

PORTSMOUTH APPROVAL CONDITIONS NOTE: ALL CONDITIONS ON THIS PLAN SET SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE CITY OF PORTSMOUTH SITE PLAN REVIEW REGULATIONS.

APPROVED BY THE PORTSMOUTH PLANNING BOARD

CHAIRMAN

DATE

LEGEND: PROPOSED



EROSION CONTROL NOTES & DETAILS

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DEMOLITION PLAN

UTILITY PLAN AND PROFILE

SEWER & WATER: PORTSMOUTH DEPARTMENT OF PUBLIC WORKS 680 PEVERLY HILL ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 427-1530 ATTN: JIM TOW

COMMUNICATIONS: CONSOLIDATED COMMUNICATIONS JOE CONSIDINE 1575 GREENLAND ROAD GREENLAND, N.H. 03840 Tel. (603) 427-5525

155 COMMERCE WAY PORTSMOUTH, N.H. 03801 Tel. (603) 679-5695 (X1037) ATTN: MIKE COLLINS

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PROPERTY LINE SETBACK EDGE OF PAVEMENT (EP CONTOUR SPOT ELEVATION UTILITY POLE WALL MOUNTED EXTERIOR LIGHTS TRANSFORMER ON CONCRETE PAD ELECTRIC HANDHOLD SHUT OFFS (WATER/GAS) GATE VALVE HYDRANT CATCH BASIN SEWER MANHOLE DRAIN MANHOLE TELEPHONE MANHOLE PARKING SPACE COUNT PARKING METER LANDSCAPED AREA TO BE DETERMINED CAST IRON PIPE COPPER PIPE DUCTILE IRON PIPE POLYVINYL CHLORIDE PIPE REINFORCED CONCRETE PIPE ASBESTOS CEMENT PIPE VITRIFIED CLAY PIPE EDGE OF PAVEMENT ELEVATION FINISHED FLOOR INVERT SLOPE FT/FT

PROPOSED RESIDENTIAL DEVELOPMENT 201 KEARSARGE WAY PORTSMOUTH, N.H.

TYPICAL

TEMPORARY BENCH MARK



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

PLAN SET SUBMITTAL DATE: 13 FEBRUARY 2020

PLAN REFERENCES 1) "SUBDIVISION PLAN TAX MAP 218-LOT 5, OWNER RICHARD P. FUSEGNI", BY AMBIT ENGINEERING, INC, SCALE: 1"=30', APRIL 2018 201 KEARSARGE WAY, PORTSMOUTH, NH, ROCKINGHAM COUNTY. RCRD-D-41295. TENNIS COURT 3 LOCATION MAP SCALE: 1'' = 300'LEGEND: N/F NOW OR FORMERLY RECORD OF PROBATE RP RCRD ROCKINGHAM COUNTY REGISTRY OF DEEDS RR SPK RAILROAD SPIKE (218) $\begin{pmatrix} 11\\ 21 \end{pmatrix}$ MAP 11/LOT 21 (1)N/F CITY OF PORTSMOUTH DPW CARTE IRON ROD FOUND OIR FND P.O. BOX 628 IRON PIPE FOUND O IP FND PORTSMOUTH, N.H. 03802 IRON ROD SET 2745/792 • IR SET DRILL HOLE FOUND • DH FND DRILL HOLE SET ODH SET NHDOT BOUND FOUND • NHHB OF EDGE OF PAVEMENT -----OVERHEAD WIRE () LEDGE OUTCROP 15,00 15,00 10,00 10,00 FND $\underbrace{\begin{array}{c} 218\\ 4 \end{array}}$ N/F DAVID T. MURRAY 182 BIRCH STREET PORTSMOUTH, N.H. 03801 3432/2644 "I CERTIFY THAT THIS PLAN WAS PREPARED UNDER MY DIRECT SUPERVISION, THAT IT IS THE RESULT OF A FIELD SURVEY BY THIS OFFICE AND HAS AN ACCURACY OF THE CLOSED TRAVERSE THAT EXCEEDS THE PRECISION OF CURVE TABLE 1:15,000." CURVE RADIUS 2-13-20 C1 JOHN R. CHAGNON, LLS DATE C2 C3 APPROVED BY THE PORTSMOUTH PLANNING BOARD JOHN CRAGNON DATE CHAIRMAN



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315 \circ NOTES: 1) PARCEL IS SHOWN ON THE CITY OF PORTSMOUTH ASSESSOR'S MAP 218 AS LOT 5. 2) OWNER OF RECORD: RICHARD P. FUSEGNI 201 KEARSARGE WAY PORTSMOUTH, N.H. 03801 5476/2661 (5979/2783) RCRD PLAN 0245 3) PARCEL IS NOT IN A FLOOD HAZARD ZONE AS SHOWN ON FIRM PANEL 33015C0259E, EFFECTIVE MAY 17, 2005. 4) EXISTING LOT AREA: 47,062 S.F. 1.0804 AC. 5) PARCEL IS LOCATED IN THE SINGLE RESIDENCE B (SRB) DISTRICT. 6) DIMENSIONAL REQUIREMENTS: MIN. LOT AREA: 15,000 S.F. FRONTAGE: 100 FT. SETBACKS: FRONT: 30 FT. SIDE: 10 FT. REAR: 30 FT. MAXIMUM STRUCTURE HEIGHT: 35 FT. MAXIMUM STRUCTURE COVERAGE: 20% MINIMUM OPEN SPACE: 40% 7) THE PURPOSE OF THIS PLAN IS TO SHOW THE SUBDIVISION OF TAX MAP 218 LOT 5 INTO 3 LOTS. 8) OAK STREET WAS CREATED BY A PLAN DATED 1919 AND WAS NEVER CONSTRUCTED. BY OPERATION OF LAW THE AREAS SHOWN BELONG TO THE RESPECTIVE LOTS BY WAY OF APPROPRIATION OF REVERSION RIGHTS. AREAS SHOWN ARE NOT INCLUDED IN EXISTING LOT AREA. 9) PROPOSED CONSERVATION EASEMENT AREA RESTRICTIONS SUBJECT TO REVIEW AND APPROVAL BY THE CITY OF PORTSMOUTH. CONSERVATION EASEMENT RESTRICTIONS WILL ALLOW FOR INSTALLATION AND MAINTENANCE OF PROPOSED DRAINAGE. 10) MAP 218 LOT 5-2 (PROPOSED LOT 1) WILL GRANT AN EASEMENT TO THE CITY OF PORTSMOUTH TO TURN AROUND IN DRIVEWAY OF LOT 5-2 FOR PLOWING AND ACCESS 11) STREET ADDRESSES SHALL MATCH DRIVEWAY LOCATIONS. 4 NOTE 10, 11 2/13/20 ADD NOTE 10 & CONSERVATION EASEMENT MARKER NOTE 2/4/20 ISSUED FOR APPROVAL 1/21/20 REVISED FOR SUBMISSION 10/8/19 0 ISSUED FOR COMMENT 4/16/19 DESCRIPTION DATE REVISIONS SUBDIVISION PLAN TAX MAP 218 - LOT 5 OWNER RICHARD P. FUSEGNI 201 KEARSARGE WAY CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM STATE OF NEW HAMPSHIRE SCALE: 1'' = 30'APRIL 2019 FB 311, PG 1 2258



JOBS2\JN2200s\JN2250s\JN2258\2019 Subdivision Lot 2\Plans & Specs\Site\2258 SUBDIVISION 2019.dwg, C1 S







AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

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NOTES:

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) THERE ARE NO WETLANDS ON SUBJECT PROPERTY.

5) BUILDINGS WILL BE GUTTERED TO DIRECT ROOF RUNOFF TO THE SIDE INFILTRATION TRENCH OR REAR OF THE LOT.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

4	STORM CHAMBERS, GRADING	2/13/20
3	EXTEND TRENCH	2/4/20
2	ISSUED FOR APPROVAL	1/21/20
1	SITE LAYOUT	12/13/19
0	ISSUED FOR COMMENT	10/8/19
NO.	DESCRIPTION	DATE
	REVISIONS	







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4) THERE ARE NO WETLANDS ON SUBJECT PROPERTY.

5) PROPOSED UTILITY CONNECTIONS TO BE REVIEWED AND APPROVED BY PORTSMOUTH DPW PRIOR TO ISSUANCE OF BUILDING PERMIT.

6) TEMPORARY WATER SERVICE SHALL BE PROVIDED TO MAP 218 LOTS 2 & 3 DURING CONSTRUCTION OF NEW WATER LINE.

7) BIRCH STREET SHALL BE RECLAIMED AND RECONSTRUCTED TO CITY STANDARDS.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

2	ELEC/CABLE, WATER, NOTE 7	2/13/20
1	AC, NOTES	2/4/20
0	ISSUED FOR COMMENT	1/21/20
NO.	DESCRIPTION	DATE
	REVISIONS	



SCALE: 1" = 20'

FB. 311, PG 1

JANUARY 2020

C3

2258

UTILITY PLAN

DEMOLITION NOTES

- A) THE LOCATIONS OF UNDERGROUND UTILITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT GUARANTEED BY THE OWNER OR THE DESIGNER. IT IS THE CONTRACTORS' RESPONSIBILITY TO LOCATE UTILITIES AND ANTICIPATE CONFLICTS. CONTRACTOR SHALL REPAIR EXISTING UTILITIES DAMAGED BY THEIR WORK AND RELOCATE EXISTING UTILITIES THAT ARE REQUIRED TO BE RELOCATED PRIOR TO COMMENCING ANY WORK IN THE IMPACTED AREA OF THE PROJECT.
- B) ALL MATERIALS SCHEDULED TO BE REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTORS UNLESS OTHERWISE SPECIFIED. THE CONTRACTOR SHALL DISPOSE OF ALL MATERIALS OFF-SITE IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS, ORDINANCES AND CODES. THE CONTRACTOR SHALL COORDINATE REMOVAL, RELOCATION, DISPOSAL, OR SALVAGE OF UTILITIES WITH THE OWNER AND APPROPRIATE UTILITY COMPANY.
- C) ANY EXISTING WORK OR PROPERTY DAMAGED OR DISRUPTED BY CONSTRUCTION/ DEMOLITION ACTIVITIES SHALL BE REPLACED OR REPAIRED TO THE ORIGINAL EXISTING CONDITIONS BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
- D) THE CONTRACTOR SHALL VERIFY LOCATION OF ALL EXISTING UTILITIES AND CALL DIG SAFE AT LEAST 72 HOURS PRIOR TO THE COMMENCEMENT OF ANY DEMOLITION/CONSTRUCTION ACTIVITIES.

GRAPHIC SCALE

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- E) SAWCUT AND REMOVE PAVEMENT ONE FOOT OFF PROPOSED EDGE OF PAVEMENT TRENCH IN AREAS WHERE PAVEMENT IS TO BE REMOVED.
- F) IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE CONDITIONS OF ALL THE PERMIT APPROVALS.
- G) THE CONTRACTOR SHALL OBTAIN AND PAY FOR ADDITIONAL CONSTRUCTION PERMITS. NOTICES AND FEES NECESSARY TO COMPLETE THE WORK AND ARRANGE FOR AND PAY FOR ANY INSPECTIONS AND APPROVALS FROM THE AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL AND OFF-SITE DISPOSAL OF MATERIALS REQUIRED TO COMPLETE THE WORK.
- H) THE CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL EXISTING STRUCTURES, CONCRETE, UTILITIES, VEGETATION, PAVEMENT, AND CONTAMINATED SOIL WITHIN THE WORK LIMITS SHOWN UNLESS SPECIFICALLY IDENTIFIED TO REMAIN. ANY EXISTING DOMESTIC / IRRIGATION SERVICE WELLS IN THE PROJECT AREA IDENTIFIED DURING THE CONSTRUCTION AND NOT CALLED OUT ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER FOR PROPER CAPPING / RE-USE.
- I) ALL WORK WITHIN THE CITY OF PORTSMOUTH RIGHT OF WAY SHALL BE COORDINATED WITH THE CITY OF PORTSMOUTH DEPARTMENT OF PUBLIC WORKS (DPW).
- J) REMOVE TREES AND BRUSH AS REQUIRED FOR COMPLETION OF WORK. CONTRACTOR SHALL GRUB AND REMOVE ALL SLUMPS WITHIN LIMITS OF WORK AND DISPOSE OF OFF-SITE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS.
- K) CONTRACTOR SHALL PROTECT ALL PROPERTY MONUMENTATION THROUGHOUT DEMOLITION AND CONSTRUCTION OPERATIONS. SHOULD ANY MONUMENTATION BE DISTURBED, THE CONTRACTOR SHALL EMPLOY A NH LICENSED LAND SURVEYOR TO REPLACE THEM.
- L) PROVIDE INLET PROTECTION BARRIERS AT ALL CATCH BASINS WITHIN CONSTRUCTION LIMITS AND MAINTAIN FOR THE DURATION OF THE PROJECT. INLET PROTECTION BARRIERS SHALL BE HIGH FLOW SILT SACK BY ACF ENVIRONMENTAL OR APPROVED EQUAL. INSPECT BARRIERS WEEKLY AND AFTER EACH RAIN OF 0.25 INCHES OR GREATER. CONTRACTOR SHALL COMPLETE A MAINTENANCE INSPECTION REPORT AFTER EACH INSPECTION. SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT OR MORE OFTEN IF WARRANTED OR FABRIC BECOMES CLOGGED. EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE START OF ANY CLEARING OR DEMOLITION ACTIVITIES.
- M) THE CONTRACTOR SHALL PAY ALL COSTS NECESSARY FOR TEMPORARY PARTITIONING, BARRICADING, FENCING, SECURITY AND SAFELY DEVICES REQUIRED FOR THE MAINTENANCE OF A CLEAN AND SAFE CONSTRUCTION SITE.

N) ANY CONTAMINATED MATERIAL REMOVED DURING THE COURSE OF THE WORK WILL REQUIRE HANDLING IN ACCORDANCE WITH NHDES REGULATIONS. CONTRACTOR SHALL HAVE A HEALTH AND SAFETY PLAN IN PLACE, AND COMPLY WITH ALL APPLICABLE PERMITS, APPROVALS, AUTHORIZATIONS, AND REGULATIONS

APPROVED BY THE PORTSMOUTH PLANNING BOARD

CHAIRMAN

DATE



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114

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3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

1/21/20 DATE

JANUARY 2020

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AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

NOTES:

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY WITHIN 100 FEET OF UNDERGROUND UTILITIES. THE EXCAVATOR IS RESPONSIBLE TO MAINTAIN MARKS. DIG SAFE TICKETS EXPIRE IN THIRTY DAYS.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

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4) ALL UTILITY MATERIALS TO BE APPROVED BY PORTSMOUTH WATER AND SEWER DEPARTMENTS.

5) UTILITY INSTALLATION SHALL BE WITNESSED BY THE C.O.P.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

2	WATER LINE	2/13/20
1	NOTES 4&5, STUB	2/4/20
0	ISSUED FOR COMMENT	1/21/20
NO.	DESCRIPTION	DATE
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- FB. 311, PG 1

EROSION CONTROL NOTES

CONSTRUCTION SEQUENCE

DO NOT BEGIN CONSTRUCTION UNTIL ALL LOCAL, STATE AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED.

INSTALL INLET PROTECTION AND PERIMETER CONTROLS, i.e., SILT FENCING OR SILTSOXX AROUND THE LIMITS OF DISTURBANCE BEFORE ANY EARTH MOVING OPERATIONS.

CUT AND GRUB ALL TREES, SHRUBS, SAPLINGS, BRUSH, VINES AND REMOVE OTHER DEBRIS AND RUBBISH AS REQUIRED.

REMOVE EXISTING HOUSE AND SHED

CONSTRUCT SITE UTILITIES.

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE "STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS.

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT EROSION.

DUST CONTROL: IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND MULCHING, DO NOT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILT FENCES AND SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM. ALL DAMAGED SILT FENCES AND SILTSOXX SHALL BE REPAIRED. SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

AVOID THE USE OF FUTURE OPEN SPACES (LOAM AND SEED AREAS) WHEREVER POSSIBLE DURING CONSTRUCTION. CONSTRUCTION TRAFFIC SHALL USE THE ROADBEDS OF FUTURE ACCESS DRIVES AND PARKING AREAS.

ADDITIONAL TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN AMOUNTS NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS--CONSTRUCT SILT FENCE OR SILTSOXX AROUND TOPSOIL STOCKPILE.

AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL. STUMPS SHALL BE DISPOSED OF IN AN APPROVED FACILITY.

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE. SETTLEMENT. SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH, WOODY DEBRIS, LEAVES. BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED, ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED: - BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED

- A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED

- A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED

- EROSION CONTROL BLANKETS HAVE BEEN INSTALLED

VEGETATIVE PRACTICE

FOR PERMANENT MEASURES AND PLANTINGS:

LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF 2 TONS PER ACRE.

FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 500 POUNDS PER ACRE OF 10-20-20 FERTILIZER.

SEED SHALL BE SOWN AT THE RATES SHOWN IN THE TABLE BELOW. IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AT A RATE OF 1.5 TO 2 TONS PER ACRE, AND SHALL BE HELD IN PLACE USING APPROPRIATE TECHNIQUES FROM THE EROSION AND SEDIMENT CONTROL HANDBOOK.

THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED. ANY AREAS WHICH ARE NOT SATISFACTORILY COVERED SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED.

A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE:

GENERAL COVER	PROPORTION SEEDING RATE
CREEPING RED FESCUE	50% 100 LBS/ACRE
KENTUCKY BLUEGRASS	50%
<u>SLOPE SEED</u> (USED ON AL	L SLOPES GREATER THAN OR EQUAL TO 3:1)
CREEPING RED FESCUE	42%
TALL FESCUE	42% 48 LBS/ACRE
BIRDSFOOT TREFOIL	16%

IN NO CASE SHALL THE WEED CONTENT EXCEED ONE PERCENT BY WEIGHT, ALL SEED SHALL COMPLY WITH APPLICABLE STATE AND FEDERAL SEED LAWS.

FOR TEMPORARY PROTECTION OF DISTURBED AREAS: MULCHING AND SEEDING SHALL BE APPLIED AT THE FOLLOWING RATES: PERENNIAL RYE: 0.7 LBS/1,000 S.F. 1.5 TONS/ACRE MULCH:

MAINTENANCE AND PROTECTION

THE CONTRACTOR SHALL MAINTAIN ALL LOAM & SEED AREAS UNTIL FINAL ACCEPTANCE AT THE COMPLETION OF THE CONTRACT. MAINTENANCE SHALL INCLUDE WATERING, WEEDING, REMOVAL OF STONES AND OTHER FOREIGN OBJECTS OVER 1/2 INCHES IN DIAMETER WHICH MAY APPEAR AND THE FIRST TWO (2) CUTTINGS OF GRASS NO CLOSER THEN TEN (10) DAYS APART. THE FIRST CUTTING SHALL BE ACCOMPLISHED WHEN THE GRASS IS FROM 2 1/2 TO 3 INCHES HIGH. ALL BARE AND DEAD SPOTS WHICH BECOME APPARENT SHALL BE PROPERLY PREPARED, LIMED AND FERTILIZED, AND RESEEDED BY THE CONTRACTOR AT HIS EXPENSE AS MANY TIMES AS NECESSARY TO SECURE GOOD GROWTH. THE ENTIRE AREA SHALL BE MAINTAINED, WATERED AND CUT UNTIL ACCEPTANCE OF THE LAWN BY THE OWNER'S REPRESENTATIVE.

THE CONTRACTOR SHALL TAKE WHATEVER MEASURES ARE NECESSARY TO PROTECT THE GRASS WHILE IT IS DEVELOPING.

TO BE ACCEPTABLE, SEEDED AREAS SHALL CONSIST OF A UNIFORM STAND OF AT LEAST 90 PERCENT ESTABLISHED PERMANENT GRASS SPECIES, WITH UNIFORM COUNT OF AT LEAST 100 PLANTS PER SQUARE FOOT.

SEEDED AREAS WILL BE FERTILIZED AND RESEEDED AS NECESSARY TO INSURE VEGETATIVE ESTABLISHMENT.

THE SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATION IS ESTABLISHED.

THE SILT FENCE OR SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL.

SILT FENCING AND SILTSOXX SHALL BE REMOVED ONCE VEGETATION IS ESTABLISHED, AND DISTURBED AREAS RESULTING FROM SILT FENCE AND SILTSOXX REMOVAL SHALL BE PERMANENTLY SEEDED.

WINTER NOTES

FILTREXX®

ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1. AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.

- 2" x 2" HARDWOOD

STAKES SPACED 10'

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.

AFTER NOVEMBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.



AREA TO BE PROTECTED

ELEVATION

- ALL MATERIAL TO MEET FILTREXX SPECIFICATIONS. FILLTREXX SYSTEM SHALL BE INSTALLED BY A CERTIFIED
- FILTREXX INSTALLER. THE CONTRACTOR SHALL MAINTAIN THE COMPOST FILTRATION SYSTEM IN A FUNCTIONAL CONDITION AT ALL TIMES. IT WILL BE
- ROUTINELY INSPECTED AND REPAIRED WHEN REQUIRED. 4. SILTSOXX DEPICTED IS FOR MINIMUM SLOPES, GREATER SLOPES MAY REQUIRE ADDITIONAL PLACEMENTS.
- THE COMPOST FILTER MATERIAL WILL BE DISPERSED ON SITE WHEN NO LONGER REQUIRED, AS DETERMINED BY THE ENGINEER.







CURB STOP

(120 PSI) ON

COPPER TO COPPER

-1"ø TYPE K COPPER

CUSTOMER'S PROPERTY

WATER MAIN IN STREET

WATER SERVICE CONNECTION (PORTSMOUTH) NTS







AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) ALL WATER LINE INSTALLATION WORK SHALL BE TO CITY OF PORTSMOUTH WATER DEPARTMENT STANDARDS. DETAILS MAY OR MAY NOT BE UP-TO-DATE.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

1	DETAIL J	2/4/20				
0	ISSUED FOR APPROVAL	1/21/20				
NO.	DESCRIPTION	DATE				
	REVISIONS					
	JOHN R. CHNCNON NO. COL					
SC	ALE AS NOTED JANUA	RY 2020				
	DETAILS	D2				











END VIEW

PART NO.	PIPE SIZE	Α	B (MAX.)	Н	L	W
1210-NP	12" / 300mm	6.5" / 165mm	10" / 254mm	6.5" / 165mm	25" / 635mm	29" / 735mm
1510-NP	15" / 375mm	6.5" / 165mm	10" / 254mm	6.5" / 165mm	25" / 635mm	29" / 735mm
1810-NP	18" / 450mm	7.5" / 190mm	15" / 380mm	6.5" / 165mm	32" / 812mm	35" / 890mm
2410-NP	24" / 600mm	7.5" / 190mm	18" / 450mm	6.5" / 165mm	36" / 900mm	45" / 1140mm
3012-NP	30" / 750mm	10.5" / 266mm	N/A	7.0" / 178mm	53" / 1345mm	68" / 1725mm
3612-NP	36" / 900mm	10.5" / 266mm	N/A	7.0" / 178mm	53" / 1345mm	68" / 1725mm

NOTE: PE THREADED ROD w/ WING NUTS PROVIDED FOR END SECTIONS 12"-24", 30" & 36" END SECTIONS TO BE WELDED TO PIPE PER MANUFACTURER'S RECOMMENDATIONS.





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

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PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.





JANUARY 2020

D4

DETAILS

AMBIT ENGINEERING, INC. CIVIL ENGINEERS AND LAND SURVEYORS

200 Griffin Road, Unit 3, Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

14 February 2020

Dexter Legg, Planning Board Chair City of Portsmouth 1 Junkins Avenue Portsmouth, NH 03801

RE: Request for Site Plan Approval at 201 Kearsarge Way, Tax Map 218 / Lot 5

Dear Chairman Legg and Planning Board Members:

On behalf of Richard Fusegni we hereby submit the attached Plans and supporting material for Planning Board Approval for the Proposed Subdivision at 201 Kearsarge Way. The project proposes the construction of 3 new single family homes with the associated and required site improvements. The site is currently a single family home on an oversized lot. The applicant, on his own, wishes to preserve the rear portion of the property as treed open space and is proposing a Conservation Easement in the location as shown on the plan set.

The following plans are included in our submission:

- Cover Sheet This shows the Development Team, Legend, Site Location, and Site Zoning.
- Subdivision Plan This plans show the proposed lot division lines and areas.
- Subdivision Site Plan C1 This plan shows the proposed driveway locations and conceptual building footprints.
- Grading and Erosion Control Plan C2 This plan shows the proposed grading, erosion control, and run-off treatment / mitigation.
- Utility Plan C3 This plan shows the proposed site utilities including sewer and water infrastructure and individual service connections.
- Demolition Plan C4 This plan shows site demolition. The existing single family residence will be removed from the property.
- Utility Plan and Profile P1 This plans shows utility improvements in Birch Street.
- Detail Sheets D1 to D4 These plans show the associated construction details.

Also included herewith is the following Supplemental Information to assist in the review of the project: Subdivision Checklist, Easement and Conservation Restrictions Example Deeds, and Drainage Analysis.

The project received a recommendation for Planning Board approval at the February 7, 2020 Technical Advisory Committee Meeting subject to stipulations. The stipulations, as well as our responses to the stipulations, are listed below:

- 1. The driveway to Lot 5-3 should be shifted as far from the intersection with Kearsarge Way as possible. The driveway has been relocated as far from the intersection as possible.
- 2. The drainage for the houses should be incorporated into the back yard areas where they can be maintained without impacting the portion of the property designated to be a conservation area. The drainage design has been reengineered as discussed at the TAC meeting. See Sheet C2, Details P and Q, and the revised Drainage Analysis.
- 3. Plans shall show 2" water main being abandoned. The plan has been revised to show the abandonment of the existing 2 inch water main. See Sheet C3 and P1.
- 4. Birch Street is to be reclaimed and reconstructed to City Standards. **This has been noted as Note 7 on Sheet C3.**
- 5. Owner shall provide easement to turn around in driveway of Lot 5-2 for plowing/city access. Note 10 has been be added to the subdivision plan in that regard.
- 6. Add location of gas, electric, and communication lines to plan. The electric and communication lines and services have been added to Sheet C3. Currently there are no gas lines along Kearsarge Way in this vicinity.
- 7. Applicant shall provide more detail about how the conservation restriction area is proposed to be monumented to prevent future encroachment and clearing by the lot owners. The conservation easement edge will be marked with placards in trees and ground markers. A note has been added to the subdivision plan.
- 8. Street addresses shall match the driveway locations. Note 11 has been added to the Subdivision Plan.
- 9. Driveway radii on Kearsarge Way shall be tightened. **The driveway radii has been tightened on the plans.**

We look forward to your review of this submission, and request approval of the site plan as submitted.

Sincerely,

John Chagnon

John R. Chagnon, PE CC: Richard Fusegni, Bernie Pelech

DRAINAGE ANALYSIS

SUBDIVISION PLAN 201 KEARSARGE WAY PORTSMOUTH, NH



JANUARY 14, 2020

Revised February 14, 2020



Ambit Engineering, Inc.

Civil Engineers and Land Surveyors 200 Griffin Road, Unit 3 Portsmouth, NH 03801 Phone: 603.430.9282; Fax: 603.436.2315 E-mail: jlm@ambitengineering.com (Ambit Job Number 2258)

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Post-Development Drainage	5
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References	8

APPENDIX

- A. Vicinity (Tax) Map
- B. Tables, Charts, Etc.
- C. HydroCAD Drainage Analysis Calculations
- D. Soil Survey Information
- E. FEMA FIRM Map
- F. INSPECTION & MAINTENANCE PLAN

ATTACHMENTS

Pre Development Drainage Plan - W1 Post Development Drainage Plan - W2

EXECUTIVE SUMMARY

This drainage analysis examines the pre-development (existing) and post-development (proposed) stormwater drainage patterns for the proposed subdivision of a residential lot and associated future site improvements at 201 Kearsarge Way in Portsmouth, NH. The site is shown on the City of Portsmouth Assessor's Tax Map 218 as Lot 5. The project proposes to subdivide the existing single lot into three lots. The total size of all the lots together is 47,062 square-feet (1.0804 acres).

The subdivision will provide for the future construction of a single-family residential structure on each lot, with associated landscaping, utilities and driveways. The new buildings will be serviced by public water and sewer. The development has the potential to increase stormwater runoff to adjacent properties, and therefore must be designed in a manner to prevent that occurrence. This will be done primarily by capturing stormwater runoff and routing it through appropriate stormwater facilities, designed to ensure that there will be no increase in peak runoff from the site as a result of this project.

SUBDIVISION PLAN

201 KEARSARGE WAY

PORTSMOUTH, NH

INTRODUCTION / PROJECT DESCRIPTION

This drainage report is designed to assist the owner, planning board, contractor, regulatory reviewer, and others in understanding the impact of the proposed development project on local surface water runoff and quality. The project site is shown on the City of Portsmouth, NH Assessor's Tax Map 218 as Lot 5. Bounding the site to north is an existing single family residential property and Mangrove Street. Bounding the site to east is Kearsarge Way and existing single family residential properties beyond. Bounding the site to south is Birch Street and an existing single family residential property beyond. Bounding the site to the west is forested land and beyond is the City of Portsmouth Spinnaker Point Recreation Center. The property is situated in the Single Residence B (SRB) District. A vicinity map is included in the Appendix to this report.

The proposed subdivision will demolish an existing single family residential structure and demolish other associated improvements such as an existing driveway. Once the land is subdivided, the new houses may not be built until a number of years in the future. Therefore this report makes some concept assumptions as to the future impervious coverage of the proposed lots; as requested by the City.

This report includes information about the existing site and the proposed subdivision necessary to analyze stormwater runoff and to design any required mitigation. The report includes maps of pre-development and post-development watersheds, subcatchment areas and calculations of runoff. The report will provide a narrative of the stormwater runoff and describe numerically and graphically the surface water runoff patterns for this site. Proposed stormwater management and treatment structures and methods will also be described, as well as erosion and sediment control practices. To fully understand the proposed site development the reader should also review a complete site plan set in addition to this report.

METHODOLOGY

"Extreme Precipitation" values from The Northeast Regional Climate Center (Cornell University) have been used for modeling purposes. Further, these values have been increased by 15% as prescribed by the NHDES Alteration of Terrain Bureau when constructing within the Great Bay Region. These values have been used in this analysis.

This report uses the US Soil Conservation Service (SCS) Method for estimating stormwater runoff. The SCS method is published in The National Engineering Handbook (NEH), Section 4 "Hydrology" and includes the Technical Release No. 20, (TR-20) "Computer Program for Project Formulation Hydrology", and Technical Release No. 55 (TR-55) "Urban Hydrology for Small Watersheds" methods. This report uses the HydroCAD version 10.0 program, written by HydroCAD Software Solutions LLC, Chocorua, N.H., to apply these methods for the calculation of runoff and for pond modeling. Rainfall data and runoff curve numbers are taken from "The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire."

Time of Concentration (Tc) is calculated by entering measured flow path data such as flow path type, length, slope and surface characteristics into the HydroCAD program. For the purposes of this report, a minimum time of concentration of 5 minutes is used.

The storm events used for the calculations in this report are the 2-year, 10-year, 25-year and 50-year (24-hour) storms. Watershed basin boundaries have been delineated using topographic maps prepared by Ambit Engineering and field observations to confirm.

SITE SPECIFIC INFORMATION

Based on the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS), Soil Survey of Rockingham County, New Hampshire the site is made up of one soil type:

799 – Urban land – Canton Complex (3-15% slopes), well drained with a typical depth to restrictive feature of more than 80 inches. This soil has a Hydrologic Soil Group (HSG) classification of A, with a Low runoff class. The percolation rate used for the infiltration calculations was 4 inches per hour.

A copy of the custom soil survey for this project site is included in the Appendix to this report.

The physical characteristics of the site consist of flat (1-3%) to moderate (10-20%) grades that generally slope downward from the high point at the center of the lot to the north (rear) and southeast (front). Elevations on the site range from 62 to 74 feet above sea level. The existing

site is partially developed and includes an existing building located at the front of the lot, with an asphalt driveway. Vegetation around the developed portion of the lot consists of established grasses, shrubs and trees. The rear of the lot is mostly undeveloped, forested land.

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 33015C0259E (effective date May 17, 2005), the project site is located in Zone X and is determined to be outside of the 0.2% annual chance floodplain. A copy of the FIRM map is included in the Appendix.

PRE-DEVELOPMENT DRAINAGE

In the pre-development condition, the site has been analyzed as three watershed basins (ES1, ES2 and ES3) based on localized topography and discharge location. As mentioned earlier in this report, there is a high point in the middle of the site that bisects the lot. Runoff that drains to the south, towards Kearsarge Way is analyzed as watershed basin ES1. The majority of this basin consists of previously developed land and includes a portion of the existing house, lawn area and asphalt pavement. The discharge location is identified in this report as Design Point 1 (DP1). The runoff curve number (CN) for basin ES1 is calculated to be 57 with impervious coverage of 33.16%.

Runoff that drains to the northwest is analyzed as watershed basin ES2. The majority of this basin consists of undeveloped land and includes a portion of the existing house, lawn area and undeveloped forest land. The discharge location is identified in this report as Design Point 2 (DP2). The runoff curve number (CN) for basin ES1 is calculated to be 37 with impervious coverage of 7.23%.

Runoff that drains to the northeast is analyzed as watershed basin ES3. The majority of this basin consists of undeveloped land and includes lawn area and undeveloped forest land. The discharge location is identified in this report as Design Point 3 (DP3). The runoff curve number (CN) for basin ES1 is calculated to be 35 with impervious coverage of 0.00%.

There is no existing stormwater detention or treatment on the site (aside from the minimal treatment achieved from infiltration).

 Table 1: Pre-Development Watershed Basin Summary

Watershed Basin ID	Basin Area (SF)	Tc (MIN)	CN	10-Year Runoff (CFS)	50-Year Runoff (CFS)	Design Point
ES1	15,047	5.0	57	0.53	1.37	DP1

ES2	29,568	5.0	37	0.04	0.59	DP2
ES3	2,453	5.0	35	0.00	0.03	DP3

POST-DEVELOPMENT DRAINAGE

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. In the post-development condition, the site has been analyzed as three major watershed basins, (PS1, PS2 and PS3 (PS2 has been broken down into two sub-watershed basins PS2a and PS2b based on future land use)). Since the design of the future houses is currently conceptual, some assumptions were made regarding the delineation of watershed basins and the determination of land usage. The delineation between Basins PS1, PS2 and PS3 correspond to approximately the same as the delineation between ES1, ES2 and ES3.

Similar to the pre-development condition, Basin PS1 consists of the area of the site that drains towards Kearsarge Way. The curve number for the concept design was applied to this basin. All runoff from the basin that does not infiltrate into the soil, is discharged to Design Point 1 (DP1). This allows for a direct review of Design Points to show the comparison of runoff from the site in the pre-development and post-development conditions.

The remainder of the site is analyzed as sub-watershed basins PS2a and PS2b. The rear of the lot is planned to be dedicated as a conservation area, free from future development and is modelled as sub-watershed basin PS2a. This basin consists entirely of undeveloped land and will be a mixture of forested and grassed areas.

Ten Stormtech Underground Storage Chambers have been designed to control stormwater runoff from the site. These chambers are enveloped in stone and have an open bottom to allow infiltration of stormwater from the system providing treatment. The chambers provide volume for storage within the system. Additionally, there is an "Isolator" row that provides pre-treatment for the removal of much of the larger suspended solids often found in stormwater runoff.

Basin PS2b is the portion of Basin PS2 that is outside of the proposed conservation area, and will include portions of the proposed houses. Similar to Basin PS1, the curve number for the current design was applied to this basin. Runoff from this basin will be combined with the runoff from PS2a and will be routed to the proposed pond for infiltration. This pond is analyzed as Design Point 2 (DP2).

Basin PS3 is similar to ES3 and is analyzed as Design Point 3 (DP3).

Watershed	Basin	Tc (MIN)	CN	10-Year	50-Year	Design Deint
Dasin ID	Area (Sr)	(\mathbf{WIIN})	CN	Kunon (CFS)	Kulloll (CFS)	Design Point
PS1	14,572	5.0	56	0.48	1.27	DP1
PS2a	17,879	5.0	32	0.00	0.12	DP2
PS2b	13,638	5.0	52	0.34	1.0	DP2
PS3	979	5.0	39	0.00	0.03	DP3

The overall impervious coverage of the area analyzed in this report for all basins **increases** from 7,126 square feet (15.1%) in the pre-development condition to 7,145 square feet (15.2%) in the post-development condition. Since the site represents an increase in impervious area, the project proposes the construction of ten StormTech Stormwater Infiltration Chambers to provide treatment, storage and infiltrate stormwater back into the soil. The design infiltration rate was determined from reviewing the NRCS Soil Conservation Service's published rates for the given soil conditions for the site. These rates were then reduced by half as prescribed by Alteration of Terrain Bureau permit guidance. This design infiltration rate is then 3.0 inches per hour.

Table 3 shows a summary of the comparison between pre-developed flows and post-developed flows for the design point. The comparison takes into account the reduced flows as a result of infiltration.

1 a D C J C C C C C D C C C D C C C C D C C C D C C C D C C C C D C C C C D C C C C D C C C C D C	Table 3: Pre-Develo	opment to Post-Deve	elopment Comparison
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	Q2 (CFS)	Q10 (CFS)	Q25 ((CFS)	Q50 (CFS)			
Design Point	Pre	Post	Pre	Post	Pre	Post	Pre	Post		
DP 1	0.12	0.10	0.53	0.48	0.94	0.87	1.37	1.27		
DP 2	0.00	0.00	0.04	0.03	0.20	0.15	0.59	0.36		
DP3	0.00	0.00	0.00	0.00	0.01	0.01	0.03	0.03		

EROSION AND SEDIMENT CONTROL PRACTICES

The erosion potential for this site as it exists is low due to the existing vegetation. During construction, the major potential for erosion is wind and stormwater runoff. The contractor will be required to inspect and maintain all necessary erosion control measures, as well as installing any additional measures as required. All erosion control practices shall conform to "The Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire." Some examples of erosion and sediment control measures to be utilized for this project during construction may include:

- Silt soxx located at the toe of disturbed slopes
- Stabilized construction entrances at all access points to the site
- Temporary mulching and seeding for disturbed areas
- Spraying water over disturbed areas to minimize wind erosion

After construction, permanent stabilization will be accomplished by permanent seeding, landscaping and surfacing the access drives and parking areas with either compacted gravel or asphalt paving.

CONCLUSION

The proposed development has been designed to match the pre-development drainage patterns to the greatest extent feasible. With the design of the holding pond and the infiltration, the post-development runoff rates are reduced to be below the pre-development runoff rates and will provide a level of treatment. Erosion and sediment control practices will be implemented for both the temporary condition during construction and for final stabilization after construction. Therefore, there are no negative impacts to downstream receptors or adjacent properties anticipated as a result of this project.

REFERENCES

- 1. City of Portsmouth, NH. Site Plan Review Regulations, amended December 18, 2014.
- 2. Comprehensive Environmental Inc. and New Hampshire Department of Environmental Services. *New Hampshire Stormwater Manual (Volumes 1, 2 and 3)*, December 2008 (Revision 1.0).
- Minnick, E.L. and H.T. Marshall. Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing Areas in New Hampshire, prepared by Rockingham County Conservation District, prepared for New Hampshire Department of Environmental Services, in cooperation with USDA Soil Conservation Service, August 1992.
- 4. HydroCAD Software Solution, LLC. *HydroCAD Stormwater Modeling System Version* 10.0, dated 2007.

APPENDIX A VICINITY (TAX) MAP



APPENDIX B

TABLES, CHARTS, ETC.

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New Hampshire
Location	
Longitude	70.776 degrees West
Latitude	43.086 degrees North
Elevation	0 feet
Date/Time	Tue, 14 Jan 2020 16:14:06 -0500

Q2 = 3.20 in. X 1.15 = 3.68 in. Q10 = 4.85 in. X 1.15 = 5.58 in. Q25 = 6.15 in. X 1.15 = 7.07 in. Q50 = 7.36 in. X 1.15 = 8.46 in. Q100 = 8.82 in. X 1.15 = 10.14 in.

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.04	1yr	0.70	0.98	1.21	1.56	2.02	2.65	2.91	1yr	2.35	2.80	3.21	3.93	4.53	1yr
2yr	0.32	0.50	0.62	0.81	1.02	1.30	2yr	0.88	1.18	1.51	1.93	2.48	3.20	3.56	2yr	2.83	3.42	3.92	4.66	5.31	2yr
5yr	0.37	0.58	0.73	0.97	1.24	1.60	5yr	1.07	1.46	1.88	2.42	3.13	4.05	4.56	5yr	3.59	4.39	5.02	5.91	6.68	5yr
10yr	0.41	0.65	0.82	1.11	1.44	1.88	10yr	1.25	1.72	2.22	2.88	3.73	4.85	5.51	10yr	4.29	5.30	6.06	7.08	7.95	10yr
25yr	0.48	0.76	0.96	1.33	1.76	2.32	25yr	1.52	2.13	2.76	3.61	4.72	6.15	7.07	25yr	5.44	6.80	7.76	8.98	10.01	25yr
50yr	0.53	0.85	1.09	1.53	2.06	2.74	50yr	1.77	2.51	3.27	4.30	5.64	7.36	8.55	50yr	6.51	8.22	9.37	10.76	11.93	50yr
100yr	0.60	0.97	1.25	1.76	2.40	3.22	100yr	2.07	2.96	3.86	5.11	6.72	8.82	10.34	100yr	7.80	9.94	11.31	12.89	14.22	100yr
200yr	0.67	1.09	1.41	2.03	2.80	3.80	200yr	2.41	3.49	4.58	6.09	8.03	10.56	12.50	200yr	9.35	12.02	13.66	15.46	16.95	200yr
500yr	0.79	1.30	1.69	2.46	3.44	4.72	500yr	2.97	4.34	5.71	7.64	10.16	13.42	16.08	500yr	11.88	15.46	17.53	19.66	21.41	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.36	0.44	0.59	0.73	0.88	1yr	0.63	0.87	0.92	1.32	1.67	2.21	2.48	1yr	1.96	2.39	2.85	3.16	3.86	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.16	1.37	1.82	2.34	3.05	3.44	2yr	2.70	3.31	3.81	4.53	5.06	2yr
5yr	0.35	0.54	0.67	0.92	1.17	1.40	5yr	1.01	1.37	1.61	2.12	2.74	3.78	4.17	5yr	3.34	4.01	4.70	5.51	6.22	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.56	1.81	2.40	3.07	4.36	4.84	10yr	3.86	4.66	5.41	6.38	7.16	10yr

precip.eas.cornell.edu/data.php?1579036456492

1/14/2020

Extreme Precipitation Tables: 43.086°N, 70.776°W

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
25yr	0.44	0.67	0.83	1.18	1.56	1.90	25yr	1.34	1.86	2.10	2.77	3.55	4.67	5.87	25yr	4.13	5.64	6.61	7.75	8.64	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.16	50yr	1.52	2.12	2.35	3.09	3.95	5.27	6.77	50yr	4.66	6.51	7.67	8.99	9.97	50yr
100yr	0.53	0.81	1.01	1.46	2.00	2.47	100yr	1.73	2.41	2.62	3.44	4.38	5.91	7.82	100yr	5.23	7.52	8.91	10.43	11.50	100yr
200yr	0.59	0.89	1.12	1.63	2.27	2.81	200yr	1.96	2.75	2.93	3.81	4.83	6.61	9.02	200yr	5.85	8.67	10.34	12.13	13.28	200yr
500yr	0.68	1.02	1.31	1.90	2.70	3.36	500yr	2.33	3.29	3.40	4.36	5.51	7.66	10.89	500yr	6.78	10.47	12.58	14.82	16.06	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.44	0.54	0.72	0.89	1.08	1yr	0.77	1.06	1.25	1.74	2.21	2.98	3.15	1yr	2.64	3.03	3.57	4.37	5.03	1yr
2yr	0.33	0.52	0.64	0.86	1.06	1.26	2yr	0.92	1.24	1.48	1.96	2.51	3.42	3.69	2yr	3.03	3.55	4.07	4.82	5.62	2yr
5yr	0.40	0.61	0.76	1.05	1.33	1.61	5yr	1.15	1.58	1.88	2.53	3.24	4.32	4.94	5yr	3.83	4.75	5.36	6.35	7.13	5yr
10yr	0.47	0.72	0.89	1.24	1.60	1.97	10yr	1.38	1.92	2.27	3.10	3.94	5.32	6.18	10yr	4.71	5.94	6.79	7.81	8.72	10yr
25yr	0.57	0.87	1.08	1.55	2.03	2.55	25yr	1.75	2.50	2.94	4.06	5.13	7.79	8.31	25yr	6.89	7.99	9.10	10.29	11.37	25yr
50yr	0.67	1.01	1.26	1.81	2.44	3.10	50yr	2.11	3.04	3.58	4.98	6.28	9.76	10.41	50yr	8.64	10.01	11.37	12.67	13.91	50yr
100yr	0.78	1.18	1.48	2.14	2.93	3.78	100yr	2.53	3.69	4.35	6.13	7.70	12.22	13.05	100yr	10.81	12.55	14.22	15.62	17.03	100yr
200yr	0.91	1.37	1.74	2.52	3.52	4.61	200yr	3.03	4.50	5.31	7.54	9.45	15.34	16.37	200yr	13.57	15.74	17.80	19.26	20.85	200yr
500yr	1.13	1.68	2.16	3.15	4.47	5.97	500yr	3.86	5.84	6.89	9.95	12.41	20.74	22.11	500yr	18.36	21.26	23.96	25.39	27.26	500yr



SCS METHODS

Technical Release - 55 Urban Hydrology for Small Watersheds

TR-55 presents simplified procedures to calculate storm runoff volume, peak rate of discharge, partial hydrographs and storage volumes for water control structures. The procedures are applicable to small watersheds, especially urbanizing watersheds with time of concentration between 0.1 hours and 10.0 hours. TR-55 is an approximation of the more detailed TR-20 method and does not have TR-20's capability to flood route. The user should examine the sensitivity of the analysis being conducted to ensure that the degree of error is tolerable. TR-55 contains two methods, the Tabular Hydrograph method and the Graphical Peak Discharge method. The accuracy of both methods is comparable; they differ only in their output. Both methods are based on open and unconfined flow over land and in channels.

The TR-55 Tabular Method can develop partial composite flood hydrographs at any point in a watershed by dividing the watershed into homogeneous subareas. By doing this, the method can estimate runoff from a larger nonhomogeneous watershed. The method is especially applicable for estimating the effects of land use change in a portion of a watershed. It can also be used to estimate the effects of proposed structures. The TR-55 Graphical Peak Discharge method calculates peak discharge using an assumed unit hydrograph and a thorough, but rapid, evaluation of the soils, slope, and surface cover characteristics of the watershed. This method is recommended for use in the design of all erosion and sediment control measures and simple stormwater management practices. When more detail and accuracy are required or when an accurate simulation of natural conditions is required, one of the other appropriate methods should be used. The TR-55 Graphical Peak Discharge method is the method that is discussed in this manual.

SCS TR-55 Graphical Peak Discharge Method

The peak discharge equation used in this method is:

$$q_p = q_u A_m Q F_p$$

where:

 q_p is the peak discharge in cubic feet per second (cfs).

 q_u is the unit peak discharge in cubic feet per second per square mile per inch of runoff (csm/in).

 A_m is the drainage area in square miles.

Q is the runoff from the watershed in inches.

 F_p is a pond and swamp adjustment factor that can be applied for ponds or swamps that are spread throughout the watershed and not in the time of concentration flow path.

Technical Release-20 Computer Program for Project Formulation Hydrology

The TR-20 computer program assists the engineer in hydrologic evaluation of flood events for use in analysis of water resource projects. The program is a single event model which computes direct runoff resulting from any natural or synthetic rainstorm. It develops flood hydrographs from runoff and routes the flow through stream channels and reservoirs. It combines the routed hydrograph with those from tributaries and computes the peak discharges, their times of occurrence and the water surface elevations at any desired cross section or structure. The program provides for the analysis of up to nine different rainstorm distributions over a watershed under various combinations of land treatment. The analysis can be performed on as many as 200 reaches and 99 structures in any one continuous run. The procedure should probably not be used for subarea drainage areas less than 5 acres nor more than 20 square miles.

Input Data Required

The following information is required to use TR-20:

Drainage Area - The drainage area of each subwatershed in square miles.

Runoff Curve Number - A factor that relates mass rainfall to mass runoff. It is based on soil characteristics, cover type, and land treatment. Tables 6-4.1 - 6-4.3 provides runoff curve numbers for urban areas and agricultural areas.

Time of Concentration - The time which would be required for the surface runoff from the hydraulically most remote part of the drainage area to reach the point being evaluated. A more detailed discussion of time of concentration is found later in this chapter.

Reach Length - The length of the stream or valley in feet selected for generally constant hydraulic characteristics for use in the study. A watershed may have several reaches in the flow path.

Cross Section Information - This information consists of either surveyed valley and channel sections with appropriate Manning's "n" values or "x" and "m" discharge coefficient values obtained from nomographs in the TR-20 documentation for the valley and channel reach.

Rainfall Data - The average depth, in inches, of rainfall occurring over a watershed or subwatershed for a given design frequency and duration storm event.

Structural Data - Information on any culverts, bridges, or reservoirs in the watershed that includes elevations, discharges, and storage behind the structures.

Output Data

The type and amount of output can be controlled by options within the program. In general the output data will provide estimates of peak flow, hydrographs, peak times, runoff volumes, and water surface elevations at any location within the watershed.

ESTIMATING RUNOFF FACTORS

The major factors that must be taken into consideration when estimating runoff from a given watershed include soils, rainfall, and the land use characteristics.

<u>Soils</u>

Infiltration rates of soils vary widely and are affected by subsurface permeability as well as surface intake rates. Soils are divided into four hydrologic soil groups: A, B, C, and D, according to their minimum infiltration rate:

Group A soils have low runoff potential and high infiltration rates even when thoroughly wetted. They consist chiefly of deep, well to excessively drained sands or gravels and have a high rate of water transmission (greater than 0.3 in/hr). Soil textures in this group include: sands, loamy sands, or sandy loams.

Group B soils have moderate infiltration rates and consist chiefly of moderately well to well drained soils with moderately fine to moderately coarse textures. The soils have a moderate rate of water transmission (0.15 - 0.30 in/hr). Soil textures in this group include: silt loams and loams.

Group C soils have low infiltration rates and consist chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine to fine texture. These soils have a low rate of water transmission (0.05 - 0.15 in/hr). Soil textures in this group are the sandy clay loams.

Group D soils have high runoff potential. They have very low infiltration rates and consist chiefly of clay soils with a high swelling potential, soils with a permanent high water table, soils with a clay pan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very low rate of water transmission (0 - 0.05 in/hr). Soil textures in this group include: clay loams, silty clay loams, sandy clays, silty clays and clays.

Hydrologic soil groups for various soils are identified in soil surveys published by the Soil Conservation Service which are available at the local County Conservation District office. Two lists, "Hydrologic Soil Groups for Determining Runoff in New Hampshire" by group type and by soil series, are located at the end of this chapter.

Rainfall

The highest peak discharges from small watersheds are usually caused by intense brief rainfalls that may occur as distinct events or a part of a longer storm. The intensity of the rainfall varies considerably during a storm as well as over geographic regions. SCS developed four synthetic 24-hour rainfall distributions for TR-55. Figure 6-5 shows the distribution boundary found in New Hampshire. Type II is the most intense short duration rainfall and type III represents the area of New Hampshire where tropical storms bring large 24-hour rainfall amounts.

Figures 6-6 through 6-11 provide rainfall maps for New Hampshire for the 2-, 5-, 10-, 25-, 50-, and 100-year frequency 24-hour duration storm events.



Source: USDA Soil Conservation Service



Source: USDA Soil Conservation Service



Source: USDA Soil Conservation Service



Source: USDA Soil Conservation Service



Source: USDA Soil Conservation Service


Source: USDA Soil Conservation Service



Runoff Curve Number (RCN)

The runoff curve number is a factor that relates mass rainfall to mass runoff. It is based on soil characteristics, cover type, hydrologic condition, and land treatment. Tables 6-4.1 through 6-4.3 provide runoff curve numbers for urban areas, cultivated agricultural areas, and other agricultural areas for various hydrologic conditions

Cover type relates to the kind of cover found on the soil such as vegetation, bare soil, and impervious surfaces such as parking areas, roofs, streets, and roads.

Hydrologic condition indicates the effects of cover type and treatment on infiltration and runoff rates. It is generally estimated from the density of plant and crop residue on the area. Good hydrologic condition indicates that the soil usually has low runoff potential for that specific hydrologic soil group, cover type and treatment. Some factors to consider in estimating the effect of cover on infiltration and runoff are: canopy or density of leaves, amount of year-round cover, amount of grass or close-seeded legumes in a rotation, percent of residue cover, and the degree of surface roughness.

Treatment is a cover type modifier used to describe the management of cultivated agricultural lands. It includes mechanical practices such as contouring and terracing, and management practices, such as crop rotations and reduced or no tillage.

Use for the design of temporary measures during grading and construction. Impervious area percent for urban areas under development vary considerably. The user will determine the percent impervious. Then using the newly graded area RCN and Table 6-4, the composite RCN can be computed for any degree of development. GROUP impervious areas is For land uses with impervious areas, curve numbers are computed assuming that 100% of runoff from impervious areas directly connected to the drainage system. Pervious areas (lawn) are considered to be equivalent to lawns in good condition and the impervious areas have an RCN of 98. CURVE NUMBERS FOR HYDROLOGIC SOIL 8 2 8 98 8285 88 23 828283 2 Δ 228 98 98 92 92 22 8 32825 c 2 388 86 88288 82 ខ 88333 8 œ 843 98 8228 85 2 < 22723 4 Average percent² impervious area² 58 \$ **42238** drs, open spaces, parks, golf courses, cemeteries, etc. good condition; grass cover on 75% or more of the area fair condition; grass cover on 50% to 75% of the area poor condition; grass cover on 50% or less of the area FULLY DEVELOPED URBAN AREAS¹ (Vegetation Established) DEVELOPING URBAN AREAS³ (No vegetation Established) Cover type and hydrologic condition Paved parking lots, roofs, driveways, etc. Row houses, town houses, and residential COVER DESCRIPTION paved with curbs and storm sewers with lot sizes 1/8 acre or less Connercial and business areas Includes paved streets. paved with open ditches Industrial districts Average Lot size Streets and roads; Newly graded area 1/4 BCre 1/3 BCre 1/2 BCre 1 acre 2 acre Residential gravel dirt Lawns, -~ m

RUNOFF CURVE NUMBERS (Average Watershed Condition)

1

TABLE 6-4.1

Source: USDA Soil Conservation Service

Soil Conservation Service USDA Source:

5

1

TABLE 6-4.3 -- RUNOFF CURVE NUMBERS (Average Watershed Condition)

휭							10			N 0 -		2			2				
OIL GR			6	62	58	R	20	βK.		81	22	20	86		x				
ologic s	ы		2	8 R	2	7	8	22		2	5.2	2	23		82	24			
FOR HYDR	<u>.</u>		ł	ۍ و	5	53	R	5 8 <u>5</u>		67	0 89 0	8	85		74				
RVE NUMBERS	<		ę	2 03 0 7	6 M	30	22	35 35		89	ያይ	45	2 R		59				
Hydrologic 6	condition			poor fair	good		poor	good		poor	good	poor	fair good		1	ent ground cover density. percent ground cover density. ent ground cover density.			
COVER DESCRIPTION	Cover type and hydrologic condition	NON-CULTIVATED AGRICULTURAL LAND	Pasture, grassland, or range - continuous forage	tor grazing		Meadow - continuous grass, protected from grazing and generally mowed for hay	Woods-grass combination (orchard or tree farm)		Brush - brush-weed-grass mixture with brush the	major element		Woods		Farmsteads - buildings, lanes, driveways, and	surrounding lots	 Poor hydrologic condition has less than 50 perc Fair hydrologic condition has between 50 and 75 Good hydrologic condition has more than 75 perc 			

Source: USDA Soil Conservation Service

APPENDIX C HYDROCAD DRAINAGE ANALYSIS CALCULATIONS



Area Listing (selected nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
14,182	39	>75% Grass cover, Good, HSG A (ES1, ES2, ES3)
3,062	98	Paved parking, HSG A (ES1, ES2)
4,066	98	Roofs, HSG A (ES1, ES2)
25,758	30	Woods, Good, HSG A (ES1, ES2, ES3)
47,068	43	TOTAL AREA

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
47,068	HSG A	ES1, ES2, ES3
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
47,068		TOTAL AREA

2258 Existing Conditions

Prepared by Ambit Engine	eering, Inc.
HydroCAD® 10.00 s/n 00801	© 2013 HydroCAD Software Solutions LLC

Printed 2/14/2020 Page 4

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Sub
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nur
 14,182	0	0	0	0	14,182	>75% Grass	
						cover, Good	
3,062	0	0	0	0	3,062	Paved parking	
4,066	0	0	0	0	4,066	Roofs	
25,758	0	0	0	0	25,758	Woods, Good	
47,068	0	0	0	0	47,068	TOTAL AREA	

Ground Covers (selected nodes)

2258 Existing Conditions	Type III 24-hr 2-Year Rainfall=3.68"
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HydroCAD® 10.00 s/n 00801 © 2013 HydroCA	D Software Solutions LLC Page 5
Time span=0.00- Runoff by SCS TR Reach routing by Stor-Ind+Tra	48.00 hrs, dt=0.01 hrs, 4801 points -20 method, UH=SCS, Weighted-CN ans method - Pond routing by Stor-Ind method
Subcatchment ES1: ES1	Runoff Area=15,047 sf 33.16% Impervious Runoff Depth=0.49" Tc=5.0 min CN=57 Runoff=0.12 cfs 608 cf
Subcatchment ES2: ES2	Runoff Area=29,568 sf 7.23% Impervious Runoff Depth=0.00" Tc=5.0 min CN=37 Runoff=0.00 cfs 11 cf
Subcatchment ES3: ES3	Runoff Area=2,453 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=35 Runoff=0.00 cfs 0 cf
Reach DP1: DP1	Inflow=0.12 cfs 608 cf Outflow=0.12 cfs 608 cf
Reach DP2: DP2	Inflow=0.00 cfs 11 cf Outflow=0.00 cfs 11 cf
Reach DP3: DP3	Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf

Total Runoff Area = 47,068 sfRunoff Volume = 619 cfAverage Runoff Depth = 0.16"84.86% Pervious = 39,940 sf15.14% Impervious = 7,128 sf

Summary for Subcatchment ES1: ES1

Runoff = 0.12 cfs @ 12.11 hrs, Volume= 608 cf, Depth= 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.68"

Area (sf)	CN	Description							
2,014	98	Roofs, HSG A							
2,975	98	Paved parking, HSG A							
3,007	30	Woods, Good, HSG A	Noods, Good, HSG A						
7,051	39	>75% Grass cover, Go	od, HSG A						
15,047	15,047 57 Weighted Average								
10,058		66.84% Pervious Area							
4,989		33.16% Impervious Are	a						
Tc Length	Slop	e Velocity Capacity	Description						
(min) (feet)	(ft/) (ft/sec) (cfs)							
5.0			Direct Entry,						
		Subcatc	hment ES1: ES1						
	Hydrograph								



Summary for Subcatchment ES2: ES2

Runoff = 0.00 cfs @ 23.65 hrs, Volume= 11 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.68"

	Area (sf)	CN	Description				
	2,052	98	Roofs, HSG	A A			
	87	98	Paved park	ing, HSG A			
	21,660	30	Woods, Go	od, HSG A			
	5,769	39	>75% Gras	s cover, Go	od, HSG A		
	29,568	37	Weighted A	verage			
	27,429		92.77% Per	vious Area			
	2,139		7.23% Impe	ervious Area	а		
	Tc Length	n Slope	Velocity	Capacity	Description		
(n	nin) (feet)) (ft/̈ft)	(ft/sec)	(cfs)	•		
	5.0				Direct Entry,	,	
				Subcato	hment ES2:	ES2	
				Hydrog	graph		
	0.001-	·					Dupoff
	0.001	·		- + + - <mark>0.(</mark>	00 cfs	iiiiiiiiii-	
	0.001	·/				Type III 24-hr	
	0.001		- L		2-Y	ear Rainfall=3.68"	
	0.001	 	 			off Aroa-20 568 of	
	0.000				Kun		
(s	0.000					noff Volume=11 cf	
(cf	0.000		 			unoff Depth=0.00"	
<u> 0</u>	0.000	i i l					
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			///////////////////////////////////////				-

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Summary for Subcatchment ES3: ES3

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.68"



Summary for Reach DP1: DP1

Inflow A	Area	=	15,047 sf,	, 33.16% In	npervious,	Inflow Depth =	0.49"	for 2-	Year event
Inflow	=	=	0.12 cfs @	12.11 hrs,	Volume=	608 c	f		
Outflow	v =	=	0.12 cfs @	12.11 hrs,	Volume=	608 c	f, Atte	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	i =		29,568 sf.	, 7.23% lr	npervious,	Inflow Depth =	0.00	" for 2-`	Year event
Inflow		=	(0.00 cfs @	23.65 hrs,	Volume=	11 0	of		
Outflov	N	=		0.00 cfs @	23.65 hrs,	Volume=	11 c	of, Att	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area	=	2,453 sf,	0.00% Impervious,	Inflow Depth = $0.00"$	for 2-Year event
Inflow	=	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Outflow	/ =	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

2258 Existing Conditions Prepared by Ambit Engineering, Inc. HydroCAD® 10.00 s/n 00801 © 2013 HydroCA	Type III 24-hr 10-Year Rainfall=5.58" Printed 2/14/2020 D Software Solutions LLC Page 12
Time span=0.00- Runoff by SCS TR Reach routing by Stor-Ind+Tra	48.00 hrs, dt=0.01 hrs, 4801 points -20 method, UH=SCS, Weighted-CN ans method - Pond routing by Stor-Ind method
Subcatchment ES1: ES1	Runoff Area=15,047 sf 33.16% Impervious Runoff Depth=1.43" Tc=5.0 min CN=57 Runoff=0.53 cfs 1,789 cf
Subcatchment ES2: ES2	Runoff Area=29,568 sf 7.23% Impervious Runoff Depth=0.25" Tc=5.0 min CN=37 Runoff=0.04 cfs 607 cf
Subcatchment ES3: ES3	Runoff Area=2,453 sf 0.00% Impervious Runoff Depth=0.17" Tc=5.0 min CN=35 Runoff=0.00 cfs 35 cf
Reach DP1: DP1	Inflow=0.53 cfs 1,789 cf Outflow=0.53 cfs 1,789 cf
Reach DP2: DP2	Inflow=0.04 cfs 607 cf Outflow=0.04 cfs 607 cf
Reach DP3: DP3	Inflow=0.00 cfs 35 cf Outflow=0.00 cfs 35 cf

Total Runoff Area = 47,068 sf Runoff Volume = 2,431 cfAverage Runoff Depth = 0.62"84.86% Pervious = 39,940 sf15.14% Impervious = 7,128 sf

Summary for Subcatchment ES1: ES1

Runoff = 0.53 cfs @ 12.09 hrs, Volume= 1,789 cf, Depth= 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.58"

A	Area (sf)	CN	Description			
	2,014	98	Roofs, HSC	βA		
	2,975	98	Paved park	ing, HSG A		
	3,007	30	Woods, Go	od, HSG A		
	7,051	39	>75% Gras	s cover, Go	od, HSG A	
	15,047	57	Weighted A	verage		
	10,058		66.84% Pe	rvious Area		
	4,989		33.16% Imp	pervious Ar	ea	
_				•		
TC	Length	Slop	e Velocity	Capacity	Description	
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)		
5.0					Direct Entry,	

Subcatchment ES1: ES1



Summary for Subcatchment ES2: ES2

Runoff = 0.04 cfs @ 12.44 hrs, Volume= 607 cf, Depth= 0.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.58"

Area (sf)	CN	Description		
2,052	98	Roofs, HSG	βA	
87	98	Paved park	ing, HSG A	Α
21,660	30	Woods, Go	od, HSG A	N Contraction of the second seco
5,769	39	>75% Gras	s cover, Go	ood, HSG A
29,568	37	Weighted A	verage	
27,429)	92.77% Pei	vious Area	a
2,139		7.23% Impe	ervious Area	ea
Tc Lengt	h Sloj	pe Velocity	Capacity	Description
(min) (fee	t) (ft/	ft) (ft/sec)	(cfs)	
5.0				Direct Entry,

Subcatchment ES2: ES2



Summary for Subcatchment ES3: ES3

Runoff = 0.00 cfs @ 13.75 hrs, Volume= 35 cf, Depth= 0.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.58"



Summary for Reach DP1: DP1

Inflow A	Area	=	15,047 sf,	, 33.16% In	npervious,	Inflow Depth =	1.43"	for 10	-Year event
Inflow	=	=	0.53 cfs @	12.09 hrs,	Volume=	1,789 c	f		
Outflow	/ =	=	0.53 cfs @	12.09 hrs,	Volume=	1,789 c	f, Atte	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	a =		29,568 sf	, 7.23% l	mpervious,	Inflow Depth =	0.25"	for 10)-Year event
Inflow		=	(0.04 cfs @	12.44 hrs	, Volume=	607 c	f		
Outflov	v	=	(0.04 cfs @	12.44 hrs	, Volume=	607 c	f, Atte	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area	a =	2,453 sf.	, 0.00% Ir	npervious,	Inflow Depth =	0.17"	for 10-Year ev	ent
Inflow		=	0.00 cfs @	13.75 hrs,	Volume=	35 c	f		
Outflow	v	=	0.00 cfs @	13.75 hrs,	Volume=	35 c	f, Atte	n= 0%, Lag= 0.0) min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

2258 Existing Conditions	Type III 24-hr 25-Year Rainfall=7.07"
Prepared by Ambit Engineering, Inc.	Printed 2/14/2020
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Time span=0.00-4 Runoff by SCS TR- Reach routing by Stor-Ind+Tra	48.00 hrs, dt=0.01 hrs, 4801 points 20 method, UH=SCS, Weighted-CN ins method . Pond routing by Stor-Ind method
Subcatchment ES1: ES1	Runoff Area=15,047 sf 33.16% Impervious Runoff Depth=2.36" Tc=5.0 min CN=57 Runoff=0.94 cfs 2,959 cf
Subcatchment ES2: ES2	Runoff Area=29,568 sf 7.23% Impervious Runoff Depth=0.65" Tc=5.0 min CN=37 Runoff=0.20 cfs 1,599 cf
Subcatchment ES3: ES3	Runoff Area=2,453 sf 0.00% Impervious Runoff Depth=0.51" Tc=5.0 min CN=35 Runoff=0.01 cfs 105 cf
Reach DP1: DP1	Inflow=0.94 cfs 2,959 cf Outflow=0.94 cfs 2,959 cf
Reach DP2: DP2	Inflow=0.20 cfs 1,599 cf Outflow=0.20 cfs 1,599 cf
Reach DP3: DP3	Inflow=0.01 cfs 105 cf Outflow=0.01 cfs 105 cf

Total Runoff Area = 47,068 sfRunoff Volume = 4,663 cfAverage Runoff Depth = 1.19"84.86% Pervious = 39,940 sf15.14% Impervious = 7,128 sf

Summary for Subcatchment ES1: ES1

Runoff = 0.94 cfs @ 12.08 hrs, Volume= 2,959 cf, Depth= 2.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=7.07"

Ar	ea (sf)	CN	Description			
	2,014	98	Roofs, HSC	Э А 🗌		
	2,975	98	Paved park	ing, HSG A	۱.	
	3,007	30	Woods, Go	od, HSG A		
	7,051	39	>75% Gras	s cover, Go	ood, HSG A	
	15,047	57	Weighted A	verage		
	10,058		66.84% Pei	vious Area		
	4,989		33.16% Imp	pervious Are	ea	
Тс	Length	Slop	e Velocity	Capacity	Description	
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)		
5.0					Direct Entry,	

Subcatchment ES1: ES1



Summary for Subcatchment ES2: ES2

Runoff = 0.20 cfs @ 12.28 hrs, Volume= 1,599 cf, Depth= 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=7.07"

Area (sf) CN	Description			
2,0	52 98	Roofs, HSC	θA		
	87 98	Paved park	ing, HSG A	l l	
21,6	60 30	Woods, Go	od, HSG A		
5,7	69 39	>75% Gras	s cover, Go	ood, HSG A	
29,5	68 37	Weighted A	verage		
27,4	29	92.77% Pe	rvious Area		
2,1	39	7.23% Impe	ervious Area	а	
Tc Ler	igth Slo	pe Velocity	Capacity	Description	
<u>(min)</u> (fe	eet) (ft	/ft) (ft/sec)	(cfs)		
5.0				Direct Entry,	

Subcatchment ES2: ES2



Summary for Subcatchment ES3: ES3

Runoff = 0.01 cfs @ 12.33 hrs, Volume= 105 cf, Depth= 0.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=7.07"



Summary for Reach DP1: DP1

Inflow A	Area =	=	15,047 sf,	, 33.16% In	npervious,	Inflow Depth =	2.36"	for 25	-Year event
Inflow	=	:	0.94 cfs @	12.08 hrs,	Volume=	2,959 c	f		
Outflow	/ =	:	0.94 cfs @	12.08 hrs,	Volume=	2,959 c	f, Atte	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	a =	29,568 sf,	, 7.23% lr	npervious,	Inflow Depth =	0.65	" for 25	5-Year event
Inflow		=	0.20 cfs @	12.28 hrs,	Volume=	1,599 c	f		
Outflov	v	=	0.20 cfs @	12.28 hrs,	Volume=	1,599 c	of, At	ten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow /	Area	a =	2,45	3 sf, 0.00	% Imperv	rious, Inflo	ow Depth =	0.51"	for 25	-Year event
Inflow		=	0.01 cfs	@ 12.33	hrs, Volu	me=	105 c	f		
Outflov	N	=	0.01 cfs	@ 12.33	hrs, Volu	me=	105 c	f, Atter	i= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

2258 Existing Conditions Prepared by Ambit Engineering, Inc.	Type III 24-hr 50-Year Rainfall=8.46" Printed 2/14/2020
	D Soliware Solutions LLC Page 26
Time span=0.00-4 Runoff by SCS TR- Reach routing by Stor-Ind+Tra	18.00 hrs, dt=0.01 hrs, 4801 points 20 method, UH=SCS, Weighted-CN ins method - Pond routing by Stor-Ind method
Subcatchment ES1: ES1	Runoff Area=15,047 sf 33.16% Impervious Runoff Depth=3.33" Tc=5.0 min CN=57 Runoff=1.37 cfs 4,180 cf
Subcatchment ES2: ES2	Runoff Area=29,568 sf 7.23% Impervious Runoff Depth=1.16" Tc=5.0 min CN=37 Runoff=0.59 cfs 2,851 cf
Subcatchment ES3: ES3	Runoff Area=2,453 sf 0.00% Impervious Runoff Depth=0.97" Tc=5.0 min CN=35 Runoff=0.03 cfs 197 cf
Reach DP1: DP1	Inflow=1.37 cfs 4,180 cf Outflow=1.37 cfs 4,180 cf
Reach DP2: DP2	Inflow=0.59 cfs 2,851 cf Outflow=0.59 cfs 2,851 cf
Reach DP3: DP3	Inflow=0.03 cfs 197 cf Outflow=0.03 cfs 197 cf

Total Runoff Area = 47,068 sf Runoff Volume = 7,228 cfAverage Runoff Depth = 1.84"84.86% Pervious = 39,940 sf15.14% Impervious = 7,128 sf

Summary for Subcatchment ES1: ES1

Runoff = 1.37 cfs @ 12.08 hrs, Volume= 4,180 cf, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"

	Area (sf)	CN	Description						
	2,014	98	Roofs, HSG A						
	2,975	98	Paved parking, HSG A						
	3,007	30	Woods, Good, HSG A						
	7,051	39	>75% Grass cover, Good, HSG A						
	15,047	15,047 57 Weighted Average							
10,058 66.84% Pervious Area									
	4,989		33.16% Impervious Area						
	Tc Length	Slop	e Velocity	Capacity	Description				
((min) (feet)	(ft/f	it) (ft/sec)	(cfs)					
	5.0				Direct Entry,				





Summary for Subcatchment ES2: ES2

Runoff = 0.59 cfs @ 12.11 hrs, Volume= 2,851 cf, Depth= 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"

A	rea (sf)	CN	Description						
	2,052	98	8 Roofs, HSG A						
	87	98	Paved parking, HSG A						
	21,660	30	Woods, Good, HSG A						
	5,769	39	>75% Grass cover, Good, HSG A						
	29,568 37 Weighted Average								
	27,429 92.77% Pervious Area								
	2,139	7.23% Impervious Area							
Tc (min)	Length (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				
Subsetshment ES2, ES2									





Summary for Subcatchment ES3: ES3

Runoff = 0.03 cfs @ 12.12 hrs, Volume= 197 cf, Depth= 0.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"


Summary for Reach DP1: DP1

Inflow A	rea =	:	15,047 sf	f, 33.16% Ir	npervious,	Inflow Depth =	3.3	3" for 50	-Year event
Inflow	=		1.37 cfs @	12.08 hrs,	Volume=	4,180 c	f		
Outflow	=		1.37 cfs @	12.08 hrs,	Volume=	4,180 c	f, A	tten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow A	rea =	29,568 sf,	, 7.23% Impervious,	Inflow Depth = 1.16"	for 50-Year event
Inflow	=	0.59 cfs @	12.11 hrs, Volume=	2,851 cf	
Outflow	- =	0.59 cfs @	12.11 hrs, Volume=	2,851 cf, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area =	2,453 sf	, 0.00% Impervious,	Inflow Depth = 0.97"	for 50-Year event
Inflow	=	0.03 cfs @	12.12 hrs, Volume=	197 cf	
Outflow	/ =	0.03 cfs @	12.12 hrs, Volume=	197 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3



Area Listing (selected nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
12,138	54	1/2 acre lots, 25% imp, HSG A (PS2b)
16,974	39	>75% Grass cover, Good, HSG A (PS1, PS2a, PS2b, PS3)
4,109	98	Paved parking, HSG A (PS1)
13,847	30	Woods, Good, HSG A (PS2a)
47,068	45	TOTAL AREA

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
47,068	HSG A	PS1, PS2a, PS2b, PS3
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
47,068		TOTAL AREA

2258 Proposed Conditions

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Sub Nun	Ground Cover	Total (sq-ft)	Other (sq-ft)	HSG-D (sq-ft)	HSG-C (sq-ft)	HSG-B (sq-ft)	HSG-A (sq-ft)	
	1/2 acre lots,	12,138	0	0	0	0	12,138	
	25% imp >75% Grass	16,974	0	0	0	0	16,974	
	cover, Good							
	Paved parking	4,109	0	0	0	0	4,109	
	Woods, Good	13,847	0	0	0	0	13,847	
	TOTAL AREA	47,068	0	0	0	0	47,068	

Ground Covers (selected nodes)

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			Pipe L	listing (se	elected	nodes)			
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1P	65.00	64.50	6.0	0.0833	0.013	12.0	0.0	0.0

Pipe Listing (selected nodes)

2258 Proposed Conditions	Type III 24-hr 2-Year Rainfall=3	3.68"
Prepared by Ambit Engineering, Inc.	Printed 2/14/2	2020
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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PS1: PS1	Runoff Area=14,572 sf 28.20% Impervious Runoff Depth=0.45" Tc=5.0 min CN=56 Runoff=0.10 cfs 542 cf
Subcatchment PS2a: PS2a	Runoff Area=17,879 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=32 Runoff=0.00 cfs 0 cf
Subcatchment PS2b: PS2b	Runoff Area=13,638 sf 22.25% Impervious Runoff Depth=0.30" Tc=5.0 min CN=52 Runoff=0.04 cfs 345 cf
Subcatchment PS3: PS3	Runoff Area=979 sf 0.00% Impervious Runoff Depth=0.02" Tc=5.0 min CN=39 Runoff=0.00 cfs 2 cf
Reach DP1: DP1	Inflow=0.10 cfs 542 cf Outflow=0.10 cfs 542 cf
Reach DP2: DP2	Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Reach DP3: DP3	Inflow=0.00 cfs 2 cf Outflow=0.00 cfs 2 cf
Pond 1P: 10 StormTech Chambers - Dis	Model SC-740 Peak Elev=65.07' Storage=25 cf Inflow=0.04 cfs 345 cf carded=0.03 cfs 345 cf Primary=0.00 cfs 0 cf Outflow=0.03 cfs 345 cf

Total Runoff Area = 47,068 sf Runoff Volume = 889 cf Average Runoff Depth = 0.23" 84.82% Pervious = 39,925 sf 15.18% Impervious = 7,144 sf

Summary for Subcatchment PS1: PS1

Runoff = 0.10 cfs @ 12.12 hrs, Volume= 542 cf, Depth= 0.45"

А	rea (sf)	CN	Description	า								
	10,463	39	>75% Gras	ss cover, Go	ood, HS	GΑ						
	4,109	98	Paved par	king, HSG A	٠ ١							
	14,572	56	Weighted /	Average								
	10,463		71.80% Pe	ervious Area	l							
	4,109		28.20% Im	pervious Ar	ea							
				-								
Tc	Length	Slope	 Velocity 	Capacity	Descr	iption						
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)								
5.0					Direc	t Entry	',					
				-		_	_					
				Subcate	chmen	t PS1	: PS1					
				Hydro	graph							
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0.105	1/1		0.10 cfs									Runoii
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	0 2 4	0 0	10 12 14 1	Tim	e (hours)	20 30	52 54	30 30	J 40	74 44	+0 40	

Summary for Subcatchment PS2a: PS2a

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Depth= 0.00"



Summary for Subcatchment PS2b: PS2b

Runoff = 0.04 cfs @ 12.30 hrs, Volume= 345 cf, Depth= 0.30"

A	rea (sf)	CN	Description								
	12,138	54	1/2 acre lot	s, 25% imp	o, HSG A						
	1,500	39	>75% Gras	s cover, Go	ood, HSG A						
	13,638	52	Weighted A	verage							
	10,604 77.75% Pervious Area										
	3,035 22.25% Impervious Area										
Тс	Tc Length Slope Velocity Capacity Description										
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)							
5.0					Direct Entry,						
Subcatchment PS2b: PS2b											
Hydrograph											



Summary for Subcatchment PS3: PS3

Runoff = 0.00 cfs @ 21.15 hrs, Volume= 2 cf, Depth= 0.02"



Summary for Reach DP1: DP1

Inflow /	Area	a =		14,572 sf	, 28.20% Ir	npervious,	Inflow Depth =	0.45"	for 2-	Year event
Inflow		=	(0.10 cfs @	12.12 hrs,	Volume=	542 c	f		
Outflov	N	=	(0.10 cfs @	12.12 hrs,	Volume=	542 c	f, Atte	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	=		31,517 sf,	9.63% lr	npervious,	Inflow Depth =	0.0	00" for 2	2-Year event	
Inflow		=	(0.00 cfs @	0.00 hrs,	Volume=	. 0	cf			
Outflov	N	=	(0.00 cfs @	0.00 hrs,	Volume=	0	cf, /	Atten= 0%	, Lag= 0.0 min	I

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area	=	979 sf,	0.00% In	npervious,	Inflow Depth =	0.02"	for 2-Year event
Inflow		=	0.00 cfs @	21.15 hrs,	Volume=	2 0	f	
Outflow	V	=	0.00 cfs @	21.15 hrs,	Volume=	2 c	f, Atten	= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

Summary for Pond 1P: 10 StormTech Chambers - Model SC-740

Inflow Area	a =	13,638 sf,	22.25% Imperv	/ious, Ir	nflow Depth =	0.30"	for 2-Y	ear event
Inflow	=	0.04 cfs @	12.30 hrs, Volu	ime=	345 cf	:		
Outflow	=	0.03 cfs @	12.30 hrs, Volu	ime=	345 cf	, Atten	= 37%,	Lag= 0.0 min
Discarded	=	0.03 cfs @	12.30 hrs, Volu	ime=	345 cf	:		
Primary	=	0.00 cfs @	0.00 hrs, Volu	ime=	0 cf	:		

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 65.07' @ 12.53 hrs Surf.Area= 374 sf Storage= 25 cf

Plug-Flow detention time= 11.3 min calculated for 345 cf (100% of inflow) Center-of-Mass det. time= 11.3 min (961.8 - 950.5)

Volume	Invert	Avail.Stora	age Storage	e Description					
#1	65.00'	1,218	B cf Custom	n Stage Data (Prismatic)Listed below (Recalc)					
			1,683 cf	f Overall - 465 cf Embedded = 1,218 cf					
#2	65.50'	46	5 cf ADS_St	tormTech SC-740 x 10 Inside #1					
			Effective	e Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf					
			Overall S	Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap					
Row Length Adjustment= +0.44' x 6.45 sf x 2 rows									
		1,683	3 cf Total Av	vailable Storage					
Flevatio	n Si	urf Area	Inc Store	Cum Store					
(for	of)	(sa-ft) ((cubic-feet)	(cubic-feet)					
	20	<u>(34-11)</u> (
69 50 374			1 000	1 (0)					
69.3	50	374	1,683	1,083					
Device	Routing	Invert	Outlet Devices	es					
#1	Primary	65.00'	12.0" Round	d Culvert	_				
	,		L= 6.0' CPP,	, projecting, no headwall, Ke= 0.900					
			Inlet / Outlet li	Invert= 65.00' / 64.50' S= 0.0833 '/' Cc= 0.900					
			n= 0.013 Cor	rrugated PE, smooth interior, Flow Area= 0.79 sf					
#2	Device 1	65.50'	1.0" Vert. Ori	ifice/Grate X 2.00 C= 0.600					
#3	Device 1	66.00'	1.0" Vert. Ori	ifice/Grate X 3.00 C= 0.600					
#4	Device 1	67.25'	1.5" Vert. Ori	ifice/Grate X 3.00 C= 0.600					
#5	Primary	68.50'	5.0' long Sha	0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)					
#6	Discarded	65.00'	3.000 in/hr Ex	Exfiltration over Surface area					
	2.0001000	20100	0.000 m/m E/						
Discard	led OutFlow	Max=0.03 cfs	@ 12.30 hrs	HW=65.05' (Free Discharge)					

-6=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=65.00' (Free Discharge)

-1=Culvert (Controls 0.00 cfs)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Controls 0.00 cfs)

4=Orifice/Grate (Controls 0.00 cfs)

5=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Pond 1P: 10 StormTech Chambers - Model SC-740

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2258 Proposed Conditions	Type III 24-hr 1	0-Year Rainfall=5.58"					
Prepared by Ambit Engineering, Inc.	CAD Software Solutions LLC						
Hydrocade 10.00 s/1100801 @ 2013 Hydrocad Soltware Solutions ELC							
Time span=0.0 Runoff by SCS T Reach routing by Stor-Ind+	00-48.00 hrs, dt=0.01 hrs, 4801 points FR-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Inc	I method					
Subcatchment PS1: PS1	Runoff Area=14,572 sf 28.20% Impervi Tc=5.0 min CN=56	ous Runoff Depth=1.35" Runoff=0.48 cfs 1,644 cf					
Subcatchment PS2a: PS2a	Runoff Area=17,879 sf 0.00% Impervi Tc=5.0 min CN=32	ous Runoff Depth=0.08" Runoff=0.00 cfs 117 cf					

Runoff Area=13,638 sf 22.25% Impervious Runoff Depth=1.08" Tc=5.0 min CN=52 Runoff=0.32 cfs 1,222 cf

Inflow=0.48 cfs 1,644 cf

Inflow=0.03 cfs 414 cf

Inflow=0.00 cfs 27 cf

Outflow=0.00 cfs 27 cf

Outflow=0.03 cfs 414 cf

Outflow=0.48 cfs 1,644 cf

Subcatchment PS3: PS3Runoff Area=979 sf0.00% ImperviousRunoff Depth=0.33"Tc=5.0 minCN=39Runoff=0.00 cfs27 cf

Reach DP1: DP1

Subcatchment PS2b: PS2b

Reach DP2: DP2

Reach DP3: DP3

Pond 1P: 10 StormTech Chambers - Model Peak Elev=65.91' Storage=341 cf Inflow=0.32 cfs 1,222 cf Discarded=0.03 cfs 925 cf Primary=0.03 cfs 298 cf Outflow=0.06 cfs 1,222 cf

> Total Runoff Area = 47,068 sf Runoff Volume = 3,010 cf Average Runoff Depth = 0.77" 84.82% Pervious = 39,925 sf 15.18% Impervious = 7,144 sf

Summary for Subcatchment PS1: PS1

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,644 cf, Depth= 1.35"

Area (sf) CN Description	
10,463 39 >75% Grass cover, Good, HSG A	
4,109 98 Paved parking, HSG A	
14,572 56 Weighted Average	
10,463 /1.80% Pervious Area	
4,109 20.20% Impervious Area	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Subcatchment PS1: PS1	
Hydrograph	
	Runoff
0.5 0.48 cfs	
0.45	
10-Year Rainfall=5.58"	
^{0.4} Runoff Area=14.572 sf	
^{0.35} Runoff Volume=1 644 cf	
2° 0.3 2° D un off D on the 1.2 C	
ể ^{0.25} -	
0.2 CN=56	
0.05	
0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)	

Summary for Subcatchment PS2a: PS2a

Runoff = 0.00 cfs @ 15.32 hrs, Volume= 117 cf, Depth= 0.08"



Summary for Subcatchment PS2b: PS2b

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,222 cf, Depth= 1.08"

Ar	rea (sf)	CN	Description	า													
	12,138	54	1/2 acre lo	ts, 25%	imp,	, HS	GΑ										
1,500 39 >75% Grass cover, Good, HSG A																	
13,638 52 Weighted Average																	
10,604 77.75% Pervious Area																	
3,035 22.25% Impervious Area																	
т.	1			0													
IC (min)	Length	Slope		Capa	CITY	Des	scripti	on									
<u>(mm)</u>	(leet)	(11/11) (11/Sec)	(cis)	D :											
5.0						Dir	ect El	ntry,									
				Subca	atch	me	nt PS	32b:	PS	2b							
				Cabo													
	/				yaro	grapn	 	4 4		L _	_		↓				
0.36-	[]	 !	 				!				_!	 					Runoff
0.34	(0.32 cfs		_ <u>+</u>			<u></u>	· ·				<u> </u>				
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0.26		+ +		+ -	- +	·	i	Kul	not	t-Al	rea	=1	3,6	36	3-S	TI	
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					Time	e (hou	ırs)		0	20			-		-	-	

Summary for Subcatchment PS3: PS3

Runoff = 0.00 cfs @ 12.37 hrs, Volume= 27 cf, Depth= 0.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.58"

А	rea (sf)	CN	Description							
	979	39	39 >75% Grass cover, Good, HSG A							
	979	979 100.00% Pervious Area								
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

Subcatchment PS3: PS3



Summary for Reach DP1: DP1

Inflow A	Area :	=	14,572 sf,	, 28.20% In	npervious,	Inflow Depth =	1.35"	for 10	-Year event
Inflow	=	:	0.48 cfs @	12.09 hrs,	Volume=	1,644 c	f		
Outflow	' =	:	0.48 cfs @	12.09 hrs,	Volume=	1,644 c	f, Attei	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	a =	31,517 sf,	, 9.63% Ir	npervious,	Inflow Depth =	0.1	6" for 10)-Year event
Inflow		=	0.03 cfs @	12.85 hrs,	Volume=	414 c	f		
Outflov	N	=	0.03 cfs @	12.85 hrs,	Volume=	414 c	sf, A	tten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area =	=	979 sf	, 0.00% li	mpervious,	Inflow Depth =	0.33"	for 10-Year event
Inflow	=		0.00 cfs @	12.37 hrs,	Volume=	27 c	f	
Outflow	v =		0.00 cfs @	12.37 hrs,	Volume=	27 c	f, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

Summary for Pond 1P: 10 StormTech Chambers - Model SC-740

Inflow Area	a =	13,638 sf,	22.25% Imper	vious, Infle	ow Depth =	1.08"	for 10-`	Year event	
Inflow	=	0.32 cfs @	12.09 hrs, Vol	ume=	1,222 ct	f			
Outflow	=	0.06 cfs @	12.85 hrs, Vol	ume=	1,222 ct	f, Atten=	= 82%,	Lag= 45.7 m	nin
Discarded	=	0.03 cfs @	11.96 hrs, Vol	ume=	925 ct	f			
Primary	=	0.03 cfs @	12.85 hrs, Vol	ume=	298 cf	f			

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 65.91' @ 12.85 hrs Surf.Area= 374 sf Storage= 341 cf

Plug-Flow detention time= 79.1 min calculated for 1,222 cf (100% of inflow) Center-of-Mass det. time= 79.1 min (971.8 - 892.8)

Volume	Invert	Avail.Stor	age Storage Description								
#1	65.00'	1,21	8 cf Custom Stage Data (Prismatic) Listed below (Recalc) 1,683 cf Overall - 465 cf Embedded = 1,218 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 1.683 cf Overall - 465 cf Embedded = 1.218 cf							
#2	65.50'	46	35 cf ADS StormTech SC-740 x 10 Inside #1								
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap Row Length Adjustment= +0.44' x 6.45 sf x 2 rows								
		1,68	33 cf Total Available Storage								
Elevatio	on Su	urf.Area	Inc.Store Cum.Store								
(fee	et)	(sq-ft)	(cubic-feet) (cubic-feet)								
65.0	00	374	0 0								
69.5	50	374	1,683 1,683								
Device	Routing	Invert	Outlet Devices								
#1	Primary	65.00'	12.0" Round Culvert L= 6.0' CPP, projecting, no headwall, Ke= 0.900								
			Inlet / Outlet Invert= 65.00' / 64.50' S= 0.0833 '/' Cc= 0.900								
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf								
#2	Device 1	65.50'	1.0" Vert. Orifice/Grate X 2.00 C= 0.600								
#3	Device 1	66.00'	1.0" Vert. Orifice/Grate X 3.00 C= 0.600								
#4	Device 1	67.25'	1.5" Vert. Orifice/Grate X 3.00 C= 0.600								
#5	Primary	68.50'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)								
#6	Discarded	65.00'	3.000 in/hr Exfiltration over Surface area								
Discard	led OutFlow	Max=0.03 cfs	3 @ 11.96 hrs HW=65.05' (Free Discharge)								

6=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.03 cfs @ 12.85 hrs HW=65.91' (Free Discharge)

-1=Culvert (Passes 0.03 cfs of 1.93 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.03 cfs @ 2.93 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Pond 1P: 10 StormTech Chambers - Model SC-740

2258 Proposed Conditions	Type III 24-hr 25-Year Rainfall=7.07"
Prepared by Ambit Engineering, Inc.	Printed 2/14/2020
HydroCAD® 10.00 s/n 00801 © 2013 HydroCAD Software Solutions	LLC Page 26

d 2/14/2020 Page 26

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PS1: PS1	Runoff Area=14,572 sf 28.20% Impervious Runoff Depth=2.26" Tc=5.0 min CN=56 Runoff=0.87 cfs 2,749 cf
Subcatchment PS2a: PS2a	Runoff Area=17,879 sf 0.00% Impervious Runoff Depth=0.33" Tc=5.0 min CN=32 Runoff=0.03 cfs 492 cf
Subcatchment PS2b: PS2b	Runoff Area=13,638 sf 22.25% Impervious Runoff Depth=1.89" Tc=5.0 min CN=52 Runoff=0.65 cfs 2,146 cf
Subcatchment PS3: PS3	Runoff Area=979 sf 0.00% Impervious Runoff Depth=0.79" Tc=5.0 min CN=39 Runoff=0.01 cfs 65 cf
Reach DP1: DP1	Inflow=0.87 cfs 2,749 cf Outflow=0.87 cfs 2,749 cf
Reach DP2: DP2	Inflow=0.15 cfs 1,507 cf Outflow=0.15 cfs 1,507 cf
Reach DP3: DP3	Inflow=0.01 cfs 65 cf Outflow=0.01 cfs 65 cf
Pond 1P: 10 StormTech Chambers - Model Discarded=0.03 cfs	Peak Elev=66.70' Storage=634 cf Inflow=0.65 cfs 2,146 cf 1,131 cf Primary=0.12 cfs 1,014 cf Outflow=0.15 cfs 2,146 cf

Total Runoff Area = 47,068 sf Runoff Volume = 5,452 cf Average Runoff Depth = 1.39" 84.82% Pervious = 39,925 sf 15.18% Impervious = 7,144 sf

Summary for Subcatchment PS1: PS1

Runoff = 0.87 cfs @ 12.08 hrs, Volume= 2,749 cf, Depth= 2.26"

Aı	rea (sf)	CN I	Description								
	10,463	39 :	>75% Gras	s cover, Go	ood, HSG	А					
	4,109	98 I	Paved park	ing, HSG A	١						
	14,572	56 \	Neighted A	verage							
	10,463	-	71.80% Pe	rvious Area	l						
	4,109	4	28.20% Imp	pervious Ar	ea						
Тс	Lenath	Slope	Velocitv	Capacity	Descrip	tion					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	_ 000p						
5.0					Direct I	Entry,					
				Subcato	hment		PS1				
				Hydro	graph		0.				
0.95							- +		- + +		Runoff
0.9	/	<mark>-</mark>					- + + -				
0.85	(Table	9-111-24	4-nr_	
0.8						25-Y	ear F	Rainf	all=7	.07"	
0.75						Dub	∖ff ∧	roo-'	1 / 57	2 cf	
0.65						NUIR	ЛІА	rea-	14,37	2 31	
0.6	/				R	unof	[Vol	ume	=2,74	9_cf	
(sj 0.55	/			+ +		R	inoff	Den	fh=2	26"	
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0.45							- -	IC	=5.0	min	
0.35				-			- T		CN	=56	
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Summary for Subcatchment PS2a: PS2a

Runoff = 0.03 cfs @ 12.42 hrs, Volume= 492 cf, Depth= 0.33"



Summary for Subcatchment PS2b: PS2b

Runoff = 0.65 cfs @ 12.08 hrs, Volume= 2,146 cf, Depth= 1.89"

	Area	a (sf)	С	N	De	scri	ptio	n																		
	12	2,138	5	54	1/2	ac	re lo	ots,	25	% iı	mp,	HS	G A	١.												
1,500 39 >75% Grass cover, Good, HSG A																										
13,638 52 Weighted Average																										
	10	0,604	•		11.	.75%	% P	erv	IOU	s Ai	rea	_														
	Ċ	3,035	1		22.	25%	% In	npe	ervic	bus	Are	a														
T	Ċ L	engt	h S	Slope	è,	Velo	ocity	ý	Cap	baci	ity	De	scr	iptic	n											
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	Ó	2 4	4 6	8	10	12	14	16	18	20	22 Time	24 (hoi	26 urs)	28	30	32	34	36	38	40	42	44	46	48		

Summary for Subcatchment PS3: PS3

Runoff = 0.01 cfs @ 12.12 hrs, Volume= 65 cf, Depth= 0.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=7.07"

Area (sf)	CN	Description			
979	39	>75% Gras	s cover, Go	ood, HSG A	
979		100.00% P	ervious Are	a	
Tc Length (min) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description	
5.0				Direct Entry,	

Subcatchment PS3: PS3



Summary for Reach DP1: DP1

Inflow A	Area	=	14,572 sf,	, 28.20% Impervious	Inflow Depth = 2	.26" for 25	-Year event
Inflow		=	0.87 cfs @	12.08 hrs, Volume=	2,749 cf		
Outflow	/	=	0.87 cfs @	12.08 hrs, Volume=	2,749 cf,	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	i =	31,517 sf,	9.63% Ir	npervious,	Inflow Depth =	0.57"	for 25-Year event	
Inflow		=	0.15 cfs @	12.46 hrs,	Volume=	1,507 c	f		
Outflov	N	=	0.15 cfs @	12.46 hrs,	Volume=	1,507 c	f, Atte	en= 0%, Lag= 0.0 mir	ſ

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area =	979 sf,	0.00% Impervious,	Inflow Depth = 0.79"	for 25-Year event
Inflow	=	0.01 cfs @	12.12 hrs, Volume=	65 cf	
Outflow	/ =	0.01 cfs @	12.12 hrs, Volume=	65 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3
Summary for Pond 1P: 10 StormTech Chambers - Model SC-740

Inflow Area	a =	13,638 sf,	22.25% Im	pervious,	Inflow Depth =	1.89" f	or 25-Y	'ear event	
Inflow	=	0.65 cfs @	12.08 hrs,	Volume=	2,146 c	f			
Outflow	=	0.15 cfs @	12.54 hrs,	Volume=	2,146 c	f, Atten=	:78%, l	Lag= 27.6 mi	n
Discarded	=	0.03 cfs @	11.75 hrs,	Volume=	1,131 c	f			
Primary	=	0.12 cfs @	12.54 hrs,	Volume=	1,014 c	f			

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 66.70' @ 12.54 hrs Surf.Area= 374 sf Storage= 634 cf

Plug-Flow detention time= 79.1 min calculated for 2,145 cf (100% of inflow) Center-of-Mass det. time= 79.1 min (952.0 - 872.9)

Volume	Inver	t Avail.Sto	rage St	orage	Description	
#1	65.00	' 1,2'	18 cf C	iston	Stage Data (P	rismatic)Listed below (Recalc) f Embedded = 1 218 cf
#2	65 50	. 46	5.5 cf A	DS S	tormTech SC-7	40 x 10 Inside #1
<i></i>	00100		Ef	fective	e Size= 44.6"W	x 30.0 "H => 6.45 sf x 7.12'L = 45.9 cf
			0	/erall	Size= 51.0"W x	30.0"H x 7.56'L with 0.44' Overlap
			R	w Le	ngth Adjustment	t= +0.44' x 6.45 sf x 2 rows
		1,68	33 cf To	tal A	ailable Storage	
Elevatio	on S	Surf.Area	Inc.St	ore	Cum.Store	
(fee	et)	(sq-ft)	(cubic-fe	et)	(cubic-feet)	
65.0	00	374		0	0	
69.5	50	374	1,6	83	1,683	
Device	Routing	Invert	Outlet E	evice	S	
#1	Primary	65.00'	12.0" F	ounc	I Culvert	
			L= 6.0'	CPP	, projecting, no l	headwall, Ke= 0.900
			Inlet / C	utlet I	nvert= 65.00' / 6	64.50' S= 0.0833 '/' Cc= 0.900
			n= 0.01	3 Co	rrugated PE, sm	ooth interior, Flow Area= 0.79 sf
#2	Device 1	65.50'	1.0" Ve	rt. Or	ifice/Grate X 2.	00 $C = 0.600$
#3	Device 1	66.00'	1.0" Ve	rt. Or	ifice/Grate X 3.	00 $C = 0.600$
#4	Device 1	67.25'	1.5" Ve	rt. Or	ifice/Grate X 3.	00 $C = 0.600$
#5	Primary	68.50'	5.0' lon	g Sha	arp-Crested Red	ctangular Weir 2 End Contraction(s)
#6	Discarded	65.00'	3.000 ir	/hr E	xfiltration over	Surface area
Discard	led OutFlov	v Max=0.03 cfs	s @ 11.7	5 hrs	HW=65.05' (F	ree Discharge)

6=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.12 cfs @ 12.54 hrs HW=66.70' (Free Discharge)

-1=Culvert (Passes 0.12 cfs of 3.26 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.06 cfs @ 5.17 fps)

-3=Orifice/Grate (Orifice Controls 0.06 cfs @ 3.89 fps)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Pond 1P: 10 StormTech Chambers - Model SC-740

2258 Proposed Conditions Prepared by Ambit Engineering, Inc. HydroCAD® 10.00 s/n 00801 © 2013 HydroCA	Type III 24-hr 50-Year Rainfall=8.46" Printed 2/14/2020 AD Software Solutions LLC Page 36
Time span=0.00- Runoff by SCS TR Reach routing by Stor-Ind+Tr	48.00 hrs, dt=0.01 hrs, 4801 points 2-20 method, UH=SCS, Weighted-CN ans method - Pond routing by Stor-Ind method
Subcatchment PS1: PS1	Runoff Area=14,572 sf 28.20% Impervious Runoff Depth=3.22" Tc=5.0 min CN=56 Runoff=1.27 cfs 3,908 cf
Subcatchment PS2a: PS2a	Runoff Area=17,879 sf 0.00% Impervious Runoff Depth=0.70" Tc=5.0 min CN=32 Runoff=0.12 cfs 1,037 cf
Subcatchment PS2b: PS2b	Runoff Area=13,638 sf 22.25% Impervious Runoff Depth=2.76" Tc=5.0 min CN=52 Runoff=1.00 cfs 3,138 cf

Subcatchment PS3: PS3Runoff Area=979 sf0.00% ImperviousRunoff Depth=1.36"Tc=5.0 minCN=39Runoff=0.03 cfs111 cf

Reach DP1: DP1

Reach DP2: DP2

Reach DP3: DP3

Inflow=0.03 cfs 111 cf Outflow=0.03 cfs 111 cf

Inflow=1.27 cfs 3,908 cf

Inflow=0.36 cfs 2,890 cf Outflow=0.36 cfs 2,890 cf

Outflow=1.27 cfs 3,908 cf

Pond 1P: 10 StormTech Chambers - Model Peak Elev=67.54' Storage=951 cf Inflow=1.00 cfs 3,138 cf Discarded=0.03 cfs 1,285 cf Primary=0.26 cfs 1,852 cf Outflow=0.28 cfs 3,138 cf

Total Runoff Area = 47,068 sf Runoff Volume = 8,193 cf Average Runoff Depth = 2.09" 84.82% Pervious = 39,925 sf 15.18% Impervious = 7,144 sf

Summary for Subcatchment PS1: PS1

Runoff = 1.27 cfs @ 12.08 hrs, Volume= 3,908 cf, Depth= 3.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"



Summary for Subcatchment PS2a: PS2a

Runoff = 0.12 cfs @ 12.31 hrs, Volume= 1,037 cf, Depth= 0.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"



Summary for Subcatchment PS2b: PS2b

Runoff = 1.00 cfs @ 12.08 hrs, Volume= 3,138 cf, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"

	Area (sf)	CN	Description	า									
	12,138	54	1/2 acre lo	ts, 25% imp	, HSG	A							
	1,500	39	>75% Gra	ss cover, Go	ood, HS	SG A							
	13,638	52	Weighted /	Average									
	10,604		77.75% Pe	ervious Area	1								
	3,035		22.25% III	pervious Ar	ea								
Тс	Length	Slop	e Velocity	Capacity	Desc	riptio	n						
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)		•							
5.0					Direc	t En	try,						
				Cubaatak			0L. D	COL					
				Subcatcr	iment	P2	2D: P	520					
				Hydro	graph								
			4 00 (i i			Runoff
1-	,i	i 	1.00 cfs		++	+ +	·	i-	- + + +		· - ' ~ 4		
							1		іур	e¦III	24	-nr	
						5	0-Ye	ear l	Rain	fall:	=8.4	 6 "	
	- 1 1					R	Runc	off A	rea=	=13.	638	sf	
						Du	noff		lumo	'?,	120	cf	
(s						Πų	non L			≠=J,	130		
(ct							Ru	Inof	f De	pth	=2.7	6"	
Flow						i i i i	i I		T	c=5	.0 n	nin	
_											żΝ+	52	
							-				, , ,		
										I I			
					i i I I	i i I I	i			i	i i I I	i I	
							-			-		-	
0-	0 2 4	6 8	10 12 14 16	5 18 20 22	24 26	28	30 32	34 36	38 4	10 42	44 4	16 48	
				Time	e (hours)	'							

Summary for Subcatchment PS3: PS3

Runoff = 0.03 cfs @ 12.10 hrs, Volume= 111 cf, Depth= 1.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=8.46"

Are	ea (sf)	CN E	Description						
	979	39 >	39 >75% Grass cover, Good, HSG A						
	979	9 100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)						
5.0	5.0 Direct Entry,								
Subcatchment PS3: PS3									



Summary for Reach DP1: DP1

Inflow A	Area =	=	14,572 sf	, 28.20% In	npervious,	Inflow Depth =	3.22	" for 50	-Year event
Inflow	=	=	1.27 cfs @	12.08 hrs,	Volume=	3,908 c	f		
Outflow	/ =	=	1.27 cfs @	12.08 hrs,	Volume=	3,908 c	f, Att	en= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP1: DP1

Summary for Reach DP2: DP2

Inflow /	Area	i =	31,517 sf,	, 9.63% lr	npervious,	Inflow Depth =	1.10"	for 50	-Year event
Inflow		=	0.36 cfs @	12.41 hrs,	Volume=	2,890 cf			
Outflov	v	=	0.36 cfs @	12.41 hrs,	Volume=	2,890 cf	, Atte	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP2: DP2

Summary for Reach DP3: DP3

Inflow A	Area =	=	979 sf	, 0.00% Ir	npervious,	Inflow Depth =	1.36"	for 50-Year event
Inflow	=		0.03 cfs @	12.10 hrs,	Volume=	111 cf		
Outflow	v =		0.03 cfs @	12.10 hrs,	Volume=	111 cf	, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach DP3: DP3

Summary for Pond 1P: 10 StormTech Chambers - Model SC-740

Inflow Area	a =	13,638 sf,	, 22.25% lm	pervious,	Inflow Depth =	2.76"	for 50-`	Year event	
Inflow	=	1.00 cfs @	12.08 hrs, \	Volume=	3,138 c	f			
Outflow	=	0.28 cfs @	12.48 hrs, \	Volume=	3,138 c	f, Atten:	= 72%,	Lag= 23.7	min
Discarded	=	0.03 cfs @	11.53 hrs, \	Volume=	1,285 c	f			
Primary	=	0.26 cfs @	12.48 hrs, \	Volume=	1,852 c	f			

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 67.54' @ 12.48 hrs Surf.Area= 374 sf Storage= 951 cf

Plug-Flow detention time= 78.6 min calculated for 3,137 cf (100% of inflow) Center-of-Mass det. time= 78.6 min (939.3 - 860.8)

Volume	Invert	Avail.Stora	age Storage Description		
#1	65.00'	1,218	8 cf Custom Stage Data (Prismatic)Listed below (Recalc)		
<i>щ</i> о		466	1,083 Cl Overall - 405 Cl Embedded = 1,218 Cl		
#2 65.50 465 CI ADS_Storm I ech SC-740 X 10 Inside #1 Effective Size= 44.6 "W x 20.0"H => 6.45 sf x 7.12"L = 45.0					
			Overall Size= 51 0 W x 30 0 H x 7 56'L with 0 44' Overlap		
			Row Length Adjustment= $+0.44' \times 6.45$ sf x 2 rows		
		1,683	3 cf Total Available Storage		
Elevatio	on Su	rf Area	Inc Store Cum Store		
(fee	et)	(sa-ft) ((cubic-feet) (cubic-feet)		
65 (<u>)</u> 0	374			
69.5	50	374	1,683 1,683		
Device	Routing	Invert	Outlet Devices		
#1	Primary	65.00'	12.0" Round Culvert		
	2		L= 6.0' CPP, projecting, no headwall, Ke= 0.900		
			Inlet / Outlet Invert= 65.00' / 64.50' S= 0.0833 '/' Cc= 0.900		
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf		
#2	Device 1	65.50'	1.0" Vert. Orifice/Grate X 2.00 C= 0.600		
#3	Device 1	66.00'	1.0" Vert. Orifice/Grate X 3.00 C= 0.600		
#4	Device 1	67.25'	1.5" Vert. Orifice/Grate X 3.00 C= 0.600		
#5	Primary	68.50'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)		
#6	Discarded	65.00'	3.000 in/hr Exfiltration over Surface area		
Discard	led OutFlow	Max=0.03 cfs	@ 11.53 hrs HW=65.05' (Free Discharge)		

-6=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.26 cfs @ 12.48 hrs HW=67.54' (Free Discharge)

1=Culvert (Passes 0.26 cfs of 4.27 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.07 cfs @ 6.81 fps)

-3=Orifice/Grate (Orifice Controls 0.10 cfs @ 5.90 fps)

4=Orifice/Grate (Orifice Controls 0.09 cfs @ 2.31 fps)

-5=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Pond 1P: 10 StormTech Chambers - Model SC-740

APPENDIX D

SOIL SURVEY INFORMATION



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Rockingham County, New Hampshire



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP L	EGEND)	MAP INFORMATION
Area of Int	erest (AOI)	33	Spoil Area	The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	۵	Stony Spot	1:24,000.
Soils		0	Very Stony Spot	Warning: Soil Man may not be valid at this scale
	Soil Map Unit Polygons	Ŷ	Wet Spot	
~	Soil Map Unit Lines	~	Other	Enlargement of maps beyond the scale of mapping can cause
	Soil Map Unit Points		Special Line Features	line placement. The maps do not show the small areas of
Special I	pecial Point Features		atures	contrasting soils that could have been shown at a more detailed
0	Biowout	~	Streams and Canals	scale.
	Borrow Pit	Transport	ation	Please rely on the bar scale on each map sheet for map
×	Clay Spot	+++	Rails	measurements.
\diamond	Closed Depression	~	Interstate Highways	Source of Man. Natural Resources Conservation Service
X	Gravel Pit	~	US Routes	Web Soil Survey URL:
0 0 0	Gravelly Spot	~	Major Roads	Coordinate System: Web Mercator (EPSG:3857)
0	Landfill	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
A.	Lava Flow	Backgrou	ind	projection, which preserves direction and shape but distorts
عليه	Marsh or swamp	The second	Aerial Photography	Albers equal-area conic projection, should be used if more
Ŕ	Mine or Quarry			accurate calculations of distance or area are required.
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data as
0	Perennial Water			of the version date(s) listed below.
~	Rock Outcrop			Soil Survey Area: Rockingham County New Hampshire
+	Saline Spot			Survey Area Data: Version 21, Sep 16, 2019
• •	Sandy Spot			Soil man units are labeled (as snace allows) for man scales
-	Severely Eroded Spot			1:50,000 or larger.
ô	Sinkhole			Data(a) aprial images were photographed: Dec 21, 2000 Sep
à	Slide or Slip			9, 2017
Ŕ	Sodic Spot			-
שן	•			I ne orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
799	Urban land-Canton complex, 3 to 15 percent slopes	13.3	100.0%
Totals for Area of Interest		13.3	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Rockingham County, New Hampshire

799—Urban land-Canton complex, 3 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9cq0 Elevation: 0 to 1,000 feet Mean annual precipitation: 42 to 46 inches Mean annual air temperature: 45 to 48 degrees F Frost-free period: 120 to 160 days Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 55 percent Canton and similar soils: 20 percent Minor components: 25 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton

Setting

Parent material: Till

Typical profile

H1 - 0 to 5 inches: gravelly fine sandy loam *H2 - 5 to 21 inches:* gravelly fine sandy loam *H3 - 21 to 60 inches:* loamy sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Udorthents

Percent of map unit: 5 percent Hydric soil rating: No

Squamscott and scitico

Percent of map unit: 4 percent Landform: Marine terraces Hydric soil rating: Yes

Boxford and eldridge

Percent of map unit: 4 percent Hydric soil rating: No

Chatfield

Percent of map unit: 4 percent Hydric soil rating: No

Scituate and newfields

Percent of map unit: 4 percent *Hydric soil rating:* No

Walpole

Percent of map unit: 4 percent Landform: Depressions Hydric soil rating: Yes

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APPENDIX E

FEMA FIRM MAP



APPENDIX F

INSPECTION & MAINTENANCE PLAN

INSPECTION & MAINTENANCE PLAN FOR

SUBDIVISION PLAN 201 KEARSARGE WAY PORTSMOUTH, NH

Introduction

The intent of this plan is to provide Richard Fusegni (herein referred to as "owner") with a list of procedures that document the inspection and maintenance requirements of the stormwater management system for this development. Specifically, infiltration system and associated structures on the project site (collectively referred to as the "Stormwater Management System").

The following inspection and maintenance program is necessary to keep the stormwater management system functioning properly. These measures will also help minimize potential environmental impacts. By following the enclosed procedures, the owner will be able to maintain the functional design of the stormwater management system and maximize its ability to remove sediment and other contaminants from site generated stormwater runoff.

Annual Report

The owner shall prepare an annual Inspection & Maintenance Report. The report shall include a summary of the system's maintenance and repair by transmission of the Inspection & Maintenance Log and other information as required. A copy of the report shall be delivered annually to the City of Portsmouth Code Enforcement Officer.

Inspection & Maintenance Checklist/Log

The following pages contain a Stormwater Management System Inspection & Maintenance Checklist and a blank copy of the Stormwater Management System Inspection & Maintenance Log. These forms are provided to the owner as a guideline for performing the inspection and maintenance of the Stormwater Management System. This is a guideline and should be periodically reviewed for conformance with current practice and standards.

STORMWATER MANAGEMENT SYSTEM COMPONENTS

The Stormwater Management System is designed to mitigate both the quantity and quality of sitegenerated stormwater runoff. As a result, the design includes the following elements:

Non-Structural BMP's

Non-Structural best management practices (BMP's) include temporary and permanent measures that typically require less labor and capital inputs and are intended to provide protection against erosion of soils. Examples of non-structural BMP's on this project include but are not limited to: temporary and permanent mulching, temporary and permanent grass cover, trees, shrubs and ground covers, miscellaneous landscape plantings, dust control, tree protection, topsoiling, sediment barriers, and a stabilized construction entrance.

Structural BMP's

Structural BMP's are more labor and capital-intensive structures or installations that require more specialized personnel to install. Examples on this project include but are not limited to: catch basins, pipes, underdrains, infiltration trenches and stormwater storage chambers.

Inspection and Maintenance Requirements

The following summarizes the inspection and maintenance requirements for the various BMP's that may be found on this project.

- **1. Grassed areas:** After each rain event of 0.5" or more during a 24-hour period, inspect grassed areas for signs of disturbance, such as erosion. If damaged areas are discovered, immediately repair the damage. Repairs may include adding new topsoil, lime, seed, fertilizer and mulch.
- 2. Plantings: Planting and landscaping (trees, shrubs) shall be monitored bi-monthly during the first year to insure viability and vigorous growth. Replace dead or dying vegetation with new stock and make adjustments to the conditions that caused the dead or dying vegetation. During dryer times of the year, provide weekly watering or irrigation during the establishment period of the first year. Make the necessary adjustments to ensure long-term health of the vegetated covers, i.e. provide more permanent mulch or compost or other means of protection.
- **3.** Storm Drain and Catch Basin Inlets/Outlets: Monitor drain inlets and outlet aprons for excessive accumulation of sediments or missing stone/riprap. Remove sediments as required to maintain filtering capabilities of the stone—replace missing riprap.
- 4. StormTech Subsurface Stormwater System Maintenance: In general, the intent of this subsurface system is to provide for storage of runoff from developed areas, in this case the construction of new homes at the site. This system is designed to slow the peak runoff from a 50 year frequency rainfall event (8.46" of rain in a 24 hour period). In order to keep the subsurface system functioning properly, it is important to keep the system unplugged by debris. Installation of this StormTech subsurface system includes an "Isolator Row". This is a row of chambers dedicated to settling out particulate matter in the stormwater run-off stream before the water reaches the rest of the subsurface system. The system should perform for many years without clogging. Regular inspection of the Isolator Row.

Specific Maintenance Procedures

<u>Inspections-</u> The "Isolator Row" of the subsurface system may become clogged with sediment, and organic matter with time.

If the subsurface system is not functioning, the system would be repaired by the removal of accumulated debris including sand and silt(s) to return the subsurface system to a functioning condition. Accumulated sediment can be removed with culvert cleaning device which will allow the removal of the accumulated debris by power washing the material back to the open end of the system for removal.

5. Stone Trenches: Monitor for settlement of stone and accumulation of excessive debris and sediment. Monitor during rainfall for excessive flow as this would be an indication that clogging of the system may have occurred and maintenance is necessary.

Invasive Species

Monitor Stormwater Management System for signs of invasive species growth. If caught earlier enough, their eradication is much easier. The most likely places where invasions start are in wetter, disturbed soils. Measures that do not require the application of chemical herbicides should be the first line of defense.

Stormwater Management System

Inspection & Maintenance Checklist for Post Construction Condition—for Richard Fusegni, 201 Kearsarge Way, Portsmouth, N.H.

BMP/System Component	Minimum Inspection Frequency	Minimum Inspection Requirements	Maintenance/Cleanout Threshold
Closed Drainage System			
Catch Basins	Every other Month	Check for sediment accumulation Check for floatable contaminants	Remove sediments when they accumulate half way up the sump. 3 in. floatable depth in WQU(if present)
Drainage Pipes	Yearly	Check for sediment clogging, or soiled runoff.	Clean entire drainage system and remove all sediments if discovered in piping.
Stormtech Subsurface Chamber System	2 times per year	During Light Rains Inspect Discharge to Rain Garden #1 is Functioning Properly.	Repair / Clean as needed
Stone Trenches	2 X Annually	Check for sediment and debris	Remove any sediment or debris. Check outlet pipe for clogging.
Annual Report	Yearly	Prepare Annual Report, including all Inspection & Maintenance Logs. Provide to Town (if required).	N/A
Stormwater Management System Maintenance Summary

BMP/System Component	Date Inspected	Inspector	Problems Noted, Required Maintenance (List Items/Comments)	Date of Maintenance	Performed By

Inspection & Maintenance Log-for Richard Fusegni, 201 Kearsarge Way, Portsmouth, N.H.

Data Sheets



PROPOSED EXISTING ----- D -----D ----------_____X _____X _____ ____x____x____ 100 98x0 97x3 -----(PS1) ES 1234 1234 WOODS WOODS 1DP1 DP1 L..... -----1-V-V--- \sim \mathbb{W} (W)EL. EL. EΡ EP FF FF INV INV TBM TBM TYP TYP ----------SF SF SCF SCF CHANNEL CHANNEL HSG HSG

PROPERTY LINE STORM DRAIN SILT FENCE CONTOUR SPOT ELEVATION EDGE OF PAVEMENT (EP) SUBCATCHMENT LINE SUBCATCHMENT NUMBER AREA IN SQUARE FEET DESCRIPTION OF COVER POND (DESIGN MODEL) REACH (DESIGN MODEL) DRAINAGE VECTOR EDGE OF WOODS / TREES CATCH BASIN DRAIN MANHOLE WELL ELEVATION EDGE OF PAVEMENT FINISHED FLOOR INVERT TEMPORARY BENCH MARK TYPICAL Tc PATH SHEET FLOW SHALLOW CONCENTRATED FLOW CHANNEL FLOW

HYDROLGIC SOIL GROUP

N/F CITY OF PORTSMOUTH DPW P.O. BOX 628 PORTSMOUTH, N.H. 03802 2745/792 Other Contraction Other Contr







AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES: 1) PARCEL IS SHOWN ON THE CITY OF PORTSMOUTH ASSESSOR'S MAP 218 AS LOT 5.

2) OWNER OF RECORD: RICHARD P. FUSEGNI

- 201 KEARSARGE WAY
- PORTSMOUTH, N.H. 03801 5476/2661 (5979/2783)
- RCRD PLAN 0245

3) PARCEL IS NOT IN A FLOOD HAZARD ZONE AS SHOWN ON FIRM PANEL 33015C0259E, EFFECTIVE MAY 17, 2005.

4) EXISTING LOT AREA: 47,062 S.F. 1.0804 AC.

NOTES:

1) THIS PLAN IS INTENDED FOR RUNOFF ANALYSIS ONLY AND SHALL NOT BE USED FOR CONSTRUCTION.

2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

3) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

4) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

SUBDIVISION PLAN TAX MAP 218 - LOT 5 201 KEARSARGE WAY PORTSMOUTH, N.H.





FB 311, PG 01

2258



N/F DAVID T. MURRAY 182 BIRCH STREET PORTSMOUTH, N.H. 03801 3432/2644



RTH \bigcirc



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

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SUBDIVISION PLAN TAX MAP 218 - LOT 5 201 KEARSARGE WAY PORTSMOUTH, N.H.

1	ADDED STORM CHAMBERS	2/14/20
0	ISSUED FOR COMMENT	1/21/20
NO.	DESCRIPTION	DATE
	REVISIONS	



FB 311, PG 01

2258

EASEMENT DEED

Richard P. Fusegni, a single person, with a mailing address of 6 Spring Lane, Eliot Maine 03903, (herein called "Grantor") for consideration paid, grants to the **CITY OF PORTSMOUTH**, a municipal body with a mailing address of 1 Junkins Avenue, Portsmouth New Hampshire 03901 (hereinafter "Grantee"), with **QUITCLAIM COVENANTS**, upon the conditions hereafter set forth, a permanent access easement (hereinafter the "Easement") over and upon land of the Grantor located in the City of Portsmouth, County of Rockingham State of New Hampshire.

Said Easement being shown as "Proposed Access Easement to the City of Portsmouth." on a plan entitled, "Subdivision Plan Tax Map 218-Lot 5, Owner: Richard P. Fusegni, 201 Kearsarge Way, City of Portsmouth, County of Rockingham, State of New Hampshire", prepared by Ambit Engineering, Inc. dated `January 21, 2020 and recorded in the Rockingham County Registry of Deeds as Plan #______ said Easement being more particularly bounded and described as follows:

The above described easement containing square feet, more or less (hereinafter "Easement Area").

Grantor grants to Grantee such access easement for all purposes for which roads are customarily used, including but not limited to vehicular, pedestrian and equipment access and travel and the installation and maintenance of utilities above the Easement Area. The Grantee shall have the obligation to construct, maintain in good order and promptly repair damage to all portions of the roadway built within the Easement Area, at Grantee's sole cost and expense. Any land or property of the Grantor disturbed or damaged by the Grantee's installation, maintenance or repair of the roadway within the Easement Area, shall be immediately restored or replaced to the condition of such land of property prior to the disturbance or damage. The Grantee shall be sole responsibility for any liability, damage, costs, or loss to persons, including death, and property, of any kind whatsoever, arising from or relating to the installation, maintenance, repair and use of the Easement Area, and hereby agrees to indemnify, defend and hold harmless the Grantor from any and all such claims, causes, demands and actions.

Reserving to Grantor, their successors and assigns, and Grantee, their successors and assigns, access and utility rights in the Easement Area, together with the use and enjoyment of said Easement Area for such purposes only as will in no way interfere with the perpetual use thereof by the Grantee, its successors and assigns for the purposes contained herein; and to that end, the Grantor, its successors and assigns shall not erect any building, structures sidewalks, parking areas, surface curbs, landscaping and other similar improvements on said Easement Area; provided however, that Grantor may install underground utility structures or systems within the Easement Area which do not interfere with Grantee's use of the Easement Areas and Grantor reserves all rights to cross the Easement Area and all rights and easements necessary or desirable for the use, occupation, repair, maintenance and replacement of any improvements now or hereafter located upon Grantor's remaining land.

This Easement Deed and the rights and privileges granted hereby are perpetual and shall run with the land.

The easements, covenants and conditions herein shall be binding and/or to the benefit of the parties hereto, their heir, successors and assigns.

Meaning and intending to convey an easement over a portion of the premises conveyed to Richard P. Fusegni by Elda Fusegni dated September 5, 2013 and recorded on September 6, 2013 in the Rockingham County Registry of Deeds at Book 5476 Page 2661.

Executed this _____ day of March, 2020.

Witness:

Richard P. Fusegni

State of New Hampshire County of Rockingham

This instrument was acknowledged before me on this _____ day of March, 2020 by Richard P. Fusegni.

Notary Public

CONSERVATION RESTRICTIVE COVENANT

THIS DECLARATION OF CONSERVATION RESTRICTIVE COVENANT is made this _____ day of March, 2020, by Richard Fusegni (hereinafter referred to as the "Declarant", which includes the plural of the word where the context requires, and shall unless the context clearly indicates otherwise, include the Declarant's executors, administrators, legal representatives, devisees, heirs, successors and assigns);

WITNESSETH:

WHEREAS, the Declarant is the sole owner in fee simple of certain real property in the City of Portsmouth, County of Rockingham, and State of New Hampshire located at 201 Kearsarge Way and described in Exhibit A attached hereto (the "Property"); and on a plan entitled "Subdivision Plan, Tax Map 218 – Lot 5, owner Richard P. Fusegni, 201 Kearsarge Way, City of Portsmouth, County of Rockingham, State of New Hampshire", prepared by Ambit Engineering, Inc, dated January , 2020.

WHEREAS, the property possesses natural, scenic, open space, forest, and wildlife habitat values (hereinafter referred to as "conservation values") of great importance to the Declarant, and the City of Portsmouth; and

WHEREAS, the Declarant further intends, as owner of the Property, to restrict certain uses upon a portion of the property to preserve and protect the conservation values of the Land (as hereinafter defined) in perpetuity; and

NOW, THEREFORE, in consideration of the above and the mutual covenants, terms, conditions, and restrictions contained herein, and pursuant to the laws of New Hampshire and in particular New Hampshire RSA 477:45-47, the Declarant hereby voluntarily places a conservation restriction in perpetuity over the specified portion of the Property ("Conservation Land") of the nature and character and to the extent hereinafter set forth. See attached Exhibit B for description of the Conservation Land.

1. CONSERVATION PURPOSE(S)

- A. To protect the natural ecosystem of the Conservation Land.
- B. To protect the natural habitat of plants and wildlife.
- C. To preserve open spaces, particularly the land of which the Conservation Land consists, for the scenic enjoyment of the general public.

2. <u>USE LIMITATIONS</u>

- A. The Conservation Land shall be maintained in perpetuity as open space free from industrial or commercial activities, except for those activities and structures reserved by Declarant in Section 3 hereof.
- B. The Conservation Land shall not be subdivided or otherwise divided into parcels of separate ownership, other than as shown on the herein referenced Subdivision Plan and may only be sold, conveyed, transferred, or devised as is shown on said Subdivision Plan as the rear portion of each of the 3 Lots shown on said Subdivision Plan.
- C. Other than in connection with development and operation of the Subdivision Plan, no structure or improvement of any kind, except for ancillary structures shall be constructed, placed or introduced onto the Conservation Land.
 - i. Ancillary structures and improvements such as drainage structures,, or culverts, may be constructed, placed or introduced onto the Conservation Land as necessary in the accomplishment of the agricultural, forestry or noncommercial outdoor recreational uses of the Conservation Land so long as they are not detrimental to the purposes of this Declaration.
- D. Other than in connection with the development and operation of the Subdivision Plan, no removal, filling, or other disturbances of soils surface, nor any changes in topography, surface or subsurface water systems, or natural habitat shall be permitted on the Conservation Land, except:
 - i. As is necessary for the accomplishment of the conservation purpose(s) of this Restriction; and
- E There shall be no mining, quarrying, excavation, or removal of rocks, minerals, gravel, sand, topsoil, or other similar materials on the Conservation Land.
- F. There shall be no dumping, injection, burning or burial of materials of any kind.

- G. There shall be no cutting of trees or vegetation other than in the normal course of silvaculture.
- H. No hunting, firearms or weapons of any type shall be allowed on the Conservation Land.

3. **DECLARANT'S RESERVED RIGHTS**

- A. Declarant reserves the right to install, maintain, repair, or replace in kind vegetation and stormwater treatment swales and drainage structures
- B. Declarant reserves the right to repair any damage caused to the Conservation Land by natural or other causes provided said repairs are in conformance with the conservation values/purposes stated herein.

4. **GENERAL PROVISIONS**

- A. <u>Controlling Law</u>. The interpretation and performance of this Conservation Restrictive Covenant shall be governed by the laws of the State of New Hampshire.
- B. <u>Liberal Construction</u>. Any general rule of construction to the contrary notwithstanding, this Conservation Restrictive Covenant shall be liberally construed in favor of the Declaration to affect the conservation purpose of this Declaration. If any provision in this instrument is found to be ambiguous, an interpretation consistent with the purpose of this Declaration that would render the provision valid shall be favored over any interpretation that would render it invalid.
- C. <u>Severability</u>. If any provision of this Declaration, or the application thereof to any person or circumstance is found to be invalid by a court of competent jurisdiction, by confirmation of an arbitration award or otherwise, the remainder of the provisions of this Declaration or the application of such provision to persons or circumstances other than those to which it is found to be invalid, as the case may be, shall not be affected thereby.
- D. <u>Termination of Rights and Obligations</u>. A party's rights and obligations under this Declaration terminate upon transfer of the party's interest in the Declaration or Conservation Land, except that liability for acts or omissions occurring prior to transfer shall survive transfer.
- E. <u>No Merger of Interests</u>. The Declarant explicitly agrees that the provisions set forth in this Declaration are intended to last in perpetuity, and that to that end no

purchase or transfer of the underlying fee interest in the Conservation Land by or to any successor or assign of the Declarant shall be deemed to eliminate the provisions set forth hereunder under the doctrine of "merger" or any other legal doctrine.

F. <u>Captions</u>. The captions in this instrument have been inserted solely for convenience of reference and are not a part of this instrument and shall have no effect upon construction or interpretation.

WHEREBY the Declarant, by recording this Declaration of Conservation Restriction for himself, his successors and assigns, agrees to be bound by, to observe and enforce its provisions, and to assume the rights and responsibilities granted to and incumbent upon him, his heirs successors and assigns, all in furtherance of the conservation purpose(s) for which this Declaration is delivered.

IN WITNESS WHEREOF, Declarant has set his hand on this _____day of March, 2020.

DECLARANT:

Richard P. Fusegni

The State of New Hampshire County of

Personally appeared Richard P.Fusegni, who acknowledged the foregoing to be his voluntary act and deed.

Before me,

Notary Public/Justice of the Peace

STATEMENT OF AUTHORIZATION

The undersigned, Richard Fusegni, owner of property at 201 Kearsarge Way, Portsmouth, NH, do hereby authorize Bernard W. Pelech, and Bosen and Associates as attorneys, and Ambit Engineering, Inc as project engineers to prepare and file any and all applications for the City of Portsmouth Planning Board, and further authorizes Bernard W. Pelech and Bosen and Associates, and Ambit Engineering to represent my interests before the Planning Board for the City of Portsmouth with regard to the subdivision of the property located at 201 Kearsarge Way.

achard Arseyn

Signature

13-2019

Date



City of Portsmouth, New Hampshire

Subdivision Application Checklist

This subdivision application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. <u>The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of</u> <u>all subdivision review requirements</u>. Please refer to the Subdivision review regulations for full details.

Applicant Responsibilities (Section III.C): Applicable fees are due upon application submittal along with required number of copies of the Preliminary or final plat and supporting documents and studies. Please consult with Planning staff for submittal requirements.

Owner:Richard P. Fusegni		Date Submitted:	020		_
Applicant: Ambit Engineering, Inc.					
Phone Number:603-430-9282	E-mail:	jrc@ambitengineering.com			
Site Address 1: _201 Kearsarge Way			_ Map: _ ²¹⁸ _	_Lot: <u>5</u>	_
Site Address 2:			_ Map:	_Lot:	

	Application Requirements		
Ø	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested
Ď	Completed Application form. (III.C.2-3)		N/A
	All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF). (III.C.4)		N/A

	Requirements for Pr	eliminary/Final Plat		
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Name and address of record owner, any option holders, descriptive name of subdivision, engineer and/or surveyor or name of person who prepared the plat. (Section IV.1/V.1)	Cover sheet	☑ Preliminary Plat ☑ Final Plat	N/A

	Requirements for Pr	eliminary/Final Plat		
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Preliminary Plat Names and addresses of all adjoining property owners. (Section IV.2) Final Plat Names and addresses of all abutting property owners, locations of buildings within one hundred (100) feet of the parcel, and any new house numbers within the subdivision. (Section V.2)	Subdivision plan House # TBD	☑ Preliminary Plat ☑ Final Plat	N/A
	North point, date, and bar scale. (Section IV.3/V3)	Required on all Plan Sheets	 ✓ Preliminary Plat ✓ Final Plat 	N/A
	Zoning classification and minimum yard dimensions required. (Section IV.4/V.4)	Subdivision plan	☑ Preliminary Plat ☑ Final Plat	N/A
	Preliminary Plat Scale (not to be smaller than one hundred (100) feet = 1 inch) and location map (at a scale of 1" = 1000'). (Section IV.5) Final Plat Scale (not to be smaller than 1"=100'), Location map (at a scale of 1"=1,000') showing the property being subdivided and its relation to the surrounding area within a radius of 2,000 feet. Said location map shall delineate all streets and other major physical features that my either affect or be affected by the proposed development. (Section V.5) Location and approximate dimensions of all existing and proposed property lines including the entire area proposed lots, and any adjacent parcels in the same ownership. (Section IV.6)	Cover sheet Subdivision plan	 ☑ Preliminary Plat ☑ Final Plat ☑ Preliminary Plat ☑ Final Plat 	N/A
	Dimensions and areas of all lots and any and all property to be dedicated or reserved for schools, parks, playgrounds, or other public purpose. Dimensions shall include radii and length of all arcs and calculated bearing for all straight lines. (Section V.6/ IV.7)	N/A	☑ Preliminary Plat ☑ Final Plat	N/A
	Location, names, and present widths of all adjacent streets, with a designation as to whether public or private and approximate location of existing utilities to be used. Curbs and sidewalks shall be shown. (Section IV.8/V.7)	Subdivision plan	☑ Preliminary Plat ☑ Final Plat	

	Requirements for Pr	eliminary/Final Plat		
Ð	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Location of significant physical features, including bodies of water, watercourses, wetlands, railroads, important vegetation, stone walls and soils types that my influence the design of the subdivision. (Section IV.9/V.8)	Sheet C1	☑ Preliminary Plat ☑ Final Plat	
	Preliminary Plat Proposed locations, widths and other dimensions of all new streets and utilities, including water mains, storm and sanitary sewer mains, catch basins and culverts, street lights, fire hydrants, sewerage pump stations, etc. (Section IV.10) Final Plat Proposed locations and profiles of all proposed streets and utilities, including water mains, storm and sanitary sewer mains, catchbasins and culverts, together with typical cross sections. Profiles shall be drawn to a horizontal scale of 1"=50' and a vertical scale of 1"=5', showing existing centerline grade, existing left and right sideline grades, and proposed centerline grade	Sheet C3	☑ Preliminary Plat ☑ Final Plat	
	(Section V.9) When required by the Board, the plat shall be accompanied by profiles of proposed street grades, including extensions for a reasonable distance beyond the subject land; also grades and sizes of proposed utilities. (Section IV.10)	N/A	☑ Preliminary Plat ☑ Final Plat	
	Base flood elevation (BFE) for subdivisions involving greater than five (5) acres or fifty (50) lots. (Section IV.11) For subdivisions of five (5) lots or more, or at the discretion of the Board otherwise, the	Subdivsion plan	 ☑ Preliminary Plat ☑ Final Plat ☑ Preliminary Plat ☑ Final Plat 	
	preliminary plat shall show contours at intervals no greater than two (2) feet. Contours shall be shown in dotted lines for existing natural surface and in solid lines for proposed final grade, together with the final grade elevations shown in figures at all lot corners. If existing grades are not to be changed, then the contours in these areas shall be solid lines. (Section IV.12/ V.12)	Cheet C2		

	Requirements for Pro	eliminary/Final Plat		
Ŋ	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Dates and permit numbers of all necessary permits from governmental agencies from which approval is required by Federal or State law. (Section V.10)	N/A	□ Preliminary Plat ☑ Final Plat	
	For subdivisions involving greater than five (5) acres or fifty (50) lots, the final plat shall show hazard zones and shall include elevation data for flood hazard zones. (Section V.11)	Not in flood zone	□ Preliminary Plat ☑ Final Plat	
	Location of all permanent monuments. (Section V.12)	Subdivision plan	 □ Preliminary Plat ☑ Final Plat 	

	General Requirement	ts ¹	
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	 Basic Requirements: (VI.1) a. Conformity to Official Plan or Map b. Hazards c. Relation to Topography d. Planned Unit Development 	Sheet C1	
	 2. Lots: (VI.2) a. Lot Arrangement b. Lot sizes c. Commercial and Industrial Lots 	Subdivision plan	
	 Streets: (VI.3) a. Relation to adjoining Street System b. Street Rights-of-Way c. Access d. Parallel Service Roads e. Street Intersection Angles f. Merging Streets g. Street Deflections and Vertical Alignment h. Marginal Access Streets i. Cul-de-Sacs j. Rounding Street Corners k. Street Name Signs l. Street Names m. Block Lengths n. Block Widths o. Grade of Streets 	N/A	
	4. Curbing: (VI.4)	N/A	
	5. Driveways: (VI.5)	Sheet C1	
	6. Drainage Improvements: (VI.6)	Sheet C2	
	7. Municipal Water Service: (VI.7)	Sheet C3 / P1	
	8. Municipal Sewer Service: (VI.8)	Sheet C3 / P1	
	 9. Installation of Utilities: (VI.9) a. All Districts b. Indicator Tape 	Sheet C3 / P1	
	10. On-Site Water Supply: (VI.10)	N/A	
	11. On-Site Sewage Disposal Systems: (VI.11)	N/A	
	 12. Open Space: (VI.12) a. Natural Features b. Buffer Strips c. Parks d. Tree Planting 	Subdivision plan	
	 13. Flood Hazard Areas: (VI.13) a. Permits b. Minimization of Flood Damage c. Elevation and Flood-Proofing Records d. Alteration of Watercourses 	N/A	
	14. Erosion and Sedimentation Control (VI.14)	Sheet C2	

Subdivision Application Checklist/April 2019

Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	15. Easements (VI.15)a. Utilitiesb. Drainage	Subdivision plan	
	16. Monuments: (VI.16)	Subdivision plan	
	17. Benchmarks: (VI.17)	Sheet C2	
	18. House Numbers (VI.18)	TBD	

	Design Standards		
	Required Items for Submittal	Indicate compliance and/or	Waiver
		provide explanation as to	Requested
		alternative design	
1.	Streets have been designed according to the design		
	standards required under Section (VII.1).		
	a. Clearing		
	b. Excavation		
	c. Rough Grade and Preparation of Sub-Grade	N/A	
	d. Base Course		
	e. Street Paving		
	t. Side Slopes		
	g. Approval Specifications		
	i. Sidewalks		
	i. Inspection and Methods		
2	J. Inspection and Methods Storm water Sewers and Other Drainage Annurtenances		
۷.	have been designed according to the design standards		
	required under Section (VII 2)	Drainage Analysis	
	a Design		
	h Standards of Construction		
3	Sanitary Sewers have been designed according to the		
5.	design standards required under Section (VII 3)		
	a Design	Shoot D1	
	h Lift Stations	Sheet PT	
	c. Materials		
	d. Construction Standards		
4.	Water Mains and Fire Hydrants have been designed		
	according to the design standards required under		
	Section (VII.4).	Shoot P1	
	a. Connections to Lots		
	b. Design and Construction		
	c. Materials		
	d. Notification Prior to Construction		

1-21-20

Date:

¹ See City of Portsmouth, NH Subdivision Rules and Regulations for details. Subdivision Application Checklist/April 2019

AMBIT ENGINEERING, INC. CIVIL ENGINEERS AND LAND SURVEYORS

200 Griffin Road, Unit 3, Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

4 February 2020

Juliet Walker, Planning Director City of Portsmouth 1 Junkins Avenue Portsmouth, NH 03801

RE: Request for TAC Site Plan Approval at 201 Kearsarge Way, Tax Map 218 / Lot 5

Dear Ms. Walker:

On behalf of Richard Fusegni we hereby submit the attached and enclosed revisions for TAC Approval for the Proposed Subdivision at 201 Kearsarge Way. This letter is intended to address email comments received on February 3, 2020. Comments from the email, along with responses **in bold** are listed below:

- 1. The driveway for Lot 5-4 needs to extend to the street. The sidewalk will need to be modified. This should be shown on the plans. Plan Sheet C1 has been modified to show the drive extended and tip downs at the drive entrance.
- 2. The driveway to Lot 5-3 should be shifted as far from the intersection with Kearsarge Way as possible. The driveway has been relocated as far from the intersection as possible.
- 3. The drainage for the houses should be incorporated into the back yard areas where they can be maintained. The drainage proposal uses the natural terrain in the rear of the property to control run-off. This was approved in the prior design. This will save more of the trees in their natural state.
- 4. All water services need to be at least 1". What is the size of the existing service that you hope to reuse? The water service note on Sheet C1 states "1" Copper Water Service (TYP)". A note has been added to require verification that the existing service is at least a 1 inch service.
- 5. Show 2" water main being abandoned. The plan has the new water main in the same trench as the existing 2 inch water main. We prefer to keep that design rather than leave a pipe in place; we understand that means we will need to provide temporary water services to any impacted abutters and added Note 6 on Sheet C3 in that regard.
- 6. Birch Street is to be reclaimed and reconstructed to City Standards. **This has been noted on Sheet C3.**
- 7. Provide easement to turn around in driveway of Lot 5-2 for plowing/city access. Note 10 has been be added to the subdivision plan in that regard.
- 8. Show gas lines. We believe that there are no gas lines along Kearsarge Way in this vicinity.
- 9. Show electric and cable lines. These will be added to the plan for the

Planning Board submission; Eversource is in the process of relocating some lines in the project vicinity.

- 10. 2' stub out of manhole is sufficient. The Plan and Profile has been revised.
- 11. Materials to be approved by Portsmouth Water / Sewer. Note 4 on Sheet P1 has been added.
- 12. Installation of utilities to be witnessed. Note 5 on Sheet P1 has been added.
- 13. Remove detail F / C3, it is incorrect and doesn't meet standards. The detail has been removed.
- 14. Provide City standard detail for drop sewer manholes on plan set. This has been added as Detail O on Sheet D3.
- **15.** What is the existing sewer in Kearsarge? It is shown as PVC, AC and Clay on various plans and details. **The existing sewer is listed as AC on the city utility maps; designations have been updated to be consistent.**
- 16. If not already completed, demolition of the existing structure may require review by the Demolition Review Committee. **Agreed.**
- It appears that many mature trees are located within and adjacent to the proposed limit of work. They should be shown on the existing conditions plan. Trees are shown on Sheet C1.
- 18. How is the conservation restriction area proposed to be monumented to prevent future encroachment and clearing by the lot owners? The conservation easement edge will be marked with placards in trees and ground markers. A note has been added to the subdivision plan.

We look forward to the TAC Committee's review of the submission and the response to comments herein.

Sincerely,

John Chagnon

John R. Chagnon, PE CC: Richard Fusegni, Bernie Pelech



	AMBIT ENGINEERING Civil Engineers & Land Sur	G, INC. veyors
КТН 0245	200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315	
PLAN	NOTES: 1) PARCEL IS SHOWN ON THE CITY OF PORTSM ASSESSOR'S MAP 218 AS LOT 5.	OUTH
RCRD	2) OWNER OF RECORD: RICHARD P. FUSEGNI 201 KEARSARGE WAY PORTSMOUTH, N.H. 03801 5476/2661 (5979/2783) RCRD PLAN 0245	:
	3) PARCEL IS NOT IN A FLOOD HAZARD ZONE A ON FIRM PANEL 33015C0259E, EFFECTIVE MAY 1	AS SHOWN 7, 2005.
	4) EXISTING LOT AREA: 47,062 S.F. 1.0804 AC.	
	5) PARCEL IS LOCATED IN THE SINGLE RESIDEN (SRB) DISTRICT.	CE B
	6) DIMENSIONAL REQUIREMENTS: MIN. LOT AREA: 15,000 S.F. FRONTAGE: 100 FT. SETBACKS: FRONT: 30 FT. SIDE: 10 FT. REAR: 30 FT. MAXIMUM STRUCTURE HEIGHT: 35 FT. MAXIMUM STRUCTURE COVERAGE: 20%	
	7) THE PURPOSE OF THIS PLAN IS TO SHOW T	HE
	8) OAK STREET WAS CREATED BY A PLAN DATE	D 1919
	AND WAS NEVER CONSTRUCTED. BY OPERATION OF THE AREAS SHOWN BELONG TO THE RESPECTIVE WAY OF APPROPRIATION OF REVERSION RIGHTS. A SHOWN ARE NOT INCLUDED IN EXISTING LOT ARE	DF LAW LOTS BY AREAS A.
	9) PROPOSED CONSERVATION EASEMENT AREA RESTRICTIONS SUBJECT TO REVIEW AND APPROVA CITY OF PORTSMOUTH. CONSERVATION EASEMENT RESTRICTIONS WILL ALLOW FOR INSTALLATION AND MAINTENANCE OF PROPOSED DRAINAGE	L BY THE
	10) MAP 218 LOT 5-2 (PROPOSED LOT 1) WIL A TURNING EASEMENT TO THE CITY OF PORTSMO SNOW PLOWING & ACCESS IN THE LOCATION OF DRIVEWAY.	L GRANT UTH FOR THE
N , 301		
	3 ADD NOTE 10 & CONSERVATION EASEMENT MARKER NOTE 2 ISSUED FOR APPROVAL 1 REVISED FOR SUBMISSION	2/4/20 1/21/20 10/8/19
	0 ISSUED FOR COMMENT NO. DESCRIPTION REVISIONS	4/16/19 DATE
	SUBDIVISION PLAN TAX MAP 218 – LOT OWNER	5
	CITY OF PORTSMOUTH COUNTY OF ROCKINGHA STATE OF NEW HAMPSHI	NI M RE









AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road – Unit 3 Portsmouth, N.H. 03801–7114 Tel (603) 430–9282 Fax (603) 436–2315

NOTES:

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1–888–DIG–SAFE (1–888–344–7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BÉST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) THERE ARE NO WETLANDS ON SUBJECT PROPERTY.

5) BUILDINGS WILL BE GUTTERED TO DIRECT ROOF RUNOFF TO THE SIDE INFILTRATION TRENCH OR REAR OF THE LOT.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

3	EXTEND TRENCH	2/4/20
2	ISSUED FOR APPROVAL	1/21/20
1	SITE LAYOUT	12/13/19
0	ISSUED FOR COMMENT	10/8/19
NO.	DESCRIPTION	DATE
	REVISIONS	



SCALE: 1" = 20'JANUARY 2020



C2







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4) THERE ARE NO WETLANDS ON SUBJECT PROPERTY.

5) PROPOSED UTILITY CONNECTIONS TO BE REVIEWED AND APPROVED BY PORTSMOUTH DPW PRIOR TO ISSUANCE OF BUILDING PERMIT.

6) TEMPORARY WATER SERVICE SHALL BE PROVIDED TO MAP 218 LOTS 2 & 3 DURING CONSTRUCTION OF NEW WATER LINE.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.







C3





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

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KEARSARG

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY WITHIN 100 FEET OF UNDERGROUND UTILITIES. THE EXCAVATOR IS RESPONSIBLE TO MAINTAIN MARKS. DIG SAFE TICKETS EXPIRE IN THIRTY DAYS.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) ALL UTILITY MATERIALS TO BE APPROVED BY PORTSMOUTH WATER AND SEWER DEPARTMENTS.

5) UTILITY INSTALLATION SHALL BE WITNESSED BY THE C.O.P.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

1

		, - · · -		
н 946 73				
	1	NOTES 4&5, STUB	2/4/20	C
	0	ISSUED FOR COMMENT	1/21/2	0
	NO.	DESCRIPTION	DATE	
	REVISIONS			
= 20' = 2'				
F	SC	ALE: AS SHOWN JAN	UARY 2020	0
60 80 HORIZONTAL 6 8		UTILITY PLAN AND PROFILE	P 1	
	FB	. 311, PG 1	225	58

EROSION CONTROL NOTES

CONSTRUCTION SEQUENCE

DO NOT BEGIN CONSTRUCTION UNTIL ALL LOCAL, STATE AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED.

INSTALL INLET PROTECTION AND PERIMETER CONTROLS, i.e., SILT FENCING OR SILTSOXX AROUND THE LIMITS OF DISTURBANCE BEFORE ANY EARTH MOVING OPERATIONS.

CUT AND GRUB ALL TREES, SHRUBS, SAPLINGS, BRUSH, VINES AND REMOVE OTHER DEBRIS AND RUBBISH AS REQUIRED.

REMOVE EXISTING HOUSE AND SHED

CONSTRUCT SITE UTILITIES.

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE "STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT FROSION.

DUST CONTROL: IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND MULCHING, DO NOT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILT FENCES AND SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM. ALL DAMAGED SILT FENCES AND SILTSOXX SHALL BE REPAIRED. SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

AVOID THE USE OF FUTURE OPEN SPACES (LOAM AND SEED AREAS) WHEREVER POSSIBLE DURING CONSTRUCTION. CONSTRUCTION TRAFFIC SHALL USE THE ROADBEDS OF FUTURE ACCESS DRIVES AND PARKING AREAS.

ADDITIONAL TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN AMOUNTS NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS -- CONSTRUCT SILT FENCE OR SILTSOXX AROUND TOPSOIL STOCKPILE.

AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL. STUMPS SHALL BE DISPOSED OF IN AN APPROVED FACILITY.

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT. SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH, WOODY DEBRIS, LEAVES, BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED. ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL.

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED: - BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED

- A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED - A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS

BFEN INSTALLED - EROSION CONTROL BLANKETS HAVE BEEN INSTALLED

VEGETATIVE PRACTICE

FOR PERMANENT MEASURES AND PLANTINGS

LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF 2 TONS PER ACRE.

FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 500 POUNDS PER ACRE OF 10-20-20 FERTILIZER.

SEED SHALL BE SOWN AT THE RATES SHOWN IN THE TABLE BELOW. IMMEDIATELY BEFORE SEEDING. THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AT A RATE OF 1.5 TO 2 TONS PER ACRE, AND SHALL BE HELD IN PLACE USING APPROPRIATE TECHNIQUES FROM THE EROSION AND SEDIMENT CONTROL HANDBOOK.

THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED. ANY AREAS WHICH ARE NOT SATISFACTORILY COVERED SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED.

A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE:

GENERAL COVER	PROPORTION SEEDING RATE
CREEPING RED FESCUE KENTUCKY BLUEGRASS	50% 100 LBS/ACRE 50%
<u>SLOPE SEED</u> (USED ON	ALL SLOPES GREATER THAN OR EQUAL TO 3:1)
CREEPING RED FESCUE TALL FESCUE BIRDSFOOT TREFOIL	42% 42% 48 LBS/ACRE 16%

IN NO CASE SHALL THE WEED CONTENT EXCEED ONE PERCENT BY WEIGHT, ALL SEED SHALL COMPLY WITH APPLICABLE STATE AND FEDERAL SEED LAWS.

FOR TEMPORARY PROTECTION OF DISTURBED AREAS: MULCHING AND SEEDING SHALL BE APPLIED AT THE FOLLOWING RATES: PERENNIAL RYE: 0.7 LBS/1,000 S.F. 1.5 TONS/ACRE MULCH:

MAINTENANCE AND PROTECTION

THE CONTRACTOR SHALL MAINTAIN ALL LOAM & SEED AREAS UNTIL FINAL ACCEPTANCE AT THE COMPLETION OF THE CONTRACT. MAINTENANCE SHALL INCLUDE WATERING, WEEDING, REMOVAL OF STONES AND OTHER FOREIGN OBJECTS OVER 1/2 INCHES IN DIAMETER WHICH MAY APPEAR AND THE FIRST TWO (2) CUTTINGS OF GRASS NO CLOSER THEN TEN (10) DAYS APART. THE FIRST CUTTING SHALL BE ACCOMPLISHED WHEN THE GRASS IS FROM 2 1/2 TO 3 INCHES HIGH. ALL BARE AND DEAD SPOTS WHICH BECOME APPARENT SHALL BE PROPERLY PREPARED, LIMED AND FERTILIZED, AND RESEEDED BY THE CONTRACTOR AT HIS EXPENSE AS MANY TIMES AS NECESSARY TO SECURE GOOD GROWTH. THE ENTIRE AREA SHALL BE MAINTAINED, WATERED AND CUT UNTIL ACCEPTANCE OF THE LAWN BY THE OWNER'S REPRESENTATIVE.

THE CONTRACTOR SHALL TAKE WHATEVER MEASURES ARE NECESSARY TO PROTECT THE GRASS WHILE IT IS DEVELOPING.

TO BE ACCEPTABLE, SEEDED AREAS SHALL CONSIST OF A UNIFORM STAND OF AT LEAST 90 PERCENT ESTABLISHED PERMANENT GRASS SPECIES, WITH UNIFORM COUNT OF AT LEAST 100 PLANTS PER SQUARE FOOT.

SEEDED AREAS WILL BE FERTILIZED AND RESEEDED AS NECESSARY TO INSURE VEGETATIVE ESTABLISHMENT.

THE SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATION IS ESTABLISHED.

THE SILT FENCE OR SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL.

SILT FENCING AND SILTSOXX SHALL BE REMOVED ONCE VEGETATION IS ESTABLISHED, AND DISTURBED AREAS RESULTING FROM SILT FENCE AND SILTSOXX REMOVAL SHALL BE PERMANENTLY SEEDED.

WINTER NOTES

FILTREXX®

ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.

AFTER NOVEMBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.

 $-2" \times 2"$ HARDWOOD

STAKES SPACED 10'



ALL MATERIAL TO MEET FILTREXX SPECIFICATIONS. 2. FILLTREXX SYSTEM SHALL BE INSTALLED BY A CERTIFIED

- FILTREXX INSTALLER. THE CONTRACTOR SHALL MAINTAIN THE COMPOST FILTRATION
- SYSTEM IN A FUNCTIONAL CONDITION AT ALL TIMES. IT WILL BE ROUTINELY INSPECTED AND REPAIRED WHEN REQUIRED. 4. SILTSOXX DEPICTED IS FOR MINIMUM SLOPES, GREATER SLOPES
- MAY REQUIRE ADDITIONAL PLACEMENTS. THE COMPOST FILTER MATERIAL WILL BE DISPERSED ON SITE WHEN NO LONGER REQUIRED, AS DETERMINED BY THE ENGINEER.









SUITABLE BACKFILL PER UTILITY COMPANY SPECIFICATIONS OR WATER LINES 2"Ø PVC FOR PHONE & CABLE TV (SEE NOTE 1)

NTS



PIPE SADDLE (TAPPED w/ C.C. THREADS)

PAINT COLORS BARREL: GREEN CAPS: GREEN TOP: REFLECTIVE WHITE

> FINISH GRADE CURB BOX & ROD - BALL VALVE CURB STOP FLOW FROM

NTS



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) ALL WATER LINE INSTALLATION WORK SHALL BE TO CITY OF PORTSMOUTH WATER DEPARTMENT STANDARDS. DETAILS MAY OR MAY NOT BE UP-TO-DATE.

PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.

1	DETAIL J	2/4/20
0	ISSUED FOR APPROVAL	1/21/20
NO.	DESCRIPTION	DATE
	REVISIONS	



SCALE AS NOTED

DETAILS

JANUARY 2020



2258





 O
 DROP
 MANHOLE
 (INSTALLATION
 OVER
 EXISTING
 MAIN)

 C3
 NTS



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

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PROPOSED SUBDIVISION 201 KEARSARGE WAY PORTSMOUTH, N.H.





DETAILS

AS NOTED

JANUARY 2020



City of Portsmouth, New Hampshire

Subdivision Application Checklist

This subdivision application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. <u>The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of</u> <u>all subdivision review requirements</u>. Please refer to the Subdivision review regulations for full details.

Applicant Responsibilities (Section III.C): Applicable fees are due upon application submittal along with required number of copies of the Preliminary or final plat and supporting documents and studies. Please consult with Planning staff for submittal requirements.

Owner:Richard P. Fusegni	r: Richard P. Fusegni Date Submitted: 1/21/2020			_	
Applicant: Ambit Engineering, Inc.					
Phone Number:603-430-9282	E-mail:	jrc@ambitengineering.com			
Site Address 1: _201 Kearsarge Way			_ Map: _ ²¹⁸ _	_Lot: <u>5</u>	_
Site Address 2:			_ Map:	_Lot:	

	Application Requirements				
Ø	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested		
Ď	Completed Application form. (III.C.2-3)		N/A		
	All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF). (III.C.4)		N/A		

	Requirements for Preliminary/Final Plat			
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Name and address of record owner, any option holders, descriptive name of subdivision, engineer and/or surveyor or name of person who prepared the plat. (Section IV.1/V.1)	Cover sheet	☑ Preliminary Plat ☑ Final Plat	N/A

Requirements for Preliminary/Final Plat				
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Preliminary Plat Names and addresses of all adjoining property owners. (Section IV.2) Final Plat Names and addresses of all abutting property owners, locations of buildings within one hundred (100) feet of the parcel, and any new house numbers within the subdivision. (Section V.2)	Subdivision plan House # TBD	☑ Preliminary Plat ☑ Final Plat	N/A
	North point, date, and bar scale. (Section IV.3/V3)	Required on all Plan Sheets	 ✓ Preliminary Plat ✓ Final Plat 	N/A
	Zoning classification and minimum yard dimensions required. (Section IV.4/V.4)	Subdivision plan	☑ Preliminary Plat ☑ Final Plat	N/A
	Preliminary Plat Scale (not to be smaller than one hundred (100) feet = 1 inch) and location map (at a scale of 1" = 1000'). (Section IV.5) Final Plat Scale (not to be smaller than 1"=100'), Location map (at a scale of 1"=1,000') showing the property being subdivided and its relation to the surrounding area within a radius of 2,000 feet. Said location map shall delineate all streets and other major physical features that my either affect or be affected by the proposed development. (Section V.5) Location and approximate dimensions of all existing and proposed property lines including the entire area proposed lots, and any adjacent parcels in the same ownership. (Section IV.6)	Cover sheet Subdivision plan	 ☑ Preliminary Plat ☑ Final Plat ☑ Preliminary Plat ☑ Final Plat 	N/A
	Dimensions and areas of all lots and any and all property to be dedicated or reserved for schools, parks, playgrounds, or other public purpose. Dimensions shall include radii and length of all arcs and calculated bearing for all straight lines. (Section V.6/ IV.7)	N/A	☑ Preliminary Plat ☑ Final Plat	N/A
	Location, names, and present widths of all adjacent streets, with a designation as to whether public or private and approximate location of existing utilities to be used. Curbs and sidewalks shall be shown. (Section IV.8/V.7)	Subdivision plan	☑ Preliminary Plat ☑ Final Plat	

Requirements for Preliminary/Final Plat				
Ð	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Location of significant physical features, including bodies of water, watercourses, wetlands, railroads, important vegetation, stone walls and soils types that my influence the design of the subdivision. (Section IV.9/V.8)	Sheet C1	☑ Preliminary Plat ☑ Final Plat	
	Preliminary Plat Proposed locations, widths and other dimensions of all new streets and utilities, including water mains, storm and sanitary sewer mains, catch basins and culverts, street lights, fire hydrants, sewerage pump stations, etc. (Section IV.10) Final Plat Proposed locations and profiles of all proposed streets and utilities, including water mains, storm and sanitary sewer mains, catchbasins and culverts, together with typical cross sections. Profiles shall be drawn to a horizontal scale of 1"=50' and a vertical scale of 1"=5', showing existing centerline grade, existing left and right sideline grades, and proposed centerline grade	Sheet C3	☑ Preliminary Plat ☑ Final Plat	
	(Section V.9) When required by the Board, the plat shall be accompanied by profiles of proposed street grades, including extensions for a reasonable distance beyond the subject land; also grades and sizes of proposed utilities. (Section IV.10)	N/A	☑ Preliminary Plat ☑ Final Plat	
	Base flood elevation (BFE) for subdivisions involving greater than five (5) acres or fifty (50) lots. (Section IV.11) For subdivisions of five (5) lots or more, or at the discretion of the Board otherwise, the	Subdivsion plan	 ☑ Preliminary Plat ☑ Final Plat ☑ Preliminary Plat ☑ Final Plat 	
	preliminary plat shall show contours at intervals no greater than two (2) feet. Contours shall be shown in dotted lines for existing natural surface and in solid lines for proposed final grade, together with the final grade elevations shown in figures at all lot corners. If existing grades are not to be changed, then the contours in these areas shall be solid lines. (Section IV.12/ V.12)	Cheet C2		

	Requirements for Preliminary/Final Plat				
Ŋ	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested	
	Dates and permit numbers of all necessary permits from governmental agencies from which approval is required by Federal or State law. (Section V.10)	N/A	□ Preliminary Plat ☑ Final Plat		
	For subdivisions involving greater than five (5) acres or fifty (50) lots, the final plat shall show hazard zones and shall include elevation data for flood hazard zones. (Section V.11)	Not in flood zone	□ Preliminary Plat ☑ Final Plat		
	Location of all permanent monuments. (Section V.12)	Subdivision plan	 □ Preliminary Plat ☑ Final Plat 		

	General Requirements ¹			
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested	
	 Basic Requirements: (VI.1) a. Conformity to Official Plan or Map b. Hazards c. Relation to Topography d. Planned Unit Development 	Sheet C1		
	 2. Lots: (VI.2) a. Lot Arrangement b. Lot sizes c. Commercial and Industrial Lots 	Subdivision plan		
	 Streets: (VI.3) a. Relation to adjoining Street System b. Street Rights-of-Way c. Access d. Parallel Service Roads e. Street Intersection Angles f. Merging Streets g. Street Deflections and Vertical Alignment h. Marginal Access Streets i. Cul-de-Sacs j. Rounding Street Corners k. Street Name Signs l. Street Names m. Block Lengths n. Block Widths o. Grade of Streets 	N/A		
	4. Curbing: (VI.4)	N/A		
	5. Driveways: (VI.5)	Sheet C1		
	6. Drainage Improvements: (VI.6)	Sheet C2		
	7. Municipal Water Service: (VI.7)	Sheet C3 / P1		
	8. Municipal Sewer Service: (VI.8)	Sheet C3 / P1		
	 9. Installation of Utilities: (VI.9) a. All Districts b. Indicator Tape 	Sheet C3 / P1		
	10. On-Site Water Supply: (VI.10)	N/A		
	11. On-Site Sewage Disposal Systems: (VI.11)	N/A		
	 12. Open Space: (VI.12) a. Natural Features b. Buffer Strips c. Parks d. Tree Planting 	Subdivision plan		
	 13. Flood Hazard Areas: (VI.13) a. Permits b. Minimization of Flood Damage c. Elevation and Flood-Proofing Records d. Alteration of Watercourses 	N/A		
	14. Erosion and Sedimentation Control (VI.14)	Sheet C2		

Subdivision Application Checklist/April 2019

Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	15. Easements (VI.15)a. Utilitiesb. Drainage	Subdivision plan	
	16. Monuments: (VI.16)	Subdivision plan	
	17. Benchmarks: (VI.17)	Sheet C2	
	18. House Numbers (VI.18)	TBD	

Design Standards				
		Required Items for Submittal	Indicate compliance and/or	Waiver
			provide explanation as to	Requested
			alternative design	
	1.	Streets have been designed according to the design		
		standards required under Section (VII.1).		
		a. Clearing		
		b. Excavation		
		c. Rough Grade and Preparation of Sub-Grade	N/A	
		d. Base Course		
		e. Street Paving		
		t. Side Slopes		
		g. Approval Specifications		
		i. Sidewalks		
		i. Inspection and Mathods		
	2	J. Inspection and Methods Storm water Sewers and Other Drainage Annurtenances		
	۷.	have been designed according to the design standards		
		required under Section (VII 2)	Drainage Analysis	
		a Design		
		h Standards of Construction		
	3	Sanitary Sewers have been designed according to the		
	5.	design standards required under Section (VII 3)		
		a Design	Shoot D1	
		h Lift Stations	Sheet PT	
		c. Materials		
		d. Construction Standards		
	4.	Water Mains and Fire Hydrants have been designed		
		according to the design standards required under		
		Section (VII.4).	Shoot D1	
		a. Connections to Lots		
		b. Design and Construction		
		c. Materials		
		d. Notification Prior to Construction		

1-21-20

Date:

¹ See City of Portsmouth, NH Subdivision Rules and Regulations for details. Subdivision Application Checklist/April 2019