOR SAMUEL W. & SUMNER L. POORVU*, BY JOHN W. DURGIN, DATED JANUARY 1956, ON FILE AT JAMES VERRA AND ASSOCIATES.
- 15. "PLAN OF PROPERTY IN PORTSMOUTH, NH OWNED BY R.I. SUGDEN", BY WM A. GROVER, DATED APRIL 15, 1919, ON FILE AT JAMES VERRA AND ASSOCIATES.
- 16. "LAND ON VAUGHAN STREET PORTSMOUTH, NH, ESTATE OF CARRIE HAM TO LAWRENCE V. REGAN" BY JOHN W. DURGIN, DATED AUGUST 6, 1937, ON FILE AT JAMES VERRA AND ASSOCIATES.
- "LAND IN PORTSMOUTH, NH, BOSTON & MAINE RAILROAD TO GEORGE D. EMERSON COMPANY", DATED JUNE 1954, R.C.R.D. BOOK 1339, PAGE 305.
- VAUGHAN STREET URBAN RENEWAL PROJECT N.H. R-10 PORTSMOUTH, NH, APPROVED AS SHOWING VAUGHAN STREET URBAN RENEWAL PROJECT BOUNDARIES AND AREA ONLY, CONDEMNATION MAP", BY ANDERSON-INCHOLS & CO., INC., DATED FERRIVARY 1971, R.C.R.D, PAIN 2425.
- "SURVEY OF HARBORSIDE & HARBORPARK LAND IN PORTSMOUTH, NH", BY BRIGGS ASSOCIATES, INC., DATED AUGUST 13, 1985, REV. AUGUST 27, 1985, R.C.R.D. PLAN 14043.
- "SUBDIVISION PLAN OF TAX MAP 123, LOT 15 FOR 299 VAUGHAN STREET, LLC", BY DOUCET SURVEY, INC., DATED MAY 19, 2017, R.C.R.D. PLAN D-40759.
- *LICENSE, EASEMENT & LAND TRANSFER PLAN FOR VAUGHAN STREET, LLC AND VAUGHAN STREET HOTEL, LLC*, BY DOUCET SURVEY, INC., DATED AUGUST 2017, R.C.R.D. PLAN D-40760.
- 23. "LOT MERGER PLAN FOR VAUGHAN STREET HOTEL, LLC", BY DOUCET SURVEY, INC., DATED SEPTEMBER 2017.
- "VAUGHAN STREET PROJECT, PROJECT NO. N.H. R-10, RIGHT OF WAY ADJUSTMENT", BY METCALF & EDDY, DATED MAY 5, 1966, R.C.R.D. PLAN D-2413.
- 26. "SKETCH OF RAILROAD CONVEYANCE, SEE R.C.R.D. BOOK 446, PAGE 164A.
- "VAUGHAN STREET URBAN RENEWAL PROJECT N.H. R-10, PORTSMOUTH, NH, DISPOSITION PLAN, PARCEL 28", BY ANDERSON-NICHOLS & CO., INC., DATED APRIL 1974, R.C.R.D. PLAN DC-4518.
- 28. "SEWER EASEMENT PLAN, TAX MAP 119, LOT 4, PROPERTY OF NORTH END MASTER DEVELOPMENT LP, GREEN MARKET & RUSSELL STREETS, PORTSMOUTH, NEW HAMPSHIRE, COUNTY OF ROCKINGHAM", BY 1FM, DATED JULY 16, 2019.
- 29. "SUBDIVISION PLAN OF PARCELS 1 & 2 IN PORTSMOUTH, NH FOR THE CITY OF PORTSMOUTH", BY BRIGGS ASSOCIATES INC., DATED AUGUST 1, 1984. R.C.R.D. PLAN D-13798.
- 30. "VAUGHAN STREET PROJECT, PROJECT NO. N.H. R-10, PROPERTY MAP-A, PORTSMOUTH HOUSING AUTHORITY PORTSMOUTH, NEW HAMPSHIRE, ROCKINGHAM COUNTY", BY METCALF & EDDY, DATED MAY 5, 1966, R.C.R.D. PLAN D-2410.
- "LAND IN PORTSMOUTH, NH, BOSTON & MAINE RAILROAD TO ROSE R. WOLFSON", DATED JUNE 1954, R.C.R.D. PLAN 2282.



EXISTING CONDITIONS PLAN

TIGHE & BOND

STONE CREEK REALITY LLC (TAX MAP 119, LOT 2) 53 GREEN STREET PORTSMOUTH. NEW HAMPSHIRE

H					
H					
NO.	DATE	DESCRIPTION		BY	

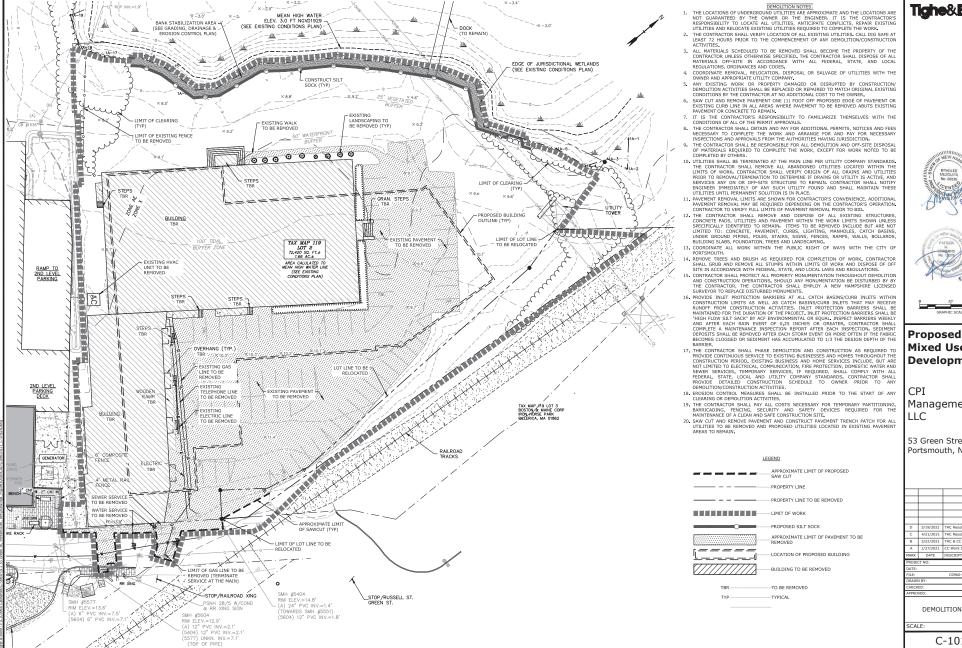
RAWN BY:	E.D.P.	DATE: NOVEMBER 2019
HECKED BY:	M.W.F.	DRAWING NO. 4383F
08 NO.	4383	SHEET 2 OF 2



mmerce Drive (Suite 202) Bedford, NH 03110 (603) 614-40 torer Street (Riverview Suite) Kennebunk, ME (207) 502-70

I CERTIFY THAT THES UNIVEY FAY IS NOT A SAEDWISON PARSUANT FOR THE THAT THE SUPPLY FAY IS NOT A SAEDWISON PARSUANT FAY IS NOT A SAEDWISON PARSUANT FAY IS NOT A SAEDWISON PARSUANT STREETS OF WAY, AND EAST SAEDWISON PARSUANT IS NOT AND FALS UNDER THE PARSUANT FAY IS NOT THOSE UNDER WY DESCT SAEDWISON AND FALS UNDER THE PARSUANT FAY IS NOT THOSE UNDER WY DESCT SAEDWISON AND FALS UNDER THE PARSUANT FAY IS NOT THE SAEDWISON FAR IS NOT THE SAEDWISON FAR IS NOT THE SAEDWISON FALSE WITH THE SAEDWISON FAST SAEDWISON FAS





Tighe&Bond







Mixed Use Development

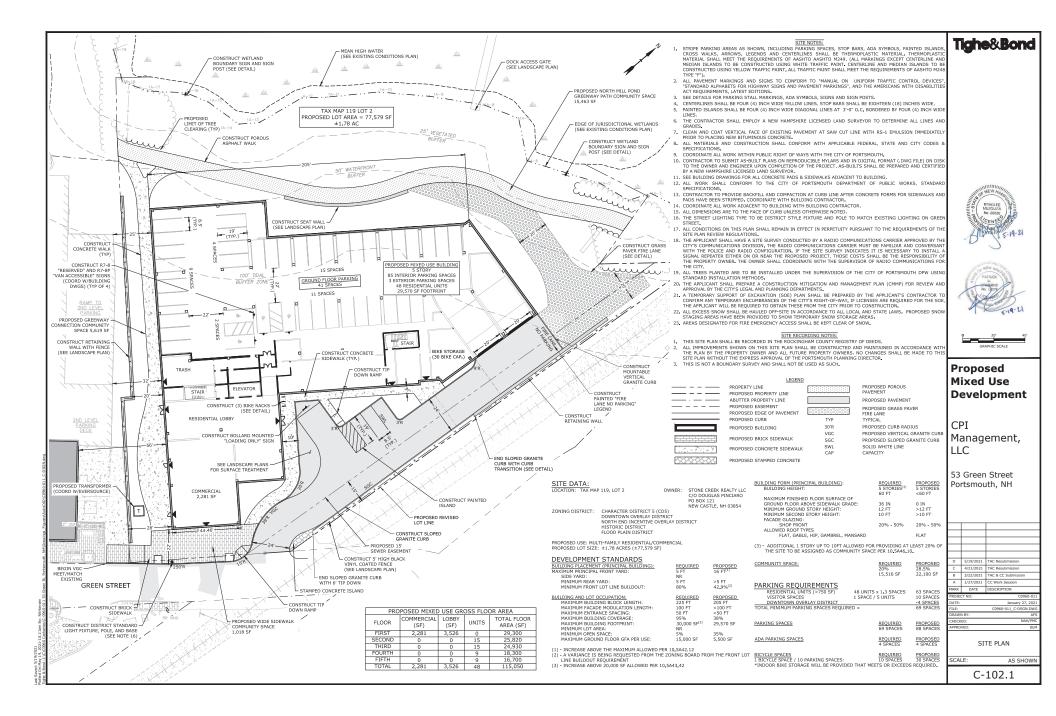
Management,

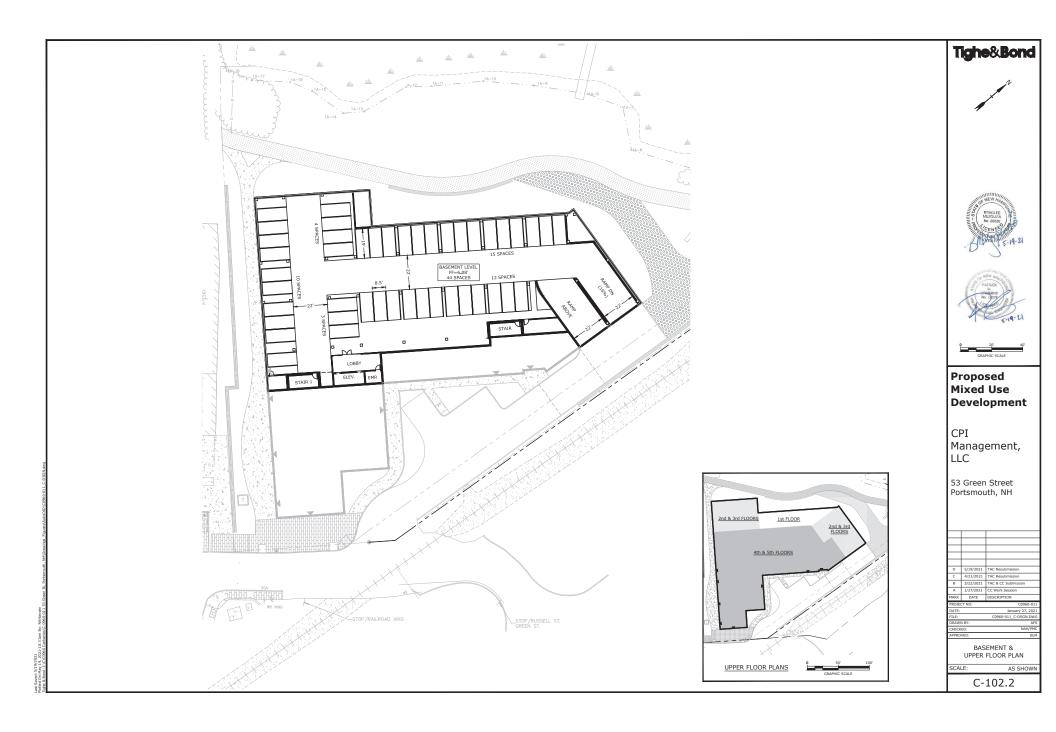
53 Green Street Portsmouth, NH

D	5/19/2021	TAC Resubmission
С	4/21/2021	TAC Resubmission
В	3/22/2021	TAC & CC Submission
A	1/27/2021	CC Work Session
MARK	DATE	DESCRIPTION
PROJEC	T NO:	C0960-0
DATE:		January 27, 20

DEMOLITION PLAN

AS SHOWN

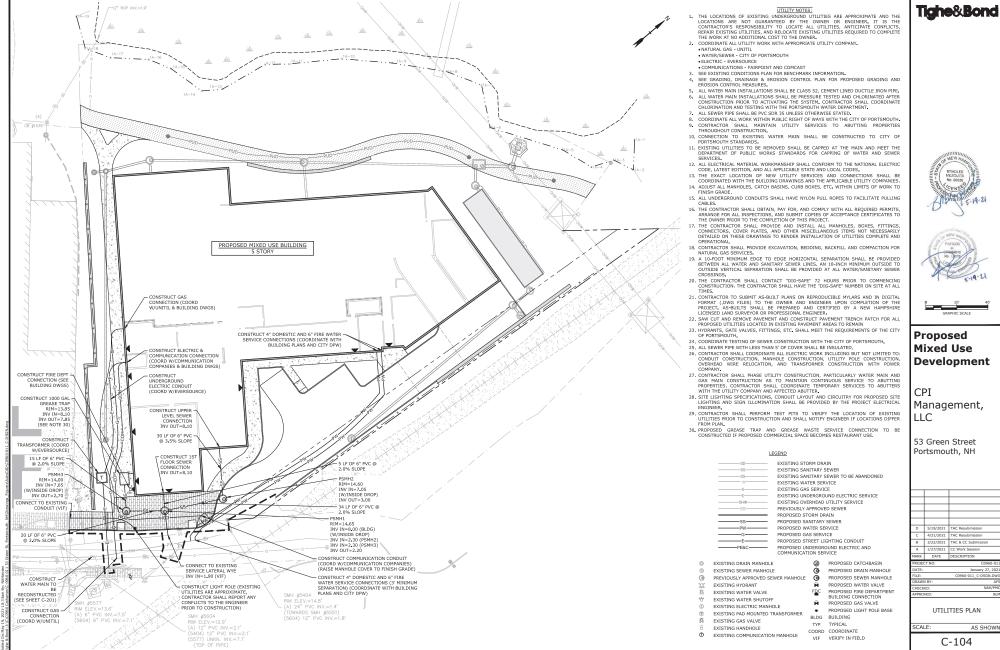




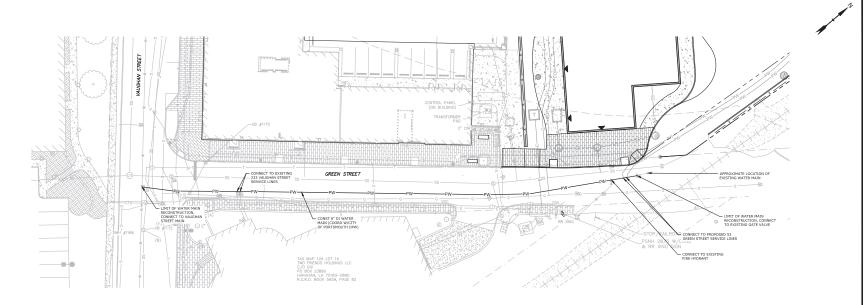








r NO:	C0960-011
	January 27, 2021
	C0960-011_C-DSGN.DWG
BY:	AFS
D:	NAH/PMC
ED:	BLM



THE LOCATIONS OF EXISTING UNDERGROUND LITLITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT
GURANTIED BY THE OWNER OR BENGIESE, IT IS THE CONTRACTOR'S REPOSIBILITY TO LOCATE ALL
UTILITIES, ANTICIPATE CONFLICTS, REPAIR EXISTING UTILITIES, AND RELOCATE EXISTING UTILITIES REQUIRED
TO COMPLETE THE WORK AT NO ADDITIONAL COST TO THE OWNER.

2. COORDINATE ALL UTILITY WORK WITH APPROPRIATE UTILITY COMPANY.

NATURAL GAS - UNITIL

WATENSEWER - CITY OF PORTSMOUTH

ELECTRIC - EVERSOURCE

COMMUNICATIONS - FAIRPOINT AND COMCAST

- COMMONICATIONS FAMILY ON BENCHMARK INFORMATION.
 SEE GRADING, DRAINAGE & EROSION CONTROL PLAN FOR PROPOSED GRADING AND EROSION CONTROL PLAN FOR PROPOSED GRADING AND EROSION CONTROL MEASURES.

 ALL WATER MAIN INSTALLATIONS SHALL BE CLASS 52, CEMENT LINED DUCTILE IRON PIPE.
- ALL WATER MAIN INSTALLATIONS SHALL BE PRESSURE TESTED AND CHLORINATED AFTER CONSTRUCTION PRIOR TO ACTIVATING THE SYSTEM. CONTRACTOR SHALL COORDINATE CHLORINATION AND TESTING WITH THE PORTSMOUTH WATER DEPARTMENT.
- COORDINATE ALL WORK WITHIN PUBLIC RIGHT OF WAYS WITH THE CITY OF PORTSMOUTH.

- COORDINATE ALL WORK WITHIN PUBLIC RIGHT OF WAX WITH THE CITY OF PORTSMOUTH.
 CONTRACTOR SHALL MAINTAIN UTILITY SERVICES TO ABUTTING PROPERTIES THROUGHOUT CONSTRUCTION.
 CONNECTIONS TO EXISTING WATER LIBES SHALL BE CONSTRUCTED TO CITY OF PORTSMOUTH STANDARDS.
 CEXISTING UTILITIES TO BE REMOVED SHALL BE CONSTRUCTED TO CITY OF PORTSMOUTH STANDARDS.
 CONNES STANDARDS FOR CAPPING OF WATER AND SEWER SERVICES.
 THE EXACT LOCATION OF NEW UTILITY SERVICES AND COMMECTIONS SHALL BE COORDINATED WITH THE BUILDING DRAWINGS AND THE APPLICABLE UTILITY COMPANIES.
 ADUIST ALL MAINOLES, CATCH BASINS, CURB BOXES, ETC. WITHIN LIMITS OF WORK TO TRINSH GRADE.
- THE CONTRACTOR SHALL OBTAIN, PAY FOR, AND COMPLY WITH ALL REQUIRED PERMITS, ARRANGE FOR ALL INSPECTIONS, AND SUBMIT COPIES OF ACCEPTANCE CERTIFICATES TO THE OWNER PRIOR TO THE COMPLETION OF THIS PROJECT.
- INSPECTIONS, AND SUBMIT COPIES OF ACCEPTANCE CERTIFICATES TO THE OWNER PRIOR TO THE COMPLETION
 OF THIS PROVIDES. THE PROVIDE AND INSTALL ALL MANIOLES, BOXES, FITTINGS, CONNECTORS, COVER
 14. THE CONTRACTION FROM THE PROVIDE AND INSTALL AND THE PROVIDED BOTH THE PROVIDED BETWEEN ALL WATER AND
 INSTALLATION OF UTILITIES CONNECTED AND PROVIDED BETWEEN ALL WATER AND
 SANITARY SEVER LINES. AN IS-INCH MINIMUM OUTSIDE OF OUTSIDE VERTICAL SEPARATION SHALL BE
 ASANITARY SEVER LINES. AN IS-INCH MINIMUM OUTSIDE OF OUTSIDE VERTICAL SEPARATION SHALL BE
 15. THE CONTRACTOR SEVER AND THE CONTRACTOR SHALL BEEN BEEN SHALL BEEN SHA

GREEN STREET PAVING:

AFTER UTILITY CONSTRUCTION, CONTRACTOR SHALL MILL GREEN STREET PAVEMENT AT A DEPTH OF 1.5" AND PAVE WEARING COURSE TO EXISTING GRADE. LIMITS OF MILL AND PAVING SHALL BE COORDINATED WITH THE CITY PRIOR TO CONSTRUCTION.

LEGEND



- EXISTING DRAIN MANHOLE
- EXISTING SEWER MANHOLE
- PREVIOUSLY APPROVED SEWER MANHOLE PROPOSED SEWER MANHOLE
- EXISTING HYDRANT
- EXISTING WATER VALVE EXISTING WATER SHUTOFF
- EXISTING ELECTRIC MANHOLE
- EXISTING PAD MOUNTED TRANSFORMER
- EXISTING GAS VALVE EXISTING HANDHOLE
- EXISTING COMMUNICATION MANHOLE
- PROPOSED GAS VALVE ■ PROPOSED LIGHT POLE BASE

PROPOSED CATCHBASIN

PROPOSED DRAIN MANHOLE

PROPOSED WATER VALVE

BLDG BUILDING TYP TYPICAL

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- COORD COORDINATE VERIFY IN FIELD
- DWGS DRAWINGS

Tighe&Bond







Proposed Mixed Use Development

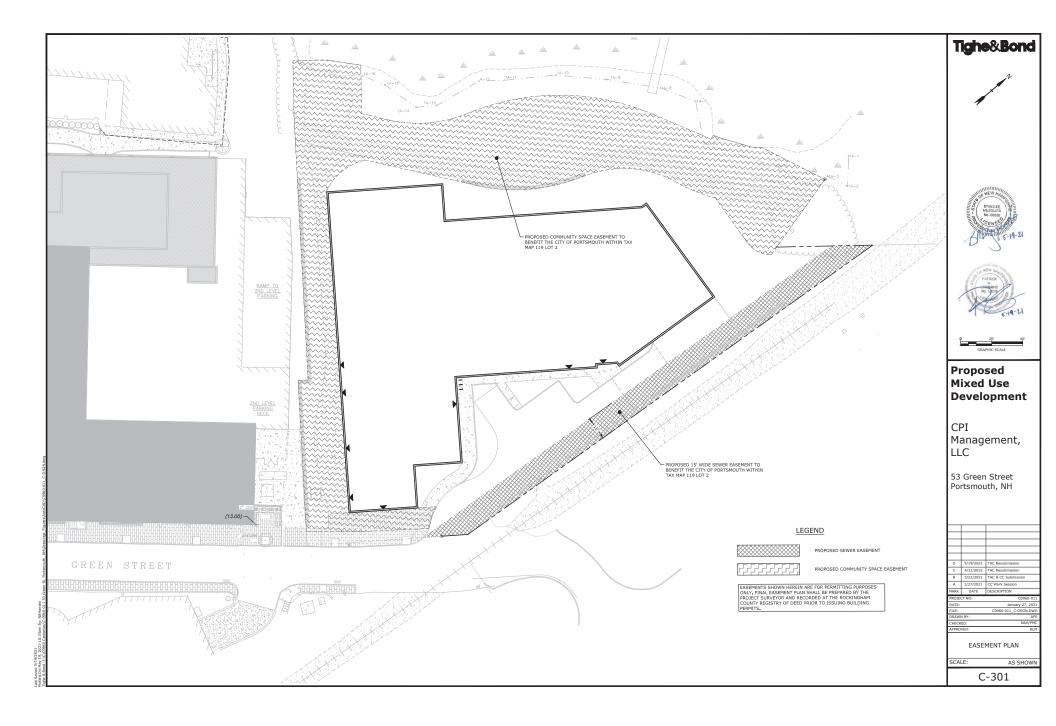
CPI Management, LLC

53 Green Street Portsmouth, NH

D	5/19/2021	TAC Resubmission
С	4/21/2021	TAC Resubmission
В	3/22/2021	TAC & CC Submission
А	1/27/2021	CC Work Session
MARK	DATE	DESCRIPTION
PROJE	T NO:	C0960

WATER MAIN

AS SHOWN



439-04'-4R"N

PROJECT DESCRIPTION
THE PROJECT CONSISTS OF THE CONSTRUCTION OF A FIVE-STORY MIXED USE RESIDENTIAL BUILDING WITH ASSOCIATED SITE IMPROVEMENTS.

<u>DISTURBED AREA</u>
THE TOTAL AREA TO BE DISTURBED IS APPROXIMATELY 1.75 ACRES.

SOIL CHARACTERISTICS

PAGED ON THE NRCS WEB SOIL SURVEY FOR ROCKINGHAM COUNTY - NEW HAMPSHIRE, THE SOILS ON SITE CONSIST OF URBAN LAND

NAME OF RECEIVING WATERS
THE STORMWATER RUNOFF FROM THE SITE WILL BE DISCHARGED VIA A PROPOSED OUTLET PIPE TO

CONSTRUCTION SEQUENCE OF MAJOR ACTIVITIES:

- CUT AND CLEAR TREES.
 CONSTRUCT TEMPORARY AND PERMANENT SEDIMENT, EROSION AND DETENTION CONTROL
 FACILITIES. EROSION, SEDIMENT AND DETENTION MEASURES SHALL BE INSTALLED PRIOR TO
 ANY EARTH MOVING OPERATIONS THAT WILL INFLUENCE STORMWATER RUNOFF SUCH AS:

 NEW CONSTRUCTION
 CONTROL OF DUST
- NEARNESS OF CONSTRUCTION SITE TO RECEIVING WATERS
- CONSTRUCTION DURING LATE WINTER AND EARLY SPRING
 ALL PERMANENT DITCHES, SWALES, DETENTION, RETENTION AND SEDIMENTATION BASINS TO BE STABILIZED USING THE VEGETATIVE AND NON-STRUCTURAL BMPS PRIOR TO DIRECTING

- RUNDET TO THEM.

 A. CLEAR AND ISSOSE OF DEBIS.

 B. CONSTRUCT TEMPORARY CULVER'S AND DIVERSION CHANNELS AS REQUIRED.

 GRADE AND GRAVE RADOWN'S AND PARKING AREAS ALL ROADS AND PARKING AREA SHALL

 BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

 B. CLIT AND FILL SLOPES SHALL

 B. ESIGH PERMANENT AND TEMPORARY SEEDING AND MULCHING, ALE CLIT AND FILL SLOPES SHALL

 B. ESIGH PERMANENT AND TEMPORARY SEEDING AND MULCHING, ALE CLIT AND FILL SLOPES SHALL

 REQUIRED, CONSTRUCT TEMPORARY SERMS, DRAINS, DITCHES, PREIMETER EROSION CONTROL MEASURES, SEDIMENT TRAPS, ETC., MULCH AND SEED AS REQUIRED FOR SHALL AND MANY AND PARKING LOTS.

 B. FINISH PAVING ALL ROADWAYS AND PARKING LOTS.

 INSPECT AND MAINTAIN ALL RESOLON AND SEDIMENT CONTROL MEASURES.

 10. COLOR TEMPORARY SEEDING AND LOSCOMES.

 TEMPORARY SERVICES OF THE MEASURE SHALL AND SHALL AND THEN REMOVE TEMPORARY SERVICES AND REVIEWS AND THEN REMOVE TEMPORARY SERVICES AND REVIEWS AND THEN REMOVE TEMPORARY SERVICES AND REVIEWS AND THEN REMOVE TEMPORARY SERVICES AND PROPRIATE AND THEN REMOVE TEMPORARY SERVICES.
- TEMPORARY FROSION CONTROL MEASURES.

SPECIAL CONSTRUCTION NOTES:

1. THE CONSTRUCTION SEQUENCE MUST LIMIT THE DURATION AND AREA OF DISTURBANCE.

2. THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

- EROSION CONTROL NOTES:

 1. ALL EROSIONE COMPON LIBESURES AND PRACTICES SHALL CONFORM TO THE "NEW HAMPSHIRE EROSIONE COMPON LIBESURES AND PRACTICES SHALL CONFORM TO THE "NEW HAMPSHIRE EROSION CONTROL TO MAN HAMBOR AND THE HAMES.

 2. PRIOR TO ANY WORK OR SOIL DISTURBANCE, CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR EROSION CONTROL MEASURES AS REQUIRED IN THE PROPECT MANUEL, INCLUDING HAY BALE, CONTRACTOR SHALL INSTALL TEMPORARY EROSION CONTROL BARRIERS, INCLUDING HAY BALE, SILT FERCES, MULCH BERMS; SILT SACKA AND SILT SOCKA AS SHOWN IN THESE DRAWINGS AS
- THE FIRST ORDER OF WORK
- SILT SACK INLET PROTECTION SHALL BE INSTALLED IN ALL EXISTING AND PROPOSED CATCH BASIN INLETS WITHIN THE WORK LIMITS AND BE MAINTAINED FOR THE DURATION OF THE
- BASIN NULETS WITHIN THE WORK LIMITS AND BE MAINTAINED FOR THE DURATION OF THE PREMIETE CONTROLS INCLUDING SILT FENCES, MULCH BERM, SILT SOCK, AND/OR HAY BALE BANKIERS SHALL BE MAINTAINED FOR THE DURATION OF THE PROJECT UNTIL NON-PAVED AREAS THE CONTRACTOR SHALL REMOVE AND PROPERTY DISPOSE OF ALL TEMPORARY EROSION CONTROL DEVICE UPPO COMPLETION OF CONSTRUCTION. ALL DISTURBED AREAS NOT OTHERWISE BEING TREATED SHALL RECEIVE 6" LOAM, SEED AND FERTILIZER.

- INSPECT ALL INLET PROTECTION AND PERIMETER CONTROLS WEEKLY AND AFTER EACH RAIN INSPECIALL INC. PROJECTION INDEPENDED FOR CONTINUE WEEKS IN AN AFTER BOGH RAIL STORM OF 0.25 INCH OR GREATER, REPAIR/MODIFY PROTECTION AS NECESSARY TO MAXIMIZE EFFICIENCY OF FILTER, REPLACE ALL FILTERS WHEN SEDIMENT IS 1/3 THE FILTER HEIGHT. CONSTRUCT REOSION CONTROL BLANKETS ON ALL SLOPES STEEPER THAN 3:1.

- STABILIZATION:

 1. AN AREA SHALL BE CONSIDERED STABLE WHEN ONE OF THE FOLLOWING HAS OCCURRED:

 A. BASE COURSE GRAVELS HAVE BEEN INSTALLED IN AREAS TO BE PAVED;

 B. A MINIMUM OF 50'S W VEGETATED GROWTH HAS BEEN ESTABLISHED;

 C. A MINIMUM OF 30' OF NON-EROSIDE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN

- A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN RISTALLED:
 RISTALLED:
 RISTALLED:
 RINGE RESEARCH STANDER STAND
- B. ALL DITCHES OR SWALES WHICH DO NOT ENHIBIT A MINIMUM OF 85 RECENT WEETATIVE CONTINUED CITIZENS 15, ON MICH ARE DISTURBED AFFER OCTORES 15, SMALL BE CONTINUED CITIZENS 15, SMALL BE CONTINUED CONTINUED
- DAYS BY THE FOURTEENTH (14TH) DAY AFTER CONSTRUCTION ACTIVITY HAS PE TEMPORARILY CEASED IN THAT AREA. STABILIZATION MEASURES TO BE USED INCLUDE: TEMPORARY SEEDING:
- B. MILCHING.

 WHEN CONSTRUCTION ACTIVITY PERMANENTLY OR TEMPORARILY CEASES WITHIN 100 FEET OF NEARBY SURFACE WATERS OR DELINEATED WETLANDS, THE AREA SHALL BE STABILIZED WITHIN SEVEN (7) DAYS OR PRIOR TO A RANN EVENT, ONCE CONSTRUCTION ACTIVITY CEASES SEVEN (7) DAYS OR PRIOR TO A RANN EVENT, ONCE CONSTRUCTION ACTIVITY CEASES AND ANY EARTHOUSES SHALL BE REMOVED ONCE PERMANENT MEASURES ARE ESTABLISHED. AND EARTHOUSES SHALL BE REMOVED ONCE PERMANENT MEASURES ARE ESTABLISHED, PROPRIED ONCE PERMANENT MEASURES ARE ESTABLISHED, PRIVAL BE DIVERTED AROUND THE SITE WITH EARTH DIVERS, PIPPING OR STABILIZED CHANNELS WHERE POSSIBLE, SHEET RUNOFF FROM THE SITE WILL BE THERED THROUGH SLIT PRIVALS MUCH BERRIES, MAJE BE ANGERDE, OR SLIT SOCKS, ALL STOOM DAIL DESIGN HERE SHALL BE STABLISHED FOR THE WITH EARTH ON SECTIONS AND TRASH ROCKS. THE STATE BY SHALL BE STABLISHED FOR THE WITH EARTH SAY HOUSE AND TRASH ROCKS. THE STATE BY SHALL BE STABLISHED FOR THE WITH EART SAY OF STATES AND SHALL BE STABLISHED FOR THE WITH EARTH SAY OF STATES AND SHALL BE STABLISHED FOR THE WITH EARTH SAY OF STATES.

DUST CONTROL:

ONTRACTOR SHALL BE RESPONSIBLE TO CONTROL DUST THROUGHOUT THE

CONSTRUCTION PERIOD.

DUST CONTROL METHODS SHALL INCLUDE, BUT BE NOT LIMITED TO SPRINKLING WATER ON EXPOSED AREAS, COVERING LOADED DUMP TRUCKS LEAVING THE SITE, AND TEMPORARY

MULCHING.
3. DUST CONTROL MEASURES SHALL BE UTILIZED SO AS TO PREVENT THE MIGRATION OF DUST FROM THE SITE TO ABUTTING AREAS.

- STOCKPILES:

 1. LOCATE STOCKPILES A MINIMUM OF 50 FEET AWAY FROM CATCH BASINS, SWALES, AND
- CULVERTS. ALL STOCKPILES SHOULD BE SURROUNDED WITH TEMPORARY EROSION CONTROL MEASURES
- PRIOR TO THE OWSET OF PRECIPITATION.

 PRIOR TO THE OWSET OF PRECIPITATION.

 PRIMETER BARRIES SHOULD BE WAINTAINED AT ALL TIMES, AND ADJUSTED AS NEEDED TO ACCOMMODATE THE DELLIVERY AND REMOVAL OF MATERIALS FROM THE STOCKPILE. THE INTERCRIT OF THE PRIOR FEEL OF WORKING DAY.

 INTERCRIT OF THE BARRIES HOULD BE INSPECTED AT THE ERNO OF FACH WORKING DAY.

 MEASURES SUCH AS BERNS, SLIT SOCK, OR OTHER APPROVED PRACTICE TO PREVENT MIGRATION OF MATERIAL BEYOND THE IMPORTANCE FORCHIES OF THE STOCKPILES.

OFF SITE VEHICLE TRACKING:

ONSTRUCT STABILIZED CONSTRUCTION ENTRANCE(S) PRIOR TO ANY EXCAVATION ACTIVITIES.

VEGETATION:

- TERPURARY WASSO LUVING
 THE AND THE ARTHOUGH FOR HEAD FOR ACRE OF 10-10-10, APPLY LIMESTONE
 APPLY FERRITLIZER AT THE RATE OF 600 POUNDS PER ACRE OF 10-10-10, APPLY LIMESTONE
 CQUIVALENT TO 50 PERCENT CALCIUM PIUS MAGNESIUM OXIDE) AT A RATE OF THREE (3)
 TORIS PER ACRE;

 SEEDING
 UTILIZE ANNUAL RYE GRASS AT A RATE OF 40 LBS/ACRE;
 UTILIZE ANNUAL RYE GRASS AT A RATE OF 40 LBS/ACRE;
 OF THE ACRE OF THE

- UTILIZE ANNUAL RYE GRASS AT A RATE OF 40 LBS/ACRE;
 WHEBE THE SOIL HAS BERG COMPACTED BY CONSTRUCTION OPERATIONS, LOOSEN SOIL
 TO A DEPTH OF TWO (2) INCHES BEFORE APPLYING FERTILIZER, LIME AND SEED;
 APPLY SEED UNIFORMLY BY HAND, CYCLONE SEEDER, OR HYDROSEEDER (SLURRY
 INCLUDING SEED AND FERTILIZER), HYDROSEEDINGS, WHICH INCLUDE WILCH, MAY BE LEFT ON SOIL SURFACE, SEEDING RATES MUST BE INCREASED 10% WHEN HYDROSEEDING
- INTENANCE: TEMPORARY SEEDING SHALL BE PERIODICALLY INSPECTED. AT A MINIMUM, 95% OF THE SOIL SURFACE SHOULD BE COVERED BY VEGETATION. IF ANY EVIDENCE OF EROSION OR SEDIMENTATION IS APPARENT, REPAIRS SHALL BE MADE AND OTHER TEMPORARY MEASURES USED IN THE INTERIM (MULCH, FILTER BARRIERS, CHECK DAMS, ETC.).
- WEGSURES USED IN THE INTERIM (MULCH, FILTER BARRIERS, CHECK DAMS, ETC.).

 VEGETATIVE PRACE DESCRIPTION DE AUTHORS:

 FOR THE STANDARD STAND
- SOIL CONDITIONERS AND FERTILIZER STRAIL BE APPLIED AT THE RECOMPRINDED ARE AND SHALL BE THOROUGHLY WORKED INTO THE LOAM. LOAM SHALL BE RAKED UNTIL THE SURFACE IS FINELY PULVERIZED, SMOOTH AND EVEN, AND THEN COMPACTED TO AN EVEN SURFACE CONFORMING TO THE REQUIRED LINES AND GRADES WITH APPROVED ROLLERS WEIGHING BETWEEN 4-1/2 POUNDS AND 5-1/2 POUNDS PER INCH OF WIDTH;
- WEIGHING BETWEEN 4-1/2 POLINOS AND 5-1/2 POUNDS FINCH OF WIDTH:

 SEED SHALL BE SOWN AT THE ARTS SHOWN BELOW. SOWING SHALL BE DONE ON A CALM, DRY DAY, PREFERABLY BY MACHINE, BUT IF BY HAND, ONLY BY EXPERIENCED WORKMEN, IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT O'VER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT O'VER 109 POUNDS FER HAND HALF SHALL BE APPLIED IMPROJATELY SHATE SEEDING AS INDICATED ABOVE; THE SURFACE SHALL BE WATERD AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT WASHING AWAY THE SOIL, WITH THE GRASS IS WELL ESTABLISHED, ANY AREAS WHICH ARE NOT SATISFACTORILLY COVERED WITH GRASS SHALL BE RESEDED, AND ALL NOXIOUS WEEDS REMOVED:
- NOXIOUS WEEDS REMOVED;
 THE CONTRACTOR SHALL PROTECT AND MAINTAIN THE SEEDED AREAS UNTIL ACCEPTED:
- h. A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE APPLIED AT THE INDICATED RATE:

SEED MIX APPLICATION RATE
CREEPING RED FESCUE 20 LBS/ACRE

TALL FESCUE 20 LBS/ACRE

REDTOP 2 LBS/ACRE
IN NO CASE SHALL THE WEED CONTENT EXCEED ONE (1) PERCENT BY WEIGHT, ALL SEED
SHALL COMPLY WITH STATE AND FEDERAL SEED LAWS, SEEDING SHALL BE DONE NO LATER
THAN SEPTIMERE ITS, IN NO CASE SHALL SEEDING TAKE PLACE OVER SHOW.

ORMANT SEEDING (SEPTIMERE ITS TO FIRST SNOWFALL):
FOLLOW PERSMAKENT HEASURES SLOVE, LIME, FERTILIZER AND GRADING REQUIREMENTS.

APPLY SEED MIXTURE AT TWICE THE INDICATED RATE, APPLY MULCH AS INDICATED FOR
PERSMAKENT HEASURES.

- CONCRETE WASHOUT AREA:

 1. THE PRODUCTION OF THE PROPERTY OF T

ALLOWABLE NON-STORMWATER DISCHARGES:

- FIRE HYDRANT FLUSHING; WATERS USED TO WASH VEHICLES WHERE DETERGENTS ARE NOT USED;
- WATER USED TO CONTROL DUST; POTABLE WATER INCLUDING UNCONTAMINATED WATER LINE FLUSHING; POTABLE WATER INCLUDING UNCONTAMINATED WATER LINE FLUSHING; ROUTINE EXTERNAL BUILDING WASH DOWN WHERE DETERGENTS ARE NOT USED; PAVEMENT WASH WATERS WHERE DETERGENTS ARE NOT USED; UNCONTAMINATED AIR CONDITIONING/COMPRESSOR CONDENSATION; UNCONTAMINATED AIR CONDITIONING/COMPRESSOR CONDENSATION; UNCONTAMINATED AIR CONTINUE DATA WHICH ARE UNCONTAMINATED; UNCONTAMINATED EXCUATION DEWATERING; LUNGSCAPE RINGATION.

- A ALL WASTE MATERIALS SHALL BE COLLECTED AND STORED IN SECURELY LIDDED RECEPTACLES, ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE SHALL BE DEPOSITED
- IN A DUMPSTER;
 NO CONSTRUCTION WASTE MATERIALS SHALL BE BURIED ON SITE;
 ALL PERSONNEL SHALL BE INSTRUCTED REGARDING THE CORRECT PROCEDURE FOR WASTE DISPOSAL BY THE SUPERINTENDENT.
- ALL SANITARY WASTE SHALL BE COLLECTED FROM THE PORTABLE UNITS A MINIMUM OF ONCE PER WEEK BY A LICENSED SANITARY WASTE MANAGEMENT CONTRACTOR.

SPILL PREVENTION:

- CONTRACTOR SHALL BE FAMILIAR WITH SPILL PREVENTION MEASURES REQUIRED BY LOCAL,
 STATE AND FEDERAL AGENCIES. AT A MINIMUM, CONTRACTOR SHALL FOLLOW THE BEST
 MANAGEMENT SPILL PREVENTION PRACTICES OUTLINED BELOW.
 THE FOLLOWING ARE THE MATERIAL MANAGEMENT PRACTICES THAT SHALL BE USED TO REDUCE
- THE RISK OF SPILLS OR OTHER ACCIDENTAL EXPOSURE OF MATERIALS AND SUBSTANCES
- DURING CONSTRUCTION TO STORMWATER RUNOFF:
 A. GOOD HOUSEKEEPING THE FOLLOWING GOOD HOUSEKEEPING PRACTICE SHALL BE
- FOLLOWED ON SITE DURING CONSTRUCTION:
 ONLY SUFFICIENT AMOUNTS OF PRODUCTS TO DO THE JOB SHALL BE STORED ON SITE; ALL MATERIALS STORED ON SITE SHALL BE STORED IN A NEAT, ORDERLY MANNER IN THEIR PROPER (ORIGINAL IF POSSIBLE) CONTAINERS AND, IF POSSIBLE, UNDER A ROOF OR OTHER ENCLOSURE;
 MANUFACTURER'S RECOMMENDATIONS FOR PROPER USE AND DISPOSAL SHALL BE
- FOLLOWED; THE SITE SUPERINTENDENT SHALL INSPECT DAILY TO ENSURE PROPER USE AND DISPOSAL
- OF MATERIALS; SUBSTANCES SHALL NOT BE MIXED WITH ONE ANOTHER UNLESS RECOMMENDED BY THE f WHENEVER POSSIBLE ALL OF A PRODUCT SHALL BE USED UP REFORE DISPOSING OF THE
- CONTAINER. B. HAZARDOUS PRODUCTS - THE FOLLOWING PRACTICES SHALL BE USED TO REDUCE THE RISKS
- ASSOCIATED WITH HAZARDOUS MATERIALS:
 PRODUCTS SHALL BE KEPT IN THEIR ORIGINAL CONTAINERS UNLESS THEY ARE NOT
- RESEALABLE;

 h. ORIGINAL LABELS AND MATERIAL SAFETY DATA SHALL BE RETAINED FOR IMPORTANT
- D. DICLINEL DEELS AND PAIR ENLIS SHE'LD VALUE TO SHALL BE DEFINED FOR THE PROPORTION TO SHALL BE DISCARDED ACCORDING TO THE MANUFACTURER'S RECOMMENDED METHODS OF DISPOSAL. C. PRODUCT SPECIFIC PRACTICES THE POLLOWING PRODUCT SPECIFIC PRACTICES SHALL BE FOLLOWED ON SITE:

 9. PETROLEUM PRODUCTS:
- a.2.
- TROLEUM PRODUCTS:
 ALL ON SITE VEHICLES SHALL BE MONITORED FOR LEAKS AND RECEIVE REGULAR
 PREVENTIVE MAINTENANCE TO REDUCE LEAKAGE;
 PETROLEUM PRODUCTS SHALL BE STORED IN TIGHTLY SEALED CONTAINERS WHICH
 ARE CLEARLY LABELED. ANY ASPHALT BASED SUBSTANCES USED ON SITE SHALL BE
 APPLIED ACCORDING TO THE MAUNTACTURERS RECOMMENDATIONS. b.1. FERTILIZERS USED SHALL BE APPLIED ONLY IN THE MINIMUM AMOUNTS DIRECTED BY
- THE SPECIFICATIONS;
 b.2. ONCE APPLIED FERTILIZER SHALL BE WORKED INTO THE SOIL TO LIMIT EXPOSURE TO
- STORWMATER;
 STORAGE SHALL BE IN A COVERED SHED OR ENCLOSED TRAILERS. THE CONTENTS OF ANY PARTIALLY USED BAGS OF FERTILIZER SHALL BE TRANSFERRED TO A SEALABLE PLASTIC BIN TO AVOID SPILLS.
- c.1. ALL CONTAINERS SHALL BE TIGHTLY SEALED AND STORED WHEN NOT REQUIRED FOR USE:
- EXCESS PAINT SHALL NOT BE DISCHARGED TO THE STORM SEWER SYSTEM
- C.3. EXCESS PAINT SHALL BE DISPOSED OF PROPERLY ACCORDING TO MANUFACTURER'S INSTRUCTIONS OR STATE AND LOCAL REGULATIONS.

 D. SPILL CONTROL PRACTICES IN ADDITION TO GOOD HOUSEKEEPING AND MATERIAL
- SPILL CONTROL PRACTICES IN ADDITION TO GOOD HOUSEKEEPING AND MATERIAL MANAGEMENT PRACTICES DISCISSED IN THE PREVIOUS SECTION, THE FOLLOWING MANAGEMENT PRACTICES DISCISSED IN THE PROVIDED SECTION, THE FOLLOWING ANALYSECTION OF THE PROSPECTION OF THE PROPERTY SHALL BE CLEARLY PROSPED AND STEE PRESONNES SHALL BE MADE PRAVILED FOR THE PROPERTY OF THE PROPERTY OF
- ALL SPILLS SHALL BE CLEANED UP IMMEDIATELY AFTER DISCOVERY: THE SPILL AREA SHALL BE KEPT WELL VENTILATED AND PERSONNEL SHALL WEAR APPROPRIATE PROTECTIVE CLOTHING TO PREVENT INJURY FROM CONTACT WITH A
- HAZARDOUS SUBSTANCE; SPILLS OF TOXIC OR HAZARDOUS MATERIAL SHALL BE REPORTED TO THE APPROPRIATE
- 6. SPILLS OF TOXIC OR HAZARDOUS MATERIAL SHALL BE REPORTED TO THE APPROPRIATE LOCAL, STATE OR FEDERAL AGENCIES AS REQUIRED;
 7. THE SITE SUPERINTENDENT RESPONSIBLE FOR DAY-TO-9 STIFE OPERATIONS SHALL BE THE SPILL PREVENTION AND CLEANING COMBINATION.
 8. CONTRACTOR SHALL NAME AN EFFORT TO PERFORM EQUIPTMENT/VEHICAL FUELING AND MAINTENANCE AT AN OFF-STIFE FACILITY;
 8. CONTRACTOR SHALL REPOYDE AN ON-SITE FUELING AND MAINTENANCE AREA THAT IS CLEAN AND DRY;
 9. CLEAN AND DRY;
 9. CLEAN AND DRY;
 9. CLEAN AND SHALL KEEP ASHLL KEEP AREA COVERED;
 9. CONTRACTOR SHALL RESULARLY INSPECT VEHICLES FOR LEANS AND DAYAGE;
 9. CONTRACTOR SHALL RESULARLY INSPECT VEHICLES FOR LEANS AND DAYAGE;
 9. CONTRACTOR SHALL RESULARLY INSPECT VEHICLES FOR LEANS AND DAYAGE;
 9. EVENTACTOR SHALL RESULARLY INSPECT VEHICLES FOR LEANS AND DAYAGE;
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 9. EVENTACTOR SHALL RESULARLY INSPECT VEHICLES FOR LEANS AND DAYAGE;
 9. EVENTACTOR SHALL RESULARLY INSPECT.

EROSION CONTROL OBSERVATIONS AND MAINTENANCE PRACTICES

REPLACING SPENT FLUID.

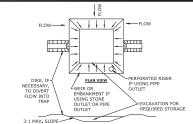
- THIS MODECT EXCESS ONE (1) AGRE OF DISTURBANCE AND THUS REQUIRES A SWPPP. THE SWPPP SHALB BE PREMARED SY THE CONTRACTOR. THE CONTRACTOR SHALL BE FAMILIAR WITH THE SWPPP AND KEEP AN UPDATED COPY OF THE SWPPP ONSTE AT ALL TIMES.

 SHALL BE FOLLOWED AS PART OF THIS PROJECT:

 A. DISSENSATIONS OF THE PROJECT FOR COMPLIANCE WITH THE SWPPP SHALL BE MADE BY THE CONTRACTOR AT LEAST DUCK OF WEEK ON WITHOUT AT HORS OF A STORM AZE SINCHES OR
- B. AN OBSERVATION REPORT SHALL BE MADE AFTER EACH OBSERVATION AND DISTRIBUTED TO THE ENGINEER, THE OWNER, AND THE CONTRACTOR;

 C. A REPRESENTATIVE OF THE SITE CONTRACTOR, SHALL BE RESPONSIBLE FOR MAINTENANCE
- D. IF A REPAIR IS NECESSARY, IT SHALL BE INITIATED WITHIN 24 HOURS OF REPORT.
- VISTING FULL
 VISTING PRIVE VIDTH
 (10" MIN) 1. THE ENTRANCE SHALL BE EXISTING PAVEMENT THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OF SEDIMENT FROM THE SITE. WHEN WASHING IS REQUIRED, IT SHALL BE DONE SO RUNOFF DRAINS INTO AN APPROVED PLAN VIEW DIVERSION BERM (OPTIONAL) 'S' (MIN) (W/O BERM) 50' (MIN) WITH 3"-6" SEDIMENT TRAPPING DEVICE. ALL SEDIMENT SHALL BE PREVENTED FR DITCHES, OR WATERWAYS MIRAFI FW-700 SIDE VIEW

STABILIZED CONSTRUCTION ENTRANCE



NOTES:

1. THE TRAP SHALL BE INSTALLED AS CLOSE TO THE DISTURBED AREA AS POSSIBLE

STATE OF THE TRAP SHALL BE INSTALLED AS CLOSE TO THE DISTURBED AREA AS POSSIBLE

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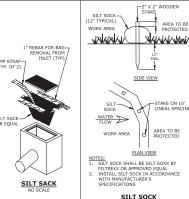
THE TRAP SHALL BE INSTALLED AS CLOSE TO THE DISTURBED AS CLOSE TO THE DIS

ACRES.
THE MINIMUM VOLUME OF THE TRAP SHALL BE 3,600 CUBIC FEET OF STORAGE

SIDE SLOPES TO BE STABILIZED

- THE PRINTING VOLUME OF THE TANS STRUCKE SYMMETERS OF BELOW THE CREST OF THE TRAP. TRAP OUTLIET SHALL BE MINIMUM OF ONE FOOT BELOW THE CREST OF THE TRAP. TRAP SHALL DISCHARGE TO A STABILIZED AREA. TRAP SHALL BE CLEANED WHEN 50 PERCENT OF THE ORIGINAL VOLUME IS FILLED.
- MATERIALS REMOVED FROM THE TRAP SHALL BE PROPERLY DISPOSED OF AND SEDIMENT TRAPS MUST BE USED AS NEEDED TO CONTAIN RUNGER UNTIL SOILS ARE STABILIZED.

SEDIMENT TRAP

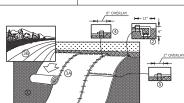




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Proposed Mixed Use Development

CPI Management, II C



PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING ANY NECESSARY

- DREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING ANY NECESSARY APPLICATION OF UME, FERTILIZER AND SEED.
 BEGIN AT THE TOP OF THE SLOPE, 36° OVER THE GRADE BREAK, BY ANCHORING HEB BLANKET IN A 6° DEEP X° TWIDE TRENCH WITH APPROXIMATELY 12° OF BLANKET EXTENDED BEYOND THE URSLOPE PORTION OF THE TRENCH. ANCHOR THE BLANKET WITH A ROW OF TRALES/STAKES 12° APART IN THE BOTTOM OF STATE AND THE BLANKET WITH A ROW OF THE SLOPE AND THE STATE ST

EROSION CONTROL BLANKET

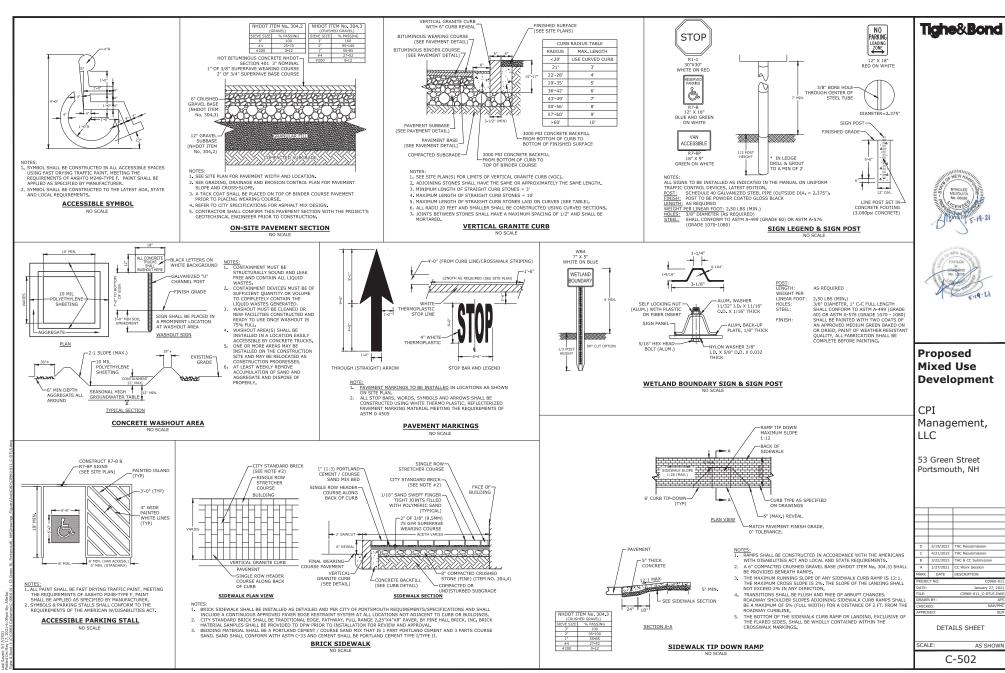


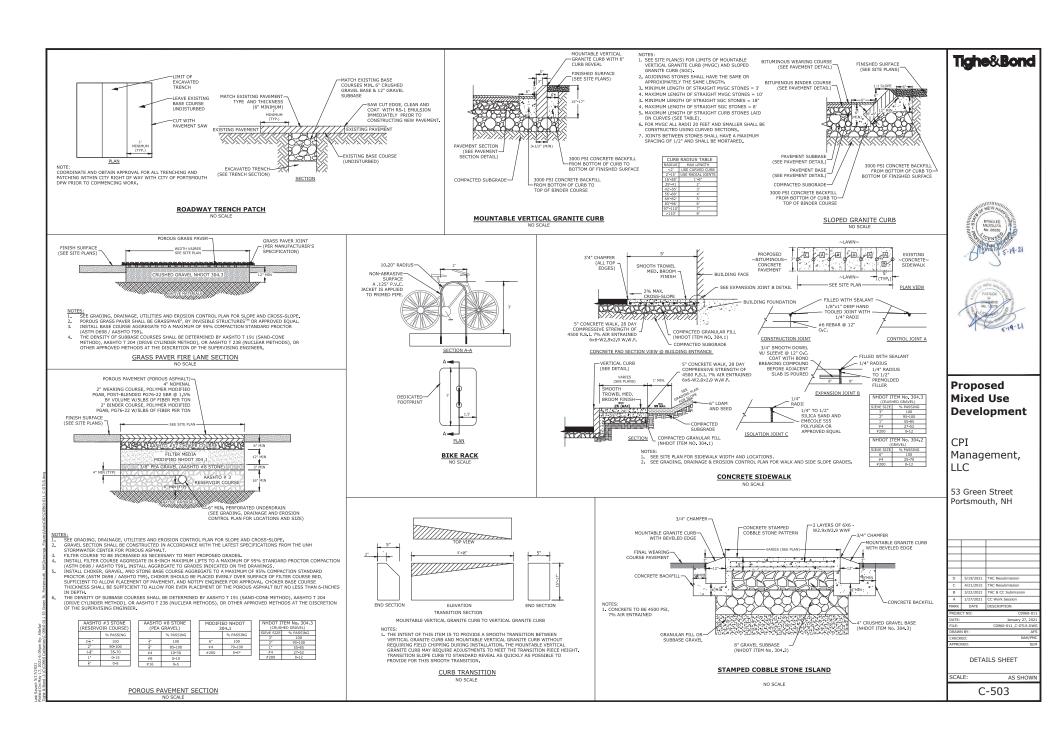
53 Green Street Portsmouth, NH

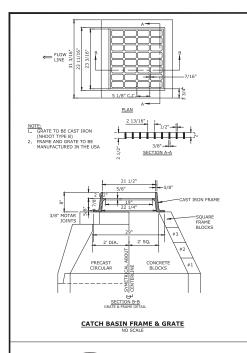
4/21/2021 TAC Resu 3/22/2021 TAC & CC Sub

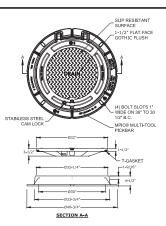
FROSION CONTROL NOTES

AS SHOWN



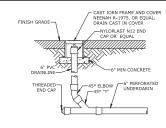




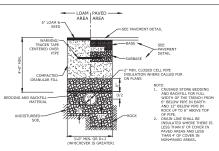


- MANHOLE FRAME AND COVER SHALL BE 32" HINGED ERGO XL
- MANHOLE FRAME AND COVER SHALL BE 32" HINGED ERGO XL PE OF CONTROL ARE NORMAL.
 FRAMES USING MARROWER DIMENSIONS FOR THICKNESS ARE ALLOWED PROVIDED:
 A. THE FRAMES MEET OR EXCRED THE SECLIFEL LOAD PATING. BY THE PROVIDED HE FRAMES FAMELY OF THE PROVIDED HE OF EXISTING GRATES/COVERS AS THE EXISTING FRAMES ALLOW, WITHOUT SHENS OR OTHER MODIFICATIONS OR ALLOW, WITHOUT SHENS OR TOTHER MODIFICATIONS OR CALLOW, WITHOUT SHENS OR TOTHER MODIFICATIONS OR CALLOW, WITHOUT SHENS OR TOTHER MODIFICATIONS OR SHEN SHENDED HE OF THE PROVIDED HE OF THE PROVIDE
- SPECIFICATIONS ARE MET.

 LABEL TYPE OF MANHOLE WITH 3" HIGH LETTERS IN HE CENTER OF THE COVER. DRAIN MANHOLE FRAME & COVER
 NO SCALE











- NOTES:

 1 ALL CATCH BASIN OUTLETS TO HAVE
 "ELIMINATOR" OIL AND FLOATING DEBRIS
 TRAP MANUFACTURED BY KLEANSTREAM (NO
- EQUAL)

 INSTALL DEBRIS TRAP TIGHT TO INSIDE OF STRUCTURE.

 1. 1/4" HOLE SHALL BE DRILLED IN TOP OF DEBRIS TRAP

"ELIMINATOR" OIL FLOATING DEBRIS TRAP

Feet Meters 787 **Mounting Styles and Configurations** Downstream Clamp



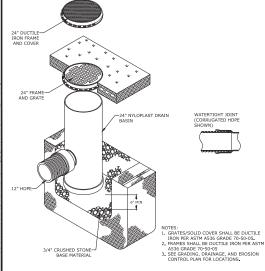


TYPICAL BACK FLOW PREVENTER

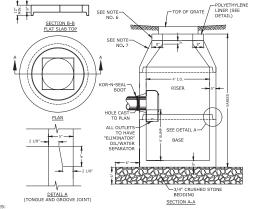


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YARD DRAIN



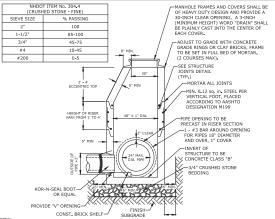
- (UNROUSE AND ORCOVERING)

 SECTION S-AA

 SECTION S-AAL

 SECTION S-A

4' DIAMETER CATCHBASIN



- NOTES: SECTIONS SHALL BE 4,000 PSI CONCRETE.
 2. CIRCLIMPRETRIAL REINFORCEMENT SHALL BE 0,12 SQUARE INCHES PER LINRAR FOOT IN ALL SECTIONS AND SHALL BE PLACED.
 IN THE CENTER THIRD OF THE WALL.
 3. THE TONGUE AND THE GROUP OF THE JOINT SHALL CONTAIN ONE LINE OF CIRCLIMPRENTIAL REINFORCEMENT EQUAL TO 0,12.

4' DIAMETER DRAIN MANHOLE

Proposed Mixed Use Development

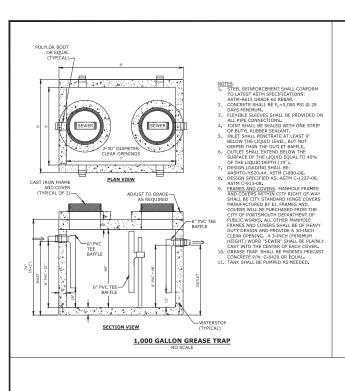
Management, LLC

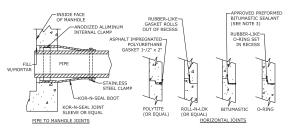
53 Green Street Portsmouth, NH

D	5/19/2021	TAC Resubmission
С	4/21/2021	TAC Resubmission
В	3/22/2021	TAC & CC Submission
А	1/27/2021	CC Work Session
MARK	DATE	DESCRIPTION
ROJE	T NO:	C0960-011
DATE:		January 27, 2021

DETAILS SHEET

AS SHOWN





- NOTES:

 1. HORIZONTAL JOINTS BETWEEN THE SECTIONS OF PRECAST CONCRETE BARRELS SHALL BE PER CITY OF PORTSMOUTH DPW STANDARD AND SHALL BE SEALED FOR WATERTIGHTNESS USING A ODDUBE ROW PLASTOPHIC OR MASTICL. HE GASKET.

 2. FOR BITUMASTIC TYPE JOINTS THE AMOUNT OF SEALANT SHALL BE SUFFICIENT TO FILL AT LEAST 75% OF THE JOINT CAUTY.

 4. ALL GASKETS, SEALANTS, MORTAR, ETC. SHALL BE INSTALLED IN ACCORDANCE WITH MADIURACTURES'S WILTEN IN PRINCIPIONS.

MANHOLE JOINTS

PAVEMENT DETAIL TRACER TAPE CENTERED OVER PIPE =2-2" MIN, CLOSED CELL PIPE INSULATION WHERE CALLED FOR ON COMPACTED-GRANULAR FILL PLANS UNDISTURBED

NOTE:

1. CRUSHED STOME BEDDING FOR FULL WIDTH OF THE TRENCH
FOR 6' BELOW PIPE IN EARTH AND 12' BELOW PIPE IN
ROCK, CRUSHED STOME SHALL ASC COMPLETELY INCASE
THE PIPE AND COVER THE PIPE TO A GRADE 6' OVER THE
PIPE AND COVER THE PIPE TO A GRADE 6' OVER THE
CORRONATE ALL INSTALLATIONS WITH THE CITY OF
PORTSMOUTH.

SEWER SERVICE TRENCH

ADJUST TO GRADE WITH NOT MORE THAN 12" OF BRICK MASONRY, FRAME TO BE SET IN FULL BED OF MORTAR.

-30" CLEAR OPENING INCLUDING FRAME AND COVER

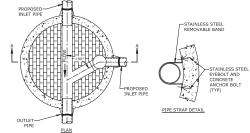


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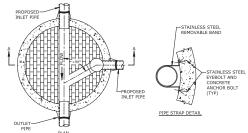


PVC BELL (REMOVE-TO CLEAN HORIZONTAL LINE -INLET PIPE DVC BELL -RUBBER BOOT SECTION GASKET REMOVED (SEE MANHOLE DETAIL) STAINLESS STEEL STAINLESS STEEL EYEBOLT AND CONCRETE ANCHOR BOLT (TYP) MODIFIED PVC ELBOW (WITH BELL REMOVED) -3/4" CRUSHED OUTLET PIPE-

SECTION A-A



INSIDE DROP MANHOLE NO SCALE



NOTES:

I. RISER PIPE AND FITTINGS SHALL BE THE SAME DIAMETER AS THE INLET PIPE AND SHALL BE CONSTRUCTED OF SDR35 PVC PIPE.

SANITARY SEWER SHALL BE INSTALLED PER THE CITY OF PORTSMOUTH DEPARTMENT OF FUBILE WORKS STANDARDS.

COORDINATE ALL INSTALLATIONS WITH THE CITY OF PORTSMOUTH.

INVERT

PLAN

BRICK MASONRY

SECTION A-A

TOP OF SHELF SHALL BE 1" ABOVE CROWN OF HIGHEST PIPE

NOTES:

1. TYPICAL SECTION

1. TO SHEET AND SHELE TO BE PLACED AFTER EACH LEAKAGE TEST.

1. TOWES SHALL BE LAVE TO TO THE UP THE BRICK INVERT IS A SMOOTH CONTINUATION OF THE SEWER INVERT.

2. INVERT BRICKS SHALL BE LAD ON EDGG.

4. TWO (2) COATS OF BITUMINOUS WATERPROOF COATING SHALL BE APPLIED TO ENTIRE EXTERIOR OF MANHOLE.

5. FRAMES AND COVERS: MANHOLE FRAMES AND COVERS WITHIN CITY RIGHT OF MAY SHALL BE CITY TO FRAMED HINGE COVERS MANUFACTURED BY EJ, FRAMES AND COVERS WILL BE PURCHASED FROM THE CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS, ALL OTHER MANHOLE FRAMES AND COVERS SHALL BE OF HEAVY DUTY DESIGN AND PROVIDE A 30-INCH CLEAR OPENING. A 3-TOKIC (MINIMAN HERE'S WALL BE ADDRESS WAS ALL OTHER MANHOLE FRAMES AND COVERS SHALL BE OF HEAVY DUTY DESIGN AND PROVIDE A 30-INCH CLEAR OPENING. A 3-TOKIC (MINIMAN HERE'S WALL DE ADDRESS WAS ALL OTHER MANHOLE FRAMES AND COVERS SHALL BE OF HEAVY DUTY DESIGN AND PROVIDE A 30-INCH CLEAR OPENING. A 3-TOKIC (MINIMAN HERE'S WALL DE ADDRESS WAS ALL OTHER MANHOLE FRAMES AND COVERS SHALL BE OF HEAVY DUTY DESIGN AND PROVIDE A 30-INCH CLEAR OPENING. A 3-TOKIC (MINIMAN HERE'S WALL DE TOKICH WAS TOKICLED SEALANT.

7. BARREL AND COME SECTIONS SHALL BE SPECAST REINFORCED CONCRETE DESIGNED FOR ADLOCATION, AND CONFORMING TO ASTRI C478-06.

SEWER MANHOLE

SUBGRADE

SEWER MANHOLE NO SCALE

Proposed Mixed Use Development

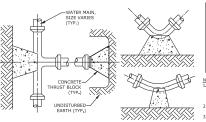
CPI Management, LLC

53 Green Street Portsmouth, NH

D	5/19/2021	TAC Resubmission
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ARK	DATE	DESCRIPTION
ROJE	T NO:	C0960-01
ATE:		January 27, 202

DETAILS SHEET

AS SHOWN



THRUST BLOCKING DETAIL

SEE PAVEMENT DETAIL

STANDARDS. COORDINATE ALL

INSTALLATIONS WITH UNITIL AND THE CITY OF PORTSMOUTH.

LOAM | PAVED ---

GAS TRENCH

TRACER TARE

COMPACTED

BEDDING AND BACKFILL MATERIAL

UNDISTURBED-

GRANULAR

FILL

CENTERED OVER PIPE

200psi	SQUARE FEET			UST BLOC MATERIAI		RING ON
	REACTION			PIPE SIZE		
ш	TYPE	4"	6"	8"	10"	12"
PRESSURE	A 90°	0.89	2.19	3.82	11.14	17.24
SES	B 180°	0.65	1.55	2.78	8.38	12.00
	C 45°	0.48	1.19	2.12	6.02	9.32
TEST	D 22-1/2°	0.25	0.60	1.06	3.08	4.74
ľ	E 11-1/4°	0.13	0.30	0.54	1.54	2.38

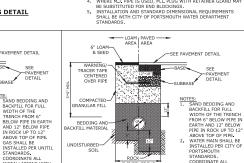
- NOTES:

 1. POLE THRUST BLOCKS AGAINST UNDISTURBED MATERIAL.

 1. POLE THRUST BLOCKS AGAINST UNDISTURBED, EXCAVATE LOOSE
 MATERIAL AND EXTEND THRUST BLOCK TO UNDISTURBED
 MATERIAL. NO JOINTS SHALL BE COVERED WITH CONCRETE.

 2. ON BERONS AND TEES, CETTED THRUST BLOCKS TO UNDISTURBED
 MATERIAL PLANT SHALL BE COVERED WITH CONCRETE.

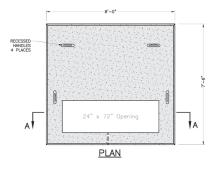
 3. PLACE BOARD IN FRONT OF ALL PLUGS BEFORE POURING THRUST
 PLACES.
- BLOCKS.
 WHERE M.J. PIPE IS USED, M.J. PLUG WITH RETAINER GLAND MAY

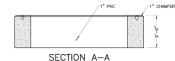


WATER TRENCH

INSTALLATIONS WITH THE CITY OF

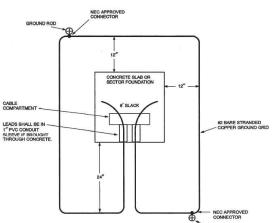
PORTSMOUTH.





NOTES:
1. DIMENSIONS SHOWN REPRESENT TYPICAL REQUIREMENT:
COORDINATED WITH EVERSQUECE PRIOR TO CONSTRUCT
2. CONCRETE MINIMUM STRENGTH - 4,000 PSI @ 28 DAYS
3. STEEL REINFORCEMENT - ASTM A615, GRADE 60
4. PAD MESTS OR EXCEEDS EVERSQUIRCE SPECIFICATIONS INTS, MANHOLE LOCATIONS AND REQUIREMENTS SHALL BE

3-PHASE TRANSFORMER PAD



THE GROUND GRID SHALL BE SUPPLIED AND INSTALLED BY THE CONTRACTOR AND IS TO BE BURIED AT LEAST 12 INCHES BELOW GRADE. EIGHT FEET OF EXTRA WINE FOR REATH GROUND GRID LEG SHALL BE LIEFT EXPOSED IN THE CABLE COMPARTMENT TO A LOW FOR THE CONNECTION TO THE TRANSPORMER. THE TWO S-FOOT GROUND RODS MAY BE EITHER GALVANIZED STEEL OR COPPERWELD AND THEY SHALL BE CONNECTED TO THE GRID WITH NEC APPROVED CONNECTIONS.

PAD-MOUNTED EQUIPMENT GROUNDING GRID DETAIL





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CPI Management, LLC

53 Green Street Portsmouth, NH

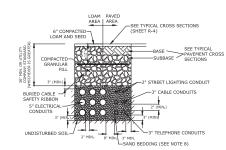
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	1/27/2021	CC Work Session
K	DATE	DESCRIPTION
JE(T NO:	C0960-011
E:		January 27, 2021

DETAILS SHEET

AS SHOWN

C-506

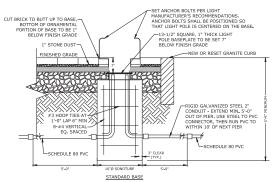


- NOTES:

 1. NUMBER, MATERIAL, AND SIZE OF UTILITY CONDUITS TO BE DETERMINED BY LOCAL UTILITY OR AS SHOWN ON ELECTRICAL DRAWINGS. CONTRACTOR TO PROVIDE ONE SPARE CONDUIT FOR EACH 2. DIMENSIONS SHOWN REPRESENT OWNERS MINIMUM REQUIREMENTS. ACTUAL DIMENSIONS NAY BE GRAATE BASED ON UTILITY COMPANY STANDARDS, BUT SHALL NOT BE LESS THAN THOSE SHOWN, 3. NO CONDUIT RIM SHALL EXCEED 360 DEGREES IN TOTAL BENDS.

 3. NO CONDUIT RIM SHALL EXCEED 360 DEGREES IN TOTAL BENDS. THE RIM THE CONDUIT RIM SHALL BY THE CONDUIT OF THE CONDUIT AFTER THE RIM IS ASSEMBLED TO AVOID BONDING THE STRING SHOULD BE BLOWN INTO THE CONDUIT AFTER THE RIM IS ASSEMBLED TO AVOID BONDING THE STRING TO THE CONDUIT AFTER THE RIM IS ASSEMBLED TO AVOID BONDING THE STRING TO THE CONDUIT INTO THE CONDUIT AFTER THE RIM IS ASSEMBLED TO AVOID BONDING THE STRING TO THE CONDUIT INTO THE AVOID AND THE CONDUIT INTO THE CONDUIT INTO
- ALL 90° SWEEPS WILL BE MADE USING RIGID GALVANIZED STEEL. SWEEPS WITH A 36 TO 48 INCH
- RADIUS.
 SAND BEDDING TO BE REPLACED WITH CONCRETE ENCASEMENT WHERE COVER IS LESS THAN 3 FEET, WHEN LOCATED BELOW PAVEMENT, OR WHERE SHOWN ON THE UTILITIES PLAN.

ELECTRICAL AND COMMUNICATION CONDUIT



NOTES:

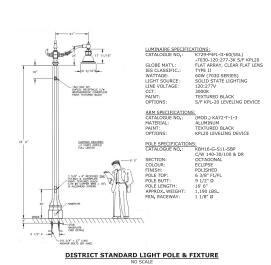
1. REFER TO ELECTRICAL PLANS FOR WIRING DETAILS.

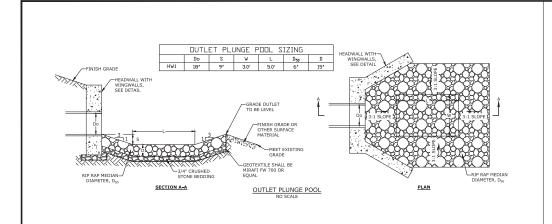
2. CONCRETE: 4000 PSI, AIR ENTRAINED STEEL: 60 KSI

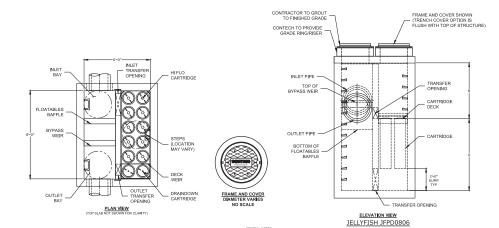
3. LIGHT POLE FOUNDATIONS SHALL BE PLACED PRIOR TO INSTALLATION OF BRICK PAVERS.

LIGHT NUMBER OF THE PROPERTY O

NORTH END LIGHT FIXTURE BASE







JELLYFISH FILTER DETAIL (JF-1)

JELLYFI	SH JFPD0806	- DESIGN NOT	ES	
JELLYFISH TREATMENT CAPACITY IS A FUNCTION OF TH STYLE WITH PRECAST TOP SLAB IS SHOWN, ALTERNATI CAPACITY TO BE DETERMINED BY ENGINEER OF RECOF CARTINDER SELECTION.	OFFLINE VAULT AND/O			
CARTRIDGE LENGTH	54"	40"	27"	15"
OUTLET INVERT TO STRUCTURE INVERT (A)	6'-6"	5'-4"	4'-3"	3'-3"
	6'-6" 0.178 / 0.089	5'-4" 0.133 / 0.067	4'-3" 0.089 / 0.045	3'-3"
OUTLET INVERT TO STRUCTURE INVERT (A)				

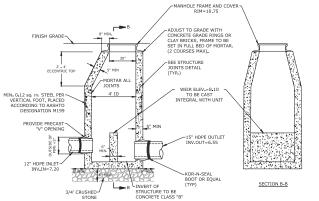
RETUGENS NOTES COVER COPTH, AMOOR ANTI-LOTATION PROVIDED AN ESTECHTICITIC CRICKI CONDIDERALIZATION CONTROLL.

A. WO RELIABLE ESCAPEL CONTROLL CONTROLL CONTROLL CONTROLL CRICKING CONTROLL CONTROL CONTROLL CONTROL CONTROLL CONTROL CONTRO

. WAY CORDIDES.COT COURLITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONFEM STRUCTURE MIET'S REQUIREMENTS OF PROJECT.

Jellyfish Filter





- NOTES:
 1. ALL SECTIONS SHALL BE 4,000 PSI CONCRETE.
 2. CIRCUMFERENTIAL REINFORCEMENT SHALL BE 0.12 SQUARE INCHES PER LINEAR FOOT IN ALL SECTIONS AND SHALL BE PLACED IN THE CENTRES THIRD OF THE WALL.
 3. THE TONGUE AND THE GROOVE OF THE JOINT SHALL CONTAIN ONE LINEO CIRCUMFERENTIAL REINFORCEMENT EQUAL TO 1.2 SQUARE INCHES PER LINEAR
- FOOT.

 THE STRUCTURES SHALL BE DESIGNED FOR H20 LOADING.
 CONSTRUCT CRUSHED STONE BEDDING AND BACKFILL UNDER (6" MINIMUM

- THE RIGHT ONES SHALL BE DESIGNED FOR HOLD AND THE RESTRICT ONES (** MINIMUM THICKNESS USSHED STORE BEDDING AND BACKFILL ONDER (** MINIMUM THICKNESS)
 THE TONGUE AND GROOVE JOINT SHALL BE SEALED WITH ONE STRIP OF BUTYL RUBBER SEALANT.

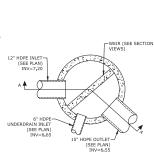
 PRESENTIAL OF SHOWN ON PARAS SHALL BE FIELD VERIFIED PRIOR TO PRESENT OF SHALL HAVE A TONGUE AND GROOVE JOINT A" HIGH AT AN 11 PARECAS TROODS SHALL HAVE A TONGUE AND GROOVE JOINT A" HIGH AT AN 11 PARECAS TROODS SHALL HAVE A TONGUE AND GROOVE JOINT A" HIGH AT AN 11 PARECAS TROODS SHALL BHAVE AND SHALL BE ASSEMBLED USING AN APPROVED FLEXIBLE SEALANT IN JOINTS.

 ALL STRUCTURES WITH HULTITE PIPES SHALL HAVE A MINIMUM OF 12" OF INSIDE SURFACE BETWEEN HOLES, MO MORE THAN 75% OF A HORIZITAL CROSS SECTION SHALL BE HOLD. AND THERE SHALL BE NO HOLES CLOSES THAN 3 TO JOINTS.

 IN HULD SHALL BE HOLD. AND THERE SHALL BE NO HOLES CLOSES THAN 3 TO JOINTS.

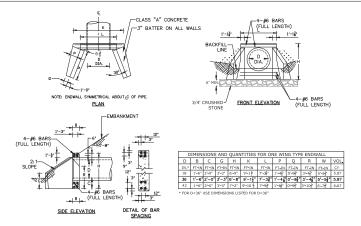
 MINIMUM FLOOR AND WALL THICKNESS, ETC.)

 MINIMUM FLOOR AND WALL THICKNESS, ETC.)



PLAN VIEW

OUTLET STRUCTURE DETAIL (POS1)



HEADWALL WITH WINGWALLS

Tighe&Bond





Proposed Mixed Use Development

Management, LLC

53 Green Street Portsmouth, NH

)	5/19/2021	TAC Resubmission
:	4/21/2021	TAC Resubmission
3	3/22/2021	TAC & CC Submission
ì	1/27/2021	CC Work Session
RK	DATE	DESCRIPTION
OJE	T NO:	C0960-0

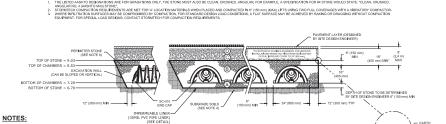
DETAILS SHEET

AS SHOWN C-507

ACCEPTABLE FILL MATERIALS: STORMTECH SC-310 CHAMBER SYSTEMS

	MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	PINAL PILL: FILL MATERIAL FOR LAYER TO STARTS FROM THE TOP OF THE C'LAYER TO THE BOTTOM OF FLEXBLE PAVEMENT OR LIMPAYED FINISHED GRADE ABOVE, NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE TO LAYER.	ANY SOLIROCK MATERIALS, NATIVE SOLIS, OR PER ENGINEER'S PLANS, CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REGUREMENTS.
С	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDDMENT STOKE TO LAYER) TO 10 (450 mm) ABOVE THE TO AND THE TOP OF THE TOP AND AND AND THE SUBBASE MAY SE A PART OF THE "C' LAYER.	GRANULAR WELL-GRADED SOLIAGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145¹ A-1, A-2-4, A-3 OR AASHTO M43¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGN COMPACTIONS AFTER 12" (500 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX. LETS TO A MIN. 500. PROCTOD BENSITY FOR DESIGNATION OF THE COMPACT ADDITIONAL STATEMENT OF THE COMPACT AND THE COMPACT AND THE COMPACT ADDITIONAL STATEMENT OF THE COMPACT AND TO EXCEED 12,000 lbs. (53 MI). DYNAMIC FORCE NOT TO EXCEED 12,000 lbs. (53 MI). DYNAMIC FORCE NOT TO EXCEED 12,000 lbs. (53 MI). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs. (69 MI).
В	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE (A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
Α	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43¹ 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. **

TO DESIGNATIONS ARE FOR GRADATIONS ONLY THE STONE MUST ALSO BE CLEAN CRUSHED ANGULAR FOR EXAMPLE A SPECIFICATION FOR MISTONE WOULD STATE: YO FAN CRUSHED



SC-310 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 STANDARDS SPECIFICATION FOR POLYPROPYLENE (PP) OF F2822
STANDARD SPECIFICATION FOR POLYETHYLENE (PE) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS. 2 SC-310 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMAN CHAMBERS"

4. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOLS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOLL MUSTURE CONDITIONS.

6. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.

2. CHAMBERS SHALL BE MANUFACTURED FROM VIRGIN POLYPROPYLENE OR POLYETHYLENE RESINS.^)

STORMTECH CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE STORMTECH SC-740, SC-310, OR APPROVED EQUAL

6. ONCE LAYER 'C' IS PLACED, NAY SO/LAMERIAL CAN BE PLACED IN LAYER OF UP TO THE FINISHED GRADE, MOST PAVEMENT SUBBASE SOLIS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR TO AT THE SITE DESIGN ENGINEER'S DISCRETION.

CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.

4. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE L FACTORS SPECIFIED IN THE ASSHTO LERD DRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS / SHORT-DURATION LIVE LOADS, BASED ON THE ASSHTO DESIGN RUCK WITH CHOSIDERATION FOR IMPACT AND MITE VEHICLE PRESENCES

5. CHAMBERS SHALL MEET ASTM F2922 (POLYETHYLENE) OR ASTM F2418 (POLYPROPYLENE), "STANDARD SPECIFICATION FOR THERMOPLASTIC

CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOADS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".

7.1. A STRUCTURAL EVALUATION SEALED BY A REGISTRAD PROFESSIONAL REGISER AND REGISERATOR THE REGISTRATE ACTIONS OF REPORTS AND REGISTRATE OF REGISTRAD PROFESSIONAL REGISERATOR REGISERATOR PROFESSIONAL REGISERATOR REGISERATOR PROFESSIONAL PROFESSIONAL REGISERATOR REGISERATOR PROFESSIONAL P

7.2. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTOLERD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE WRIT. THE 50 YEAR CREEP MODULES DATA SPECIFIED IN ASTM F2418 OR ASTM F2922 WRIST BE USED AS PARED FTH EASHTOS TRUCTURAL EVALUATION TO VERIFIC LOAD-STREAM PERFORMANCE.

ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE
FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE:

COVER ENTIRE ISOLATOR ROW WITH ADS GEOSYNTHETICS 601T NON-WOVEN GEOTEXTILE

SC-310 ISOLATOR ROW DETAIL

INSPECTION & MAINTENANCE

INSPECT ISOLATOR ROW FOR SEDIMENT

A INSPECTION FORTS IF PRESENT)
AS THE PROPERTY OF THE PRESENT OF THE PROPERTY OF THE PROPERTY

SOLATOR ROWS

SOLATOR

SOL

STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS

A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS

PREFERRED
APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
VACUUM STRUCTURE SUMP AS REQUIRED

REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM

NOTES

INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION, ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.

CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

1. STORATECH SC-310 & SC-740 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS. 1

2. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-780 CONSTRUCTION GUIDE", "1

3. CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. 1 STORMTECH RECOMMENDS 3 BACKFILL METHODS

STONESHOOTER LOCATED OFF THE CHAMBER BED.

IMPERMEABLE LINER DETAIL

BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.

BACKELL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR △1

4. THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.^J 5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE, ^]

MAINTAIN MINIMUM - 6" (150 mm) SPACING BETWEEN THE CHAMBER ROWS. ^J

7. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE 3/4-2" (20-50 mm), ^1

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-310/SC-740 SYSTEM

8. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.^J

9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

7.3 STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.

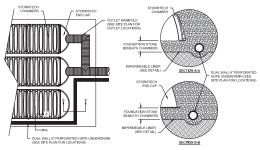
8. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY

NOTES FOR CONSTRUCTION EQUIPMENT

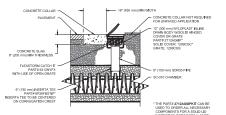
- 1. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE", "I
- 2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-310 & SC-740 CHAMBERS IS LIMITED:
- . NO RUBBER TIRED LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE*.
- WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE". ^]
- 3. FULL 36* (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-888-892-7694 WITH ANY DIJECTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT

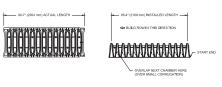


UNDERDRAIN DETAIL



SC-310 6" INSPECTION PORT DETAIL

SC-310 TECHNICAL SPECIFICATION











PART#	STUB	A	В	С
SC310EPE06T / SC310EPE06TPC	6° (150 mm)	9.6" (244 mm)	5.8" (147 mm)	_
SC310EPE06B / SC310EPE06BPC		900 (See mm)	_	0.5" (13 mm)
SC310EPE08T / SC310EPE08TPC	8" (200 mm)	11.9° (302 mm)	3.5° (89 mm)	-
SC310EPE08B / SC310EPE08BPC	0 (200 mm)		-	0.6" (15 mm)
SC310EPE10T / SC310EPE10TPC	10° (250 mm)	12,7" (323 mm)	1.4" (36 mm)	-
SC310EPE10B / SC310EPE10BPC	10 (2301111)	12.7 (02011111)		0.7" (18 mm)
SC310EPE12B	12° (300 mm)	13,5" (343 mm)		0,91(23 mm

THE STUB IS FLUSH WITH THE BOTTOM OF THE END CAP. FOR ADDITIONAL INFORMATION CONTACT STORMTECH AT

* FOR THE SC310EPE12B THE 12* (300 mm) STUB LIES BELOW THE BOTTOM OF THE END CAP APPROXIMATELY 0.25* (6 mm). BACKFILL MATERIAL SHOULD BE REMOVED FROM BELOW THE N-12 STUB SO THAT THE FITTING SITS LEVEL. NOTE: ALL DIMENSIONS ARE NOMINAL

Tighe&Bond





Proposed Mixed Use Development

CPI Management, LLC

53 Green Street Portsmouth, NH

D	5/19/2021	TAC Resubmission		
С	4/21/2021	TAC Resubmission		
В	3/22/2021	TAC & CC Submission		
А	1/27/2021	CC Work Session		
MARK	DATE	DESCRIPTION		
PROJECT NO: C0960				
DATE: January 27,				

DETAILS SHEET

AS SHOWN

Landscape Notes

- Landscape Notes

 1. Design is based on drawings by Tighe & Bond dated 5/19/2021 and may require adjustment due to actual field conditions.

 2. The control of the second control

- Contractor:

 3. The Contractor shall verify exact location and elevation of all utilities with the respective utility owners prior to construction. Call DIGSAFE at 1-680-544-723.

 3. DIG

- As also.

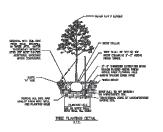
 Dip strap shall extend to 5° beyond roof ownering and shall be edged with 31°5° thick model edger.

 Dip strap shall extend to 5° beyond roof ownering and shall be edged with 31°5° thick model excellent and a shall not book be seen of a plant or or shall much ever be more than 5° bick total (including previously applied model) over the root ball of any plant.

 Secondary lateral branches of decidations trees overhapping vehicular and pedestrant travew ways shall be pruned up to a
- height of 5' to allow dear and safe passage of vehicles and pedestrians under tree canopy. Within the sight distance triangles at vehicle interactions the canopies shall be raised to 8' min.

 Snow shall be stored a minimum of 5' from shrubs and trunks of trees.

 Landscape Architect is not responsible for the means and methods of the contractor.

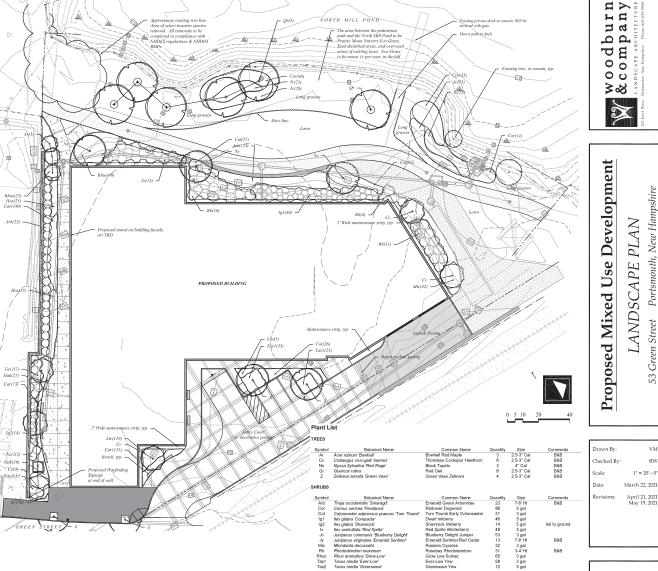




- P. S. ALL WITE AND DUMLAT DAVIS DE DESCRICO PRODUTOS DE DESCRICO DAL PRO-DI ACTUAL 1835. 2.3 THE ROOT EAST, OF THE THEE SHALL BE ROOMED 50 THAT THE ROOT DULLN'S OF THE THEE BY VEHICLE AND MIT SHIPLING HOUSE AND
- 2.4 THE BOOT COLLAR OF THE THEE SHALL BE 2'-3' MOVE STATE OF PLANTED HOME FOR PROBING COPYL
- 20) ALL PLANSAGE SHIEL SE ERSCHELLES Vann SELL HARD THE STEE AND MERCHES LES COME THEM SERVE WITH MERCHES CONFERT, THE DRAW EMBERGHES AND HELD CONFERENCIES WHERE REMOVEDED COME AS ENDER LESTO HE DEBURKETERS WITH DRAW STULM HARD SECTIO WITH THAT HARD HARD AND THAT CONFERTS.
- ALL PLANTINGS SHALL BE ENCYPLIED IN THREE LETS AND HISE OF AN SHALL BE THREEFER SO THE PLANTING INL. BE SET AND HISE OF AN STREET, THE STREET, AND THE OF AN
- 2.7 AN SIRBN REDN SHALL BE PLACES AROUND THE PERMITTER OF THE PLATEST HOLE EXCEPT CHIEFE DISERSE PLACESON SIZES OF PIES ARE CONSTRUCTED. 28 3"-3" OF MUCH SHALL BE PLACED OVER THE PLANTISC MED
- 2.9 AT THE THE OF PLANTING AS SCHOOL THE PLANTISK SHOW, STREET, ARTHOUGH, SCHOOL TO MINIST INSTITUTE WASSESS OF THE STREET, PADPILL HOUSE, AND MILES LAWS.

- 2.11 ALL PLANTING TYPICK SHALL BE STEVENEY BLAUFF, FIRST OF PETCHEN AND SHAPE SHAPE

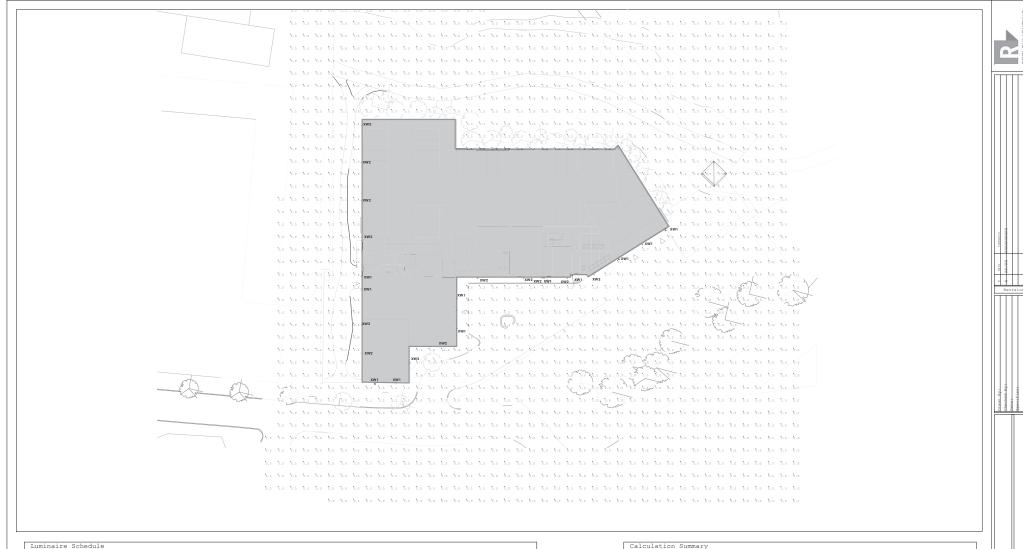




PERENNIALS, GROUNDCOVERS, VINES and ANNUALS

Botanical Name Amsonia nuonchia Carex appalachica Calamagrostis acutifolia 'Karl Foerster' Hakonechloa mecra Hosta 'Guacamole' Liriope spicata Penninton Smartseed Tall Fescue Blend Common Name
Blue Star Flower
Appalachian Sedge
Feather Reed Grass
Japanese Frost Grass
Guacamole Hosta
Lily Turf

City of Portsmouth Tree Planting Detail



- 1	Luminaire Schedule						
	Symbol	Qty	Label	Arrangement	Description		
		11	XW1	SINGLE	WS-W54614-XX		
		13	XW2	SINGLE	WP-LED119-30		

Calculation Summary						
Label	Units	Avg	Max	Min	Avg/Min	Max/Min
CalcPts 1	Fc	0.07	10.2	0.0	N.A.	N.A.

Page 1 of

Street

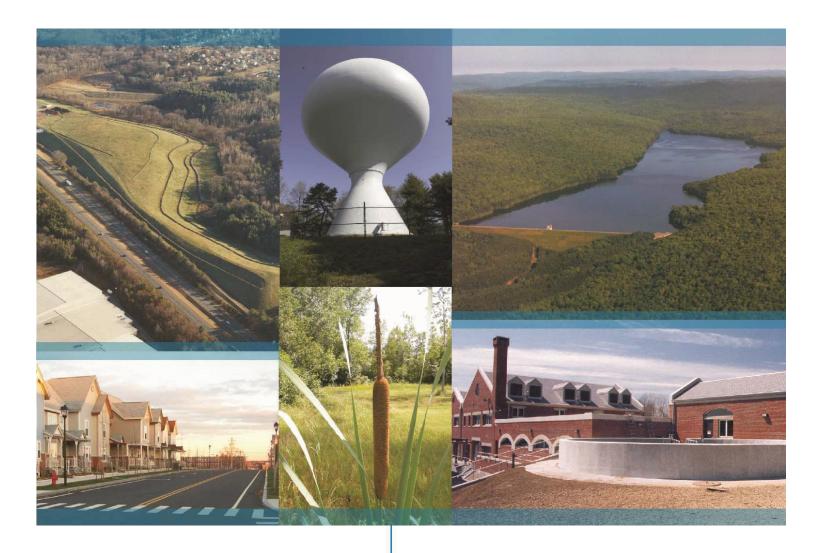




City of Portsmouth TAC, May 4, 2021:		
TAC Comment	Applicant Response	<u>Sheet</u>
TAC Comments from 5/4 Correspondence:		
1 How does this project comply with the front lot line build out requirement?	The applicant is applying for a variance from this requirement.	N/A
2 Does the driveway comply with the requirements of Section 10.5A44.33?	A dimension has been added to the plans to clarify the 24' driveway width.	C-102.1
3 What is the status of the NHDES permit for this project?	Alteration of Terrain permit application has been submitted to NHDES. The applicant has had	Cover
	multiple pre application meetings with NHDES in relation to the Wetland Impact Permit. The	
	submission of the Wetland Impact Permit is pending the completion of the technical review process.	
	Once the technical review is complete the formal wetland permit application can be submitted to	
	NHDES.	
4 Please make sure all relevant documents are included in each updated submission set (e.g. the	Comment acknowledged.	N/A
Green Building Statement was included with last month's submission, but was not incorporated into		
your latest revision, nor was the will-serve letter from Unitil)		
5 It appears you are missing a few of the will-serve letters from the private utility companies	A will serve letter from Until is included with the submission package. The project is coordinating	N/A
	with Eversource on required off site improvements.	
6 Please add a note to the bike storage room that it must provide storage for a minimum of 10 bikes.	The building's bike room has been noted to have a 30 bike capacity. Outdoor bike racks have been	C-102.1
Are you able to provide any exterior bike racks, those would be more suitable for visitors and many	included in the design.	
of the residents may prefer outdoor racks as well.		
7 Plans must stipulate that the tandem spaces meet the requirements of 10.1114.33 (must be	All tandem spaces have been removed.	C-102.1
assigned to the same dwelling unit, cannot be used for guest parking, and must measure 9' by 38'.)		
This requirement applies whether or not you exceed your minimum parking requirement. Your		
current tandem spaces are too narrow.		
8 Wetland buffer exhibits should quantify temporary impacts in the wetland buffer areas for	The Wetland Buffer Impervious Surface Exhibit has been revised to include temporary impacts in the	Wetland Buffer Impervious
construction of the pervious trail and fire access.	wetland buffer for construction.	Surface Exhibit
9 Please provide a pavement maintenance plan for the porous pavement areas as required by Section	The pavement maintenance plan in the Long Term Operation and Maintenance Plan has been	Long Term Operation &
10.1018.32.	revised to match these requirements, specifically snow maintenance requirements.	Maintenance Plan
10 Plans should include proposed locations for permanent wetland boundary markers as required by	Plans have been revised to include proposed locations for permanent wetland boundary markers as	C-102.1 & C-502
Section 10.1018.40.	required by Section 10.1018.40.	
11 Thank you for the addition to the planting plan to include shrubs along the top of bank. Please	A bank stabilization area has been added to address any active erosion in the area between the top	C-103
address the area between the top of bank and the HOTL to determine if there is active erosion and	of bank and the HOTL.	
whether bank stabilization plantings could slow any active erosion in this area.		
12 The marging of the fire access and public trail are a positive improvement	Commont salva quiled god	N/A
12 The merging of the fire access and public trail are a positive improvement. 13 The greenway trail shall be designed and constructed to accommodate periodic inundation by	Comment acknowledged. The greenway trail is designed as a porous asphalt path. Any periodic inundation by flooding will be	C-503
		C-303
flooding and to support maintenance vehicles as necessary.	able to infiltrate through the asphalt section and discharge through the underdrain. Additional the	
	porous section is deigned per UNH Stormwater Center specifications which are designed for H-20	
14 Discourant of course to Device Avenue in the project of Fig. 1 of the Device of Device of ASS Devices	loading.	Duningan Aughtria
14 Please correct references to Raynes Ave project on page 5-5 of the Drainage Report and 105 Bartlett	The Drainage Analysis has been revised.	Drainage Analysis
project on page 5-7.	<u></u>	0.400.4
15 Consider a cobble stone island for the drop-off area. If possible, consider a raised planter in the	The layout in this area has been updated and no longer has a middle island.	C-102.1
middle.		

The turning diagrams show a 22 foot long delivery truck just barely being able to turn around within the site, and that's if no vehicles are parked in the loading zone or drop-off area. A 22 foot long truck is a rather short truck. Moving vans, UPS vans and box trucks are commonly 26-30 feet long. If trucks are not able to turn around on site they will likely park on Green Street and block traffic on this narrow street. There needs to be a better way to accommodate typical size trucks on the site.	The site layout has been updated to accommodate a larger truck (SU-30).	Site Traffic Exhibit
17 The sharp angle of the driveway entrance requires entering trucks to use most or all of the driveway width. This will not be possible when a vehicle is exiting the site at the same time.	Updated turning templates have been provided.	N/A
18 A tip down ramp should be provided at the edge of the driveway at its intersection with Green Street. This may require relocating the tree pit.	A tip down ramp has been added.	C-102.1
19 The Site Traffic Exhibit shows that vehicles using the garage ramps will need to be positioned against the retaining wall in order to make the turn into the garage without interfering with other vehicles. In reality, vehicles tend to shy away from a vertical element at least a foot. The exhibit also does not show the garage doors on the plan, as are shown on the other sheets such as C-102.1. We are not convinced that a vehicle will be able to make the turns in and out of the garage under the current design. The drive aisle should be widened or other alterations made to provide easier access into and out of the garage.	An updated driveway and garage entrance has been provided to address these comments.	C-102.1, Site Traffic Exhibit
Passenger cars making the turn around on site will need to hug the curb and hope that no vehicles are hanging over the edge of the loading zone area, in order to make the 270 degree turn, and then into a 90 degree turn. It is unrealistic to think that vehicles will be able to easily execute the turning movements shown on the plan.	This area has been updated to eliminate the turning movement.	C-102.1
21 Overall, the on-site circulation seems too tight and cramped to allow for safe and efficient vehicle maneuvering.	This driveway and building have been updated to enhance circulation.	C-102.1
22 Raise Telephone Manhole to grade	A note has been added.	C-104
23 Demo plan says gas line only removed on this property. In reality the Gas line needs to be removed/terminated at the main. Show it properly.	The plans have been updated.	C-101
24 Water line services should be 3' apart at main	The plans have been updated.	C-104
25 Any required upgrades to the Russell Street electrical system will be the responsibility of the project, in coordination with Eversource	Confirmed. The project has been coordinating with Eversource on any required off site improvements.	N/A
26 Excavate ledge for tree pits, confirm size of ledge removal necessary with City Arborist. Confirm relief in ledge hole to ensure water will drain from excavation and not drown tree.	The project proposes two tree pits. One tree pit is within the existing building footprint. The other tree pit location did not find any ledge with a ledge probe in that area. Although, if ledge is encountered the applicant will coordinate with the City Arborist to confirm adequate tree pit excavation.	N/A
27 There are only two water services for 233 Vaughn and they are the two that are shown together. Remove 3rd service from water plan.	The plans have been updated.	C-201
28 Truncated dome detail – cast iron panels are required to be the width of the crosswalk. Please update	Detail has been removed.	C-502
'	Detail has been updated.	C-505
30 North End Light Fixture Base Detail – field confirm appropriate depth of base below finished grade so that finished base of light is approximately 1" below finished grade.	The detail note has been revised with the updated dimension.	C-506
31 Any drainage structures with proposed invert elevations below elev. 9 should be salt proofed on the inside.	A note has been added to salt proof these structures.	C-504
Placing a retaining wall that close to the AC's 'rain garden' that is under the ramp will make this area almost impossible to maintain. Please explain how this use will not be impacted.	The raingarden access area within the AC's property is of sufficient size to allow for maintenance items to be completed as described in the AC's Long Term Operation & Maintenance Plan.	N/A

	33	Plans shall indicate that the fire truck area shall be kept clear of snow	A note has been added as requested.	C-102.1
Г	34	On the Fire Truck Turning Exhibit, show the truck and the outrigger area with the rear of the truck	Exhibit has been revised.	Fire Truck Turning Exhibit
		clearing the corner.		





Proposed Mixed-Use Development 53 Green Street Portsmouth, NH

Drainage Analysis

Prepared For:

CPI Management, LLC 100 Summer Street Boston, Massachusetts 02110

March 22, 2021

Last Revised: May 19, 2021

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Appendices

- A Site Specific Soils Report
- B Extreme Precipitation Tables
- C "Examination of Thermal Impacts from Stormwater BMPs", By The University of New Hampshire Stormwater Center

Section 1 Project Description

The proposed project is located at 53 Green Street in Portsmouth and is identified as Map 119, Lot 2 on the City of Portsmouth's Tax Maps. This parcel is approximately 1.65 acres. As part of this project, this parcel will acquire a portion of the adjacent lot that contains the rail line, identified as Tax Map 119 Lot 3. This will result in a total acreage of approximately 1.77 acres for the proposed parcel. The parcel is bounded to the north and west by North Mill Pond, to the south by an adjacent parcel, and to the east by Green Street and the Boston and Maine (B&M) railroad.

The lot is currently occupied by two (2) single-story commercial tenant buildings, which total approximately 21,000 square feet, and associated parking. The lot is predominantly impervious and has a maintained lawn area along the North Mill Pond shoreline. There is an existing utility easement on the north corner of the parcel which contains a utility tower with overhead wire connections, not directly associated with the site.

The proposed project includes the demolition of the two existing single-story structures and construction of a single five story mixed-use building. The project will include associated site improvements that consist of below grade parking, utilities, stormwater management and treatment, landscaping, lighting, and a public recreation trail in coordination with the City. Additionally, the land associated with the public recreation trail will be deeded to the City of Portsmouth and designated as community space for the City's North Mill Pond Trail project.

1.1 On-Site Soil Description

The site is a highly disturbed site along the North Mill Pond. The property shows evidence of what appears to be very old filling and grading associated with the existing development. The site consists of terrain that is generally flat and slopes from the south to the north to North Mill Pond. The existing property has an approximate high point of elevation of 14 near Green Street

A site specific soils survey was conducted by Leonard Lord, PhD, CSS, CWS of Tighe & Bond, Inc and can be found in Appendix A of this Report. Based on the soil survey, the runoff analyzed within these studies has been modeled using mostly Hydrologic Soil Group B soils and some portions of Hydrologic Soil Group C soils, as much of the site is comprised of Udorthents with two drainage classifications, moderately poorly drained soils and portions of well drained soils.

1.2 Pre- and Post-Development Comparison

The pre-development and post-development watershed areas have been analyzed at a single point of analysis. While the point of analysis remained unchanged, its contributing sub-catchment areas varied between pre-development and post-development conditions. These adjustments were made to reflect the differences in drainage patterns between the existing and proposed conditions. The overall area analyzed as part of this drainage analysis was held constant. For reference, PA-1 assesses flows that discharge directly to North Mill Pond via overland flow or various outlets.

Since North Mill Pond is a tidal water, NHDES does not require peak runoff control requirements to be met (Env-Wq 1507.06(d)). However, a Stormtech Isolator Row and detention system is proposed on the development site for the purpose of mitigating temperature differences between the stormwater runoff and the North Mill Pond.

1.3 Calculation Methods

The design storms analyzed in this study are the 2-year, 10-year, 25-year and 50-year 24-hour duration storm events. The stormwater modeling system, HydroCAD 10.0 was utilized to predict the peak runoff rates from these storm events. The peak discharge rates were determined by analyzing Type III 24-hour storm events. The rainfall data for these storm events was obtained from the data published by the Northeast Regional Climate Center at Cornell University, with an additional 15% added factor of safety as required by Env-Wq 1503.08(I).

	1 4 2 1 2 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1				
YEAR	24-hr Estimate (inches)	+ 15% (inches)			
2	3.20	3.68			
10	4.86	5.59			
25	6.16	7.08			
50	7.37	8.48			

Table 1.2: Extreme Precipitation Estimates (NRCC)

The time of concentration was computed using the TR-55 Method, which provides a means of determining the time for an entire watershed to contribute runoff to a specific location via sheet flows, shallow concentrated flow and channel flow. Runoff curve numbers were calculated by estimating the coverage areas and then summing the curve number for the coverage area as a percent of the entire watershed.

References:

- 1. HydroCAD Stormwater Modeling System, by HydroCAD Software Solutions LLC, Chocorua, New Hampshire.
- 2. New Hampshire Stormwater Management Manual, Volume 2, Post-Construction Best Management Practices Selection and Design, December 2008.
- 3. "Extreme Precipitation in New York & New England." Extreme Precipitation in New York & New England by Northeast Regional Climate Center (NRCC), 26 June 2012.

Section 2 Pre-Development Conditions

In order to analyze the pre-development condition, the site has been divided into one (1) watershed area modeled at one (1) point of analysis. This point of analysis and watershed are depicted on the plan entitled "Pre-Development Watershed Plan", Sheets C-801.

The point of analysis and its contributing watershed area is described below:

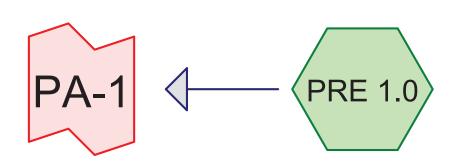
Point of Analysis (PA-1)

Point of Analysis 1 (PA-1) is the North Mill Pond which borders the northwest boundary of the site. The North Mill Pond is a tidal wetland which directly feeds into the Piscataqua River.

Pre-development Watershed 1.0 (PRE 1.0) is the single watershed analyzed in the pre-development condition. It is comprised of mostly impervious surfaces including paved parking and structures, disturbed forested areas to the north and west adjacent to the North Mill Pond shoreline and a maintained lawn between the building and shoreline. Runoff from this watershed area travels via overland flow to discharge into North Mill Pond. The runoff is currently untreated before discharge.

2.1 Pre-Development Calculations

2.2 Pre-Development Watershed Plans



POINT OF ANALYSIS 1









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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
26,605	61	>75% Grass cover, Good, HSG B (PRE 1.0)
2,659	74	>75% Grass cover, Good, HSG C (PRE 1.0)
23,291	98	Paved parking, HSG B (PRE 1.0)
21,715	98	Roofs, HSG B (PRE 1.0)
4,041	55	Woods, Good, HSG B (PRE 1.0)
78,311	82	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
75,652	HSG B	PRE 1.0
2,659	HSG C	PRE 1.0
0	HSG D	
0	Other	
78,311		TOTAL AREA

C0960-011 PRE

Type III 24-hr 2 Year Storm Rainfall=3.68"

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Page 4

Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPRE 1.0:

Runoff Area=78,311 sf 57.47% Impervious Runoff Depth=1.93" Flow Length=380' Tc=5.0 min CN=82 Runoff=4.17 cfs 12,610 cf

Link PA-1: POINT OF ANALYSIS1

Inflow=4.17 cfs 12,610 cf Primary=4.17 cfs 12,610 cf

Total Runoff Area = 78,311 sf Runoff Volume = 12,610 cf Average Runoff Depth = 1.93" 42.53% Pervious = 33,305 sf 57.47% Impervious = 45,006 sf

C0960-011 PRE

Type III 24-hr 10 Year Storm Rainfall=5.59"

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Page 5

Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPRE 1.0:

Runoff Area=78,311 sf 57.47% Impervious Runoff Depth=3.61" Flow Length=380' Tc=5.0 min CN=82 Runoff=7.74 cfs 23,570 cf

Link PA-1: POINT OF ANALYSIS1

Inflow=7.74 cfs 23,570 cf Primary=7.74 cfs 23,570 cf

Total Runoff Area = 78,311 sf Runoff Volume = 23,570 cf Average Runoff Depth = 3.61" 42.53% Pervious = 33,305 sf 57.47% Impervious = 45,006 sf HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software Solutions LLC

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Summary for Subcatchment PRE 1.0:

Runoff = 7.74 cfs @ 12.08 hrs, Volume= 23,570 cf, Depth= 3.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

	Aı	rea (sf)	CN [Description		
		21,715	98 F	Roofs, HSG	βB	
		23,291	98 F	Paved park	ing, HSG E	3
		26,605	61 >	75% Gras	s cover, Go	ood, HSG B
		4,041	55 V	Voods, Go	od, HSG B	
_		2,659	74 >	75% Gras	s cover, Go	ood, HSG C
		78,311	82 V	Veighted A	verage	
		33,305	4	2.53% Per	vious Area	
		45,006	5	57.47% lmp	pervious Ar	ea
	_		01			B
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	0.9	100	0.0330	1.80		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.68"
	1.9	223	0.0090	1.93		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.7	57	0.0400	1.40		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	3.5	380	Total	ncreased t	o minimum	$T_{\rm C} = 5.0 \text{min}$

380 Total, Increased to minimum Tc = 5.0 min

Summary for Link PA-1: POINT OF ANALYSIS 1

Inflow Area = 78,311 sf, 57.47% Impervious, Inflow Depth = 3.61" for 10 Year Storm event

Inflow = 7.74 cfs @ 12.08 hrs, Volume= 23,570 cf

Primary = 7.74 cfs @ 12.08 hrs, Volume= 23,570 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs

C0960-011 PRE

Type III 24-hr 25 Year Storm Rainfall=7.08"

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Page 1

Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPRE 1.0:

Runoff Area=78,311 sf 57.47% Impervious Runoff Depth=4.99" Flow Length=380' Tc=5.0 min CN=82 Runoff=10.58 cfs 32,572 cf

Link PA-1: POINT OF ANALYSIS1

Inflow=10.58 cfs 32,572 cf Primary=10.58 cfs 32,572 cf

Total Runoff Area = 78,311 sf Runoff Volume = 32,572 cf Average Runoff Depth = 4.99" 42.53% Pervious = 33,305 sf 57.47% Impervious = 45,006 sf C0960-011 PRE

Type III 24-hr 50 Year Storm Rainfall=8.48"

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Page 2

Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

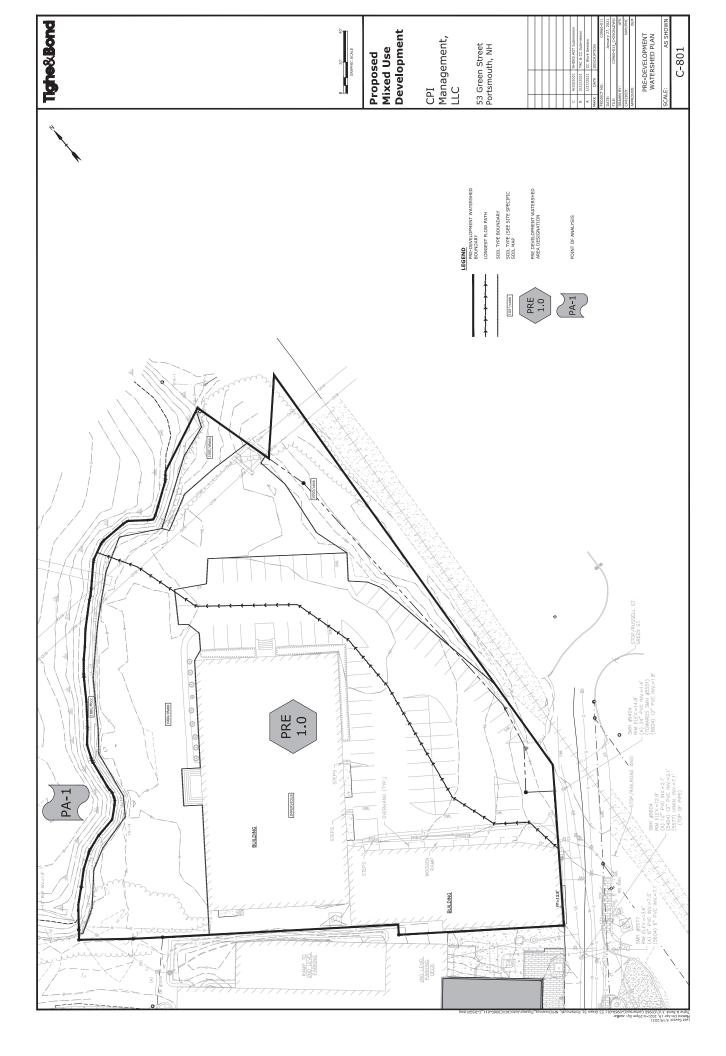
SubcatchmentPRE 1.0:

Runoff Area=78,311 sf 57.47% Impervious Runoff Depth=6.32" Flow Length=380' Tc=5.0 min CN=82 Runoff=13.25 cfs 41,222 cf

Link PA-1: POINT OF ANALYSIS1

Inflow=13.25 cfs 41,222 cf Primary=13.25 cfs 41,222 cf

Total Runoff Area = 78,311 sf Runoff Volume = 41,222 cf Average Runoff Depth = 6.32" 42.53% Pervious = 33,305 sf 57.47% Impervious = 45,006 sf



Section 3 Post-Development Conditions

The post-development condition was analyzed by dividing the watersheds into six (6) watershed areas. Stormwater runoff from these sub-catchments predominantly flows via subsurface drainage systems prior to discharging into North Mill Pond (PA-1). A negligible amount of runoff from the sidewalk along Green Street will sheet flow into the City's closed drainage system due to the existing grades of the street sloping away from the site. The City's drainage system eventually discharges into North Mill Pond (PA-1), and, therefore, has been included in the single point of analysis.

A Stormtech Isolator Row and detention system is included on the development site for the purpose of mitigating temperature differences between the stormwater runoff and the North Mill Pond. This system and outlet structure have been designed to mitigate temperature of the water quality volume (WQV). Runoff that exceeds this volume will utilize an overflow and discharge into North Mill Pond (PA-1). This detention basin is used to mitigate increased temperature of the initial surface runoff, based on data provided in a publication by the University of New Hampshire Stormwater Center (UNHSC), titled "Examination of Thermal Impacts from Stormwater BMPs" and con be found in Appendix C. Due to this system being included in the design, post-development flows from the site have been reduced from the pre-development condition. As previously described, North Mill Pond is a tidal water, therefore, NHDES does not require peak runoff control requirements to be met (per Env-Wq 1507.06(d)).

The point of analysis and sub-catchment areas are depicted on the plan entitled "Post-Development Watershed Plan," Sheet C-802. The points of analysis and its contributing watershed areas are described below:

Point of Analysis (PA-1)

Point of Analysis 1 (PA-1), North Mill Pond, has the same overall contributing area as in the pre-development condition. PA-1 includes an underground detention basin, which is designed to detain the water quality volume of the paved surface runoff. Additional impervious surface runoff will be collected and filtered prior to discharging into the North Mill Pond.

Post-development Watershed 1.1 (POST-1.1) is approximately 72% impervious surface of either pavement or concrete surface. The area includes in the site access driveway and entrance turnaround. The pervious portion of this watershed includes a porous grass paver section intended for emergency use for fire truck access. Additional pervious areas that contribute to this watershed include a small amount of landscaped areas along the building façade. The stormwater runoff created from this area is collected via offline deep-sump and hooded catch basins and conveyed via a closed drainage system to the underground stormtech chamber system (POND-1). The detention basin is equipped with an isolator row as recommended by the UNHSC publication and is lined due to high seasonal high water table in the area. The system is underdrained and treatment is attained post detention by use of a proprietary membrane filtration treatment device identified as Jellyfish Filter 1 (JF-1). All collected runoff from this catchment is discharged into the North Mill Pond (PA-1).

Post-development Watershed 1.2 (POST-1.2) is 100% impervious roof surface that is collected via internal building plumbing system and conveyed via piping to a proprietary membrane filtration treatment device identified as Jellyfish Filter 1 (JF-1). The treated runoff eventually discharges into North Mill Pond (PA-1).

Post-development Watershed 1.3 (POST-1.3) is the connection path for public access to the public recreation trail along the shoreline. The area is approximately 50% impervious surface and consists of landscaping and grassed lawn areas in the post-development condition. The runoff associated with this area is captured via yard drains and is conveyed via piping to a proprietary membrane filtration treatment device identified as Jellyfish Filter 1 (JF-1). The treated runoff eventually discharges into North Mill Pond (PA-1).

Post-development Watershed 1.4 (POST-1.4) is 100% pervious surface. The area consists mostly of lawn, wooded, and landscaped areas. Runoff from this area remains similar to existing conditions and flows overland and discharges into the North Mill Pond.

Post-development Watershed 1.5 (POST-1.5) is 100% impervious sidewalk surface and flows overland onto Green Street. This subcatchment represents a proposed city sidewalk which flows onto the city street for collection. The closed drainage system associated with Green Street eventually discharges into North Mill Pond (PA-1).

Post-development Watershed 1.6 (POST-1.6) includes a city recreation trail which the city requested that be porous pavement, as not to increase impervious area so close to the waterfront. The runoff associated with this area flows overland and is captured and treated by the porous pavement section and is conveyed via piping to discharge into North Mill Pond.

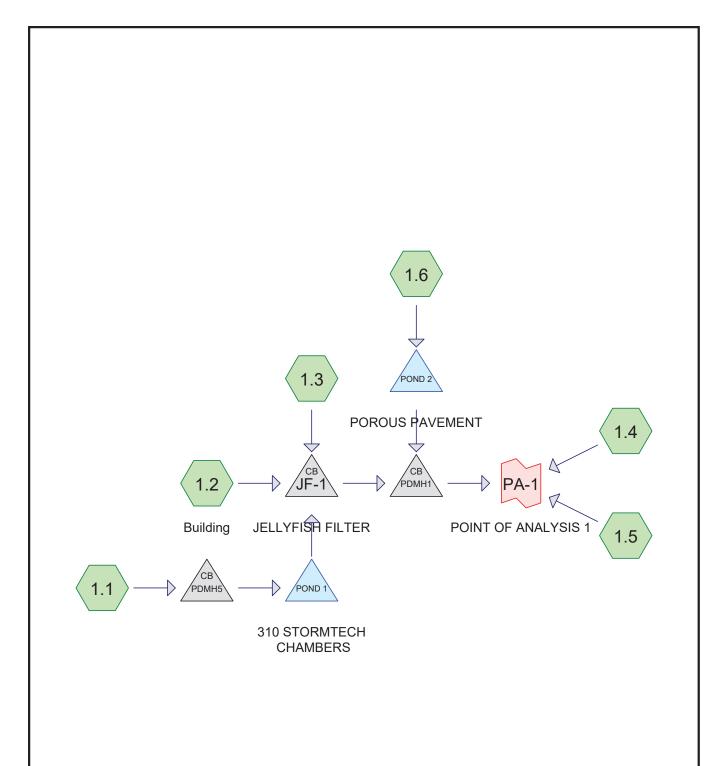
3.1 Peak Rate Comparison

The following table summarizes and compares the pre- and post-development peak runoff rates for the 2-year, 10-year, 25-year and 50-year storm events at each point of analysis. Though peak flow mitigation is not required, the following table is provided for reference.

	Pre/ Post	Pre/ Post	Pre/ Post	Pre/ Post
Point of	2-Year	10-Year	25-Year	50-Year
Analysis	Storm	Storm	Storm	Storm
	(cfs)	(cfs)	(cfs)	(cfs)
PA1	4.17/ 3.37	7.74/ 5.68	10.58/ 8.61	13.25/ 10.81

3.2 Post-Development Calculations

3.3 Post-Development Watershed Plans











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Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
25,869	61	>75% Grass cover, Good, HSG B (1.1, 1.3, 1.4, 1.6)
2,659	74	>75% Grass cover, Good, HSG C (1.4)
14,478	98	Paved parking, HSG B (1.1, 1.3, 1.5, 1.6)
3,392	98	Porous Paved Path, HSG B (1.6)
29,486	98	Roofs, HSG B (1.2)
1,427	55	Woods, Good, HSG B (1.4)
77,311	84	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
74,652	HSG B	1.1, 1.2, 1.3, 1.4, 1.5, 1.6
2,659	HSG C	1.4
0	HSG D	
0	Other	
77,311		TOTAL AREA

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Page 4

Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1.1: Runoff Area=14,379 sf 71.72% Impervious Runoff Depth=2.43"

Tc=5.0 min CN=88 Runoff=0.96 cfs 2,916 cf

Subcatchment1.2: Building Runoff Area=29,486 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=2.48 cfs 8,467 cf

Subcatchment1.3: Runoff Area=4,788 sf 50.31% Impervious Runoff Depth=1.78"

Tc=5.0 min CN=80 Runoff=0.23 cfs 710 cf

Subcatchment1.4: Runoff Area=15,732 sf 0.00% Impervious Runoff Depth=0.75"

Tc=5.0 min CN=63 Runoff=0.27 cfs 982 cf

Subcatchment1.5: Runoff Area=1,321 sf 100.00% Impervious Runoff Depth=3.45"

Tc=5.0 min CN=98 Runoff=0.11 cfs 379 cf

Subcatchment1.6: Runoff Area=11,605 sf 32.98% Impervious Runoff Depth=1.30"

Tc=5.0 min CN=73 Runoff=0.40 cfs 1,259 cf

Pond JF-1: JELLYFISH FILTER Peak Elev=7.41' Inflow=3.00 cfs 12,093 cf

24.0" Round Culvert n=0.013 L=70.0' S=0.0043 '/' Outflow=3.00 cfs 12,093 cf

Pond PDMH1: Peak Elev=7.04' Inflow=3.00 cfs 12,668 cf

24.0" Round Culvert n=0.013 L=7.0' S=0.0071'/' Outflow=3.00 cfs 12,668 cf

Pond PDMH5: Peak Elev=7.94' Inflow=0.96 cfs 2.916 cf

12.0" Round Culvert n=0.013 L=2.0' S=0.0000 '/' Outflow=0.96 cfs 2.916 cf

Pond POND 1: 310 STORMTECH CHAMBERS Peak Elev=7.64' Storage=526 cf Inflow=0.96 cfs 2,916 cf

Outflow=0.62 cfs 2,915 cf

Pond POND 2: POROUS PAVEMENT Peak Elev=6.53' Storage=800 cf Inflow=0.40 cfs 1,259 cf

Outflow=0.02 cfs 575 cf

Link PA-1: POINT OF ANALYSIS1 Inflow=3.37 cfs 14,029 cf

Primary=3.37 cfs 14,029 cf

Total Runoff Area = 77,311 sf Runoff Volume = 14,714 cf Average Runoff Depth = 2.28" 38.75% Pervious = 29,955 sf 61.25% Impervious = 47,356 sf

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Summary for Subcatchment 1.1:

Runoff = 1.62 cfs @ 12.07 hrs, Volume= 5,071 cf, Depth= 4.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

A	rea (sf)	CN I	CN Description				
	10,313	98 F	Paved park	ing, HSG B	В		
	4,066	61 >	1 0				
	14,379	88 \	Veighted A	verage			
	4,066	2	28.28% Pervious Area				
	10,313	7	1.72% lmp	ervious Ar	rea		
_				_			
	Length	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		

Summary for Subcatchment 1.2: Building

Runoff = 3.78 cfs @ 12.07 hrs, Volume= 13,152 cf, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

A	rea (sf)	CN [Description		
	29,486	98 F	Roofs, HSC	B	
	29,486	,	100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment 1.3:

Runoff = 0.45 cfs @ 12.08 hrs, Volume= 1,362 cf, Depth= 3.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

Area (sf)	CN	Description
2,409	98	Paved parking, HSG B
2,379	61	>75% Grass cover, Good, HSG B
4,788	80	Weighted Average
2,379		49.69% Pervious Area
2,409		50.31% Impervious Area

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	Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.0					Direct Entry,

Summary for Subcatchment 1.4:

Runoff = 0.79 cfs @ 12.08 hrs, Volume=

2,484 cf, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

Area (sf)	CN	Description
11,646	61	>75% Grass cover, Good, HSG B
1,427	55	Woods, Good, HSG B
2,659	74	>75% Grass cover, Good, HSG C
15,732	63	Weighted Average
15,732		100.00% Pervious Area
Tc Length	Slo	pe Velocity Capacity Description
(min) (feet)		/ft) (ft/sec) (cfs)
5.0		Direct Entry,

Summary for Subcatchment 1.5:

Runoff = 0.17 cfs @ 12.07 hrs, Volume=

589 cf, Depth= 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

_	Α	rea (sf)	CN I	Description				
		1,321	98 I	Paved parking, HSG B				
		1,321		100.00% In	npervious A	Area		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	5.0					Direct Entry.		

Summary for Subcatchment 1.6:

Runoff = 0.88 cfs @ 12.08 hrs, Volume= 2,661 cf, Depth= 2.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs Type III 24-hr 10 Year Storm Rainfall=5.59"

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	Area (sf)	CN I	CN Description				
	435	98 I	Paved park	ing, HSG E	В		
	7,778	61	>75% Ġras	s cover, Go	Good, HSG B		
*	3,392	98 I	8 Porous Paved Path, HSG B				
	11,605	73 \	Weighted A	verage			
	7,778	(67.02% Pervious Area				
	3,827	,	32.98% Impervious Area				
To	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		

Summary for Pond JF-1: JELLYFISH FILTER

Inflow Area = 48,653 sf, 86.75% Impervious, Inflow Depth = 4.83" for 10 Year Storm event

Inflow = 4.72 cfs @ 12.08 hrs, Volume= 19,584 cf

Outflow = 4.72 cfs @ 12.08 hrs, Volume= 19,584 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.72 cfs @ 12.08 hrs, Volume= 19,584 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs / 2

Peak Elev= 7.70' @ 12.08 hrs

Flood Elev= 12.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	6.45'	24.0" Round Culvert
			L= 70.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 6.45' / 6.15' S= 0.0043 '/' Cc= 0.900
			n= 0.013, Flow Area= 3.14 sf

Primary OutFlow Max=4.72 cfs @ 12.08 hrs HW=7.69' TW=7.29' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.72 cfs @ 3.28 fps)

Summary for Pond PDMH1:

[80] Warning: Exceeded Pond POND 2 by 0.84' @ 12.04 hrs (0.69 cfs 1,142 cf)

Inflow Area = 60,258 sf, 76.40% Impervious, Inflow Depth = 4.29" for 10 Year Storm event

Inflow = 4.72 cfs @ 12.08 hrs, Volume= 21,561 cf

Outflow = 4.72 cfs @ 12.08 hrs, Volume= 21,561 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.72 cfs @ 12.08 hrs, Volume= 21,561 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs / 2

Peak Elev= 7.29' @ 12.08 hrs

Flood Elev= 10.10

Device	Routing	Invert	Outlet Devices
#1	Primary	6.15'	24.0" Round Culvert
			L= 7.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 6.15' / 6.10' S= 0.0071 '/' Cc= 0.900
			n= 0.013, Flow Area= 3.14 sf

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Primary OutFlow Max=4.70 cfs @ 12.08 hrs HW=7.29' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.70 cfs @ 3.67 fps)

Summary for Pond PDMH5:

Inflow Area = 14,379 sf, 71.72% Impervious, Inflow Depth = 4.23" for 10 Year Storm event Inflow = 1.62 cfs @ 12.07 hrs, Volume= 5,071 cf

Outflow = 1.62 cfs @ 12.07 hrs, Volume= 5,071 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.62 cfs @ 12.07 hrs, Volume= 5,071 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs / 2

Peak Elev= 8.30' @ 12.14 hrs

Flood Elev= 11.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	7.30'	12.0" Round Culvert
			L= 2.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 7.30' / 7.30' S= 0.0000 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.60 cfs @ 12.07 hrs HW=8.16' TW=7.92' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.60 cfs @ 2.99 fps)

Summary for Pond POND 1: 310 STORMTECH CHAMBERS

Exfiltration Rate derived from Site Specific Soil Survey report which compares existing soil classification to Sutton Soil HSG-B, which has a low Hydraulic conductivity rate of 0.6 in/hr, per NHDES regulations shall be modeling as 0.3 in/hr.

Inflow Area = 14,379 sf, 71.72% Impervious, Inflow Depth = 4.23" for 10 Year Storm event
Inflow = 1.62 cfs @ 12.07 hrs, Volume= 5,071 cf
Outflow = 1.19 cfs @ 12.17 hrs, Volume= 5,070 cf, Atten= 27%, Lag= 5.6 min
Primary = 1.19 cfs @ 12.17 hrs, Volume= 5,070 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs / 2 Peak Elev= 8.21' @ 12.16 hrs Surf.Area= 998 sf Storage= 898 cf Flood Elev= 9.36' Surf.Area= 998 sf Storage= 1,250 cf

Plug-Flow detention time= 16.0 min calculated for 5,066 cf (100% of inflow) Center-of-Mass det. time= 16.3 min (809.3 - 793.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.70'	719 cf	14.83'W x 67.28'L x 2.33'H Field A
			2,329 cf Overall - 531 cf Embedded = 1,798 cf x 40.0% Voids
#2A	7.20'	531 cf	ADS_StormTech SC-310 +Cap x 36 Inside #1
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap
			4 Rows of 9 Chambers
		1,250 cf	Total Available Storage

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Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	6.40'	15.0" Round Culvert
	,		L= 12.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 6.40' / 6.30' S= 0.0083 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	6.70'	6.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	8.10'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 3	7.20'	12.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=1.17 cfs @ 12.17 hrs HW=8.20' TW=7.53' (Dynamic Tailwater)

1=Culvert (Passes 1.17 cfs of 4.82 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.77 cfs @ 3.93 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 0.40 cfs @ 1.02 fps)

4=Orifice/Grate (Passes 0.40 cfs of 1.18 cfs potential flow)

Summary for Pond POND 2: POROUS PAVEMENT

Inflow Area = 11,605 sf, 32.98% Impervious, Inflow Depth = 2.75" for 10 Year Storm event

Inflow = 0.88 cfs @ 12.08 hrs, Volume= 2,661 cf

Outflow = 0.30 cfs @ 12.57 hrs, Volume= 1,977 cf, Atten= 66%, Lag= 29.6 min

Primary = 0.30 cfs @ 12.57 hrs, Volume= 1,977 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs / 2

Peak Elev= 6.89' @ 12.46 hrs Surf.Area= 3,392 sf Storage= 1,295 cf

Flood Elev= 9.35' Surf.Area= 3,392 sf Storage= 2,992 cf

Plug-Flow detention time= 202.2 min calculated for 1,977 cf (74% of inflow)

Center-of-Mass det. time= 111.5 min (945.1 - 833.6)

Volume	Invert Ava	ail.Storage	Storage Descrip	otion		
#1	5.94'	2,992 cf	Custom Stage	Custom Stage Data (Prismatic)Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
5.94	3,392	0.0	0	0		
7.52	3,392	40.0	2,144	2,144		
8.52	3,392	10.0	339	2,483		
9.02	3,392	30.0	509	2,992		
9.35	3,392	0.0	0	2,992		
Dovice Pouting Invert Outlet Dovices						

Device	Routing	Invert	Outlet Devices
#1	Primary	6.44'	6.0" Vert. Underdrain C= 0.600
#2	Device 1	5.94'	10.000 in/hr Filter Media Infiltration over Surface area

Primary OutFlow Max=0.27 cfs @ 12.57 hrs HW=6.87' TW=6.77' (Dynamic Tailwater)

1=Underdrain (Orifice Controls 0.27 cfs @ 1.52 fps)

²⁼Filter Media Infiltration (Passes 0.27 cfs of 0.79 cfs potential flow)

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Summary for Link PA-1: POINT OF ANALYSIS 1

Inflow Area = 77,311 sf, 61.25% Impervious, Inflow Depth = 3.82" for 10 Year Storm event

Inflow = 5.68 cfs @ 12.08 hrs, Volume= 24,634 cf

Primary = 5.68 cfs @ 12.08 hrs, Volume= 24,634 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.04 hrs

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Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1.1: Runoff Area=14,379 sf 71.72% Impervious Runoff Depth=5.67"

Tc=5.0 min CN=88 Runoff=2.14 cfs 6,796 cf

Subcatchment1.2: Building Runoff Area=29,486 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=4.80 cfs 16,809 cf

Subcatchment1.3: Runoff Area=4,788 sf 50.31% Impervious Runoff Depth=4.77"

Tc=5.0 min CN=80 Runoff=0.62 cfs 1,903 cf

Subcatchment 1.4: Runoff Area=15,732 sf 0.00% Impervious Runoff Depth=2.96"

Tc=5.0 min CN=63 Runoff=1.27 cfs 3,882 cf

Subcatchment1.5: Runoff Area=1,321 sf 100.00% Impervious Runoff Depth=6.84"

Tc=5.0 min CN=98 Runoff=0.21 cfs 753 cf

Subcatchment1.6: Runoff Area=11,605 sf 32.98% Impervious Runoff Depth=4.00"

Tc=5.0 min CN=73 Runoff=1.28 cfs 3,873 cf

Pond JF-1: JELLYFISH FILTER Peak Elev=8.06' Inflow=7.14 cfs 25,507 cf

24.0" Round Culvert n=0.013 L=70.0' S=0.0043 '/' Outflow=7.14 cfs 25,507 cf

Pond PDMH1: Peak Elev=7.60' Inflow=7.14 cfs 28,695 cf

24.0" Round Culvert n=0.013 L=7.0' S=0.0071'/' Outflow=7.14 cfs 28,695 cf

Pond PDMH5: Peak Elev=8.63' Inflow=2.14 cfs 6.796 cf

12.0" Round Culvert n=0.013 L=2.0' S=0.0000 '/' Outflow=2.14 cfs 6.796 cf

Pond POND 1: 310 STORMTECH CHAMBERS Peak Elev=8.34' Storage=965 cf Inflow=2.14 cfs 6,796 cf

Outflow=2.01 cfs 6,795 cf

Pond POND 2: POROUS PAVEMENT Peak Elev=7.24' Storage=1,759 cf Inflow=1.28 cfs 3,873 cf

Outflow=0.51 cfs 3.188 cf

Link PA-1: POINT OF ANALYSIS1 Inflow=8.61 cfs 33,329 cf

Primary=8.61 cfs 33,329 cf

Total Runoff Area = 77,311 sf Runoff Volume = 34,014 cf Average Runoff Depth = 5.28" 38.75% Pervious = 29,955 sf 61.25% Impervious = 47,356 sf

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Time span=0.00-48.00 hrs, dt=0.04 hrs, 1201 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1.1: Runoff Area=14,379 sf 71.72% Impervious Runoff Depth=7.04"

Tc=5.0 min CN=88 Runoff=2.63 cfs 8,433 cf

Subcatchment1.2: Building Runoff Area=29,486 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=5.75 cfs 20,247 cf

Subcatchment1.3: Runoff Area=4,788 sf 50.31% Impervious Runoff Depth=6.08"

Tc=5.0 min CN=80 Runoff=0.79 cfs 2,424 cf

Subcatchment1.4: Runoff Area=15,732 sf 0.00% Impervious Runoff Depth=4.05"

Tc=5.0 min CN=63 Runoff=1.75 cfs 5,309 cf

Subcatchment1.5: Runoff Area=1,321 sf 100.00% Impervious Runoff Depth=8.24"

Tc=5.0 min CN=98 Runoff=0.26 cfs 907 cf

Subcatchment 1.6: Runoff Area=11,605 sf 32.98% Impervious Runoff Depth=5.24"

Tc=5.0 min CN=73 Runoff=1.67 cfs 5,065 cf

Pond JF-1: JELLYFISH FILTER Peak Elev=8.30' Inflow=8.80 cfs 31,104 cf

24.0" Round Culvert n=0.013 L=70.0' S=0.0043 '/' Outflow=8.80 cfs 31,104 cf

Pond PDMH1: Peak Elev=7.79' Inflow=8.80 cfs 35,484 cf

24.0" Round Culvert n=0.013 L=7.0' S=0.0071'/' Outflow=8.80 cfs 35,484 cf

Pond PDMH5: Peak Elev=8.92' Inflow=2.63 cfs 8.433 cf

12.0" Round Culvert n=0.013 L=2.0' S=0.0000 '/' Outflow=2.63 cfs 8.433 cf

Pond POND 1: 310 STORMTECH CHAMBERS Peak Elev=8.49' Storage=1,034 cf Inflow=2.63 cfs 8,433 cf

Outflow=2.46 cfs 8,433 cf

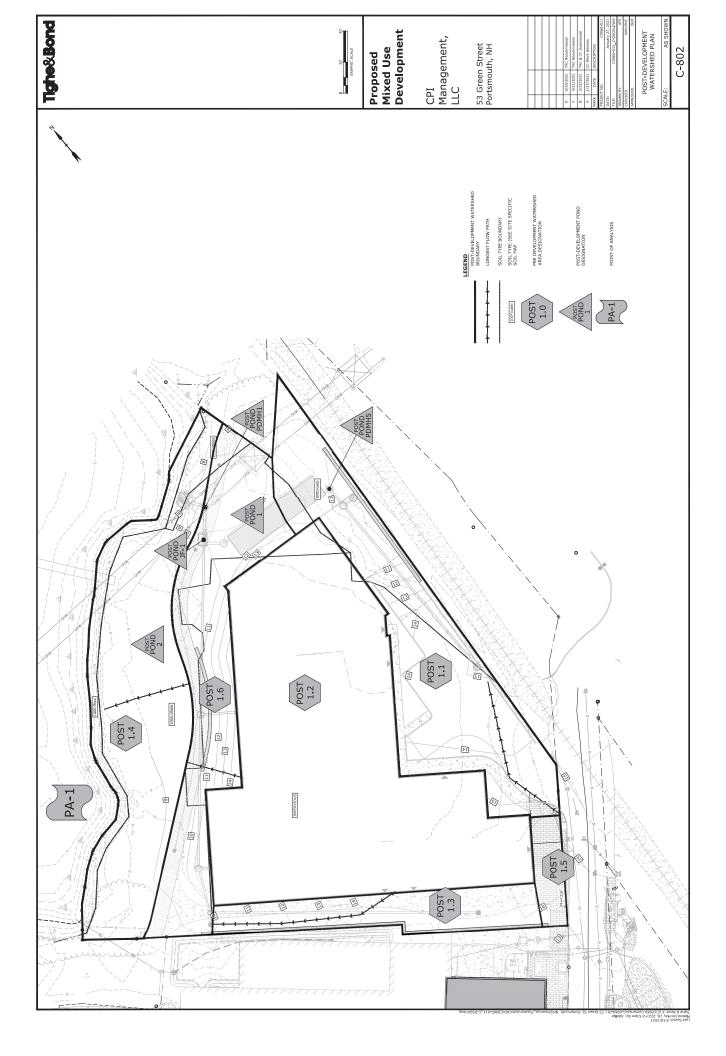
Pond POND 2: POROUS PAVEMENT Peak Elev=7.78' Storage=2,231 cf Inflow=1.67 cfs 5,065 cf

Outflow=0.72 cfs 4.380 cf

Link PA-1: POINT OF ANALYSIS1 Inflow=10.81 cfs 41,701 cf

Primary=10.81 cfs 41,701 cf

Total Runoff Area = 77,311 sf Runoff Volume = 42,386 cf Average Runoff Depth = 6.58" 38.75% Pervious = 29,955 sf 61.25% Impervious = 47,356 sf



Section 4 Stormwater Treatment

The stormwater management system has been designed to provide stormwater treatment as required by the City of Portsmouth Site Review Regulations and NHDES AoT Regulations (Env-Wq 1500).

4.1 Pre-Treatment Methods for Protecting Water Quality

Pre-treatment for the stormwater that is collected on-site is pretreated through use of offline deep-sump and hooded catch basins .

4.2 Treatment Methods for Protecting Water Quality

The runoff from proposed impervious areas will be treated by a Contech Jellyfish stormwater filtration system. The Jellyfish system is sized to treat the Water Quality Flow from the contributing subcatchment areas. The system is outfitted with an internal bypass that diverts peak flows away from treatment. The BMP worksheet for this practice has been included in Section 5 of this report.

The multiuse path along the North Mill Pond will be constructed as porous pavement with and underdrain. The underdrain will discharge to the onsite closed drainage system prior to discharging to the Pond.

Table 4.1 - Pollutant Removal Efficiencies					
ВМР	Total Suspended Solids	Total Nitrogen	Total Phosphorus		
Jellyfish Filter w/Pretreatment ¹	91%	53%	61%		
Porous Pavement w/Underdrain ²	90%	10%	45%		

- 1. Pollutant removal calculations for Jellyfish Filter with deep sump catch basin pretreatment shown in Table 4.2.
- 2. Pollutant removal efficiencies from NH Stormwater Manual Volume 2, Appendix B.

Table 4.2 - Pollutant Removal Calculations						
Contech Jellyfish Filter						
ВМР	TSS Removal Rate	Starting TSS Load	TSS Removed	Remaining TSS Load		
Deep Sump Catchbasin w/Hood ¹	0.15	1.00	0.15	0.85		
Jellyfish Filter ²	0.89	0.85	0.76	0.09		
	Total Suspended Solids Removed: 91%					
	TN Removal Rate	Starting TN Load	TN Removed	Remaining TN Load		
Deep Sump Catchbasin w/Hood ¹	0.05	1.00	0.05	0.95		
Jellyfish Filter ²	0.51	0.95	0.48	0.47		
		53%				
	TP Removal Rate	Starting TP Load	TP Removed	Remaining TP Load		
Deep Sump Catchbasin w/Hood ¹	0.05	1.00	0.05	0.95		
Jellyfish Filter ²	0.59	0.95	0.56	0.39		
	Total Phosphorus Removed: 61%					

^{1.} Pollutant removal efficiencies from NH Stormwater Manual Volume 2, Appendix E.

^{2.} Pollutant removal efficiencies from Contech Engineered Solutions, Jellyfish Filter Stormwater Treatment performance testing results.

Section 5 BMP Worksheet and Sizing Memo



GENERAL CALCULATIONS - WQV and WQF (optional worksheet)

This worksheet may be useful when designing a BMP that does not fit into one of the specific worksheets already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)

Water Quality Volume (WQV)

1.12 ac	A = Area draining to the practice
0.97 ac	A _I = Impervious area draining to the practice
0.87 decimal	I = Percent impervious area draining to the practice, in decimal form
0.83 unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)
0.93 ac-in	WQV= 1" x Rv x A
3,372 cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")

Water Quality Flow (WQF)

1	inches	P = Amount of rainfall. For WQF in NH, P = 1".
0.83	inches	Q = Water quality depth. Q = WQV/A
98	unitless	CN = Unit peak discharge curve number. CN = $1000/(10+5P+10Q-10*[Q^2+1.25*Q*P]^{0.5})$
0.2	inches	S = Potential maximum retention. S = (1000/CN) - 10
0.032	inches	Ia = Initial abstraction. Ia = 0.2S
5.0	minutes	T _c = Time of Concentration
655.0	cfs/mi²/in	\textbf{q}_{u} is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III.
0.951	cfs	WQF = $q_u \times WQV$. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by 1mi ² /640ac.

Designer's Notes:
This calculation represents the treatment train directed to the Contech Jellyfish Filter (JF-1).
This calculation represents the treatment train directed to the conteen senting in the Tri.
Full Treatment in compliance with Env-Wq 1508.10 shall be achieved by use of a proprietary flow-through
device. A Contech Jellyfish Filter model JFPD0806-5-1 will be used to treat the WQF as calculated in the above
spreadsheet. The specified device is designed to treat up to 0.80 cfs of flow.
See attached sizing calculation sheet from manufacturer.



CONTECH Stormwater Solutions Inc. Engineer: DRA
Date Prepared: 3/17/2021

Site Information

Project Name 53 Green Street

Project State NH

Project City Portsmouth

Total Drainage Area, Ad	1.12 ac
Post Development Impervious Area, Ai	0.97 ac
Pervious Area, Ap	0.15 ac
% Impervious	87%
Runoff Coefficient, Rc	0.83

Mass Loading Calculations

Mean Annual Rainfall, P 50 in Agency Required % Removal 80%
Percent Runoff Capture 90%
Mean Annual Runoff, Vt 151752 ft³
Event Mean Concentration of Pollutant, EMC 75 mg/l Annual Mass Load, M total 710.10 lbs

Filter System

Filtration Brand

Cartridge Length

54 in

Jelly Fish Sizing

Mass to be Captured by System 568.08 lbs Water Quality Flow 0.95 cfs

Method to Use FLOW BASED

		Summary
Flour	Treatment Flow Rate	0.98 cfs
Flow	Required Size	JFPD0806-5-1



GENERAL CALCULATIONS - WQV and WQF (optional worksheet)

This worksheet may be useful when designing a BMP that does not fit into one of the specific worksheets already provided (i.e. for a technology which is not a stormwater wetland, infiltration practice, etc.)

Water Quality Volume (WQV)

0.31 ac	A = Area draining to the practice
0.23 ac	A _I = Impervious area draining to the practice
0.74 decimal	I = Percent impervious area draining to the practice, in decimal form
0.72 unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)
0.22 ac-in	WQV= 1" x Rv x A
815 cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")

Water Quality Flow (WQF)

1	inches	P = Amount of rainfall. For WQF in NH, $P = 1$ ".
0.72	inches	Q = Water quality depth. Q = WQV/A
97	unitless	CN = Unit peak discharge curve number. CN = $1000/(10+5P+10Q-10*[Q^2+1.25*Q*P]^{0.5})$
0.3	inches	S = Potential maximum retention. S = (1000/CN) - 10
0.059	inches	Ia = Initial abstraction. Ia = 0.2S
5.0	minutes	T _c = Time of Concentration
655.0	cfs/mi²/in	\textbf{q}_{u} is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III.
0.230	cfs	WQF = $q_u \times WQV$. Conversion: to convert "cfs/mi ² /in * ac-in" to "cfs" multiply by 1mi ² /640ac.

esigner's Notes:
is calculation represents the treatment train directed to the underground detention pond.
etreatment is accomplished by use a offline deep sump/hooded catch basins prior to entering the
derground detention structure.
eatment is achieved by use of the Jellyfish filter strucutre (JF-1). This treatment is represented
mperature mitigation is achieved by detaining WQV and dispersing through stone and underdrain.

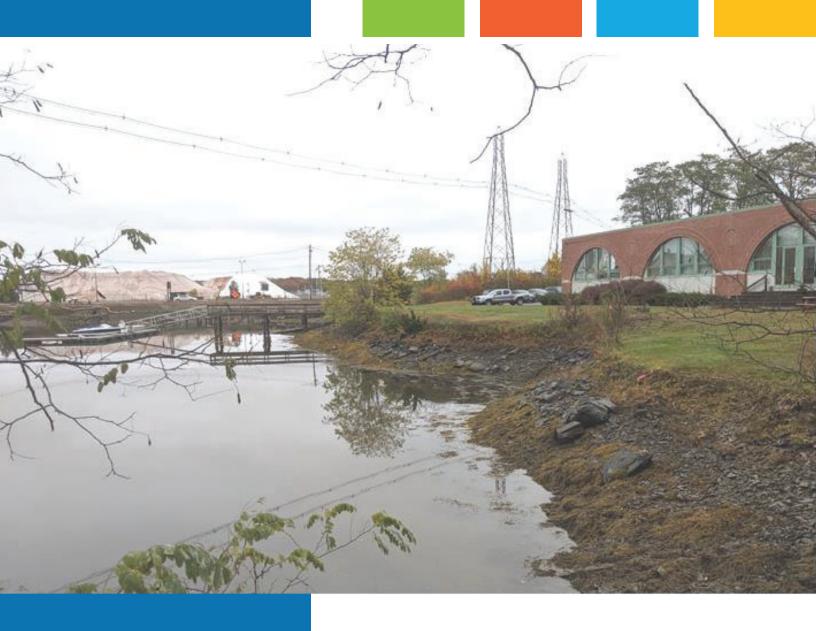
Prepared by Tighe & Bond HydroCAD® 10.00-20 s/n 03436 © 2017 HydroCAD Software Solutions LLC

Stage-Area-Storage for Pond POND 1: 310 STORMTECH CHAMBERS

Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
6.70	0	7.74	600	8.78	1,149
6.72	8	7.76	614	8.80	1,157
6.74	16	7.78	628	8.82	1,165
6.76	24	7.80	642	8.84	1,173
6.78	32	7.82	655	8.86	1,181
6.80	40	7.84	669	8.88	1,189
6.82	48	7.86	683	8.90	1,197
6.84	56	7.88	696	8.92	1,205
6.86	64	7.90	709	8.94	1,213
6.88	72	7.92	723	8.96	1,221
6.90	80	7.94	736	8.98	1,229
6.92	88	7.96	749	9.00	1,237
6.94	96	7.98	762	9.02	1,245
6.96	104	8.00	774	9.04	1,250
6.98	112	8.02	787	9.06	1,250
7.00	120	8.04	800	9.08	1,250
7.02	128	8.06	812	9.10	1,250
7.04	136	8.08	824	9.12	1,250
7.06	144	8.10	836	9.14	1,250
7.08	152	8.12	848	9.16	1,250
7.10	160	8.14	860	9.18	1,250
7.12	168	8.16	872	9.20	1,250
7.14	176	8.18	883	9.22	1,250
7.16	184	8.20	895	9.24	1,250
7.18	192	8.22	906	9.26	1,250
7.20	200	8.24	917	9.28	1,250
7.22	215	8.26	927	9.30	1,250
7.24	230	8.28	937	9.32	1,250
7.26	246	8.30	948	9.34	1,250
7.28	261	8.32	957	9.36	1,250
7.30	276	8.34	967		
7.32	292	8.36	976		
7.34	307	8.38	985		
7.36	322	8.40	994		
7.38	337	8.42	1,003		
7.40	352	8.44	1,012		
7.42	367	8.46	1,020		
7.44	382	8.48	1,028		
7.46 7.48	397 412	8.50 8.52	1,037		
7.40 7.50	427	8.54	1,045 1,053		
7.52	442	8.56	1,061		
7.54	457	8.58	1,069		
7.56	471	8.60	1,077		
7.58	486	8.62	1,085		
7.60	500	8.64	1,093		
7.62	515	8.66	1,101		
7.64	529	8.68	1,109		
7.66	544	8.70	1,117		
7.68	558	8.72	1,125		
7.70	572	8.74	1,123		
7.72	586	8.76	1,141		
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APPENDIX A



Proposed Mixed Use Development 53 Green Street, Portsmouth, NH

SITE SPECIFIC SOIL MAP REPORT

CPI Management, LLC March 2021





1.0 Introduction

This report is provided in conjunction with a 1.81 +/- acre Site Specific Soil Map (SSSM) prepared by Tighe & Bond for a parcel at 53 Green Street in Portsmouth, NH. The purpose of the mapping was to assist in the evaluation of drainage and other soil-related uses associated with site improvements, and may be used as part of an Alteration of Terrain (AoT) permit application.

2.0 Methods

Fieldwork for the soil mapping was completed October 22 and December 2, 2019 based on *Site-Specific Soil Mapping Standards for New Hampshire and Vermont, Version 5.0,* (Society of Soil Scientists of Northern New England [SSSNNE] Special Publication No. 3, December 2017). The poorly and very poorly drained soil types under this system are based on the most recent version of *Field Indicators for Identifying Hydric Soils in New England, Version 4* (New England Interstate Water Pollution Control Commission, 2018).

The soil legend for this map is based on the soil series currently mapped in the State of New Hampshire as published in the New Hampshire State-Wide Numerical Soils Legend (USDA Natural Resources Conservation Service, Issue #10, 2011). Since this soil map includes disturbed soils and may be used for an AoT application, the map symbols are composed of two major parts separated by a forward slash (/). The first part of the soil symbol includes a numerical identifier from the state-wide soil legend, followed by a letter indicating the slope class (e.g., 299A). Slope class identifiers are as follows:

Α	0-3%	D	15-25%
В	3-8%	Е	25-50%
С	8-15%	F	>50%

The second part of the symbol is based on the SSSNNE Disturbed Soil Supplemental Symbols, which are included within the Site Specific Soil Map (SSSM) standards. This portion of the symbol translates as follows:

Character 1: Drainage Class

a-Excessively Drained

b-Somewhat Excessively Drained

c-Well Drained

d-Moderately Well Drained

e-Somewhat Poorly Drained

f-Poorly Drained

g-Very Poorly Drained

h-Not Determined

Character 2: Parent Material (of naturally formed soil only, if present)

- a-No natural soil within 60 inches
- b-Glaciofluvial deposits (outwash/terraces of sand or sand and gravel)
- c-Glacial till material (active ice)
- d-Glaciolacustrine very fine sand and silt deposits (glacial lakes)
- e-Loamy/sandy over silt/clay deposits
- f-Marine silt and clay deposits (ocean waters)
- g-Alluvial deposits (floodplains)
- h-Organic materials-fresh water wetlands
- i-Organic materials-tidal wetlands

Character 3: Restrictive Properties

- a-None
- b-Bouldery surface with more than 15% of the surface covered with boulders
- c-Mineral restrictive layer(s) are present in the soil profile less than 40 inches below the soil surface such as hard pan, platy structure or clayey texture with consistence of at least firm (i.e. more than 20 newtons).
- d-Bedrock in the soil profile; 0-20 inches
- e-Bedrock in the soil profile; 20-60 inches
- f-Areas where depth to bedrock is so variable that a single soil type cannot be applied, will be mapped as a complex of soil types
- g-Subject to flooding
- h-Manufactured impervious surface including pavement, concrete, or built-up surfaces (e.g. buildings) with no morphological restrictive layer within control section

Character 4: Estimated Ksat (most limiting layer excluding symbol 3h above)

- a-High
- b-Moderate
- c-Low
- d-Not determined *See "Guidelines for Ksat Class Placement" in Chapter 3 of the Soil Survey Manual, USDA

Character 5: Hydrologic Soil Group

a-Group A

b-Group B

c-Group C

d-Group D

e-Not determined

SSSM report standards require estimates of the maximum size of *limiting* inclusions for the entire soil map and an estimate of the percentage of *dissimilar* inclusions within each map unit. *Limiting* inclusions are soils "...that differ appreciably in one or more soil properties from the named soil in a map unit. The difference in soil properties is more restrictive and may affect use and management." *Dissimilar* inclusions are "...soils that either do not share limits of some important diagnostic properties of the named taxon, or, in the professional judgment of the soil scientist, have different use or management requirements." The maximum size of any limiting inclusions in this soil map is estimated to be less than 2,000 square feet. Any dissimilar inclusions noted during the mapping are listed below within the map unit descriptions.

3.0 Site Features

The parcel is a highly disturbed site along the North Mill Pond. The property shows evidence of what appears to be very old filling and grading associated with the existing development.

4.0 Soil Map Unit Descriptions

Below are descriptions for the map unit found on the accompanying SSSM. The "*" after the numerical map unit symbol represents a placeholder for the slope class indicators described above.

100*/cfabb—Udorthents, wet substratum

Landscape Setting: Soils that have been filled over what was originally hydric soils

Drainage Class: Well drained

Parent Material: Fill over marine silts and clays at <60 inches.

Typical Textures: Gravelly sandy loam fill

Hydrologic Soil Group: B

Dissimilar Inclusions: None noted

<u>Limiting Inclusions:</u> Upper slopes along the shore are steeper than the mapped unit and are affected by tidal inundation. These areas comprise less than 10% of the unit

<u>Additional Notes:</u> Soils in these areas have properties that are similar to the Charlton soil series for Hydrologic Soil Group determination

100*/dfabb-Udorthents, wet substratum

<u>Landscape Setting:</u> Soils that have been filled and leveled over what was originally hydric soils

<u>Drainage Class:</u> Moderately well drained

<u>Parent Material</u>: Fill over marine silts and clays at <60 inches.

Typical Textures: Very gravelly sandy loam fill

Hydrologic Soil Group: B

Dissimilar Inclusions: None noted

<u>Limiting Inclusions:</u> Slopes along the shore are steeper than the mapped unit and are affected by tidal inundation. These areas comprise less than 10% of the unit

<u>Additional Notes:</u> Soils in these areas have properties that are similar to the Sutton soil series for Hydrologic Soil Group determination

100*/ffccc-Udorthents, wet substratum

Landscape Setting: Soils that have been filled over what was originally hydric soils

Drainage Class: Poorly drained

Parent Material: Fill over marine silts and clays at <60 inches.

Typical Textures: Gravelly and cobbly sandy loam fill with some anthropogenic debris,

such as bricks, over silt loam

Hydrologic Soil Group: C

<u>Dissimilar Inclusions:</u> None noted Limiting Inclusions: None noted

<u>Additional Notes:</u> Soils in these areas have properties that are similar to the Shaker soil series for Hydrologic Soil Group determination. These soils are regularly inundated by the tides.

Site Specific Soil Map Legend

53 Green Street, Portsmouth, NH

Slope	<u>Class</u>	Identif	<u>iers</u>

	010 p 0 010101	, = 41 411 411	
Α	0-3%	D	15-25%
В	3-8%	Е	25-50%
С	8-15%	F	>50%

Map Unit Symbols

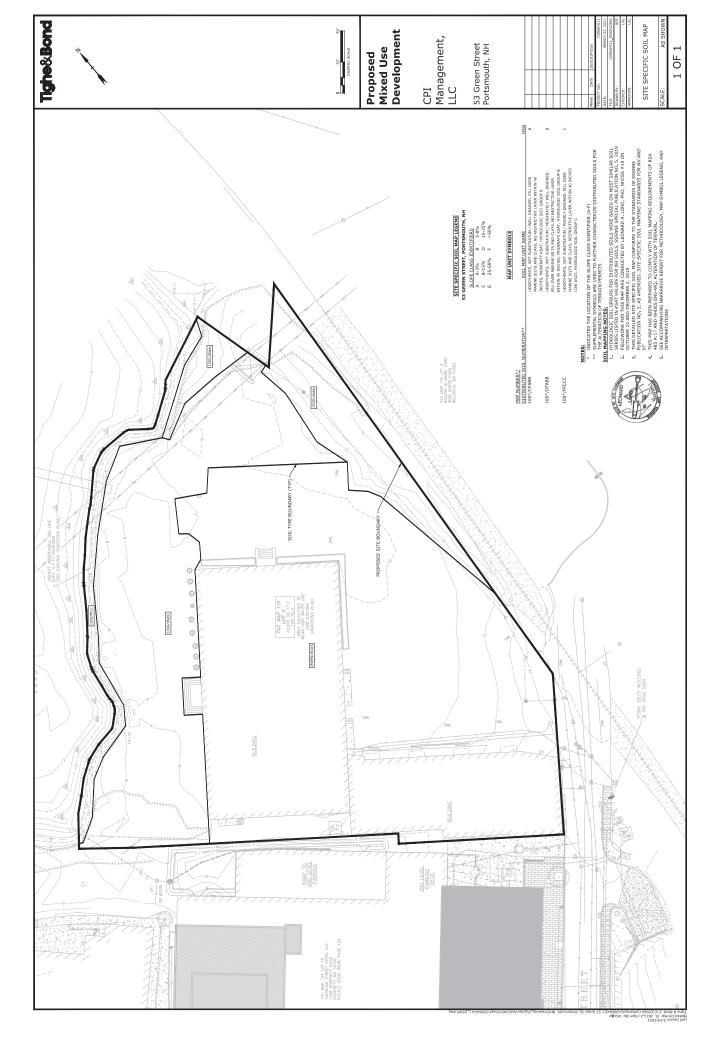
Map Number* / <u>Disturbed Soil</u> Numerator**	Soil Map Unit Name	Hydrologic Soil Group
100*/cfabb	Udorthents, wet substratum / well drained, fill over marine silts and clays, no restrictive layer within 40 inches, moderate Ksat, Hydrologic Soil Group B	В
100*/dfabb	Udorthents, wet substratum, 0-3% slopes / moderately well drained, fill over marine silts and clays, no restrictive layer within 40 inches, moderate Ksat, Hydrologic Soil Group B	В
100*/ffccc	Udorthents, wet substratum, 0-3% slopes / poorly drained, fill over marine silts and clays, restrictive layer is present within 40 inches, low Ksat, Hydrologic Soil Group C	С

^{*}Indicates the location of the slope class identifier (A-F)

Soil Mapping Notes:

- 1. Hydrologic soil groups for disturbed soils were based on most similar soil series listed in *Ksat Values for NH Soils*, SSSNNE Special Publication No. 5, 2009.
- 2. Fieldwork for this map was conducted by Leonard A. Lord, PhD, NHCSS #19 on October 22 and December 2, 2019.
- 3. This detailed Site Specific Soil Map conforms to the standards of SSSNNE Publication No. 3, as amended, Site Specific Soil Mapping Standards for NH and VT.
- 4. This map has been prepared to comply with soil mapping requirements of RSA 485 A:17 and NHDES Env-Wq, Alteration of Terrain.
- 5. See accompanying narrative report for methodology, map symbol legend, and interpretations.

^{**}Supplemental symbols are used to further characterize disturbed soils for Alteration of Terrain permits



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APPENDIX B

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing Yes

State New Hampshire

Location

Longitude 70.764 degrees West **Latitude** 43.080 degrees North

Elevation 0 feet

Date/Time Fri, 24 Jul 2020 12:23:19 -0400

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.04	1yr	0.70	0.98	1.21	1.56	2.03	2.65	2.92	1yr	2.35	2.81	3.22	3.94	4.54	1yr
2yr	0.32	0.50	0.62	0.81	1.02	1.30	2yr	0.88	1.18	1.52	1.94	2.48	3.20	3.57	2yr	2.84	3.43	3.93	4.67	5.32	2yr
5yr	0.37	0.58	0.73	0.97	1.25	1.61	5yr	1.08	1.47	1.89	2.43	3.14	4.06	4.57	5yr	3.59	4.40	5.03	5.93	6.69	5yr
10yr	0.41	0.65	0.82	1.11	1.45	1.89	10yr	1.25	1.72	2.23	2.89	3.74	4.86	5.52	10yr	4.30	5.31	6.07	7.09	7.96	10yr
25yr	0.48	0.76	0.97	1.33	1.77	2.33	25yr	1.53	2.14	2.77	3.62	4.73	6.16	7.09	25yr	5.45	6.81	7.78	9.00	10.03	25yr
50yr	0.53	0.86	1.10	1.53	2.07	2.75	50yr	1.78	2.52	3.28	4.31	5.65	7.37	8.57	50yr	6.53	8.24	9.40	10.79	11.95	50yr
100yr	0.59	0.96	1.24	1.76	2.41	3.25	100yr	2.08	2.97	3.90	5.15	6.75	8.83	10.36	100yr	7.82	9.96	11.35	12.93	14.24	100yr
200yr	0.67	1.10	1.42	2.04	2.82	3.82	200yr	2.43	3.51	4.60	6.11	8.06	10.58	12.52	200yr	9.37	12.04	13.71	15.50	16.98	200yr
500yr	0.80	1.31	1.71	2.48	3.47	4.75	500yr	2.99	4.37	5.75	7.68	10.19	13.45	16.11	500yr	11.90	15.49	17.61	19.72	21.44	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.73	0.88	1yr	0.63	0.86	0.92	1.33	1.68	2.23	2.48	1yr	1.97	2.39	2.86	3.18	3.88	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.16	1.37	1.82	2.34	3.05	3.45	2yr	2.70	3.31	3.82	4.54	5.07	2yr
5yr	0.35	0.54	0.67	0.92	1.17	1.40	5yr	1.01	1.37	1.61	2.12	2.73	3.78	4.18	5yr	3.34	4.02	4.71	5.52	6.23	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.56	1.80	2.39	3.06	4.36	4.85	10yr	3.86	4.66	5.42	6.39	7.17	10yr
25yr	0.44	0.67	0.83	1.18	1.56	1.90	25yr	1.34	1.86	2.10	2.76	3.54	4.70	5.87	25yr	4.16	5.64	6.62	7.76	8.65	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.17	50yr	1.52	2.12	2.34	3.07	3.93	5.31	6.77	50yr	4.70	6.51	7.68	9.00	9.98	50yr
100yr	0.53	0.81	1.01	1.46	2.00	2.47	100yr	1.73	2.41	2.62	3.42	4.35	5.96	7.81	100yr	5.28	7.51	8.92	10.45	11.52	100yr
200yr	0.59	0.89	1.12	1.63	2.27	2.81	200yr	1.96	2.75	2.93	3.79	4.79	6.68	9.01	200yr	5.91	8.66	10.34	12.15	13.31	200yr
500yr	0.68	1.02	1.31	1.90	2.70	3.36	500yr	2.33	3.28	3.41	4.32	5.46	7.76	10.87	500yr	6.87	10.45	12.58	14.86	16.11	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.54	0.72	0.89	1.08	1yr	0.77	1.06	1.26	1.74	2.21	2.98	3.16	1yr	2.64	3.04	3.58	4.37	5.04	1yr
2yr	0.34	0.52	0.64	0.86	1.07	1.27	2yr	0.92	1.24	1.48	1.96	2.52	3.42	3.70	2yr	3.03	3.56	4.08	4.83	5.62	2yr
5yr	0.40	0.62	0.76	1.05	1.34	1.62	5yr	1.15	1.58	1.88	2.53	3.25	4.33	4.96	5yr	3.84	4.77	5.37	6.37	7.15	5yr
10yr	0.47	0.72	0.89	1.24	1.61	1.97	10yr	1.39	1.93	2.28	3.11	3.95	5.33	6.20	10yr	4.72	5.96	6.82	7.83	8.74	10yr
25yr	0.57	0.87	1.09	1.55	2.04	2.57	25yr	1.76	2.51	2.95	4.07	5.15	7.77	8.34	25yr	6.88	8.02	9.15	10.33	11.40	25yr
50yr	0.67	1.02	1.27	1.82	2.46	3.12	50yr	2.12	3.05	3.59	5.00	6.32	9.73	10.46	50yr	8.62	10.06	11.45	12.71	13.95	50yr
100yr	0.79	1.19	1.49	2.15	2.95	3.80	100yr	2.55	3.72	4.37	6.15	7.76	12.18	13.11	100yr	10.78	12.61	14.32	15.68	17.08	100yr
200yr	0.92	1.39	1.76	2.54	3.55	4.64	200yr	3.06	4.54	5.33	7.58	9.53	15.29	16.45	200yr	13.53	15.82	17.94	19.34	20.91	200yr
500yr	1.14	1.70	2.19	3.18	4.52	6.02	500yr	3.90	5.89	6.92	10.01	12.54	20.67	22.22	500yr	18.29	21.37	24.18	25.50	27.33	500yr



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APPENDIX C



Examination of Thermal Impacts from Stormwater BMPs



In a study in Durham, New Hampshire, four years of runoff temperature data were examined for a range of stormwater best management practices (BMPs) in relation to established environmental indicators.

The stormwater BMPs examined included:

Conventional

- Vegetated Swale
- Detention Pond
- Retention Pond

Low Impact Development

- Bioretention
- Gravel Wetland

Manufactured Treatment Devices

- Storm Tech Isolator Row
- ADS Infiltration System
- Hydrodynamic Separator

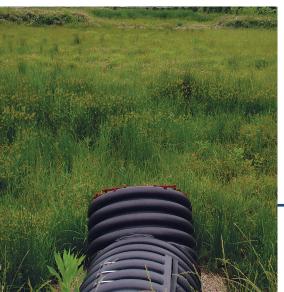


Surface systems that are exposed to direct sunlight have been shown to increase already elevated summer runoff temperatures, while systems that provide treatment by infiltration and filtration can moderate runoff temperatures by thermal exchange with cool subsurface materials.

The storm drain system in this study had an annual average event mean temperature (EMT) greater than the mean groundwater temperature of 47°F that commonly feeds coldwater streams.

The examination of BMPs indicates that outflow from the larger surface systems is warmer and more variable than from parking lots. The filtration and infiltration systems cooled stormwater runoff to temperatures close to groundwater temperature.

Top: A view of a healthy coldwater fishery. Center: Large parking areas store tremendous amounts of heat which is transferred into stormwater runoff. Bottom: Subsurface treatment systems such as gravel wetlands can buffer temperature impacts for stormwater runoff.



Thermal Extremes

The summer temperatures of the two stormwater ponds, vegetated swale, and HDS (Hydrodynamic Separators) systems, indicate that they provide little to no reduction of high runoff temperatures.

The Retention and Detention ponds have the largest variation in temperature.

The Retention Pond is the only system to exceed both the Upper Optimum Limit (UOL) and the Lethal Limit of 80°F, however, the Detention Pond with a maximum temperature of 79.4°F comes very close.

The permanent pool of water in the Retention Pond appears to act as a heat sink during periods of extreme heat.

FILTRATION & INFILTRATION SYSTEMS:

Thermal Buffers

Filtration and infiltration systems showed the strongest ability to reduce temperature variations.

The gravel wetland, the ADS (Advanced Drainage Systems™)

Infiltration System, and the StormTech Isolator Row have a strong capacity to reduce temperatures of runoff.

The Bioretention system showed minor buffering capacity and was consistently cooler in the summer and warmer in the winter than the runoff. These filtration and infiltration

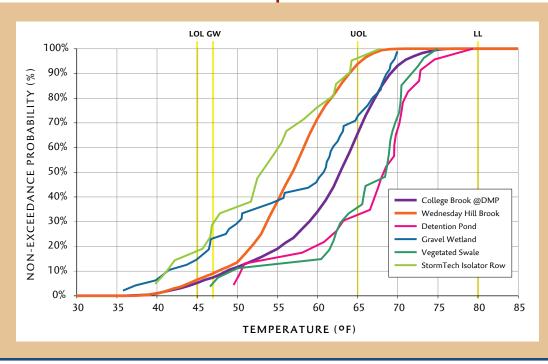


StormTech Isolator Row.

systems are, on average, reducing the summer temperatures and increasing the winter temperatures of the runoff to near the average groundwater temperature of 47°F.

The two subsurface infiltration systems, ADS and STIR, are the only systems with mean July temperatures within the optimum zone of 45°F to 65°F for coldwater aquatic species. All other systems result in runoff within the stress zone for aquatic species, between 65°F and 80°F.

The Gravel Wetland, the ADS infiltration system, and the Isolator Row systems have the lowest exceedance values of the UOL at 13.0%, 5.0%, 1.5% respectively.



Comparison of summer temperatures for two streams:
Wednesday Hill Brook (unimpacted) and College Brook (impacted); a wet and dry pond, a gravel wetland, and subsurface infiltration (Stormtech Isolator Row) with environmental indicators for cold water fisheries:

Average Annual Groundwater Temperature (GW) = 47°F

Lower Optimum Limit (LOL) = $45^{\circ}F$ Upper Optimum Limit (UOL) = $65^{\circ}F$ Lethal Limit (LL) = $80^{\circ}F$

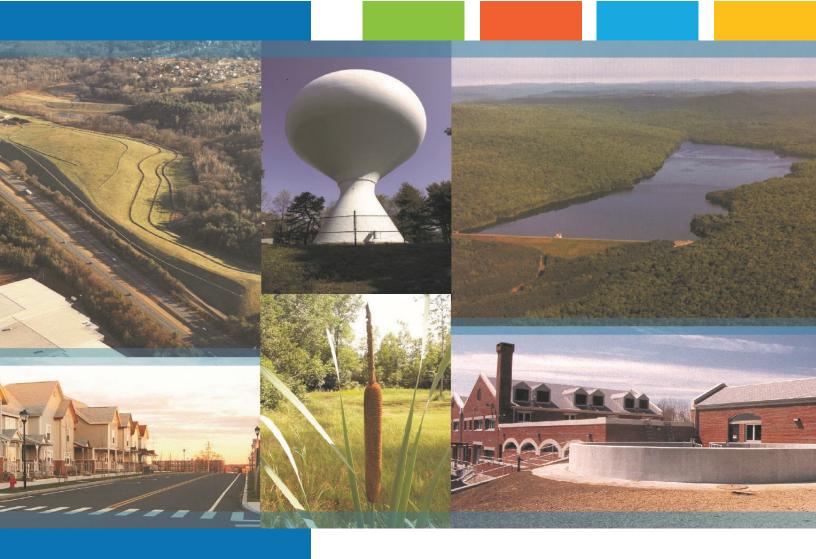












Proposed Mixed Use Development 53 Green Street Portsmouth, NH

Long-Term Operation & Maintenance Plan

CPI Management, LLC

May 19, 2021





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Section 1 Long-Term Operation & Maintenance Plan

It is the intent of this Operation and Maintenance Plan to identify the areas of this site that need special attention and consideration, as well as implementing a plan to assure routine maintenance. By identifying the areas of concern as well as implementing a frequent and routine maintenance schedule the site will maintain a high-quality stormwater runoff.

1.1 Contact/Responsible Party

Maintenance Area	Contact/Responsible Party	
Map 119 Lot 2	CPI Management, LLC 100 Summer Street, Suite 1600 Boston, MA 02110	
North Mill Pond Trail (City Easement)	City of Portsmouth DPW 680 Peverly Hill Road Portsmouth, NH 03801	

(Note: The contact information for the Contact/Responsible Party shall be kept current. If ownership changes, the Operation and Maintenance Plan must be transferred to the new party.)

1.2 Maintenance Items

Maintenance of the following items shall be recorded:

- Litter/Debris Removal
- Landscaping
- Catchbasin Cleaning
- Pavement Sweeping
- ADS Stormtech System with Isolator Row
- Contech Jellyfish Filtration System
- Porous Pavement

The following maintenance items and schedule represent the minimum action required. Periodic site inspections shall be conducted, and all measures must be maintained in effective operating condition. The following items shall be observed during site inspection and maintenance:

- Inspect vegetated areas, particularly slopes and embankments for areas of erosion. Replant and restore as necessary
- Inspect catch basins for sediment buildup
- Inspect site for trash and debris

1.3 Overall Site Operation & Maintenance Schedule

Maintenance Item	Frequency of Maintenance	Responsible Party
Litter/Debris Removal	Weekly	CPI Management, LLC
Pavement Sweeping	Bi-annually	CPI Management,
- Sweep impervious areas to remove sand and litter.		LLC
Landscaping	Maintained as required and	CPI Management,
- Landscaped islands to be maintained and mulched.	mulched each Spring	LLC
Catch Basin (CB) Cleaning	Annually	CPI Management,
- CB to be cleaned of solids and oils.		LLC
Jelly Fish Units	In accordance with Manufacturer's Recommendations	CPI Management, LLC
ADS Stormtech System with Isolator Row	In accordance with Manufacturer's	CPI Management, LLC
- Visual observation of sediment levels within system	Recommendations	
Porous Pavement	Bi-Annually	City of Portsmouth
- Clean using a vacuum sweeper		DPW

1.3.1 Disposal Requirements

Disposal of debris, trash, sediment and other waste material should be done at suitable disposal/recycling sites and in compliance with all applicable local, state and federal waste regulations.

1.4 ADS Stormtech System with Isolator Row

ADS Stormtech System w/Isolator Row Inspection/Maintenance Requirements				
Inspection/	Frequency	Action		
Maintenance				
Monitor inlet and outlet structures for sediment accumulation	Two (2) times annually	Trash, debris and sediment to be removedAny required maintenance shall be addressed		
Inspect Isolator Row for sediment	6 months for the first year, then adjust based on previous observations	- Inspect inside the isolator row through inspection ports (if provided) or through the upstream structure.		
Jetting and Vacuuming	Annually or as required by inspection.	 If sediment is 3" or above, then clean out isolator row using the jetvac process. Vacuum structure sump as required. 		



Isolator® Row O&M Manual









THE ISOLATOR® ROW

INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a technique to inexpensively enhance Total Suspended Solids (TSS) removal and provide easy access for inspection and maintenance.

THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-160LP, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC- 310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

Two different fabrics are used for the Isolator Row. A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The tough geotextile provides a media for storm water filtration and provides a durable surface for maintenance operations. It is also designed to prevent scour of the underlying stone and remain intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the SC-160LP, DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.

The Isolator Row is typically designed to capture the "first flush" and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole not only provides access to the Isolator Row but typically includes a high flow weir such that storm water flowrates or volumes that exceed the capacity of the Isolator Row overtop the over flow weir and discharge through a manifold to the other chambers.

The Isolator Row may also be part of a treatment train. By treating storm water prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured. Pre-treatment best management practices can be as simple as deep sump catch basins, oil-water separators or can be innovative storm water treatment devices. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

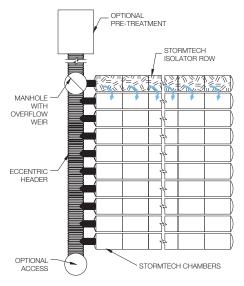
Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.



Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.



StormTech Isolator Row with Overflow Spillway (not to scale)





ISOLATOR ROW INSPECTION/MAINTENANCE

INSPECTION

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

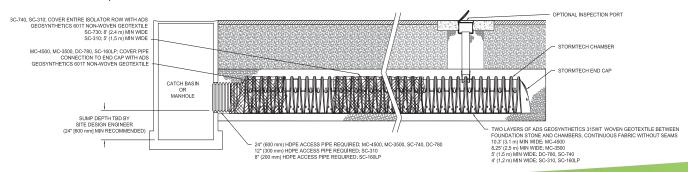
MAINTENANCE

The Isolator Row was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.

StormTech Isolator Row (not to scale)

Note: Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-4500 chamber models and is not required over the entire Isolator Row.





ISOLATOR ROW STEP BY STEP MAINTENANCE PROCEDURES

STEP 1

Inspect Isolator Row for sediment.

- A) Inspection ports (if present)
 - i. Remove lid from floor box frame
 - ii. Remove cap from inspection riser
 - iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
 - iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
- B) All Isolator Rows
 - i. Remove cover from manhole at upstream end of Isolator Row
 - ii. Using a flashlight, inspect down Isolator Row through outlet pipe
 - 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 - 2. Follow OSHA regulations for confined space entry if entering manhole
 - iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2. If not, proceed to Step 3.

STEP 2

Clean out Isolator Row using the JetVac process.

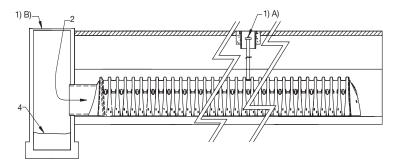
- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

STEP 3

Replace all caps, lids and covers, record observations and actions.

STEP 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



SAMPLE MAINTENANCE LOG

	Stadia Rod Readings		Sediment Depth		
Date	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)	(1)-(2)	Observations/Actions	Inspector
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	MCG
9/24/11		6.2	0.1 ft	Some grit felt	SM
6/20/13		5,8	0.5 ft	Mucky feel, debris visible in manhole and in Isolator Row, maintenance due	Ν
7/7/13	6.3 ft		0	System jetted and vacuumed	MCG





1.5 Contech Jellyfish Filter System Maintenance Requirements

Contech Jellyfish Filter System Inspection/Maintenance Requirements							
Inspection/	Frequency	Action					
Maintenance							
Inspect vault for sediment build up, static water, plugged media and bypass condition	One (1) time annually and after any rainfall event exceeding 2.5" in a 24-hr period	Maintenance required for any of the following: - >4" of sediment on the vault floor - >1/4" of sediment on top of the cartridge4" of static water above the cartridge bottom more than 24 hours after a rain event - If pore space between media is absent If vault is in bypass condition during an average rainfall event.					
Replace Cartridges	As required by inspection, 1–5 years.	 Remove filter cartridges per manufacturer methods. Vacuum sediment from vault. Install new cartridges per manufacturer methods 					



Jellyfish® Filter Owner's Manual



Jellyfish® Filter

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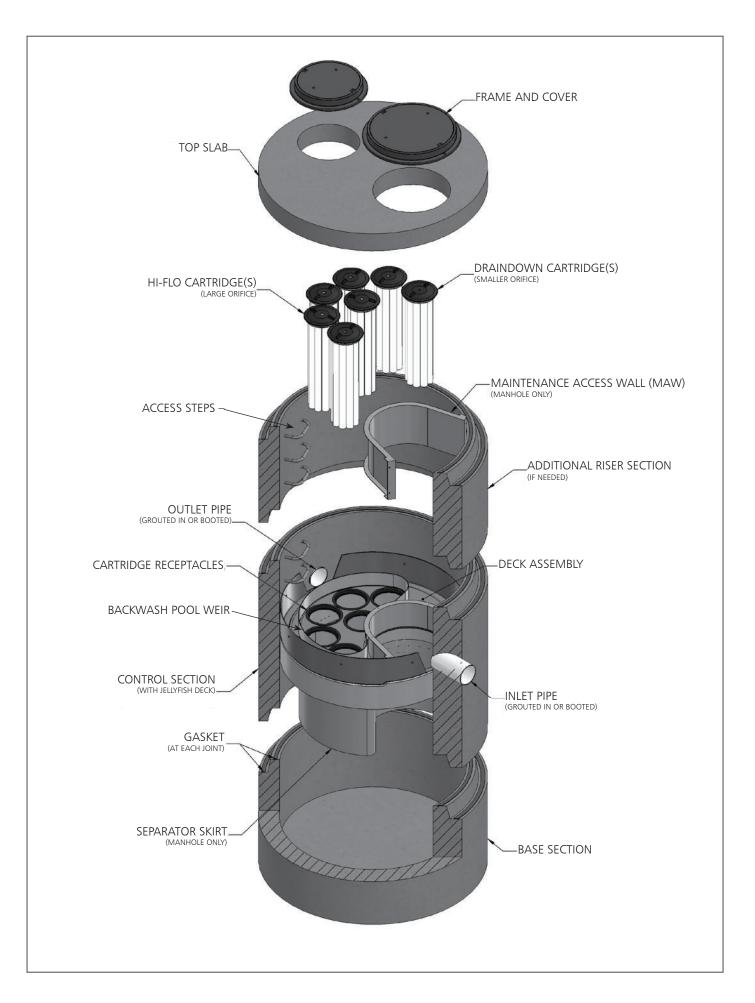
THANK YOU FOR PURCHASING THE JELLYFISH® FILTER!

Contech Engineered Solutions would like to thank you for selecting the Jellyfish Filter to meet your project's stormwater treatment needs. With proper inspection and maintenance, the Jellyfish Filter is designed to deliver ongoing, high levels of stormwater pollutant removal.

If you have any questions, please feel free to call us or e-mail us:

Contech Engineered Solutions

9025 Centre Pointe Drive, Suite 400 | West Chester, OH 45069 513-645-7000 | 800-338-1122 www.ContechES.com info@conteches.com



WARNINGS / CAUTION

- 1. FALL PROTECTION may be required.
- 2. WATCH YOUR STEP if standing on the Jellyfish Filter Deck at any time; Great care and safety must be taken while walking or maneuvering on the Jellyfish Filter Deck. Attentive care must be taken while standing on the Jellyfish Filter Deck at all times to prevent stepping onto a lid, into or through a cartridge hole or slipping on the deck.
- 3. The Jellyfish Filter Deck can be SLIPPERY WHEN WET.
- 4. If the Top Slab, Covers or Hatches have not yet been installed, or are removed for any reason, great care must be taken to NOT DROP ANYTHING ONTO THE JELLYFISH FILTER DECK. The Jellyfish Filter Deck and Cartridge Receptacle Rings can be damaged under high impact loads. This type of activity voids all warranties. All damaged items to be replaced at owner's expense.
- 5. Maximum deck load 2 persons, total weight 450 lbs.

Safety Notice

Jobsite safety is a topic and practice addressed comprehensively by others. The inclusions here are intended to be reminders to whole areas of Safety Practice that are the responsibility of the Owner(s), Manager(s) and Contractor(s). OSHA and Canadian OSH, and Federal, State/Provincial, and Local Jurisdiction Safety Standards apply on any given site or project. The knowledge and applicability of those responsibilities is the Contractor's responsibility and outside the scope of Contech Engineered Solutions.

Confined Space Entry

Secure all equipment and perform all training to meet applicable local and OSHA regulations regarding confined space entry. It is the Contractor's or entry personnel's responsibility to proceed safely at all times.

Personal Safety Equipment

Contractor is responsible to provide and wear appropriate personal protection equipment as needed including, but not limited to safety boots, hard hat, reflective vest, protective eyewear, gloves and fall protection equipment as necessary. Make sure all equipment is staffed with trained and/or certified personnel, and all equipment is checked for proper operation and safety features prior to use.

- Fall protection equipment
- Eye protection
- Safety boots
- Ear protection
- Gloves
- Ventilation and respiratory protection
- Hard hat
- Maintenance and protection of traffic plan

Chapter 1

1.0 - Owner Specific Jellyfish Filter Product Information

Below you will find a reference page that can be filled out according to your Jellyfish Filter specification to help you easily inspect, maintain and order parts for your system.

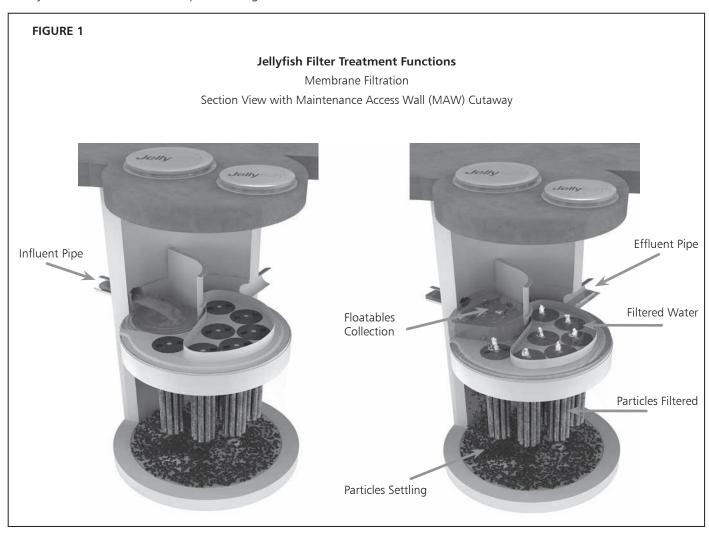
Owner Name:	
Phone Number:	
Site Address:	
Site GPS Coordinates/unit location:	
Unit Location Description:	
Jellyfish Filter Model No.:	
Contech Project & Sequence Number	
No. of Hi-Flo Cartridges	
No. of Cartridges:	
Length of Draindown Cartridges:	
No. of Blank Cartridge Lids:	
Bypass Configuration (Online/Offline):	
Notes:	

Chapter 2

2.0 – Jellyfish Filter System Operations and Functions

The Jellyfish Filter is an engineered stormwater quality treatment technology that removes a high level and wide variety of stormwater pollutants. Each Jellyfish Filter cartridge consists of eleven membrane - encased filter elements ("filtration tentacles") attached to a cartridge head plate. The filtration tentacles provide a large filtration surface area, resulting in high flow and high pollutant removal capacity.

The Jellyfish Filter functions are depicted in Figure 1 below.

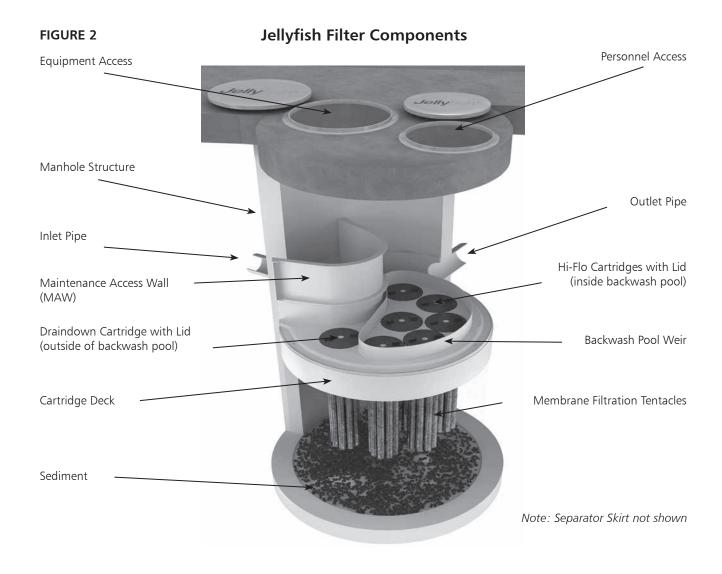


Jellyfish Filter cartridges are backwashed after each peak storm event, which removes accumulated sediment from the membranes. This backwash process extends the service life of the cartridges and increases the time between maintenance events.

For additional details on the operation and pollutant capabilities of the Jellyfish Filter please refer to additional details on our website at www.ContechES.com.

2.1 - Components and Cartridges

The Jellyfish Filter and components are depicted in Figure 2 below.



Tentacles are available in various lengths as depicted in Table 1 below.

Table 1 – Cartridge Lengths / Weights and Cartridge Lid Orifice Diameters

Cartridge Lengths	Cartridge Lengths Dry Weight		Draindown Orifice Diameter
15 inches (381 mm)	10 lbs (4.5 kg)	35 mm	20 mm
27 inches (686 mm)	14.5 lbs (6.6 kg)	45 mm	25 mm
40 inches (1,016 mm)	19.5 lbs (8.9 kg)	55 mm	30 mm
54 inches (1,372 mm)	25 lbs (11.4 kg)	70 mm	35 mm

2.2 - Jellyfish Membrane Filtration Cartridge Assembly

The Jellyfish Filter utilizes multiple membrane filtration cartridges. Each cartridge consists of removable cylindrical filtration "tentacles" attached to a cartridge head plate. Each filtration tentacle has a threaded pipe nipple and o-ring. To attach, insert the top pipe nipples with the o-ring through the head plate holes and secure with locking nuts. Hex nuts to be hand tightened and checked with a wrench as shown below.

2.3 – Jellyfish Membrane Filtration Cartridge Installation

- Cartridge installation will be performed by trained individuals and coordinated with the installing site Contractor. Flow diversion devices are required to be in place until the site is stabilized (final paving and landscaping in place). Failure to address this step completely will reduce the time between required maintenance.
- Descend to the cartridge deck (see Safety Notice and page 3).
- Refer to Contech's submittal drawings to determine proper quantity and placement of Hi-Flo, Draindown and Blank cartridges with appropriate lids. Lower the Jellyfish membrane filtration cartridges into the cartridge receptacles within the cartridge deck. It is possible that not all cartridge receptacles will be filled with a filter cartridge. In that case, a blank headplate and blank cartridge lid (no orifice) would be installed.



Cartridge Assembly

Do not force the tentacles down into the cartridge receptacle, as this may damage the membranes. Apply downward pressure on the cartridge head plate to seat the lubricated rim gasket (thick circular gasket surrounding the circumference of the head plate) into the cartridge receptacle. (See Figure 3 for details on approved lubricants for use with rim gasket.)

- Examine the cartridge lids to differentiate lids with a small orifice, a large orifice, and no orifice.
 - Lids with a <u>small orifice</u> are to be inserted into the <u>Draindown cartridge receptacles</u>, outside of the backwash pool weir.
 - Lids with a <u>large orifice</u> are to be inserted into the <u>Hi-Flo cartridge receptacles</u> within the backwash pool weir.
 - Lids with <u>no orifice</u> (blank cartridge lids) and a <u>blank headplate</u> are to be inserted into unoccupied cartridge receptacles.
- To install a cartridge lid, align both cartridge lid male threads with the cartridge receptacle female threads before rotating approximately 1/3 of a full rotation until firmly seated. Use of an approved rim gasket lubricant may facilitate installation.

3.0 Inspection and Maintenance Overview

The primary purpose of the Jellyfish® Filter is to capture and remove pollutants from stormwater runoff. As with any filtration system, these pollutants must be removed to maintain the filter's maximum treatment performance. Regular inspection and maintenance are required to insure proper functioning of the system.

Maintenance frequencies and requirements are site specific and vary depending on pollutant loading. Additional maintenance activities may be required in the event of non-storm event runoff, such as base-flow or seasonal flow, an upstream chemical spill or due to excessive sediment loading from site erosion or extreme runoff events. It is a good practice to inspect the system after major storm events.

Inspection activities are typically conducted from surface observations and include:

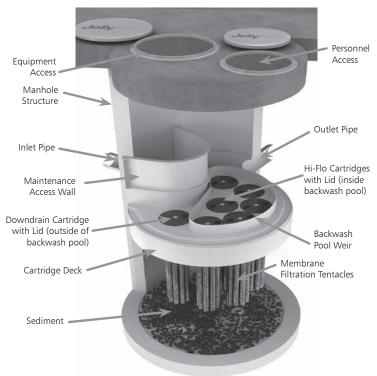
- Observe if standing water is present
- Observe if there is any physical damage to the deck or cartridge lids
- Observe the amount of debris in the Maintenance Access Wall (MAW) or inlet bay for vault systems

Maintenance activities include:

- Removal of oil, floatable trash and debris
- Removal of collected sediments
- Rinsing and re-installing the filter cartridges
- Replace filter cartridge tentacles, as needed

4.0 Inspection Timing

Inspection of the Jellyfish Filter is key in determining the maintenance requirements for, and to develop a history of, the site's pollutant loading characteristics. In general, inspections should be performed at the times indicated below; or per the approved project stormwater quality documents (if applicable), whichever is more frequent.



Note: Separator Skirt not shown

- A minimum of quarterly inspections during the first year of operation to assess the sediment and floatable pollutant accumulation, and to ensure proper functioning of the system.
- 2. Inspection frequency in subsequent years is based on the inspection and maintenance plan developed in the first year of operation. Minimum frequency should be once per year.
- 3. Inspection is recommended after each major storm event.
- Inspection is required immediately after an upstream oil, fuel or other chemical spill.

5.0 Inspection Procedure

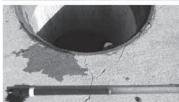
The following procedure is recommended when performing inspections:

- 1. Provide traffic control measures as necessary.
- 2. Inspect the MAW or inlet bay for floatable pollutants such as trash, debris, and oil sheen.
- Measure oil and sediment depth in several locations, by lowering a sediment probe until contact is made with the floor of the structure. Record sediment depth, and presences of any oil layers.
- Inspect cartridge lids. Missing or damaged cartridge lids to be replaced.
- Inspect the MAW (where appropriate), cartridge deck and receptacles, and backwash pool weir, for damaged or broken components.

5.1 Dry weather inspections

- Inspect the cartridge deck for standing water, and/or sediment on the deck.
- No standing water under normal operating conditions.
- Standing water inside the backwash pool, but not outside the backwash pool indicates, that the filter cartridges need to be rinsed.





Inspection Utilizing Sediment Probe

- Standing water outside the backwash pool is not anticipated and may indicate a backwater condition caused by high water elevation in the receiving water body, or possibly a blockage in downstream infrastructure.
- Any appreciable sediment (≥1/16") accumulated on the deck surface should be removed.

5.2 Wet weather inspections

- Observe the rate and movement of water in the unit.
 Note the depth of water above deck elevation within the MAW or inlet bay.
- Less than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges (i.e. cartridges located outside the backwash pool).
- Greater than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges and each of the hi-flo cartridges (i.e. cartridges located inside the backwash pool), and water should be overflowing the backwash pool weir.
- 18 inches or greater and relatively little flow is exiting the cartridge lids and outlet pipe, this condition indicates that the filter cartridges need to be rinsed.

6.0 Maintenance Requirements

Required maintenance for the Jellyfish Filter is based upon results of the most recent inspection, historical maintenance records, or the site specific water quality management plan; whichever is more frequent. In general, maintenance requires some combination of the following:

- Sediment removal for depths reaching 12 inches or greater, or within 3 years of the most recent sediment cleaning, whichever occurs sooner.
- 2. Floatable trash, debris, and oil removal.
- 3. Deck cleaned and free from sediment.
- 4. Filter cartridges rinsed and re-installed as required by the most recent inspection results, or within 12 months of the most recent filter rinsing, whichever occurs sooner.
- 5. Replace tentacles if rinsing does not restore adequate hydraulic capacity, remove accumulated sediment, or if damaged or missing. It is recommended that tentacles should remain in service no longer than 5 years before replacement.
- Damaged or missing cartridge deck components must be repaired or replaced as indicated by results of the most recent inspection.
- The unit must be cleaned out and filter cartridges inspected immediately after an upstream oil, fuel, or chemical spill.
 Filter cartridge tentacles should be replaced if damaged or compromised by the spill.

7.0 Maintenance Procedure

The following procedures are recommended when maintaining the Jellyfish Filter:

- 1. Provide traffic control measures as necessary.
- Open all covers and hatches. Use ventilation equipment as required, according to confined space entry procedures. Caution: Dropping objects onto the cartridge deck may cause damage.
- 3. Perform Inspection Procedure prior to maintenance activity.

- 4. To access the cartridge deck for filter cartridge service, descend into the structure and step directly onto the deck. Caution: Do not step onto the maintenance access wall (MAW) or backwash pool weir, as damage may result. Note that the cartridge deck may be slippery.
- 5. Maximum weight of maintenance crew and equipment on the cartridge deck not to exceed 450 lbs.

7.1 Filter Cartridge Removal

- 1. Remove a cartridge lid.
- Remove cartridges from the deck using the lifting loops in the cartridge head plate. Rope or a lifting device (available from Contech) should be used. Caution: Should a snag occur, do not force the cartridge upward as damage to the tentacles may result. Wet cartridges typically weigh between 100 and 125 lbs.
- 3. Replace and secure the cartridge lid on the exposed empty receptacle as a safety precaution. Contech does not recommend exposing more than one empty cartridge receptacle at a time.

7.2 Filter Cartridge Rinsing

- Remove all 11 tentacles from the cartridge head plate. Take care not to lose or damage the O-ring seal as well as the plastic threaded nut and connector.
- 2. Position tentacles in a container (or over the MAW), with the



threaded connector (open end) facing down, so rinse water is flushed through the membrane and captured in the container.

3. Using the Jellyfish rinse tool (available from Contech) or a low-pressure garden hose sprayer, direct water spray onto the tentacle membrane, sweeping from top to bottom along the length of the tentacle. Rinse until all sediment is removed from the membrane. Caution: Do not use a high pressure sprayer or focused stream of water on the membrane. Excessive water pressure may damage the membrane.

5. Reassemble cartridges as detailed later in this document. Reuse O-rings and nuts, ensuring proper placement on each tentacle.

7.3 Sediment and Flotables Extraction

- 1. Perform vacuum cleaning of the Jellyfish Filter only after filter cartridges have been removed from the system. Access the lower chamber for vacuum cleaning only through the maintenance access wall (MAW) opening. Be careful not to damage the flexible plastic separator skirt that is attached to the underside of the deck on manhole systems. Do not lower the vacuum wand through a cartridge receptacle, as damage to the receptacle will result.
- Vacuum floatable trash, debris, and oil, from the MAW opening or inlet bay. Alternatively, floatable solids may be removed by a net or skimmer.
- 3. Pressure wash cartridge deck and receptacles to remove all



Rinsing Cartridge with Contech Rinse Tool

sediment and debris. Sediment should be rinsed into the sump area. Take care not to flush rinse water into the outlet pipe.

- Remove water from the sump area. Vacuum or pump equipment should only be introduced through the MAW or inlet bay.
- Remove the sediment from the bottom of the unit through the MAW or inlet bay opening.
- 6. For larger diameter Jellyfish Filter manholes (≥8-ft) and some



Vacuuming Sump Through MAW

vaults complete sediment removal may be facilitated by removing a cartridge lid from an empty receptacle and inserting a jetting wand (not a vacuum wand) through the receptacle. Use the sprayer to rinse loosened sediment toward the vacuum hose in the MAW opening, being careful not to damage the receptacle.

7.4 Filter Cartridge Reinstallation and Replacement

- Cartridges should be installed after the deck has been cleaned.
 It is important that the receptacle surfaces be free from grit and debris.
- 2. Remove cartridge lid from deck and carefully lower the filter cartridge into the receptacle until head plate gasket is seated squarely in receptacle. Caution: Do not force the cartridge downward; damage may occur.
- 3. Replace the cartridge lid and check to see that both male threads are properly seated before rotating approximately 1/3 of a full rotation until firmly seated. Use of an approved rim gasket lubricant may facilitate installation. See next page for additional details.
- 4. If rinsing is ineffective in removing sediment from the tentacles, or if tentacles are damaged, provisions must be made to replace the spent or damaged tentacles with new tentacles. Contact Contech to order replacement tentacles.

7.5 Chemical Spills

Caution: If a chemical spill has been captured, do not attempt maintenance. Immediately contact the local hazard response agency and contact Contech.

7.6 Material Disposal

The accumulated sediment found in stormwater treatment and conveyance systems must be handled and disposed of in accordance with regulatory protocols. It is possible for sediments to contain measurable concentrations of heavy metals and organic chemicals (such as pesticides and petroleum products). Areas with the greatest potential for high pollutant loading include industrial areas and heavily traveled roads. Sediments and water must be disposed of in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes. This typically requires coordination with a local landfill for solid waste disposal. For liquid waste disposal a number of options are available including a municipal vacuum truck decant facility, local waste water treatment plant or on-site treatment and discharge.

Jellyfish Filter Components & Filter Cartridge Assembly and Installation

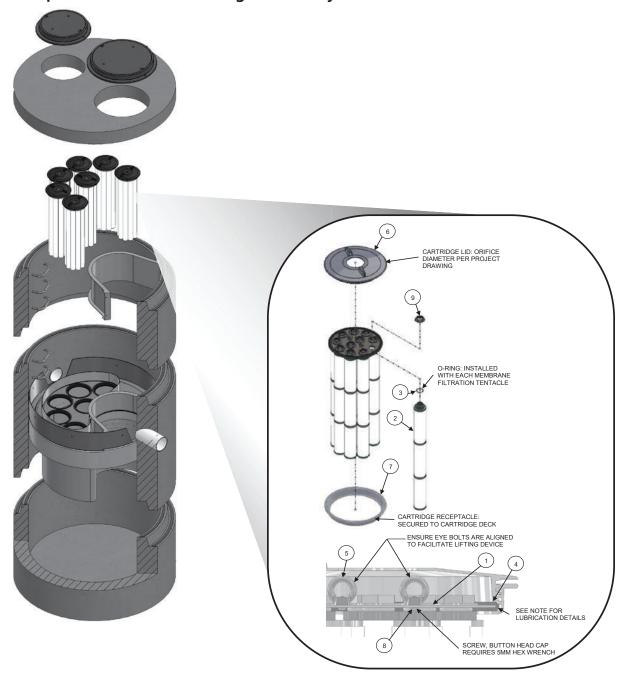


TABLE 1: BOM

TABLE I. DOM					
ITEM NO.	DESCRIPTION				
1	JF HEAD PLATE				
2	JF TENTACLE				
3	JF O-RING				
4	JF HEAD PLATE GASKET				
5	JF CARTRIDGE EYELET				
6	JF 14IN COVER				
7	JF RECEPTACLE				
	BUTTON HEAD CAP				
8	SCREW M6X14MM SS				
9	JF CARTRIDGE NUT				

TABLE 2: APPROVED GASKET LUBRICANTS

PART NO.		MFR	DESCRIPTION
	78713	LA-CO	LUBRI-JOINT
	40501	HERCULES	DUCK BUTTER
30600		OATEY	PIPE LUBRICANT
	PSI UBXI 10	PROSELECT	PIPE JOINT LUBRICANT

NOTES:

Head Plate Gasket Installation:

Install Head Plate Gasket (Item 4) onto the Head Plate (Item 1) and liberally apply a lubricant from Table 2: Approved Gasket Lubricants onto the gasket where it contacts the Receptacle (Item 7) and Cartridge Lid (Item 6). Follow Lubricant manufacturer's instructions.

Lid Assembly:

Rotate Cartridge Lid counter-clockwise until both male threads drop down and properly seat. Then rotate Cartridge Lid clock-wise approximately one-third of a full rotation until Cartridge Lid is firmly secured, creating a watertight seal.

Jellyfish Filter Inspection and Maintenance Log

Owner:			Jellyfish Mo			
Location:			GPS Coord	inates:		
Land Use:	Commercial:	Industrial:	Se	ervice Station:		
	Road/Highway:	Airport:	Re	esidential:	Parkir	ng Lot:
Date/Time:						
Inspector:						
Maintenance	Contractor:					
Visible Oil Pre						
Oil Quantity F	Removed 					
Floatable Deb	oris Present: (Y/N)					
Floatable Deb	oris removed: (Y/N)					
Water Depth	in Backwash Pool					
Cartridges ext	ternally rinsed/re-commission	oned: (Y/N)				
New tentacle	s put on Cartridges: (Y/N)					
Sediment Dep	pth Measured: (Y/N)					
Sediment Dep	pth (inches or mm):					
Sediment Rer	moved: (Y/N)					
Cartridge Lids	s intact: (Y/N)					
Observed Dar	mage:					
Comments:						

1.6 Porous Asphalt Maintenance Requirements

Porous Asphalt Inspection/Maintenance Requirements						
Inspection/ Maintenance	Frequency	Action				
Monitor for sediment build up, particularly in the winter.	Two (2) – Four (4) Times Annually.	- Clean with vacuum sweeper, bi- annually - Loose debris such as leaves or can be removed using a power/leaf blower or gutter broom. Fall and spring cleanup should be accompanied by pavement vacuuming.				
Inspect Adjacent Vegetation	Two (2) – Four (4) Times Annually.	- Repair or replace any eroded areas.				
Inspect for standing water -Within 30 minutes following a rain event.	One (1) - Two (2) Times Annually	- Use of a power washer or compressed air blower at an angle of 30 degrees or less can be effective, vacuum or vacuum sweeper if necessary.				
Damage to pavement	As needed	- Repairs should be made as identified.				

Porous Asphalt Winter Maintenance Guidelines:

- No winter sanding or salting of porous pavements is permitted
- Porous surfaces are commonly not treated and plowed until 2 or more inches of snow accumulation.
- Plow after every storm. If possible, plow with a slightly raised blade, this will help prevent pavement scarring.

Additional Porous Asphalt Operation and Maintenance Requirements:

- Never reseal or repave with impermeable materials.
- Inspect annually for pavement deterioration or spalling.
- Monitor periodically to ensure the pavement surface drains effectively after storms.

1.7 Snow & Ice Management for Standard Asphalt and Walkways

Snow storage areas shall be located such that no direct untreated discharges are possible to receiving waters from the storage site (snow storage areas have been shown on the Site Plan). The property manager will be responsible for timely snow removal from all private sidewalks, driveways, and parking areas. Snow removal will be hauled off-site and legally disposed of when snowbanks exceed 3 feet in height. Salt storage areas shall be covered or located such that no direct untreated discharges are possible to receiving waters from the storage site. Salt storage is not permitted withing the 100' wetland buffer. Salt and sand shall be used to the minimum extent practical (refer to the attached for deicing application rate guideline from the New Hampshire Stormwater Management Manual, Volume,).

Section 2 Chloride Management Plan

Winter Operational Guidelines

The following Chloride Management Plan is for the 53 Green Street, Mixed Use Development in Portsmouth, New Hampshire. The Plan includes operational guidelines including winter operator certification requirements, weather monitoring, equipment calibration requirements, mechanical removal, and salt usage evaluation and monitoring. Due to the evolving nature of chloride management efforts, the Chlorides Management Plan will be reviewed annually, in advance of the winter season, to reflect the current management standards.

2.1 Background Information

The 53 Green Street, Mixed Use Development is located along the North Mill Pond in Portsmouth, New Hampshire.

2.2 Operational Guidelines - Chloride Management

All private contractors engaged at the development site for the purposes of winter operational snow removal and surface maintenance, are responsible for assisting in meeting compliance for the following protocols. Private contractors are expected to minimize the effects of the use of de-icing, anti-icing and pretreatment materials by adhering to the strict guidelines outlined below.

The winter operational de-icing, anti-icing and pretreatment materials will adhere to the following protocols:

2.2.1 Winter Operator Certification Requirements

All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance must be current UNHT2 Green SnowPro Certified operators or equivalent and will use only pre-approved methods for spreading abrasives on private roadways and parking lots. All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance shall provide to the property management two copies of the annual UNHT2 Green SnowPro certificate or equivalent for each operator utilized on the premises. The annual UNHT2 Green SnowPro certificate or equivalent for each operator will be available on file in the Facilities Management office and be present in the vehicle/carrier at all times.

2.2.2 Improved Weather Monitoring

The property manager will coordinate weather information for use by winter maintenance contractors. This information in conjunction with site specific

air/ground surface temperature monitoring will ensure that private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance will make more informed decisions as to when and to what extent de-icing, anti-icing and pretreatment materials are applied to private roadways, sidewalks, and parking lots.

2.2.3 Equipment Calibration Requirements

All equipment utilized on the premises for the purpose of winter operational snow removal and surface maintenance will conform to the following calibration requirements.

2.2.3.1 Annual Calibration Requirements

All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance shall provide two copies of the annual calibration report for each piece of equipment utilized on the premises. Each calibration report shall include the vehicle/carrier VIN number and the serial numbers for each component including, but not limited to, spreader control units, salt aggregate spreader equipment, brining/prewetting equipment, ground speed orientation unit, and air/ground surface temperature monitor. Annual calibration reports will be available on file in the Facilities Management office and be present in the vehicle/carrier at all times.

Prior to each use, each vehicle/carrier operator will perform a systems check to verify that unit settings remain within the guidelines established by the Management Team in order to accurately dispense material. All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance will be subject to spot inspections by members of the Property Management Team to ensure that each vehicle/carrier is operating in a manner consistent with the guidelines set herein or State and Municipal regulations. All units will be recalibrated, and the updated calibration reports will be provided each time repairs or maintenance procedures affect the hydraulic system of the vehicle/carrier.

2.2.4 Increased Mechanical Removal Capabilities

All private contractors engaged at the premises will endeavor to use mechanical removal means on a more frequent basis for roadways, parking lots and sidewalks. Dedicating more manpower and equipment to increase snow removal frequencies prevents the buildup of snow and the corresponding need for deicing, anti-icing and pretreatment materials. Shortened maintenance routes, with shorter service intervals, will be used to stay ahead of snowfall. Minimized snow and ice packing will reduce the need for abrasives, salt aggregates, and/or brining solution to restore surfaces back to bare surface states after winter precipitation events.

After storm events the management team will be responsible for having the streets swept to recapture un-melted de-icing materials, when practical.

2.3 Salt Usage Evaluation and Monitoring

All private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance shall provide two copies of a storm report, which includes detailed information regarding treatment areas and the use of de-icing, anticing and pretreatment materials applied for the removal of snow and surface maintenance on the premises. The property manager will maintain copies of Summary Documents, including copies of the Storm Reports, operator certifications, equipment used for roadway and sidewalk winter maintenance, calibration reports and amount of de-icing materials used.

2.4 Summary

The above-described methodologies are incorporated into the Operational Manual and are to be used to qualify and retain all private contractors engaged at the premises for the purpose of winter operational snow removal and surface maintenance. This section of the Manual is intended to be an adaptive management document that is modified as required based on experience gained from past practices and technological advancements that reflect chloride BMP standards. All employees directly involved with winter operational activities are required to review this document and the current standard Best Management Practices published by the UNH Technology Transfer (T2) program annually. All employees directly involved with winter operational activities, and all private contractors engaged at the premises for the purposes of winter operational snow removal and surface maintenance, must be current UNHT2 Green SnowPro Certified operators or equivalent and undergo the necessary requirements to maintain this certification annually.

Deicing Application Rate Guidelines

24' of pavement (typcial two-lane road)

These rates are not fixed values, but rather the middle of a range to be selected and adjusted by an agency according to its local conditions and experience.

			Pounds per two-lane mile			
Pavement Temp. (°F) and Trend (↑↓)	Weather Condition	Maintenance Actions	Salt Prewetted / Pretreated with Salt Brine	Salt Prewetted / Pretreated with Other Blends	Dry Salt*	Winter Sand (abrasives)
	Snow	Plow, treat intersections only	80	70	100*	Not recommended
>30° ↑	Freezing Rain	Apply Chemical	80 - 160	70 - 140	100 - 200*	Not recommended
30° ↓	Snow	Plow and apply chemical	80 - 160	70 - 140	100 - 200*	Not recommended
30 V	Freezing Rain	Apply Chemical	150 - 200	130 - 180	180 - 240*	Not recommended
25°-30° ↑	Snow	Plow and apply chemical	120 - 160	100 - 140	150 - 200*	Not recommended
23 30 1	Freezing Rain	Apply Chemical	150 - 200	130 - 180	180 - 240*	Not recommended
25°-30° ↓	Snow	Plow and apply chemical	120 - 160	100 - 140	150 - 200*	Not recommended
23 30 4	Freezing Rain	Apply Chemical	160 - 240	140 - 210	200 - 300*	400
20° - 25° ↑	Snow or Freezing Rain	Plow and apply chemical	160 - 240	140 - 210	200 - 300*	400
20°-25° ↓	Snow	Plow and apply chemical	200 - 280	175 - 250	250 - 350*	Not recommended
20 - 25 ψ	Freezing Rain	Apply Chemical	240 - 320	210 - 280	300 - 400*	400
15°-20° ↑	Snow	Plow and apply chemical	200 - 280	175 - 250	250 - 350*	Not recommended
	Freezing Rain	Apply Chemical	240 - 320	210 - 280	300 - 400*	400
15°-20° ↓	Snow or Freezing Rain	Plow and apply chemical	240 - 320	210 - 280	300 - 400*	500 for freezing rain
0°-15° ↑↓	Snow	Plow, treat with blends, sand hazardous areas	Not recommended	300 - 400	Not recommended	500 - 750 spot treatment as needed
< 0°	Snow	Plow, treat with blends, sand hazardous areas	Not recommended	400 - 600**	Not recommended	500 - 750 spot treatment as needed

^{*} Dry salt is not recommended. It is likely to blow off the road before it melts ice.

^{**} A blend of 6 - 8 gal/ton MgCl₂ or CaCl₂ added to NaCl can melt ice as low as -10°.

Anti-icing Route Data Form					
Truck Station:					
Date:					
Air Temperature	Pavement Temperature	Relative Humidity	Dew Point	Sky	
Reason for applying:		, L		I	
Route:					
Chemical:					
Application Time:					
Application Amount:					
Observation (first day)):				
Observation (after eve	ent):				
Observation (before n	ext application):				
Name:					

Section 3 Invasive Species

With respect to a particular ecosystem, any species, including its seeds, eggs, spores, or other biological material capable of propagating that species, that is not native to that ecosystem is classified as an invasive species. Refer to the following fact sheet prepared by the University of New Hampshire Cooperative Extension entitled Methods for Disposing Non-Native Invasive Plants for recommended methods to dispose of invasive plant species.

UNIVERSITY of NEW HAMPSHIRE Methods for Disposing OOPERATIVE EXTENSION

Non-Native Invasive Plants

Prepared by the Invasives Species Outreach Group, volunteers interested in helping people control invasive plants. Assistance provided by the Piscataquog Land Conservancy and the NH Invasives Species Committee. Edited by Karen Bennett, Extension Forestry Professor and Specialist.



Tatarian honevsuckle

Lonicera tatarica USDA-NRCS PLANTS Database / Britton, N.L., and A. Brown. 1913. An illustrated flora of the northern United States, Canada and the British Possessions. Vol. 3: 282.

Non-native invasive plants crowd out natives in natural and managed landscapes. They cost taxpayers billions of dollars each year from lost agricultural and forest crops, decreased biodiversity, impacts to natural resources and the environment, and the cost to control and eradicate them.

Invasive plants grow well even in less than desirable conditions such as sandy soils along roadsides, shaded wooded areas, and in wetlands. In ideal conditions, they grow and spread even faster. There are many ways to remove these nonnative invasives, but once removed, care is needed to dispose the removed plant material so the plants don't grow where disposed.

Knowing how a particular plant reproduces indicates its method of spread and helps determine

the appropriate disposal method. Most are spread by seed and are dispersed by wind, water, animals, or people. Some reproduce by vegetative means from pieces of stems or roots forming new plants. Others spread through both seed and vegetative means.

Because movement and disposal of viable plant parts is restricted (see NH Regulations), viable invasive parts can't be brought to most transfer stations in the state. Check with your transfer station to see if there is an approved, designated area for invasives disposal. This fact sheet gives recommendations for rendering plant parts nonviable.

Control of invasives is beyond the scope of this fact sheet. For information about control visit www.nhinvasives.org or contact your UNH Cooperative Extension office.

New Hampshire Regulations

Prohibited invasive species shall only be disposed of in a manner that renders them nonliving and nonviable. (Agr. 3802.04)

No person shall collect, transport, import, export, move, buy, sell, distribute, propagate or transplant any living and viable portion of any plant species, which includes all of their cultivars and varieties, listed in Table 3800.1 of the New Hampshire prohibited invasive species list. (Agr 3802.01)

How and When to Dispose of Invasives?

To prevent seed from spreading remove invasive plants before seeds are set (produced). Some plants continue to grow, flower and set seed even after pulling or cutting. Seeds can remain viable in the ground for many years. If the plant has flowers or seeds, place the flowers and seeds in a heavy plastic bag "head first" at the weeding site and transport to the disposal site. The following are general descriptions of disposal methods. See the chart for recommendations by species.

Burning: Large woody branches and trunks can be used as firewood or burned in piles. For outside burning, a written fire permit from the local forest fire warden is required unless the ground is covered in snow. Brush larger than 5 inches in diameter can't be burned. Invasive plants with easily airborne seeds like black swallow-wort with mature seed pods (indicated by their brown color) shouldn't be burned as the seeds may disperse by the hot air created by the fire.

Bagging (solarization): Use this technique with softertissue plants. Use heavy black or clear plastic bags (contractor grade), making sure that no parts of the plants poke through. Allow the bags to sit in the sun for several weeks and on dark pavement for the best effect.



Japanese knotweed
Polygonum cuspidatum
USDA-NRCS PLANTS Database /
Britton, N.L., and A. Brown. 1913. An
illustrated flora of the northern United
States, Canada and the British
Possessions, Vol. 1: 676

Tarping and Drying: Pile material on a sheet of plastic and cover with a tarp, fastening the tarp to the ground and monitoring it for escapes. Let the material dry for several weeks, or until it is clearly nonviable.

Chipping: Use this method for woody plants that don't reproduce vegetatively.

Burying: This is risky, but can be done with watchful diligence. Lay thick plastic in a deep pit before placing the cut up plant material in the hole. Place the material away from the edge of the plastic before covering it with more heavy plastic. Eliminate as much air as possible and toss in soil to weight down the material in the pit. Note that the top of the buried material should be at least three feet underground. Japanese knotweed should be at least 5 feet underground!

Drowning: Fill a large barrel with water and place soft-tissue plants in the water. Check after a few weeks and look for rotted plant material (roots, stems, leaves, flowers). Well-rotted plant material may be composted. A word of caution- seeds may still be viable after using this method. Do this before seeds are set. This method isn't used often. Be prepared for an awful stink!

Composting: Invasive plants can take root in compost. Don't compost any invasives unless you know there is no viable (living) plant material left. Use one of the above techniques (bagging, tarping, drying, chipping, or drowning) to render the plants nonviable before composting. Closely examine the plant before composting and avoid composting seeds.

Suggested Disposal Methods for Non-Native Invasive Plants

This table provides information concerning the disposal of removed invasive plant material. If the infestation is treated with herbicide and left in place, these guidelines don't apply. Don't bring invasives to a local transfer station, unless there is a designated area for their disposal, or they have been rendered non-viable. This listing includes wetland and upland plants from the New Hampshire Prohibited Invasive Species List. The disposal of aquatic plants isn't addressed.

Woody Plants	Method of Reproducing	Methods of Disposal
Norway maple (Acer platanoides) European barberry (Berberis vulgaris) Japanese barberry (Berberis thunbergii) autumn olive (Elaeagnus umbellata) burning bush (Euonymus alatus) Morrow's honeysuckle (Lonicera morrowii) Tatarian honeysuckle (Lonicera tatarica) showy bush honeysuckle (Lonicera x bella) common buckthorn (Rhamnus cathartica) glossy buckthorn (Frangula alnus)	Fruit and Seeds	Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Use as firewood. Make a brush pile. Chip. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip once all fruit has dropped from branches. Leave resulting chips on site and monitor.
oriental bittersweet (Celastrus orbiculatus) multiflora rose (Rosa multiflora)	Fruits, Seeds, Plant Fragments	Prior to fruit/seed ripening Seedlings and small plants Pull or cut and leave on site with roots exposed. No special care needed. Larger plants Make a brush pile. Burn. After fruit/seed is ripe Don't remove from site. Burn. Make a covered brush pile. Chip — only after material has fully dried (1 year) and all fruit has dropped from branches. Leave resulting chips on site and monitor.

Non-Woody Plants	Method of Reproducing	Methods of Disposal
garlic mustard (Alliaria petiolata) spotted knapweed (Centaurea maculosa) Sap of related knapweed can cause skin irritation and tumors. Wear gloves when handling. black swallow-wort (Cynanchum nigrum) May cause skin rash. Wear gloves and long sleeves when handling. pale swallow-wort (Cynanchum rossicum) giant hogweed (Heracleum mantegazzianum) Can cause major skin rash. Wear gloves and long sleeves when handling. dame's rocket (Hesperis matronalis) perennial pepperweed (Lepidium latifolium) purple loosestrife (Lythrum salicaria) Japanese stilt grass (Microstegium vimineum) mile-a-minute weed (Polygonum perfoliatum)	Fruits and Seeds	Prior to flowering Depends on scale of infestation Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile. (You can pile onto or cover with plastic sheeting). Monitor. Remove any re-sprouting material. During and following flowering Do nothing until the following year or remove flowering heads and bag and let rot. Small infestation Pull or cut plant and leave on site with roots exposed. Large infestation Pull or cut plant and pile remaining material. (You can pile onto plastic or cover with plastic sheeting). Monitor. Remove any re-sprouting material.
common reed (Phragmites australis) Japanese knotweed (Polygonum cuspidatum) Bohemian knotweed (Polygonum x bohemicum) Bohemian knotweed (Polygonum x bohemicum) Bohemian knotweed (Polygonum x bohemicum) Fruits, Seeds, Plant Fragments Primary means of spread in these species is by plant parts. Although all care should be given to preventing the dispersal of seed during control activities, the presence of seed doesn't materially influence disposal activities.		Small infestation Bag all plant material and let rot. Never pile and use resulting material as compost. Burn. Large infestation Remove material to unsuitable habitat (dry, hot and sunny or dry and shaded location) and scatter or pile. Monitor and remove any sprouting material. Pile, let dry, and burn.

Managing Invasive Plants Methods of Control by Christopher Mattrick

They're out there. The problem of invasive plants is as close as your own backyard.

Maybe a favorite dogwood tree is struggling in the clutches of an Oriental bittersweet vine. Clawlike canes of multiflora rose are scratching at the side of your house. That handsome burning bush you planted few years ago has become a whole clump in practically no time ... but what happened to the azalea that used to grow right next to it?

If you think controlling or managing invasive plants on your property is a daunting task, you're not alone. Though this topic is getting lots of attention from federal, state, and local government agencies, as well as the media, the basic question for most homeowners is simply, "How do I get rid of the invasive plants in my own landscape?" Fortunately, the best place to begin to tackle this complex issue is in our own backyards and on local conservation lands. We hope the information provided here will help you take back your yard. We won't kid you—there's some work involved, but the payoff in beauty, wildlife habitat, and peace of mind makes it all worthwhile.

PLAN OF ATTACK

Three broad categories cover most invasive plant control: mechanical, chemical, and biological. Mechanical control means physically removing plants from the environment



Spraying chemicals to control invasive plants.

through cutting or pulling. Chemical control uses herbicides to kill plants and inhibit regrowth. Techniques and chemicals used will vary depending on the species. Biological controls use plant diseases or insect predators, typically from the targeted species' home range. Several techniques may be effective in controlling a single species, but there is usually one preferred method—the one that is most resource efficient with minimal impact on non-target species and the environment.

MECHANICAL CONTROL METHODS

Mechanical treatments are usually the first ones to look at when evaluating an invasive plant removal project. These procedures do not require special licensing or introduce chemicals into the environment. They do require permits in some situations, such as wetland zones. [See sidebar on page 23.] Mechanical removal is highly labor intensive and creates a significant amount of site disturbance, which can lead to rapid reinvasion if not handled properly.

Pulling and digging

Many herbaceous plants and some woody species (up to about one inch in diameter), if present in limited quantities, can be pulled out or dug up. It's important to remove as much of the root system as possible; even a small portion can restart the infestation. Pull plants by hand or use a digging fork, as shovels can shear off portions of the root

system, allowing for regrowth. To remove larger woody stems (up to about three inches in diameter), use a Weed WrenchTM, Root Jack, or Root Talon. These tools, available from several manufacturers, are designed to remove the aboveground portion of the plant as well as the entire root system. It's easiest to undertake this type of control in the spring or early summer when soils are moist and plants come out more easily.



Using tools to remove woody stems.





Volunteers hand pulling invasive plants.

Suffocation

Try suffocating small seedlings and herbaceous plants. Place double or triple layers of thick UV-stabilized plastic sheeting, either clear or black (personally I like clear), over the infestation and secure the plastic with stakes or weights. Make sure the plastic extends at least five feet past the edge of infestation on all sides. Leave the plastic in place for at least two years. This technique will kill everything beneath the plastic—invasive and non-invasive plants alike. Once the plastic is removed, sow a cover crop such as annual rye to prevent new invasions.

Cutting or mowing

This technique is best suited for locations you can visit and treat often. To be effective, you will need to mow or cut infested areas three or four times a year for up to five years. The goal is to interrupt the plant's ability to photosynthesize by removing as much leafy material as possible. Cut the plants at ground level and remove all resulting debris from the site. With this treatment, the infestation may actually appear to get worse at first, so you will need to be as persistent as the invasive plants themselves. Each time you cut the plants back, the root system gets slightly larger, but must also rely on its energy reserves to push up new growth. Eventually, you will exhaust these reserves and the plants will die. This may take many years, so you have to remain committed to this process once you start; otherwise the treatment can backfire, making the problem worse.

CHEMICAL CONTROL METHODS

Herbicides are among the most effective and resource-efficient tools to treat invasive species. Most of the commonly known invasive plants can be treated using only two herbicides—glyphosate (the active ingredient in Roundup™ and Rodeo™) and triclopyr (the active ingredient in Brush-B-GoneTM and GarlonTM). Glyphosate is non-selective, meaning it kills everything it contacts. Triclopyr is selective and does not injure monocots (grasses, orchids, lilies, etc.). Please read labels and follow directions precisely for both environmental and personal safety. These are relatively benign herbicides, but improperly used they can still cause both short- and long-term health and environmental problems. Special aquatic formulations are required when working in wetland zones. You are required to have a stateissued pesticide applicator license when applying these chemicals on land you do not own. To learn more about the pesticide regulations in your state, visit or call your state's pesticide control division, usually part of the state's Department of Agriculture. In wetland areas, additional permits are usually required by the Wetlands Protection Act. [See sidebar on page 23.]

Foliar applications

When problems are on a small scale, this type of treatment is usually applied with a backpack sprayer or even a small handheld spray bottle. It is an excellent way to treat large monocultures of herbaceous plants, or to spot-treat individual plants that are difficult to remove mechanically, such as goutweed, swallowwort, or purple loosestrife. It is also an effective treatment for some woody species, such as Japanese barberry, multiflora rose, Japanese honeysuckle, and Oriental bittersweet that grow in dense masses or large numbers over many acres. The herbicide mixture should contain no more than five percent of the active ingredient, but it is important to follow the instructions on the product label. This treatment is most effective when the plants are actively growing, ideally when they are flowering or beginning to form fruit. It has been shown that plants are often more susceptible to this type of treatment if the existing stems are cut off and the regrowth is treated. This is especially true for Japanese knotweed. The target plants should be thoroughly wetted with the herbicide on a day when there is no rain in the forecast for the next 24 to 48 hours.

Cut stem treatments

There are several different types of cut stem treatments, but here we will review only the one most commonly used. All treatments of this type require a higher concentration of the active ingredient than is used in foliar applications. A 25 to 35 percent solution of the active ingredient should be used for cut stem treatments, but read and follow all label instructions. In most cases, the appropriate herbicide is glyphosate, except for Oriental bittersweet, on which triclopyr should be used. This treatment can be used on all woody stems, as well as phragmites and Japanese knotweed.

For woody stems, treatments are most effective when applied in the late summer and autumn—between late August and November. Stems should be cut close to the ground, but not so close that you will lose track of them. Apply herbicide directly to the cut surface as soon as possible after cutting. Delaying the application will reduce the effectiveness of the treatment. The herbicide can be applied with a sponge, paintbrush, or spray bottle.



Cut stem treatment tools.

For phragmites and Japanese knotweed, treatment is the same, but the timing and equipment are different. Plants should be treated anytime from mid-July through September, but the hottest, most humid days of the summer are best

for this method. Cut the stems halfway between two leaf nodes at a comfortable height. Inject (or squirt) herbicide into the exposed hollow stem. All stems in an infestation should be treated. A wash bottle is the most effective application tool, but you can also use an eyedropper, spray bottle, or one of the recently developed high-tech injection systems.

It is helpful to mix a dye in with the herbicide solution. The dye will stain the treated surface and mark the areas that have been treated, preventing unnecessary reapplication. You can buy a specially formulated herbicide dye, or use food coloring or laundry dye.

There is not enough space in this article to describe all the possible ways to control invasive plants. You can find other treatments, along with more details on the above-described methods, and species-specific recommendations on The Nature Conservancy Web site (tncweeds.ucdavis.edu). An upcoming posting on the Invasive Plant Atlas of New England (www.ipane.org) and the New England Wild Flower Society (www.newfs.org) Web sites will also provide further details.



Hollow stem injection tools.

Biological controls—still on the horizon

Biological controls are moving into the forefront of control methodology, but currently the only widely available and applied biocontrol relates to purple loosestrife. More information on purple loosestrife and other biological control projects can be found at www.invasiveplants.net.

DISPOSAL OF INVASIVE PLANTS

Proper disposal of removed invasive plant material is critical to the control process. Leftover plant material can cause new infestations or reinfest the existing project area. There are many appropriate ways to dispose of invasive plant debris. I've listed them here in order of preference.

- **1. Burn it**—Make a brush pile and burn the material following local safety regulations and restrictions, or haul it to your town's landfill and place it in their burn pile.
- **2. Pile it**—Make a pile of the woody debris. This technique will provide shelter for wildlife as well.
- **3.** Compost it—Place all your herbaceous invasive plant debris in a pile and process as compost. Watch the pile closely for resprouts and remove as necessary. Do not use the resulting compost in your garden. The pile is for invasive plants only.



Injecting herbicide into the hollow stem of phragmites.

4. Dry it/cook it—Place woody debris out on your driveway or any asphalt surface and let it dry out for a month. Place herbaceous material in a doubled-up black trash bag and let it cook in the sun for one month. At the end of the month, the material should be non-viable and you can dump it or dispose of it with the trash. The method assumes there is no viable seed mixed in with the removed material.

Care should be taken in the disposal of all invasive plants, but several species need extra attention. These are the ones that have the ability to sprout vigorously from plant fragments and should ideally be burned or dried prior to disposal: Oriental bittersweet, multiflora rose, Japanese honeysuckle, phragmites, and Japanese knotweed.

Christopher Mattrick is the former Senior Conservation Programs Manager for New England Wild Flower Society, where he managed conservation volunteer and invasive and rare plant management programs. Today, Chris and his family work and play in the White Mountains of New Hampshire, where he is the Forest Botanist and Invasive Species Coordinator for the White Mountain National Forest.



Controlling Invasive Plants in Wetlands

Special concerns; special precautions

Control of invasive plants in or around wetlands or bodies of water requires a unique set of considerations. Removal projects in wetland zones can be legal and effective if handled appropriately. In many cases, herbicides may be the least disruptive tools with which to remove invasive plants. You will need a state-issued pesticide license to apply herbicide on someone else's property, but all projects in wetland or aquatic systems fall under the jurisdiction of the Wetlands Protection Act and therefore require a permit. Yes, even hand-pulling that colony of glossy buckthorn plants from your own swampland requires a permit. Getting a permit for legal removal is fairly painless if you plan your project carefully.

- 1. Investigate and understand the required permits and learn how to obtain them. The entity charged with the enforcement of the Wetlands Protection Act varies from state to state. For more information in your state, contact:
 - ME: Department of Environmental Protection www.state.me.us/dep/blwq/docstand/nrpapage.htm
 - NH: Department of Environmental Services www.des.state.nh.us/wetlands/
 - VT: Department of Environmental Conservation www.anr.state.vt.us/dec/waterq/permits/htm/pm_cud.htm
 - MA: Consult your local town conservation commission
 - **RI:** Department of Environmental Management www.dem.ri.gov/programs/benviron/water/permits/fresh/index.htm
 - CT: Consult your local town Inland Wetland and Conservation Commission

- 2. Consult an individual or organization with experience in this area. Firsthand experience in conducting projects in wetland zones and navigating the permitting process is priceless. Most states have wetland scientist societies whose members are experienced in working in wetlands and navigating the regulations affecting them. A simple Web search will reveal the contact point for these societies. Additionally, most environmental consulting firms and some nonprofit organizations have skills in this area.
- 3. Develop a well-written and thorough project plan. You are more likely to be successful in obtaining a permit for your project if you submit a project plan along with your permit application. The plan should include the reasons for the project, your objectives in completing the project, how you plan to reach those objectives, and how you will monitor the outcome.
- 4. Ensure that the herbicides you plan to use are approved for aquatic use. Experts consider most herbicides harmful to water quality or aquatic organisms, but rate some formulations as safe for aquatic use. Do the research and select an approved herbicide, and then closely follow the instructions on the label.
- 5. If you are unsure—research, study, and most of all, ask for help. Follow the rules. The damage caused to aquatic systems by the use of an inappropriate herbicide or the misapplication of an appropriate herbicide not only damages the environment, but also may reduce public support for safe, well-planned projects.

Section 4 Annual Updates and Log Requirements

The Owner and/or Contact/Responsible Party shall review this Operation and Maintenance Plan once per year for its effectiveness and adjust the plan and deed as necessary.

A log of all preventative and corrective measures for the stormwater system shall be kept on-site and be made available upon request by any public entity with administrative, health environmental or safety authority over the site including NHDES.

Copies of the Stormwater Maintenance report shall be submitted to the City of Portsmouth on an annual basis.

		Stormwa	Stormwater Management Report	T		
Mixed Use Development	nt	53 Green S	53 Green Street – Map 119 Lot 2			
BMP Description	Date of Inspection	Inspector	BMP Installed and Operating Properly?	Corrective Correction Action Needed	Date of Cleaning / Repair	Performed By
Deep Sump CB's			□Yes □No			
ADS Stormtech System with Isolator Row			□Yes □No			
Jellyfish Filter 1			□Yes □No			

		Stc	Stormwater Management Report	sport		
City of Portsmouth	outh	North Mill Pond Trail	Pond Trail			
BMP Description	Date of Inspection	Inspector	BMP Installed and Operating Properly?	Cleaning / Corrective Action Needed	Date of Cleaning / Repair	Performed By
Porous Pavement			□Yes □No			

1:\C\C0960 Cathartes\C-0960-011 53 Green St, Portsmouth, NH\Report_Evaluation\Applications\City of Portsmouth\20210519 TAC Resubmission\O&M\C-0960-011_Operations and Maintenance.docx

www.tighebond.com



PROPOSED MIXED USE DEVELOPMENT 53 GREEN STREET PORTSMOUTH, NEW HAMPSHIRE

SITE OVERLAY EXHIBIT

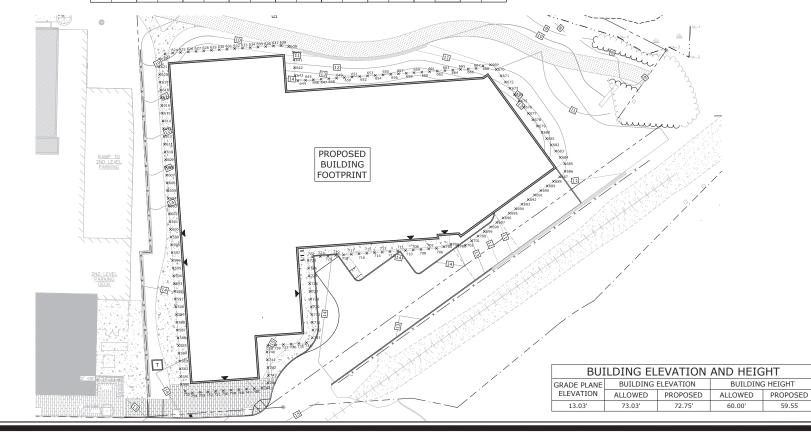


Last Save Date: May 19, 2021 9:43 AM By: NAHANSEN Plot Date: Wednesday, May 19, 2021 Plotted By: Nell A. Hansen

Poin	t Table	Point	t Table	Poin	Table	Point	Table	Point	Table	Poin	Table	Point	Table	Poin	t Table	Point	Table
Point #	Elevation	Point #	Elevation														
588	13.750	599	14.400	619	10.650	639	9.950	660	13.800	680	13.000	700	12.600	720	14.050	740	15.100
644	14.100	600	14.300	620	10.400	640	10.750	661	13.500	681	12.750	701	13.100	721	14.100	741	15.100
580	13.950	601	14.200	621	10.150	641	11.600	662	13.200	682	12.000	702	13.600	722	14.600	742	15.100
581	13.950	602	14.100	622	9.900	642	12.600	663	12.900	683	11.750	703	14.000	723	14.650	743	15.100
582	13.900	603	14.000	623	9.850	643	13.600	664	12.600	684	11.400	704	14.600	724	14,650	744	15.050
583	13.850	604	13,800	624	9.950	645	14.100	665	12,300	685	11.100	705	14,600	725	14,650	745	15,000
584	13,800	605	13,600	625	10,400	646	13,950	666	12,000	686	11,100	706	14,600	726	14,650	746	15,000
585	13.750	606	13.400	626	11.000	647	13.800	667	11.750	687	10,650	707	14.600	727	14.650	747	14.900
586	13,700	607	13.200	627	11.200	648	13.700	668	11.100	688	10,650	708	14.600	728	14.650	748	14.800
587	13.700	608	12.900	628	11.200	649	13.500	669	10.900	689	10.650	709	14.600	729	14.650	749	14.700
589	13.850	609	12.700	629	11.200	650	13.350	670	11.250	690	10.650	710	14.600	730	14.650	750	14.600
590	13.900	610	12.550	630	11.200	651	13.250	671	11.600	691	10.650	711	14.100	731	14.650	751	14.500
591	13.950	611	12.350	631	11.100	652	13.100	672	11.950	692	10.650	712	14.100	732	14.650	752	14.400
592	14.000	612	12.100	632	11.000	653	13.100	673	12.250	693	10.900	713	14.000	733	14.600	753	14.300
593	14.150	613	11.850	633	11.000	654	13.100	674	12,600	694	10.900	714	14.000	734	14,600	754	14,200
594	14.300	614	11.650	634	10.750	655	13.200	675	12.950	695	10.900	715	13.950	735	14,600	755	14,000
595	14.450	615	11.450	635	10,500	656	13.300	676	13,100	696	10.950	716	13,950	736	14,700		
596	14.500	616	11,200	636	10,350	657	13.500	677	13.100	697	11.250	717	13.900	737	14.800	AVERAGE	13.03
597	14,500	617	10.950	637	10.100	658	13.750	678	13.100	698	11.750	718	13.950	738	15.000	GRADE PLANE	13.03
598	14,500	618	10.800	638	9.950	659	13.800	679	13.100	699	12,200	719	13.950	739	15.100	1	

PROPOSED MIXED USE DEVELOPMENT 53 GREEN STREET PORTSMOUTH, NEW HAMPSHIRE

GRADE PLANE EXHIBIT





May 19, 20 C0960-011_C-DSGN.d Memorandum Tighe&Bond

53 Green Street, Portsmouth, NH: Wetland & Buffer Report

To: Patrick Crimmins, PE

FROM: Leonard A. Lord, PhD, CSS, CWS

DATE: January 6, 2020

PROJECT: P-0595-007

On October 29 and December 2, 2019, Tighe & Bond delineated and assessed tidal wetlands and their 100-foot buffers at 53 Green Street, Portsmouth, NH. This 1.81-acre parcel lies along the northwestern end of North Mill Pond.

Methods

The wetland delineation was based on criteria specified in the *Corps of Engineers Wetlands Delineation Manual, Technical Report Y-87-1* (January 1987) and the *Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region* (January 2012). The Highest Observable Tide Line (HOTL) was delineated based on the definition found in the NH Department of Environmental Services (NHDES) Wetland Rules, Env-Wt 101.49/Env-Wt 602.23. Wetlands were classified based on the *Classification of Wetlands and Deepwater Habitats of the United States* (Cowardin et al., 1979). The only wetlands located on the parcel are tidal wetlands (HOTL), which were delineated with sequentially-numbered flagging labelled 1A-1 to 1A-19.

Important wetland functions and values were also assessed and summarized in the vicinity of the parcel. The assessment was based on the *Maine Citizens Guide to Evaluating, Restoring, and Managing Tidal Marshes* (Bryan et al., 1997) and *The Highway Methodology Workbook Supplement—Wetland Functions and Values: A Descriptive Approach*, NAEEP-360-1-30a, US Army Corps of Engineers, New England Division, (September 1999).

Wetlands

Wetlands on this site were classified as estuarine intertidal rocky shore, rubble, and regularly flooded (E2RS2N). The wetland edge slopes sharply and is predominantly covered with angular stones and cobbles. Sparse halophytic vegetation along the upper portion of the tidal wetland edge includes seaside plantain (*Plantago maritima*), sea lavender (*Limonium carolinianum*), salt meadow grass (*Spartina patens*), and seaside goldenrod (*Solidago sempervirens*). Lower portions of the slopes were covered with rockweed (*Ascophyllum nodosum*) within the intertidal zone. Important wetland functions and values in this portion of North Mill Pond include recreation potential and aesthetic quality, though both are impacted by the density and character of the surrounding urban development.

Tidal Buffer

The 100-foot tidal buffer on this parcel consists primarily of maintained lawn, a commercial building, and a parking lot. There are small patches of shrubby vegetation and small trees at the tops of the slopes between the lawn and tidal wetlands, particularly near both ends of the wetland delineation. Species in these areas include black locust (*Robinia pseudoacacia*),

MEMO Tighe&Bond

eastern red cedar (*Juniperus virginiana*), staghorn sumac (*Rhus typhina*), and black cherry (*Prunus serotina*). The highly-developed tidal buffer provides some vegetated permeable surfaces to help reduce and filter runoff but otherwise does little to enhance and protect the downgradient tidal wetland.

\\tighebond.com\data\Data\Projects\P\P0595 Pro Con General Proposals\P0595-007 Raynes Ave Hotel\Raynes+Green Wetlands+Soils\Green St Wetland-Buffer Rept- 2020-1-9.pdf

Photographic Log



Client: ProCon Job Number: P-0595-007

Site: 53 Green Street, Portsmouth, NH

Photograph No.: 1 Date: 10/29/2019 Direction Taken: Northeast

Description: Intertidal rocky shore and tidal buffer viewed from the southwest end of the site.



Photograph No.: 2 Date: 10/29/2019 Direction Taken: Northeast

Description: Intertidal rocky shore and narrow shrubby portion of the tidal buffer at the northeastern end of the site.



Photographic Log 1

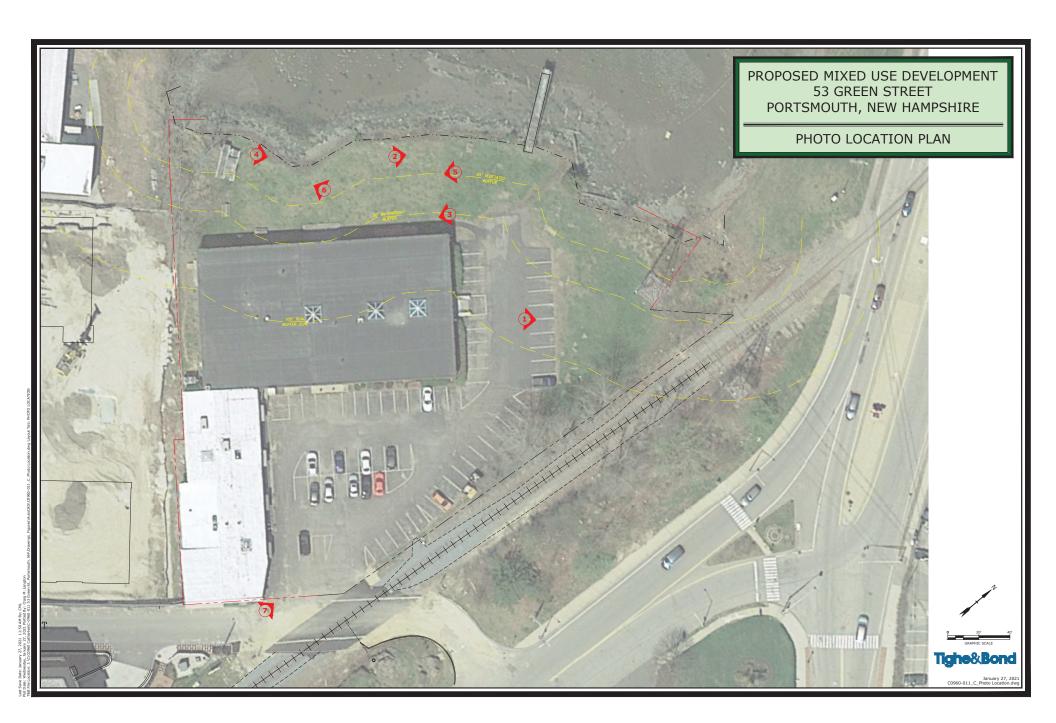




Photo #1: Looking northeast at existing utility towers and parking located in 100-foot tidal wetland buffer.



Photo #2: Looking northeast towards Market Street across existing maintained lawn area located in 100-foot tidal wetland buffer.



Photo #3: Looking southwest along existing building within 100-foot tidal wetland buffer.



Photo #4: Looking northeast toward existing building and parking located in 100-foot tidal wetland buffer.



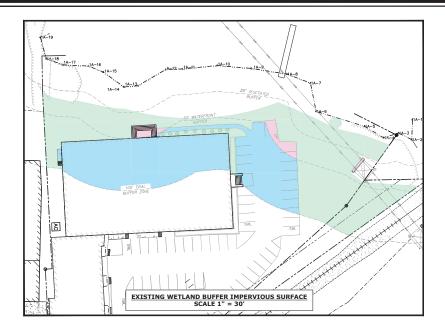
Photo #5: Looking southwest towards existing building and maintained lawn area located in 100-foot tidal wetland buffer.

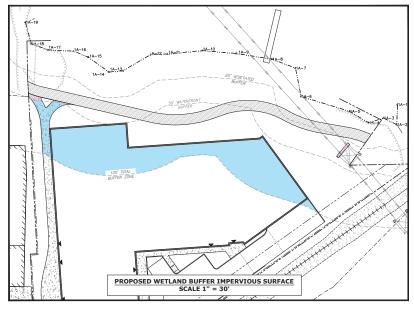


Photo #6: Looking west across existing maintained lawn area and North Mill Pond toward location of future City park.



Photo #7: Looking north toward existing parking lot.





PROPOSED MIXED USE DEVELOPMENT 53 GREEN STREET

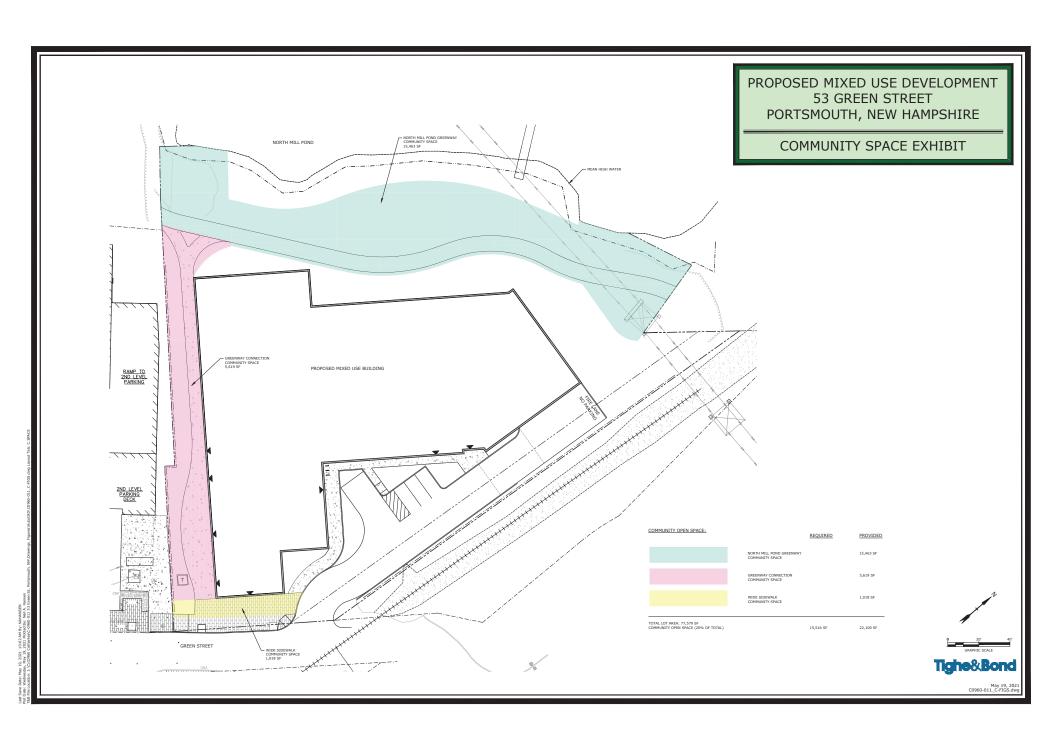
WETLAND BUFFER IMPERVIOUS SURFACE EXHIBIT

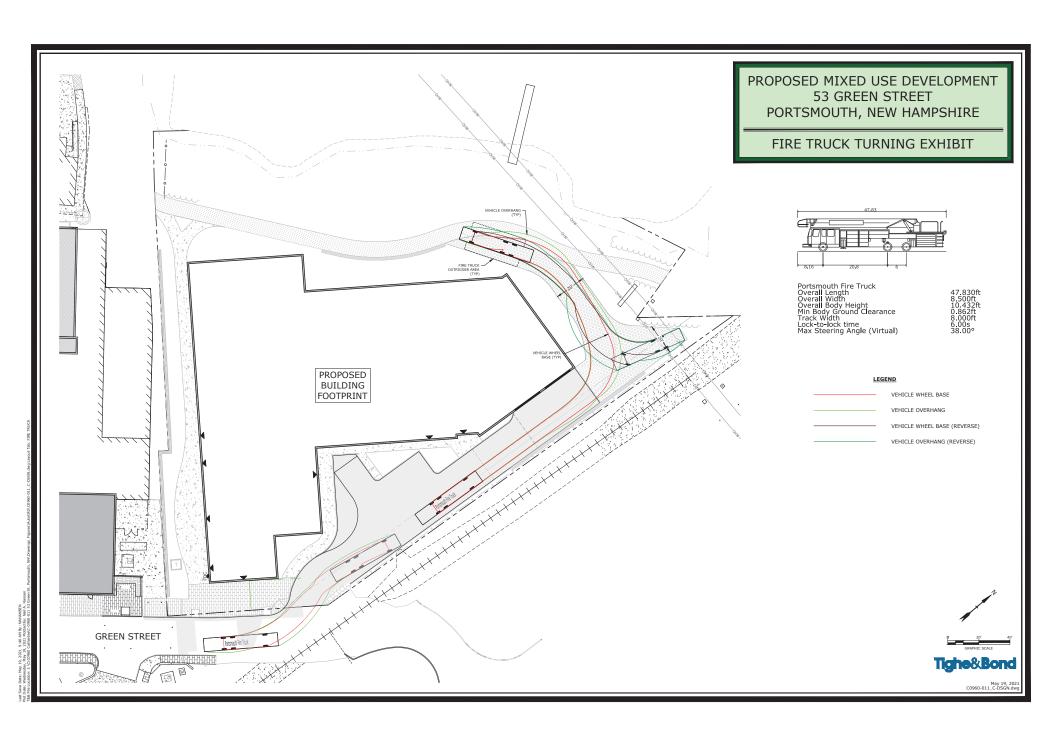
Impervio	us Surface Within Bu	ıffer Area
Local Wetland Buffer	Imperv	ious Surface
Setback	Existing Condition	Proposed Development
0 - 25 FT	0 SF	0 SF
25 - 50 FT	745 SF	110 SF
50 - 100 FT	10,836 SF	8,513 SF
Total Impervious Surface	11,581 SF	8,623 SF
Net Impervious Sruface	-2	,958 SF

AREA OF TEMPORARY WETLAND BUFFER IMPACTS FOR CONSTRUCTION



May 19, 202 0960-011_C-FIGS.dw





LEGEND

VEHICLE WHEEL BASE

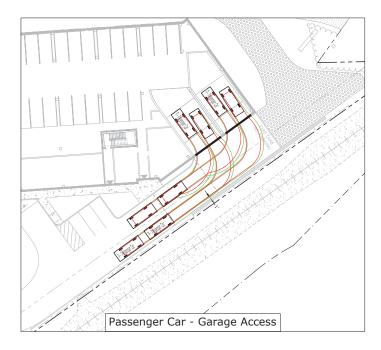
VEHICLE OVERHANG

VEHICLE WHEEL BASE (REVERSE)

VEHICLE OVERHANG (REVERSE)

PROPOSED MIXED USE DEVELOPMENT 53 GREEN STREET PORTSMOUTH, NEW HAMPSHIRE

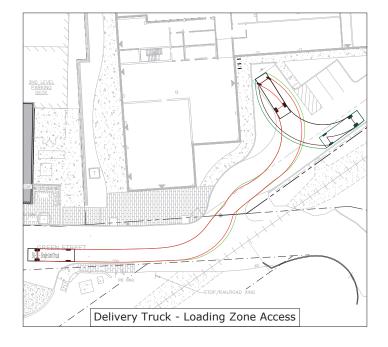
SITE TRAFFIC EXHIBIT

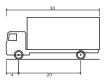




P - Passenger Car Overall Length Overall Width Overall Body Height Min Body Ground Clearance Track Width Lock-to-lock time Max Steering Angle (Virtual)

19.000ft 7.000ft 4.300ft 1.115ft 6.000ft 4.00s





SU-30 - Single Unit Truck Overall Width Overall Width Overall Body Height Min Body Ground Clearance Track Width Lock-to-lock time Max Steering Angle (Virtual)

30.000ft 8.000ft 13.500ft 1.367ft 8.000ft 5.00s



May 19, 202



C0960-011 May 19, 2021

Mr. Eric Eby, City Traffic Engineer City of Portsmouth Department of Public Works 680 Peverly Hill Road Portsmouth New Hampshire

Trip Generation Analysis

Proposed Mixed Use Development - 53 Green Street, Portsmouth, NH

Dear Eric:

Tighe & Bond has performed a trip generation analysis for traffic related to a proposed mixeduse development on a parcel of land located at 53 Green Street that is identified as Map 119 Lot 2 on the City of Portsmouth Tax Maps.

This analysis was performed utilizing Institute of Transportation Engineers (ITE) Trip Generation Manual, latest edition. For purposes of analysis, we have compared the existing and proposed uses for the parcel. The parcel's existing uses consists of 14,600 SF of office, 3,000 SF of medical office and 4,070 SF of spa with on-site parking. These buildings will be demolished. The proposed building consists 48 dwelling units with associated on-site parking. The proposed building also includes $\pm 2,300$ SF of first floor commercial space along Green Street but there are no on-site parking spaces required for this use, however it was included as part of this Trip Generation Analysis to provide a more conservative analysis.

		1	Existi		Prop	osed	
		Office	<u>Spa</u>	Medical Office	Multifamily Housing	Commercial	<u>Net</u> Trips
Wee	kday AM Peak Hour						
	Trips Entering	15	5	6	4	3	-19
	Trips Exiting	2	0	2	13	1	+10
	Total Vehicle Trips	17	5	8	17	4	-9
Wee	kday PM Peak Hour						
	Trips Entering	3	1	3	13	2	+8
	Trips Exiting	15	5	7	8	4	-15
	Total Vehicle Trips	18	6	10	21	6	-7
Satu	rday Peak Hour						
	Trips Entering	4	8	5	10	0	-7
	Trips Exiting	4	13	4	11	1	-9
	Total Vehicle Trips	8	21	9	21	1	-16

Source: Institute of Transportation Engineering, Trip Generation, 10th Edition

Land Uses - 221 Multifamily Housing (Mid-Rise), 710 General Office, 712 Small

Office Building, 720 Medical Office, 918 Hair Salon

Tighe&Bond

As depicted above, the proposed 48 residential units and 2,300 SF of small office space in place of the existing 14,600 SF of office use, 3,000 SF of medical office use and 4,070 SF of spa use will result in a reduction of 9 vehicle trips during the Weekday AM Peak Hour, 7 vehicle trips during the Weekday PM Peak Hour and 16 vehicle trips during the Saturday Peak Hour. It is anticipated there will be a reduced number of vehicle trips associated with this project resulting in no additional impact to the surrounding roadway network during peak hour times.

Please feel free to contact us if you have any questions or need any additional information.

Sincerely,

TIGHE & BOND, INC.

Neil A. Hansen, PE Project Engineer Patrick M. Crimmins, PE Senior Project Manager



February 22, 2021

Rob Simmons CPI Management, LLC 100 Summer Street, Ste 1600 Boston, MA 02109

RE: Natural Gas Availability to 53 Green St Portsmouth NH Project

Dear Rob,

Unitil's natural gas division has reviewed the requested site for natural gas service.

Unitil hereby confirms natural gas service will be available to 53 Green St Potsmouth NH Project.

Installation is pending an authorized installation agreement with CPI Management, LLC and street opening approval from the City of Portsmouth DPW.

Let me know if you have any questions. You can email me at oliver@unitil.com. My phone number is 603-294-5174.

Sincerely,

Janet Oliver Senior Business Development Representative

EMBARC

March 22, 2021

Portsmouth Planning Board 53 Green Street Portsmouth, NH 03801

Green Building Statement 53 Green Street Proposed Mixed-Use Building

- Site/Landscape: In it's current condition, the site consists of the existing building, parking to the south and east, and a mown lawn to the top of the bank by the North Mill Pond. The building has a foundation planting of mature Rhododendron. A small area of trees is found at the northwest and southeast corners of the property. The proposed landscape plan provides a pedestrian connector from Green Street to the North Mill Pond Greenway along the west side of the building. This pathway is buffered from the AC Hotel with a green wall of Arborvitae and ornamental grasses creating a garden connector to the greenway beyond. The north side of the building will be faced with a mixed evergreen screen of native shrubs (Inkberry, Rhododendron, Eastern Red Cedar) and the 25' buffer will be enhanced with the addition of Red Oaks and a fescue grass mix for disturbed areas that will be left long, mowed once a year to discourage the incursion of invasive plant material. Between the building and the greenway path will be a mown fescue lawn. The south side of the building will be reserved for vehicular access to the entry and parking garage.
- Exterior Wall Systems: The exterior wall systems will meet or exceed the 2015 IECC standards for energy efficiency and will include a continuous air barrier and continuous insulation on all exterior wall enclosing heated spaces as well as insulation within the stud cavities. The exterior cladding materials will include a combination of masonry and metal panel rain screen systems that utilize and air space outboard of the insulation layer for efficient moisture management.
- Window Systems: All window systems in the project will meet or exceed 2015 IECC standards for u-value, shading coefficient and solar heat gain coefficient, including a thermally-broken frame and insulated, high-performance, low-E glazing to reduce

- thermal transfer. Large window expanses provide plenty of natural daylight to all building occupants.
- Roofing Systems: The roofing system will include a light-colored, reflective "cool roof" over continuous, sloped rigid insulation that meets or exceeds code requirements.
- **HVAC Systems:** The dwelling units will be provided with individualized systems providing either heating and cooling or both. System may include electric heat pumps or a hydronic gas fired heating system with gas fired domestic hot water heaters.
- **Plumbing Systems:** All plumbing fixtures in the proposed project will be low-flow fixtures. Individual EnergyStar rated instantaneous hot water heaters will be used for domestic hot water and heating.
- **Lighting Systems:** Interior lighting systems will use LED fixtures throughout the building, including the use of occupancy sensors. Exterior lighting design will include energy-efficient LED cutoff fixtures to minimize light pollution.
- **Appliances:** All appliances for the project will be EnergyStar rated.

Sincerely,

Dartagnan Brown | Founder + CEO



Icon

Outdoor Wall Sconce 3000K

Fixture Type:
Catalog Number:
Project:
Location:

Model & Size	Color Temp & CRI	Finish	Watt	LED Lumens	Delivered Lumens
○ WS-W54614 14"	3000K 90	O AL Brushed Aluminum BK Black BZ Bronze	10.9W	845	458

Example: WS-W54614-BZ

For custom requests please contact customs@waclighting.com

DESCRIPTION

Like a simple reference to something greater, the up and down lights accentuate linear architectural forms. A simple shape, with infinite applications, the Icon features a shieldedlight source for great low-glare illumination. Constructed with a solid die-cast aluminum and powder coated finish. The light engine is factory sealed for maximum protection against the

FEATURES

- Weather resistant powder coat finish
- Simple shape, simple idea, infinite applications
- Up & down light
- Shielded light source for great low-glare illumination
- Driver concealed within the fixture
- 5 year warranty

SPECIFICATIONS

Color Temp: 3000K

Input: 120-277 VAC,50/60Hz

CRI: 90

Dimming: ELV: 100-10% Rated Life: 54000 Hours

Mounting: Can be mounted on wall in all orientations

Standards: ETL, cETL,IP65

Wet Location Listed

Construction: Aluminum hardware with glass diffuser

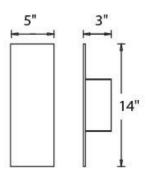


FINISHES:



Aluminum

LINE DRAWING:





Icon

Outdoor Wall Sconce 3000K

Fixture Type:	
Catalog Number:	
Project:	
Location:	

Model & Size	Color Temp & CRI	Finish	Watt	LED Lumens	Delivered Lumens
○ WS-W54620 20"	3000K 90	O AL Brushed Aluminum O BK Black O BZ Bronze	11.1W	847	478

Example: WS-W54620-BZ

For custom requests please contact customs@waclighting.com

DESCRIPTION

Like a simple reference to something greater, the up and down lights accentuate linear architectural forms. A simple shape, with infinite applications, the Icon features a shieldedlight source for great low-glare illumination. Constructed with a solid die-cast aluminum and powder coated finish. The light engine is factory sealed for maximum protection against the

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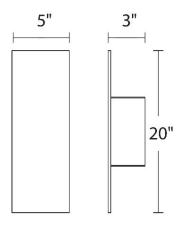
Construction: Aluminum hardware with glass diffuser



FINISHES:



LINE DRAWING:



WS-W54620

FIN - model: WP-LED1

Endurance Wallpack

WAC LIGHTING

Responsible Lighting®



S	P	EC	IF	IC	ΑT	IC	N	S
_		nc	tri		tio	n.	Di	_

Construction: Die-cast aluminum

Fixture Type:

Project:

Location:

Catalog Number:

Power: Integral driver in luminaire. Universal voltage input (120V-277V)

Dimming: 100% - 30% with 0 - 10V dimmer (120V - 277V)

100% - 15% with Electronic Low Voltage (ELV) dimmer (120V only)

Finish: Architectural Bronze, Graphite, and White **Standards:** IP66, Wet Location, ETL & cETL Listed

Total Harmonic Distortion: 35%

Operating Temperature: -40°C (-40°F) to 40°C (104°F)

PRODUCT DESCRIPTION

Die cast aluminum factory sealed housings with patent pending design for a water and dust proof IP66 rated outdoor luminaire

FEATURES

- Factory-Sealed LED Light Engine
- 20° Forward Throw Illumination
- Photo/Motion Sensor Compatible (Sold Separately)
- Built-in Level For Easy Adjustment
- Suitable to install in all directions
- Multi-Function Dimming: ELV (120V) or 0-10V
- 85 CRI
- 100,000 hour rated life

ORDER NUMBER

PHOTOMETRY 90 Left Plane Front Plane

		Power	Comparable	Colo	r Temp	Delivered Lumens	CBCP	Finish	
	WP-LED119	19W	39W HID	30 50	3000K 5000K	1390 1460	1030 1048		
5	WP-LED127	27W	70W HID	30 50	3000K 5000K	2075 2135	1461 1467	aBZ aGH aWT	Architectural Bronze Architectural Graphite Architectural White
	WP-LED135	35W	100W HID	30 50	3000K 5000K	2750 2825	1930 1921		

Example: WP-LED119-50-BZ

ACCESSORIES





WAC Lighting
www.waclighting.com
Phone (800) 526.2588 • Fax (800) 526.2585

Headquarters/Eastern Distribution Center44 Harbor Park Drive • Port Washington, NY 11050
Phone (516) 515.5000 • Fax (516) 515.5050

Western Distribution Center 1750 Archibald Avenue • Ontario, CA 91760 Phone (800) 526.2588 • Fax (800) 526.2585