

AMBIT ENGINEERING, INC. CIVIL ENGINEERS AND LAND SURVEYORS

200 Griffin Road, Unit 3, Portsmouth, NH 03801

03801 Phone (603) 430-9282

Fax 436-2315

1 August 2019

Mr. Dexter Legg, Chair City of Portsmouth Planning Board 1 Junkins Avenue Portsmouth, NH 03801

RE: Request for Approval of the Clews Subdivision located at 799 South Street, Tax Map 132 / Lot 24

Chairman Legg and Planning Board Members;

On behalf of the Noele M. Clews Revocable Trust we hereby submit a request for Subdivision Approval at 799 South Street. The project team met with the Technical Advisory Committee on July 2, 2019 to review the proposal. The Technical Advisory Committee voted to recommend Preliminary and Final Subdivision Approval with the stipulations listed herein. Responses to the stipulations are highlighted in **Bold Text**:

- Change the grass paver to use a surface that is readily apparent as a drivable surface for the Fire Truck access. Add note to plans that this area will be maintained year round to allow fire truck access as necessary. The drivable area for fire truck turn-in has been revised to granite pavers from grass pavers, with an option to provide asphalt paving. Also see Note 8 on the Subdivision Plan.
- 2. The sight distances in both directions at each driveway should be verified on a profile plan of the roadway to be reviewed and confirmed by Eric Eby, the City's Transportation and Parking Engineer. Site distance profiles have been provided in the plan revised plan set. After review, the driveway location for Lot 1 was relocated to the existing lower driveway. The plans now show that we have adequate driveway site distance in both directions at both driveways. We have followed up with the Parking and Transportation Engineer, Eric Eby, and attached his email stating that he is satisfied that the site distance requirements have been met.
- 3. Update plans to be consistent regarding the number of mature trees proposed to be removed for this project. Every effort should be made to preserve existing mature trees. Plans have been revised to consistently show 10 trees to be removed for lot development.
- 4. Relocate the utility pole on the plan to the spot approved by the City. The utility pole has been relocated to reflect the spot approved by the city, see the Utility Plan Sheet C2.

- 5. Add note to plans that construction of utilities and driveways shall impact the sidewalk for no more than 1 week. Safe access shall be restored each night. Note #8 has been added to Sheet C2.
- 6. The sidewalk in front of the property up to the nearest adjacent driveways shall be replaced with concrete meeting the City's specifications. This work shall take no more than 1 week to full restoration. All sidewalk construction shall meet ADA standards. Note #9 has been added to Sheet C2.
- 7. Update turning template for fire truck to show cars parked in proposed parking spaces. The turning template for the fire truck has been updated showing movements with cars parked in the proposed parking space and a 16' wide driveway. See the Fire Truck Turning Plan submitted herewith. We have attached an email from Deputy Fire Chief, Patrick Howe stating the 16' wide driveway is acceptable.
- 8. The plans shall note that the removal of the existing and construction of the proposed driveways shall be done in a sequence that will maintain driveway access to existing residences at all times. Note #8 of the Subdivision Plan states, "Prior to discontinuance of the driveway access to Lot 2, the new Lot 2 / 3 Driveway shall be installed and maintained for the occupants."

To be included as stipulations of Planning Board approval:

1. An easement shall be provided to benefit the City wherever the sidewalk crosses private property including a 2' paralleling the sidewalk to allow for snow storage. Note #9 on the Subdivision Plan states that an easement shall be provided.

Also submitted herewith is a Waiver Request to allow an overhead electrical service to the proposed structure on Lot 3.

We are available to meet with City Staff should you have any questions or concerns. We look forward to your approval of this proposed subdivision at the August Planning Board meeting.

Sincerely. John R. Chagnon, PE

CC: Christopher Kit Clews, Bernie Pelech, File

J:\JOBS1\JN1700s\JN1730s\JN1736\2018 Subdivision\Applications\City of Portsmouth Subdivision and Site\07 Submission for 8-8-19\Planning Board Submission Letter 08-1-19.doc

Douglas LaRosa

From:	Patrick R. Howe < prhowe@cityofportsmouth.com>
Sent:	Tuesday, July 16, 2019 12:49 PM
То:	Douglas LaRosa; Jillian Harris
Cc:	Kit Clews; John Chagnon; Bernie Pelech
Subject:	RE: Clews Subdivision, T5 Firetruck (Inferno) Turning Movement and Entrance Pavers

Follow Up Flag:Follow upFlag Status:Flagged

Thank you Doug. The 16' wide driveway is acceptable.

Patrick R. Howe Deputy Fire Chief Portsmouth Fire Department 170 Court St. Portsmouth, NH 03801 603.610.7350 prhowe@cityofportsmouth.com

From: Douglas LaRosa [mailto:djl@ambitengineering.com]
Sent: Tuesday, July 16, 2019 9:17 AM
To: Patrick R. Howe; Jillian Harris
Cc: Kit Clews; John Chagnon; Bernie Pelech
Subject: RE: Clews Subdivision, T5 Firetruck (Inferno) Turning Movement and Entrance Pavers

Good Morning Patrick,

You are correct. The T5 Ladder Truck has very little room for error.

The 12' wide driveway provided has a turning movement were the vehicle and overhang are on or over a drivable surface, but there is little room for error.

I am providing a 14' wide driveway and a 16' wide driveway exhibit in PDF 11x17 format. Please let me know if either of these have acceptable room for error.

We would be happy to meet with you to discuss if that is preferable,

Thanks,

Doug

Douglas LaRosa

From:	Eric B. Eby <ebeby@cityofportsmouth.com></ebeby@cityofportsmouth.com>
Sent:	Monday, July 29, 2019 2:19 PM
То:	Douglas LaRosa; John Chagnon
Cc:	Kit Clews; Juliet T.H. Walker; Jillian Harris
Subject:	RE: 799 South Street, Clews Subdivision, Sight distance from Existing Driveway, Lot 1

Doug

Thank you for the plans. After reviewing both of the sheets, I am satisfied that the minimum sight line requirements will be met at the Lot 1 Driveway. Let me know if you need anything else from me. Eric

Eric B. Eby, P.E.

Parking and Transportation Engineer Department of Public Works City of Portsmouth 680 Peverly Hill Road Portsmouth, NH 03801 (603) 766-1415 Cell 603-828-6695

From: Douglas LaRosa [mailto:djl@ambitengineering.com]
Sent: Friday, July 26, 2019 3:22 PM
To: Eric B. Eby <ebeby@cityofportsmouth.com>; John Chagnon <jrc@ambitengineering.com>
Cc: Kit Clews <Kit@clews.org>
Subject: RE: 799 South Street, Clews Subdivision, Sight distance from Existing Driveway, Lot 1

Eric,

Attached please find C5 Driveway Sight Distance and C7 Stopping Sight Distance (2- to 3.5') for your review. Thank you, Doug

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Douglas J. LaRosa Ambit Engineering 200 Griffin Road Unit 3 Portsmouth, NH 03801 603-766-2993 603-430-9282 (312) FAX 603-436-2315 djl@ambitengineering.com

From: Eric B. Eby <<u>ebeby@cityofportsmouth.com</u>> Sent: Thursday, July 25, 2019 5:03 PM

PROPOSED SUBDIVISION 799 SOUTH STREET PORTSMOUTH, NEW HAMPSHIRE SITE PLANS

OWNERS:

NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE 67 RIDGES COURT PORTSMOUTH, NH 03801

CIVIL ENGINEER & LAND SURVEYOR:

AMBIT ENGINEERING, INC. 200 GRIFFIN ROAD, UNIT 3 PORTSMOUTH, N.H. 03801 Tel. (603) 430-9282 Fax (603) 436-2315

TRAFFIC ENGINEER:

STEPHEN G. PERNAW & COMPANY, INC. PO BOX 1721 CONCORD, N.H. 03302 Tel. (603) 731-8500 Fax (866) 929-6094





DWG No.

INDEX OF SHEETS

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1. <u>1.</u> 1. 1. 1.	SUBDIVISION PLAN
C1	EXISTING CONDITIONS PLAN
C2	UTILITY SITE PLAN
C3	DRIVEWAY PLAN
C4	CONCEPT AND GRADING PLAN
C5-C7	SIGHT DISTANCE PLAN PROFILES
D1-D4	DETAILS

PORTSMOUTH APPROVAL CONDITIONS NOTE: ALL CONDITIONS ON THIS PLAN SET SHALL REMAIN IN EFFECT IN PERPETUITY PURSUANT TO THE REQUIREMENTS OF THE CITY OF PORTSMOUTH SUBDIVISION REGULATIONS.

APPROVED BY THE PORTSMOUTH PLANNING BOARD

CHAIRMAN

DATE

	RB		
	AL AN		
	SITE	SOUTH STREET	aroato Similar
	0		2 mg
Shell R		PINETHURST DRIVE	
SOUTH SOUTH	<u>LOCUS MAP</u> SCALE: 1" = 100'		



 \square

UTILITY CONTACTS

ELECTRIC: **EVERSOURCE** 1700 LAFAYETTE ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 436-7708, Ext. 555.5678 ATTN: MICHAEL BUSBY, P.E. (MANAGER)

SEWER & WATER: PORTSMOUTH DEPARTMENT OF PUBLIC WORKS 680 PEVERLY HILL ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 427-1530 ATTN: JIM TOW

NATURAL GAS: UNITIL 325 WEST ROAD PORTSMOUTH, N.H. 03801 Tel. (603) 294-5144 ATTN: DAVE BEAULIEU

COMMUNICATIONS: FAIRPOINT COMMUNICATIONS JOE CONSIDINE 1575 GREENLAND ROAD GREENLAND, N.H. 03840 Tel. (603) 427-5525

CABLE: COMCAST 155 COMMERCE WAY PORTSMOUTH, N.H. 03801 Tel. (603) 679–5695 (X1037) ATTN: MIKE COLLINS

PERMIT LIST: PORTSMOUTH SUBDIVISION

	LEGE	ND:
EXISTING	PROPOSED	
		PROPERTY LINE SETBACK
S SL	S SL	SEWER PIPE SEWER LATERAL
G	G	GAS LINE
W	W	WATER LINE
WS UGE	WS UGE	WATER SERVICE UNDERGROUND ELECTI
OHW	OHW UD	OVERHEAD ELECTRIC/ FOUNDATION DRAIN
		EDGE OF PAVEMENT (
97x3	98x0	CONTOUR SPOT ELEVATION UTILITY POLE
-Ŏ-		WALL MOUNTED EXTER
		TRANSFORMER ON CO
		ELECTRIC HANDHOLD
N SO 650	NSO GSO	SHUT OFFS (WATER/G
\bowtie		GATE VALVE
-@-	+ + +	HYDRANT
CB	СВ	CATCH BASIN
	€ ^{SMH}	SEWER MANHOLE
	DMH	DRAIN MANHOLE
		TELEPHONE MANHOLE
14	14	PARKING SPACE COUN
PM		PARKING METER
LSA	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	LANDSCAPED AREA
TBD	TBD	TO BE DETERMINED
COP	COP	COPPER PIPE
DI	DI	DUCTILE IRON PIPE
RCP	RCP	REINFORCED CONCRET
AC		ASBESTOS CEMENT PIR
VC FP	VC FP	VITRIFIED CLAY PIPE
EL.	EL.	ELEVATION
FF INV	FF INIV	FINISHED FLOOR
S =	S =	SLOPE FT/FT
TBM TYP	TBM TYP	TEMPORARY BENCH M. TYPICAL

ROPERTY LINE TBACK EWER PIPE EWER LATERAL AS LINE TORM DRAIN ATER LINE ATER SERVICE NDERGROUND ELECTRIC VERHEAD ELECTRIC/WIRES OUNDATION DRAIN DGE OF PAVEMENT (EP) ONTOUR POT ELEVATION JTILITY POLE

WALL MOUNTED EXTERIOR LIGHTS FRANSFORMER ON CONCRETE PAD LECTRIC HANDHOLD

SHUT OFFS (WATER/GAS)

ARKING SPACE COUNT

BE DETERMINED AST IRON PIPE OPPER PIPE UCTILE IRON PIPE OLYVINYL CHLORIDE PIPE EINFORCED CONCRETE PIPE SBESTOS CEMENT PIPE TRIFIED CLAY PIPE DGE OF PAVEMENT LEVATION NISHED FLOOR IVERT LOPE FT/FT EMPORARY BENCH MARK PICAL

CLEWS SUBDIVISION 799 SOUTH STREET PORTSMOUTH, N.H.



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

PLAN SET SUBMITTAL DATE: 8 AUGUST 2019





6" 9//24/08, NOT FOUND 8/6/18, IRON ROD TO BE SET

3/4" IRON PIPE FOUND, DOWN 3" 9/24/08, NOT FOUND 8/6/18, IRON ROD TO BE SET -

(132) 23

N/F

— E.S. 5-41

(OLD)

IRON ROD w/LLS 738 ID CAP FOUND, UP 1" 8/6/18, SET 9/5/08

PLAN REFERENCES:

GWE

1) STANDARD BOUNDARY SURVEY FOR PROPERTY AT 613 UNION STREET, ROCKINGHAM COUNTY, PORTSMOUTH, NEW HAMPSHIRE OWNED BY JOHN R. & ELIZABETH U. FEGELA. DATED 5/15/98 BY EASTERLY SURVEYING. RCRD #D−26381.

METER (GAS, WATER, ELECTRIC)

CATCH BASIN

2) SITE PLAN, BROAD PARK CONDOMINIUMS OFF BROAD STREET, PORTSMOUTH, N.H. DATED 3/10/81 BY RICHARD P. MILLETTE AND ASSOCIATES. RCRD #D-10137. 3) PLAN OF RAND AND HISLOP LAND, SOUTH, UNION AND SPRING STS., PORTSMOUTH, N.H. DATED OCT. 1943 BY JOHN W. DURGIN. RCRD #01239. 4) PLAN OF LOTS, PORTSMOUTH, N.H. OWNED BY C.W. BREWSTER. DATED OCT. 1931 BY JOHN W. DURGIN. RCRD #0687.

VARIANCE REQUEST: 1) SECTION 10.521-TO ALLOW 95.08' OF FRONTAGE FOR LOT 2, WHERE 100' IS REQUIRED. APPROVED 3-26-19

"I CERTIFY THAT THIS PLAN WAS PREPARED UNDER MY DIRECT SUPERVISION, THAT IT IS THE RESULT OF A FIELD SURVEY BY THIS OFFICE AND HAS AN ACCURACY OF THE CLOSED TRAVERSE THAT EXCEEDS THE PRECISION OF :15,000.'

DATE

-CO29 CHAGNON



-8-19

OHN R. CHAGNON, LLS



OTES:	
IES DELINEATED BY STEVEN D. RIKER,	AMBIT ENGINEERING, INC.
AND IN ACCORDANCE WITH THE DARDS:	200 Griffin Road – Unit 3
ARMY CORPS OF ENGINEERS WETLANDS	Portsmouth, N.H. 03801-7114 Tel (603) 430-9282
-ATION MANUAL. LECHNICAL REPORT -1 (JAN. 1987). AND REGIONAL	наластийн гах (оUJ) 430-2315
LEMENT TO THE CORPS OF ENGINEERS	NOTES
IND DELINEATION MANUAL: ICENTRAL AND NORTHEAST REGION.	1) PARCEL IS SHOWN ON THE CITY OF PORTSMOUTH
ON 2.0, JANUARY 2012.	ASSESSOR'S TAX MAP 132 AS LOT 24.
D STATES, VERSION 8.1, USDA-NRCS,	2) OWNERS OF RECORD:
AND (FOR DISTURBED SITES) FIELD	CHRISTOPHER CLEWS, TRUSTEE
W ENGLAND, VERSION 4. NEIWPCC	67 RIDGES COURT PORTSMOUTH, NH 03801
NDS WORK GROUP (2017).	2338/525
R IN WETLANDS: NORTHEAST (REGION	3) PARCEL NOT IN A FLOOD HAZARD ZONE AS SHOWN ON
SFWS (MAY 1988).	(A) = EXISTING + OT ADEA.
VATER HABITATS OF THE UNITED	4) EXISTING LUT AREA: 76,889 S.F.
S. USFW MANUAL FWS/OBS-79/31	1.7651 ACRES
). TIFICATION AND DOCUMENTATION OF	PROPOSED LOT AREAS: LOT 1: 9,004 S.F.
L POOLS IN NEW HAMPSHIRE" (1997).	0.2067 ACRES
TAMPSHIRE FISH AND GAME	LOT 2: 58,885 S.F. 1.3518 ACRES
	10T 3. 9 000 SE
WERE FIELD LOCATED BY AMBIT	0.2066 ACRES
	5) PARCEL IS LOCATED IN GENERAL RESIDENCE A (GRA)
'ARIF	DISTRICT.
	6) DIMENSIONAL REQUIREMENTS: MIN. LOT AREA: 7500 S.F.
16'21"E DISTANCE 8.05'	FRONTAGE: 100 FEET MIN_DEPTH 70 FEET
39'03"E 24.10'	SETBACKS:
	SIDE: 10 FEET
	MAXIMUM STRUCTURE HEIGHT: 35'
	MAXIMUM STRUCTURE COVERAGE: 25% MINIMUM OPEN SPACE: 30%
	7) THE PURPOSE OF THIS PLAN IS TO SHOW THF
	SUBDIVISION OF TAX MAP 132 LOT 24 IN THE CITY OF PORTSMOUTH INTO 3 LOTS.
131	8) PRIOR TO DISCONTINUANCE OF THE DRIVEWAY ADDRESS
	TO LOT 2 THE NEW LOTS 2 & 3 DRIVEWAY SHALL BE
N/F ′ A. LEFEBVRE	DRIVEWAY SHALL BE MAINTAINED TO ITS FULL WIDTH TO
A LEFEBVRE SOUTH ST UTH, NH 0.3801	ALLOW FIRE TRUCK ACCESS TO 779 SOUTH STREET.
28/2635	9) A SIDEWALK EASEMENT (TO INCLUDE THE AREA 2' BEYOND FOR CONSTRUCTION & SNOW STORAGE) FOR
IIIIA	EXISTING SIDEWALK ON LOTS 2 & 3 WILL BE GRANTED TO
w/LLS 738 ID CAP	
2 1" 8/6/18, SET 10/7/08	
E.S.	
5/37 w/ELECTRIC	
PSNH 5/37	
NEI 36 (OLD)	
	2 NUIES 8 & 9 7/8/19
\mathbf{b}	0 ISSUED FOR VARIANCE APPLICATION 2/02/10
ブーン	NO. DESCRIPTION DATE
- JRN &	REVISIONS
RN & ARY KORN STREET	
DRN & ARY KORN STREET NH 03801 264	SUBDIVISION PLAN
DRN & ARY KORN STREET NH 03801 264	SUBDIVISION PLAN TAX MAP 132 - LOT 24
DRN & ARY KORN STREET NH 03801 264	SUBDIVISION PLAN TAX MAP 132 - LOT 24 OWNER: NOFLE M CLEWS
DRN & ARY KORN STREET NH 03801 264	SUBDIVISION PLAN TAX MAP 132 - LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUET
DRN & ARY KORN I STREET NH 03801 I264	REVISIONS SUBDIVISION PLAN TAX MAP 132 - LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST
F DRN & ARY KORN I STREET NH 03801 I 264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE
F DRN & ARY KORN I STREET NH 03801 I 264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE PROPERTY LOCATED AT: 799 SOUTH STREFT
PRN & ARY KORN I STREET NH 03801 I264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE PROPERTY LOCATED AT: 799 SOUTH STREET CITY OF PORTSMOUTH
P NRN & ARY KORN I STREET NH 03801 I264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE PROPERTY LOCATED AT: 799 SOUTH STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM
P NRV & ARY KORN I STREET NH 03801 1264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE PROPERTY LOCATED AT: 799 SOUTH STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM STATE OF NEW HAMPSHIRE
JRN & ARY KORN I STREET NH 03801 1264	REVISIONS SUBDIVISION PLAN TAX MAP 132 – LOT 24 OWNER: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE PROPERTY LOCATED AT: 799 SOUTH STREET CITY OF PORTSMOUTH COUNTY OF ROCKINGHAM STATE OF NEW HAMPSHIRE



LAWN AREA $\begin{array}{c} 132\\ 22 \end{array}$ N/F JEFFREY B. SHELDON REVOCABLE TRUST JEFFERY B. SHELDON, TRUSTEE GINA D. SHELDON REVOCABLE TRUST GINA D. SHELDON, TRUSTEE P.O. BOX 4218 PORTSMOUTH, NH 03802 5654/0879 LAWN AREA 36" MAPLE -(ADT - RIPRAF RETAINING WALL --ſŤP#1∕ - 36" MAPLE METAL FENCE -12" MAPLE 22" MAPLE $\begin{array}{c} 132\\ 23 \end{array}$ N/F ALI S. KODAL PAMELA HENRY 845 SOUTH ST PORTSMOUTH, NH 03801 2917/1992 24" MAPLE 42" MAPLE

– E.S. 5–41

(OLD)

--52



TEST PIT 1, ELEV.: 56.0 5/30/19 DOUG LAROSA NONE

Observed Water: NONE Restrictive layer: NONE LEDGE AT 42" DEPTH DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE 20% GRAVEL

TEST PIT 2, ELEV. 59.1

5/30/19 DOUG LAROSA 49" MOTTLES 5YR 7/8 Observed Water: NONE Restrictive layer: NONE LEDGE AT 64" DESCRIPTION DEPTH 10YR 3/3 FINE SANDY LOAM. MASSIVE, FRIABLE, 15% GRAVEL 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, 20% COBBLES

TEST PIT 3, ELEV.: 62.0

5/30/19 DOUG LAROSA NONE Observed Water: NONE Restrictive layer: NONE LEDGE AT 37" DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVÉ, FRIABLE, 20% GRAVEL 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, ROOTS 36"

TEST PIT 4, ELEV. 48.4

9/30/19 DOUG LAROSA NONE Observed Water: NONE Restrictive layer: NONE LEDGE AT 29" <u>DEPTH</u> DESCRIPTION 10YR 4/3 FINE SANDY LOAM, MASSIVÉ, FRIABLE 10YR 4/6 FINE SANDY LOAM, GRANULAR, FRIABLE, ROOTS 26"

TEST PIT 5, ELEV. 53.5

	9/30/19
d by:	DOUG LAROSA
Г:	NONE
ved Wa	ter: NONE
ctive la	yer: NONE
SAL:	LEDGE AT 25"
PTH	DESCRIPTION
46"	10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE
- 72"	10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, 20% GRAVEL ROOTS 65"
46" - 72"	10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, 20% GRAVE ROOTS 65"

TEST PIT 6, ELEV. 54.5

	9/30/19
d by:	DOUG LAROSA
T:	NONE
ved Wo	iter: NONE
ctive lo	ayer: NONE
SAL:	LEDGE AT 42"
PTH	DESCRIPTION
- 42"	10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE



AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) PARCEL IS SHOWN ON CITY OF PORTSMOUTH ASSESSOR'S TAX MAP 132 AS LOT 24.

2) OWNERS OF RECORD: NOELE M. CLEWS REVOCABLE TRUST CHRISTOPHER CLEWS, TRUSTEE

- 67 RIDGES COURT
- PORTSMOUTH, NH 03801 2338/525

3) PARCEL NOT IN A FLOOD HAZARD ZONE AS SHOWN ON FIRM PANEL 33015C0259E. MAY 17, 2005.

4) EXISTING LOT AREA: 76,889 S.F. 1.7651 ACRES

5) PARCEL IS LOCATED IN GENERAL RESIDENCE A (GRA) DISTRICT.

6)	DIMENSIONAL	REQUIREMENTS:
	and the second	

MIN. LOT AREA:		- 7.,
FRONTAGE:		10
SETBACKS:	FRONT	15
	SIDE	10
	REAR	20
MAXIMUM STRUCTURE	HEIGHT:	35
MAXIMUM BUILDING C	OVERAGE:	25
MINIMUM OPEN SPACI	Ξ.	30

7,500 S.F. 00 FEET 5 FEET 0 FEET 20 FEET 5 FEET 5% 0%

7) THE PURPOSE OF THIS PLAN IS TO SHOW THE EXISTING CONDITIONS ON TAX MAP 132 LOT 24 IN THE CITY OF PORTSMOUTH.

8) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

9) VERTICAL DATUM IS MEAN SEA LEVEL NAVD88. BASIS OF VERTICAL DATUM IS REDUNDANT RTN GPS OBSERVATIONS (±0.2').

10) SEWER LOCATED FROM FIELD INSPECTED BY TED BERRY COMPANY ON APRIL 22, 2019.

CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

2	ADDED GAS LINE, REVISED WATER LINE	6/17/19		
1	ISSUED FOR CITY	4/5/19		
0	ISSUED FOR COMMENT	2/22/19		
NO.	DESCRIPTION	DATE		
5. S S	REVISIONS			



1736

FB 288 PG 22



FB 288 PG 22

1736





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

4) REQUIRED PARKING FOR LOT 2:

UNII	1:	2,119	SF:	1.3	SPACE	S
UNIT	2:	505	SF:	1 5	SPACE	
UNIT	3:	1,147	SF:	1.3	SPACE	S
UNIT	4:	1,493	SF:	1.3	SPACE	<u>S</u>
TO	TAL	REQUIR	RED:	5	SPACES	5
PR	ROVIE	ED: 7	SPAC	ES		

5) THE PURPOSE OF THIS PLAN IS TO SHOW PROPOSED DRIVEWAY LOCATIONS AND DRIVEWAY SIGHT DISTANCES FOR THE CLEWS SUBDIVISION.

CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

5	REVISED DRIVEWAYS & SIGHT DIST. #'S	8/1/19
4	ADDED PROP. SIDEWALK/ SIGHT DIST. #'S	7/8/19
3	REVISED TREES TO BE REMOVED	7/2/19
2	ISSUED FOR APPROVAL	6/17/19
1	DRIVE LOCATIONS, SIGHT DISTANCE	5/9/19
NO.	DESCRIPTION	DATE
1. F.	REVISIONS	



DRIVEWAY PLAN

FB 288 PG 22

C3





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

1) THE PURPOSE OF THIS PLAN IS TO SHOW THE CONCEPTUAL LAYOUT ASSOCIATED WITH THE SUBDIVISION OF TAX MAP 132 LOT 24 IN THE CITY OF PORTSMOUTH.

2) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

3) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

4) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "NEW HAMPSHIRE STORMWATER MANUAL, VOLUME 3, EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION. (NHDES DECEMBER 2008).

5) VERTICAL DATUM IS MEAN SEA LEVEL NAVD88. BASIS OF VERTICAL DATUM IS REDUNDANT RTN GPS OBSERVATIONS (±0.2').

6) THE PURPOSE OF THIS PLAN IS TO SHOW THE POTENTIAL DEVELOPMENT OF LOTS 1 & 3. BUILDINGS ARE CONCEPTUAL AND SUBJECT TO CHANGE. PROPOSED DRIVEWAY LOCATIONS ARE FIXED AND ARE TO BE CONSTRUCTED PER THIS PLAN.



2	REVISED DRIVES & HOUSE LOT 1	8/1/19
1	ADDED PROPOSED SIDEWALK	7/8/19
0	ISSUED FOR COMMENT	6/17/19
NO.	DESCRIPTION	DATE
·	REVISIONS	



CONCEPT AND

GRADING PLAN

FB 288 PG 22

74





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

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4) THE PURPOSE OF THIS PLAN IS TO SHOW THE PROPOSED DRIVEWAY LOCATION AND DRIVEWAY SIGHT DISTANCE FOR PROPOSED LOT 1.

CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

2	REVISED LOT 2 DRIVEWAY WIDTH	8/1/19
1	REVISED LOT 1 DRIVEWAY LOCATION	7/26/19
0	ISSUED FOR COMMENT	7/8/19
NO.	DESCRIPTION	DATE
	REVISIONS	



SIGHT DISTANCE

PLAN AND PROFILE

FB 288 PG 22

C5



JN1700s/JN1730s/JN1736/2018 Subdivision/Plans & Specs/Site/1736 SUBDIVISION 2019.

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STRI	GET .		CONCRE SIDEWAL - 40T E.S. 5- NET 56/37 PSNH 5/38	TE K -38											
	ROAD	NEHURST	(OLD)												
					2.0										
			DRIVEWA STATION	Y CL 5+46.70											
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-00	99 29 25 4+	-50	-23.01 -23.01	00	5+ 5+	50	+9 +4.25	00	4 40.55	-50	28.78 74	-00	99 95 7+	50	
		S	FOPPIN	G SIG	HT DIS	STANCE	ALON	G ROA	D						

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4) THE PURPOSE OF THIS PLAN IS TO SHOW THE PROPOSED DRIVEWAY LOCATION AND STOPPING SIGHT DISTANCE FOR PROPOSED LOT 1.

- 1736 -

FB 288 PG 22

EROSION CONTROL NOTES

CONSTRUCTION SEQUENCE

DO NOT BEGIN CONSTRUCTION UNTIL ALL LOCAL, STATE, AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED.

INSTALL PERIMETER CONTROLS, i.e., SILT FENCING OR SILTSOXX AROUND THE LIMITS OF DISTURBANCE BEFORE ANY EARTH MOVING OPERATIONS. THE USE OF HAY BALES IS NOT ALLOWED.

CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE.

PERFORM CLEARING & GRUBBING

CUT AND GRUB ALL TREES, SHRUBS, SAPLINGS, BRUSH, VINES AND REMOVE OTHER DEBRIS AND RUBBISH AS REQUIRED.

BULLDOZE TOPSOIL INTO STOCKPILES, AND CIRCLE WITH SILT FENCING OR SILTSOXX. IF EROSION IS EXCESSIVE, THEN COVER WITH MULCH.

CONSTRUCT FILTRATION BASINS AND OUTLET, BUT DO NOT ALLOW INFLOW UNTIL ALL CONTRIBUTING THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT AREAS ARE STABILIZED AND EROSION-FREE. ROUGH GRADE SITE. REMOVE AND CRUSH LEDGE, THEN BACKFILL WITH ONSITE SOILS OR GRAVEL IN 12" LIFTS, TYP. ROUGH GRADE SITE, IN LANDSCAPED AREAS OUT OF THE WAY OF SUBSEQUENT CONSTRUCTION ACTIVITY, INSTALL TOPSOIL, MULCH, SEED AND FERTILIZER. STABILIZE STEEPER SLOPES PER DETAILS.

CONSTRUCT FOUNDATIONS.

CONSTRUCT WALLS.

LAYOUT AND INSTALL ALL BURIED UTILITIES AND SERVICES TO THE PROPOSED BUILDING FOUNDATIONS. CAP AND MARK TERMINATIONS OR LOG SWING TIES.

CONSTRUCT BUILDING FRAMES.

FINISH GRADE SITE, BACKFILL DRIVEWAY & PARKING SUBBASE GRAVEL IN TWO, COMPACTED LIFTS. PROVIDE TEMPORARY EROSION PROTECTION TO DITCHES AND SWALES IN THE FORM OF MULCHING, JUTE MESH OR DITCH DAMS.

BUILDING EXTERIOR WORK: LIGHT FIXTURES

INSTALL EXTERIOR LIGHT POLE BASES, AND MAKE FINAL CONNECTIONS TO CONDUIT.

ALL PERMANENT FILTRATION BASINS, DITCHES AND SWALES SHALL BE STABILIZED PRIOR TO DIRECTING RUNOFF TO THEM.

AFTER BUILDING IS COMPLETED FINISH ALL REMAINING LANDSCAPED WORK.

CONSTRUCT ASPHALT WEARING COURSE.

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE *STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER. EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY. AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT EROSION.

DUST CONTROL: IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND IULCHING, DU NUT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILT FENCES AND SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM. ALL DAMAGED SILT FENCES AND SILTSOXX SHALL BE REPAIRED. WINTER NOTES SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

AVOID THE USE OF FUTURE OPEN SPACES (LOAM AND SEED AREAS) WHEREVER POSSIBLE DURING CONSTRUCTION. CONSTRUCTION TRAFFIC SHALL USE THE ROADBEDS OF FUTURE ACCESS DRIVES AND PARKING AREAS.

ADDITIONAL TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN AMOUNTS NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS -- CONSTRUCT SILT ADVANCE OF THAW OR SPRING MELT EVENTS. FENCE OR SILTSOXX AROUND TOPSOIL STOCKPILE.

AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL. STUMPS SHALL BE DISPOSED OF TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW IN AN APPROVED FACILITY.

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH, WOODY DEBRIS. LEAVES, BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED, ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL.

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:

- BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED - A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED

- A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS BEEN INSTALLED - EROSION CONTROL BLANKETS HAVE BEEN INSTALLED

VEGETATIVE PRACTICE

FOR PERMANENT MEASURES AND PLANTINGS:

LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF 2 TONS PER ACRE.

FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 500 POUNDS PER ACRE OF 10-20-20 FERTILIZER.

SEED SHALL BE SOWN AT THE RATES SHOWN IN THE TABLE BELOW. IMMEDIATELY BEFORE SEEDING. THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AT A RATE OF 1.5 TO 2 TONS PER ACRE, AND SHALL BE HELD IN PLACE USING APPROPRIATE TECHNIQUES FROM THE EROSION AND SEDIMENT CONTROL HANDBOOK.

WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED, ANY AREAS WHICH ARE NOT SATISFACTORILY COVERED SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED.

A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE:

GENERAL COVER	PROPORTION	SEEDING RATE	
CREEPING RED FESCUE KENTUCKY BLUEGRASS	50% 50%	100 LBS/ACRE	
SLOPE SEED (USED ON A	LL SLOPES GR	REATER THAN OR E	EQUAL TO 3:1)
	400		

CREEPING RED FESCUE	42%	
TALL FESCUE	42%	48 LBS/ACRE
BIRDSFOOT TREFOIL	16%	

IN NO CASE SHALL THE WEED CONTENT EXCEED ONE PERCENT BY WEIGHT. ALL SEED SHALL COMPLY WITH APPLICABLE STATE AND FEDERAL SEED LAWS.

FOR TEMPORARY PROTECTION OF DISTURBED AREAS: MULCHING AND SEEDING SHALL BE APPLIED AT THE FOLLOWING RATES:

PERENNIAL RYE: 0.7 LBS/1,000 S.F. MULCH: 1.5 TONS/ACRE

MAINTENANCE AND PROTECTION

THE CONTRACTOR SHALL MAINTAIN ALL LOAM & SEED AREAS UNTIL FINAL ACCEPTANCE AT THE COMPLETION OF THE CONTRACT. MAINTENANCE SHALL INCLUDE WATERING, WEEDING, REMOVAL OF STONES AND OTHER FOREIGN OBJECTS OVER 1/2 INCHES IN DIAMETER WHICH MAY APPEAR AND THE FIRST TWO (2) CUTTINGS OF GRASS NO CLOSER THEN TEN (10) DAYS APART. THE FIRST CUTTING SHALL BE ACCOMPLISHED WHEN THE GRASS IS FROM 2 1/2 TO 3 INCHES HIGH. ALL BARE AND DEAD SPOTS WHICH BECOME APPARENT SHALL BE PROPERLY PREPARED, LIMED AND FERTILIZED, AND RESEEDED BY THE CONTRACTOR AT HIS EXPENSE AS MANY TIMES AS NECESSARY TO SECURE GOOD GROWTH. THE ENTIRE AREA SHALL BE MAINTAINED, WATERED AND CUT UNTIL ACCEPTANCE OF THE LAWN BY THE OWNER'S REPRESENTATIVE.

THE CONTRACTOR SHALL TAKE WHATEVER MEASURES ARE NECESSARY TO PROTECT THE GRASS WHILE IT IS DEVELOPING.

TO BE ACCEPTABLE, SEEDED AREAS SHALL CONSIST OF A UNIFORM STAND OF AT LEAST 90 PERCENT ESTABLISHED PERMANENT GRASS SPECIES, WITH UNIFORM COUNT OF AT LEAST 100 PLANTS PER SQUARE FOOT.

SEEDED AREAS WILL BE FERTILIZED AND RESEEDED AS NECESSARY TO INSURE VEGETATIVE ESTABLISHMENT.

THE SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATION IS ESTABLISHED.

THE SILT FENCE OR SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL.

SILT FENCING AND SILTSOXX SHALL BE REMOVED ONCE VEGETATION IS ESTABLISHED, AND DISTURBED AREAS RESULTING FROM SILT FENCE AND SILTSOXX REMOVAL SHALL BE PERMANENTLY SEEDED.

ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING. ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED CONDITIONS.

AFTER NOVEMBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.

- 6) ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE CONSTRUCTION ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE.
- 7) THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, WASHED, OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED PROMPTLY.
- 8) WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.

(AS NEEDED)

AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors 200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282

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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

1) INLET BASKETS SHALL BE INSTALLED IMMEDIATELY AFTER CATCH BASIN CONSTRUCTION IS COMPLETE AND SHALL REMAIN IN PLACE AND BE MAINTAINED UNTIL PAVEMENT BINDER COURSE IS COMPLETE

2) FILTER FABRIC SHALL BE PUSHED DOWN AND FORMED TO THE SHAPE OF THE BASKET. THE SHEET OF FABRIC SHALL BE LARGE ENOUGH TO BE SUPPORTED BY THE BASKET FRAME WHEN HOLDING SEDIMENT AND, SHALL EXTEND AT LEAST 6" PAST THE FRAME. THE INLET GRATE SHALL BE PLACED OVER THE BASKET/FRAME AND WILL SERVE AS THE FABRIC ANCHOR.

3) THE FILTER FABRIC SHALL BE A GEOTEXTILE FABRIC; POLYESTER, POLYPROPYLENE, STABILIZED NYLON, POLYETHYLENE, OR POLYVINYLIDENE CHLORIDE MEETING THE FOLLOWING SPECIFICATIONS

> -RAB STRENGTH: 45 LB. MIN. IN ANY PRINCIPAL DIRECTION (ASTM D1682) -MULLEN BURST STRENGTH: MIN. 60 psi (ASTM D774)

4) THE FABRIC SHALL HAVE AN OPENING NO GREATER THAN A NUMBER 20 U.S. STANDARD SIEVE AND A MINIMUM PERMEABILITY OF 120 gpm/s.f. (MULTIPLY THE PERMITTIVITY IN SEC .- 1 FROM ASTM 54491-85 CONSTANT HEAD TEST USING THE CONVERSION FACTOR OF 74.)

5) THE INLET BASKET SHALL BE INSPECTED WITHIN 24 HOURS AFTER EACH RAINFALL OR DAILY DURING EXTENDED PERIODS OF PRECIPITATION. REPAIRS SHALL BE MADE IMMEDIATELY, AS NECESSARY, TO PREVENT PARTICLES FROM REACHING THE DRAINAGE SYSTEM AND/OR CAUSING SURFACE FLOODING.

6) SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT, OR MORE OFTEN IF THE FABRIC BECOMES CLOGGED.

CATCH BASIN INLET BASKET

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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

1	F/C4, G/C4 & I/C4	7/8/19
0	ISSUED FOR COMMENT	6/17/19
NO.	DESCRIPTION	DATE
	REVISIONS	

'00s/JN1730s/JN1736/2018 Subdivision/Plans & Specs/Site/1736 DETAILS.dwg, DETAILS

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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H. ORIGINAL OF COMMENT 6/17/19 NO. DESCRIPTION DATE REVISIONS

SCALE: AS SHOWN

FB 288 PG 22

JUNE 2019

73

DETAILS

IN PAVEMENT CROSS COUNTRY 4" HOT BITUMINOUS PAVEMENT FINISH GRADE AT CENTERLINE OF TRENCH TO BE MOUNDED (NHDOT ITEM 403.11 - MACH. METHOD) 2" WEAR COURSE, TYPE "F" (3/8") 6" ABOVE ORIGINAL GRADE 2" BASE COURSE, TYPE "B" (3/4") (OR MATCH EXISTING) _MULCH & SEED w/ SUITABLE GRASSES - 4" LOAM SUB-BASE PER NHDOT STANDARDS-COVF BACKFILL w/ SPECIFIED MATERIAL COMPACTED IN LIFTS TO 95% -------UNDER PIPE) SAND LAYER (COVER OVER PIPE, 12" MIN.) 0.00 SEWER CRUSHED STONE BEDDING (1/2 O.D. + 6" MINIMUM 0.D.+24" UNDER PIPE) TRENCHING SHALL BE PERFORMED IN ACCORDANCE WITH STATE OF N.H. STANDARDS FOR SEWER CONSTRUCTION. WATER & SEWER IN COMMON TRENCH **C4** (WHERE APPROVED ONLY)

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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.

1	REVISED DETAIL R, TRANSFORMER NEEDED	6/18/19
0	ISSUED FOR COMMENT	6/17/19
NO.	DESCRIPTION	DATE
	REVISIONS	

SCALE: AS SHOWN

FB 288 PG 22

DETAILS

JUNE 2019

D4

1736

MINIMUM COVER BACKFILL WITH EXCAVATED MATERIAL. NO BOULDERS, LEDGE, OR TRASH MATERIAL UTILITY MARKING TAPE LAID 24" ABOVE PIPE FOR ENTIRE LENGTH OF RUN SAND LAYER (SPRING LINE TO 12" + 0.D. OVER PIPE, MIN.) CRUSHED STONE BEDDING (1/2 O.D. + 6" MINIMUM)

NTS

AMBIT ENGINEERING, INC. CIVIL ENGINEERS AND LAND SURVEYORS 200 Griffin Road, Unit 3, Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

9 July, 2019

Mr. Dexter Legg, Chair City of Portsmouth Planning Board 1 Junkins Avenue Portsmouth, NH 03801

RE: Request for Waiver of the Clews Subdivision located at 799 South Street, Tax Map 132 / Lot 24 to allow relocation of existing utility pole and overhead utility service to Lot 3.

Chairman Legg and Planning Board Members;

In conjunction with the application for Subdivision Approval for the above referenced property a waiver is hereby requested to the City of Portsmouth Subdivision Regulations Section VI,9. A. 1.2. The waiver is to allow an Eversource Utility pole to be relocated just inside the property line to service the existing residential facility on Lot 2 and Lot 1. A waiver is also being requested to service Lot 3 directly from a pole on the opposite side of South Street.

We have had discussions with the PSNH and DPW regarding connections to the existing utilities in the street. Placing of the pole and allowing overhead utilities will allow for a workmanlike utility design.

We hereby respectfully request that you vote in the affirmative to grant the requested waiver.

Sincerely,

John R. Chagnon, PE

CC: Christopher Kit Clews, Bernie Pelech, File

J:\JOBS1\JN1700s\JN1730s\JN1736\2018 Subdivision\Applications\City of Portsmouth Subdivision and Site\05 Submission PB 7-9-19\PB Waiver Request from V19A UG Utilities 07.9.2019.doc

DRAINAGE ANALYSIS ADDENDUM CLEWS SUBDIVISION

799 SOUTH STREET PORTSMOUTH, NH

Revised August 1, 2019

This drainage addendum addresses the minor changes to the drainage report.

The drainage area calculations have been evaluated to include the relocation of lot 1 driveway and the change to impervious area.

The proposed revisions are very minor and these changes do not affect the drainage analysis post construction peak rate of runoff. We have included a revised Sheet W2, Post Construction Drainage Plan to replace the W2 provided, as well as a revised Area calculation worksheet.

CLEWS SUBDIVI	ISION 1736				Done	Soils	Canton B			
799 South Stree	et									
Portsmouth, NI	Т				8/1/19	В	ksat=6			
Area Calculatio	n WorkSheet									
									Area check	
Subcatchment.	Pavement	Building	Drip Edge	Gravel	Grass	Woods/grass	Misc. Impervious	Total		
ES1	587	a	a	2,003	8,903	1,376	2	12,869	12,869	
ES2	4,056	3,686	×	2,732	6,335	19,166	1,117	37,092	37,092	
ES3	ŝ	Ŀ.	e	*	8	1,130		1,130	1,130	total lot
ES4		632	10	e,	ų	10,911	782	12,325	12,325	
ES5	388	220	30	2,309	2,949	7,823	110	13,799	13,799	77,215
TOTAL	5,031	4,538		7,044	18,187	40,406	2,009	77,215	77,215	
Subcatchment.	Pavement	Building	Drip Edge	Gravel	Grass	Woods/grass	Misc. Impervious	Total		
PS1	413	,	в	303	2,586	4,200	5	7,502	7,502	
PS2	685	3,686	686	3,428	9	30,068	1,117	39,670	39,670	
PS2a		2,250						2,250	2,250	
PS3	ĩ	R.		÷		1,130	1	1,130	1,130	
PS4	Û	632	308	509	471	11,018	1	12,938	12,938	
PS4a		2,250						2,250	2,250	
PS5	110	220	36	606	464	8,526	504	10,769	11,475	
TOTAL	1,208	9,038		5,149	3,521	54,942	1,621	76,509	77,215	77,215

DRAINAGE ANALYSIS CLEWS SUBDIVISION

799 SOUTH STREET PORTSMOUTH, NH

JUNE 17, 2019

Revised July 8, 2019

Ambit Engineering, Inc.

Civil Engineers and Land Surveyors 200 Griffin Road, Unit 3 Portsmouth, NH 03801 Phone: 603.430.9282; Fax: 436.2315 E-mail: <u>djl@ambitengineering.com</u>

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Drainage Analysis	3
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ATTACHMENTS

- 1. Plan of Existing Subcatchments W1
- 2. Plan of Proposed Subcatchments W2
- 3. NRCS Soil Survey
- 4. Precipitation Table
- 5. Stormwater Management, Maintenance and Inspection Plan D1

APPENDIX A

1. Results of Drainage Analysis Calculations from the HydroCAD Program Analysis

EXECUTIVE SUMMARY

This drainage analysis examines the existing and proposed condition stormwater drainage patterns for construction of two single residential structures on South Street in Portsmouth, as shown on the City of Portsmouth Assessor's Map 132, Lot 24. The plan is to subdivide 1 lot into Proposed Lot 1, 2 and 3. The existing lot is 76,889 square-feet (1.7651 ac) in area.

The development will add two single-family residences, with buried utilities and a partially paved and partially graveled driveway on proposed Lots 1, 2 and 3. The future development of Lot 1 and 3 has been added in the post construction drainage model in a conceptual design. We have used the maximum allowable impervious building area of 25% on Lots 1 and 3 to be conservative. Lots 1, 2 and 3 will be serviced by City water and sewer.

The development has the potential to increase stormwater runoff to adjacent properties, and therefore must be designed in a manner to prevent that occurrence. This will be done primarily by removing a large portion of gravel driveway and adding infiltration around each new structure.

The design of the stormwater management system not only detains runoff but treats it to the maximum extent possible via infiltration.

DRAINAGE ANALYSIS PROPOSED 3 LOT SUBDIVISION 799 SOUTH STREET PORTSMOUTH, NH

INTRODUCTION

This drainage report is designed to assist the owner, planning board, contractor, regulatory reviewer, and others in understanding the impact of the proposed development project on local surface water runoff and quality. The project site is shown on City of Portsmouth Assessor's Map 132 as Lot 24. The proposed development will construct two new single family residences, paved driveway aprons, gravel driveways, and other improvements.

This report includes information about the existing site and the proposed site necessary to analyze stormwater runoff and design mitigation. The report includes maps of existing and proposed subcatchments and calculations of runoff. The report will provide a brief narrative description of the storm water runoff and describe numerically and graphically the surface water runoff patterns for this site. Proposed stormwater management and treatment structures and methods will also be described. To fully understand the proposed site development the reader should review a complete site plan set as well as this report.

Runoff from the impervious roof areas will be diverted to a stone drip edges that will infiltrate, cool, and outlet stormwater runoff. The design infiltration rate used for design and modeling purposes is 6" per hour. In situ tests performed on similar sites suggest a much higher infiltration rate, however, the average hydraulic conductivity value of 12 was used and a safety factor of 2 was applied for design purposes.

METHODOLOGY

This report uses the US Soil Conservation Service Method for prediction of storm water runoff. The SCS method is published in The National Engineering Handbook, Section 4 "Hydrology", in Technical Release No. 20, (TR-20) "Computer Program for Project Formulation Hydrology", and Technical Release-55 (TR-55) "Urban Hydrology for Small Watersheds". This report uses the HydroCAD program, written by Applied

Microcomputer Systems, Chocorua, N.H., to apply these methods. Rainfall data is taken from the Extreme Precipitation Tables, Northeast Regional Climate Center, Cornell University x 1.15 safety factor for NH Seacoast Communities. Runoff curve numbers are taken from the Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing areas in NH.

SITE SPECIFIC INFORMATION

Located on South Street in Portsmouth, the existing 1.765-acre site (three proposed lots) had soils examined via test pits by Douglas J. LaRosa of Ambit Engineering, Inc. on May 30, 2019.

The soils were typical of the soils shown on the site SCS soil maps.

According to the SCS soils the site is exclusively 799 Canton complex, 3 to 15 percent slopes. The existing site is approximately 14.99% impervious. The "developed" sites is approximated to have impervious cover of 16.11% or an increase of 2.12% in area of impervious across the three lots.

The building sites will be located east and west of the existing building in the front yard. Existing driveways will be removed, and two new driveways will be constructed in the area of greatest sight distance. A common drive will be used to access Lot 3 and the existing building. This drive will be sloped away from the street. Runoff from this new driveway will be captured in an infiltration basin located in rear of the new home (see sheet W2).

DRAINAGE ANALYSIS

This drainage analysis consists of two sections, an analysis of the stormwater runoff from the site in the existing or pre-developed condition, and an analysis of the stormwater runoff from the same area along with the associated proposed development. Areas and drainage information were taken from an existing conditions plan, and site topographic map prepared by Ambit Engineering. Test pits to determine soils and depth to groundwater were carried out by Douglas LaRosa on May 5, 2019 and SCS Ksat values were used to determine infiltration potential for the Stone Drip Edges.

Existing or Pre-Developed Site Runoff

The existing conditions for this site can be defined by subcatchments (ES1-ES5). Subcatchments were delineated by topography and critical areas of concern. In the predeveloped or existing conditions.

The flow paths used in the stormwater model for this site are primarily shallow concentrated flow due to the small size of the lot and a lack of any well defined drainage channels. The flow paths chosen in both the pre and post developed analysis are meant to be the longest time of concentration flow paths (woods or porous surfaces have longer times of concentration as compared to pavement or lawns), not the longest length of flow path. See "Preconstruction Drainage Plan" – W1.

<u>Proposed or Post-Developed Site Runoff</u>

The lot will be developed with the additions of two single family residences with a driveway, walkways and associated development. This will increase impervious area that will generate more stormwater runoff. To offset this increase, the stormwater will be infiltrated, so that the post development peak runoff is similar to the pre-developed conditions.

The proposed conditions for this site are defined by seven subcatchments (Ps1, Ps2, Ps2a Ps3, Ps4, Ps4a, and Ps5).

The proposed plan was designed to mimic the existing drainage patterns to the greatest extent possible. See Sheet W2 for flow paths.

See the attached drainage calculations for postconstruction drainage analysis.

Peak Flow Rates

One of the main goals of any stormwater runoff analysis is to maintain peak runoff amounts at or below pre-developed levels. For this development, this is accomplished at all property boundaries using a Stone Drip Edges and infiltration trenches which detain and infiltrate and treat runoff. The following summary describes the peak flow and runoff from the existing to developed conditions:

	Q2 (CFS)	Q10	(CFS)	Q25 ((CFS)	Q50	(CFS)
Design								
Point	Pre	Post	Pre	Post	Pre	Post	Pre	Post
DP1	0.26	0.10	0.65	0.30	1.01	0.50	1.36	0.69
DP2	1.03	0.63	2.46	1.93	3.73	3.16	4.96	4.38
DP3	0.01	0.01	0.03	0.03	0.06	0.06	0.09	0.09
DP4	0.13	0.14	0.52	0.48	0.88	0.81	1.24	1.14
DP5	0.28	0.22	0.76	0.61	1.21	0.97	1.64	1.33

The 2, 10, 25 and 50 year Post development peak rate of run-off shows the rate maintaining or decreasing from Pre-development peak rate of run-off which complies with City of Portsmouth requirements.

Conclusion

The proposed Clews Subdivision can be developed with the proposed site improvements and stormwater features described herein and create no negative impacts on abutting properties. The undeveloped Lots 1 and 3 can be developed with no negative impacts on abutting properties. This is possible because runoff from the site is being detained and infiltrated. This is consistent with NHDES goals of infiltrating runoff from new developments. This meets the requirements of the City of Portsmouth in terms of stormwater management and treatment.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
799	Urban land-Canton complex, 3 to 15 percent slopes	10.5	100.0%
Totals for Area of Interest		10.5	100.0%

AMBIT ENGINEERING, INC. CIVIL ENGINEERS AND LAND SURVEYORS 200 Griffin Road, Unit 3, Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

TECHNICAL REPORT OF WETLAND DELINEATION, CLASSIFICATION & IDENTIFICATION

Ambit Engineering Project No.: 1736 Date(s) of Delineation: 8/7/18 Date of Report: 8/16/18

Field Delineator: Steven D. Riker, CWS 219

Compiled by: Steven D. Riker, CWS 219

Project Location/Tax Map & Lot: 799 South Street, Portsmouth, NH. Tax Map 132, Lot 24.

Prepared for: Kit Clews, 67 Ridges Court, Portsmouth, NH 03801.

Site Area Observed: Entire lot.

Site Conditions: Portion of lot is developed, remainder is forested.

Weather/Seasonal Conditions: 85 sunny. Summer conditions.

Site Disturbance: Only in areas of development.

Wetlands Present: Wetlands exist in low lying depressions or drainageways.

Wetland conditions/atypical situation/problem area: None.

Hydric Soil Criterion: A11. Field Indicators of Hydric Soils in the United States, Version 8.1.

Delineation Standards Utilized:

- US Army Corps of Engineers Wetlands Delineation Manual, Technical Report Y-87-1 (Jan 1987). AND Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region, Version 2.0, January 2012.
- Field Indicators of Hydric Soils in the United States, Version 8.1, USDA-NRCS, 2010 AND (for disturbed sites) Field Indicators for Identifying Hydric Soils in New England, Version 3. NEIWPCC Wetlands Work Group (April 2004).
- 3. National List of Plant Species That Occur in Wetlands: Northeast (Region 1). USFWS (May 1988).

Ambit Engineering, Inc. delineated jurisdictional wetland boundaries utilizing fluorescent pink flagging tape, labeled alpha-numerically for aid in survey location.

Notes: A1-A5 stop would be classified as a palustrine scrub shrub broad leaved deciduous wetland system that is seasonally flooded and or saturated (PSS1E). This wetland boundary is located on the abutting lot but would be subject to the City of Portsmouth 100. Wetland Buffer.

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes	Storm Event	Rainfa	ll(Inche	s) X 1.15	II	Adjusted (inches)
State	New Hampshire	2	3.14	×	1.15	11	3.61
Location		10	4.79	×	1.15	II	5.51
Lonoitude	71 050 deorees West	25	6.10	×	1.15	II	7.02
Latitude	42.993 degrees North	50	7.32	×	1.15	II	8.42
Elevation	0 feet						
Date/Time	Mon, 03 Jun 2019 11:04:40 -0400						

Extreme Precipitation Estimates

ini10min15min30min60min120min1hr2hr3hr6hr124hr48hr14ay2day4day7d. 2.6 0.40 0.50 0.66 0.82 1.04 1yr 0.71 0.99 1.21 1.56 2.02 2.63 2.82 1yr 2.33 2.71 3.12 3.86 4.5 3.7 0.50 0.60 0.81 1.02 1.30 $2yr$ 0.88 1.18 1.71 1.92 2.45 3.14 3.48 $2yr$ 2.33 3.86 4.52 5.82 3.7 0.58 0.13 0.98 1.25 1.61 $5yr$ 1.08 1.17 1.89 2.42 3.11 4.00 4.92 5.27 3.86 4.52 5.92 7.6 4.7 0.66 0.83 1.13 1.46 1.90 $1.09r$ 1.26 1.72 2.89 3.72 4.79 5.97 5.97 5.92 7.7 4.7 0.66 0.83 1.13 1.46 1.90 1.26 1.75 2.74 2.74 2.79 2.78 3.74 4.92 5.92 7.7 4.7 0.98 1.12 $1.90r$ $1.90r$ 1.26 1.25 2.92 3.76 4.72 6.10 6.78 5.92 7.7 4.7 0.98 1.12 1.27 2.11 2.27 2.24 2.92 2.72 8.74 6.10 7.78																						
00.400.500.660.821.04 1yr 0.710.991.211.562.022.632.82 1yr 2.332.713.123.864.500.500.620.811.021.30 2yr 0.881.181.511.922.453.143.48 2yr 2.783.353.864.500.580.730.981.251.61 5yr 1.081.471.892.423.114.004.48 5yr 2.783.353.864.5500.580.730.981.251.61 5yr 1.081.471.892.423.114.004.48 5yr 2.783.544.925.927.000.770.981.361.361.9010yr1.261.732.242.893.724.795.475.445.215.927.000.770.981.361.361.201.261.252.803.644.726.106.982.975.927.000.770.981.361.361.802.372.913.923.724.726.106.988.134.925.927.000.881.121.802.372.913.923.523.543.555.406.717.559.0810.700.981.121.812.483.321.0001.822.54 </th <th>imi</th> <th>E</th> <th>10min</th> <th>15min</th> <th>30min</th> <th>60min</th> <th>120min</th> <th></th> <th>1hr</th> <th>2hr</th> <th>3hr</th> <th>6hr</th> <th>12hr</th> <th>24hr</th> <th>48hr</th> <th></th> <th>1 day</th> <th>2day</th> <th>4day</th> <th>7day</th> <th>10day</th> <th></th>	imi	E	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1 day	2day	4day	7day	10day	
2 0.50 0.62 0.81 1.02 1.30 2yr 0.88 1.18 1.51 1.92 2.45 3.14 3.48 2yr 3.35 3.86 4.5 7 0.58 0.73 0.98 1.25 1.61 5yr 1.08 1.47 1.89 2.42 3.11 4.00 4.48 5yr 3.54 4.30 4.92 5.8 2 0.66 0.83 1.13 1.46 1.90 10yr 1.26 1.73 2.24 2.89 3.72 4.79 5.42 5.40 4.73 5.92 7.0 9 0.77 0.98 1.36 1.80 2.37 25yr 1.56 2.13 4.36 5.42 5.12 6.10 6.98 5.70 6.71 7.0	0.2	9	0.40	0.50	0.66	0.82	1.04	1yr	0.71	0.99	1.21	1.56	2.02	2.63	2.82	1yr	2.33	2.71	3.12	3.82	4.43	1yr
7 0.58 0.73 0.98 1.25 1.61 5yr 1.08 1.47 1.89 2.42 3.11 4.00 4.48 5yr 3.54 4.30 4.92 5.8 2 0.66 0.83 1.13 1.46 1.90 10yr 1.26 1.73 2.24 2.89 3.72 4.79 5.42 10yr 4.24 5.21 5.92 7.0 9 0.77 0.98 1.36 1.80 2.37 25yr 1.56 2.13 2.12 6.10 6.98 5.49 5.10 7.0 7.0 6 0.88 1.12 1.57 2.11 2.80 50yr 1.82 5.66 7.3 8.45 50yr 6.48 8.13 9.08 10.3 1 0.99 1.28 2.11 2.80 50yr 1.83 5.40 6.71 7.55 9.08 10.3 1 0.99 1.28 3.34 3.36 5.66 7	0.3	5	0.50	0.62	0.81	1.02	1.30	2yr	0.88	1.18	1.51	1.92	2.45	3.14	3.48	2yr	2.78	3.35	3.86	4.58	5.22	2yr
2 0.66 0.83 1.13 1.46 1.90 10yr 1.26 1.73 2.24 2.89 3.72 4.79 5.42 10yr 4.24 5.21 5.92 7.0 9 0.77 0.98 1.36 1.80 2.37 25yr 1.56 2.15 2.80 3.64 4.72 6.10 6.98 25yr 5.40 6.71 7.55 9.0 5 0.88 1.12 1.57 2.11 2.80 50yr 1.82 2.54 3.34 4.36 5.66 7.32 8.45 50yr 6.48 8.13 9.08 10. 10 0.99 1.28 3.32 100yr 2.14 3.00 3.97 5.21 6.73 8.79 6.04 8.13 9.08 10. 1 0.99 1.28 3.32 100yr 2.14 3.03 5.21 6.73 8.79 10.74 7.78 9.85 10.93 13.15 15.	0.3	5	0.58	0.73	0.98	1.25	1.61	5yr	1.08	1.47	1.89	2.42	3.11	4.00	4.48	5yr	3.54	4.30	4.92	5.85	6.61	5yr
9 0.77 0.98 1.36 1.80 2.37 25yr 1.56 2.15 2.80 3.64 4.72 6.10 6.98 25yr 5.40 6.71 7.55 9.0 55 0.88 1.12 1.57 2.11 2.80 50yr 1.82 2.54 3.34 4.36 5.66 7.32 8.45 50yr 6.48 8.13 9.08 10. 51 0.99 1.28 1.81 2.48 3.32 100yr 2.14 3.00 3.97 5.21 6.72 8.43 50yr 7.78 9.85 10.93 13. 70 1.14 1.47 2.11 2.91 3.93 2.97 5.21 6.72 8.12 10.56 12.41 200yr 7.78 9.85 10.93 13. 70 1.14 1.47 2.11 2.91 3.55 4.72 6.22 8.12 10.56 12.41 200yr 9.35 11.93 13.15 15.3 <	0.4	5	0.66	0.83	1.13	1.46	1.90	10yr	1.26	1.73	2.24	2.89	3.72	4.79	5.42	10yr	4.24	5.21	5.92	7.05	7.91	10yr
55 0.88 1.12 1.57 2.11 2.80 50yr 1.82 2.54 3.34 4.36 5.66 7.32 8.45 50yr 6.48 8.13 9.08 10. 51 0.99 1.28 1.81 2.48 3.32 100yr 2.14 3.00 3.97 5.21 6.78 8.79 100yr 7.78 9.85 10.93 13. 70 1.14 1.47 2.11 2.91 3.93 200yr 2.51 3.55 4.72 6.22 8.12 10.56 17.41 200yr 9.35 13.15 15. 82 1.35 1.35 4.72 6.22 8.12 10.56 12.41 200yr 9.35 13.15 15. 82 1.36 3.60 4.92 500yr 3.11 4.44 5.93 7.87 10.31 15.91 15.39 15.39 15.30 15.30 13.46 16.01 500yr 15.39 16.80 20.5 15.30 15.30 15.30 15.30 15.30 15.30 15.30 15.3	ò	49	0.77	0.98	1.36	1.80	2.37	25yr	1.56	2.15	2.80	3.64	4.72	6.10	6.98	25yr	5.40	6.71	7.55	9.02	10.04	25yr
61 0.99 1.28 1.81 2.48 3.32 100yr 2.14 3.00 3.97 5.21 6.78 8.79 10.24 100yr 7.78 9.85 10.93 13. 70 1.14 1.47 2.11 2.91 3.93 200yr 2.51 3.55 4.72 6.22 8.12 10.56 12.41 200yr 9.35 13.15 15. 82 1.36 4.92 500yr 2.51 3.55 4.72 6.22 8.12 10.56 12.41 200yr 9.35 13.15 15. 82 1.36 3.60 4.92 500yr 3.11 4.44 5.93 7.87 10.33 13.46 16.01 500yr 16.80 20. 20. 20. 20.	0	55	0.88	1.12	1.57	2.11	2.80	50yr	1.82	2.54	3.34	4.36	5.66	7.32	8.45	50yr	6.48	8.13	9.08	10.88	12.04	50yr
70 1.14 1.47 2.11 2.91 3.93 200yr 2.51 3.55 4.72 6.22 8.12 10.56 12.41 200yr 9.35 11.93 13.15 15. 82 1.35 1.76 2.56 3.60 4.92 500yr 3.11 4.44 5.93 7.87 10.33 13.46 16.01 500yr 15.39 16.80 20. 20.	õ	61	0.99	1.28	1.81	2.48	3.32	100yr	2.14	3.00	3.97	5.21	6.78	8.79	10.24	100yr	7.78	9.85	10.93	13.14	14.43	100yr
82 1.35 1.76 2.56 3.60 4.92 500yr 3.11 4.44 5.93 7.87 10.33 13.46 16.01 500yr 11.91 15.39 16.80 20.	0	70	1.14	1.47	2.11	2.91	3.93	200yr	2.51	3.55	4.72	6.22	8.12	10.56	12.41	200yr	9.35	11.93	13.15	15.88	17.32	200yr
	0	82	1.35	1.76	2.56	3.60	4.92	500yr	3.11	4.44	5.93	7.87	10.33	13.46	16.01	500yr	11.91	15.39	16.80	20.41	22.05	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.36	0.44	0.60	0.73	0.89	1yr	0.63 (0.87	0.98	1.27	1.54	2.19	2.54	1yr	1.94	2.44	2.86	3.52	3.96	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.18	2yr	0.86	1.16	1.36	1.80	2.31	3.05	3.35	2yr	2.70	3.22	3.73	4.39	5.02	2yr
5yr	0.36	0.55	0.68	0.93	1.19	1.41	5yr	1.02	1.38	1.61	2.11	2.71	3.63	4.01	5yr	3.21	3.86	4.42	5.48	6.00	5yr
10yr	0.39	09.0	0.75	1.05	1.35	1.62	10yr	1.17	1.58	1.81	2.39	3.05	4.12	4.58	10yr	3.65	4.40	5.03	6.42	6.79	10yr

APPENDIX A

HydroCAD Pre & Post Runoff Models

Area Listing (all nodes)

	Area	CN	Description
(a	acres)		(subcatchment-numbers)
	0.350	61	>75% Grass cover, Good, HSG B (ES1, ES2)
	0.162	96	Gravel surface, HSG B (ES1, ES2, ES5)
	0.115	98	Paved parking, HSG B (ES1, ES2, ES5)
	0.104	98	Roofs, HSG B (ES2, ES4, ES5)
	0.020	98	Unconnected pavement, HSG B (ES4, ES5)
	0.026	98	Unconnected roofs, HSG B (ES2)
	0.995	58	Woods/grass comb., Good, HSG B (ES1, ES2, ES3, ES4, ES5)
	1.773	68	TOTAL AREA

Page 2

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
1.773	HSG B	ES1, ES2, ES3, ES4, ES5
0.000	HSG C	
0.000	HSG D	
0.000	Other	
1.773		TOTAL AREA
1736-Pre Prepared by AMBIT ENGINEERING, INC HydroCAD® 10.00 s/n 00801 © 2013 HydroCAD Software Solutions LLC

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.350	0.000	0.000	0.000	0.350	>75% Grass cover, Good	ES1,
							ES2
0.000	0.162	0.000	0.000	0.000	0.162	Gravel surface	ES1,
							ES2,
							ES5
0.000	0.115	0.000	0.000	0.000	0.115	Paved parking	ES1,
							ES2,
							ES5
0.000	0.104	0.000	0.000	0.000	0.104	Roofs	ES2,
							ES4,
							ES5
0.000	0.020	0.000	0.000	0.000	0.020	Unconnected pavement	ES4,
							ES5
0.000	0.026	0.000	0.000	0.000	0.026	Unconnected roofs	ES2
0.000	0.995	0.000	0.000	0.000	0.995	Woods/grass comb., Good	ES1,
							ES2,
							ES3,
							ES4,
							ES5
0.000	1.773	0.000	0.000	0.000	1.773	TOTAL AREA	

Ground Covers (all nodes)

Type III 24-hr 2YRX Rainfall=3.61"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=0.97" Flow Length=168' Tc=10.6 min CN=68 Runoff=0.26 cfs 0.024 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=1.08" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=1.03 cfs 0.076 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=0.50" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.01 cfs 0.001 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=0.62" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.15 cfs 0.015 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=0.86" Flow Length=141' Tc=5.6 min CN=66 Runoff=0.28 cfs 0.023 af
Link DP1: DP1	Inflow=0.26 cfs 0.024 af Primary=0.26 cfs 0.024 af
Link DP2: DP2	Inflow=1.03 cfs 0.076 af Primary=1.03 cfs 0.076 af
Link DP3: DP3	Inflow=0.01 cfs 0.001 af Primary=0.01 cfs 0.001 af
Link DP4: DP4	Inflow=0.15 cfs 0.015 af Primary=0.15 cfs 0.015 af
Link DP5: DP5	Inflow=0.28 cfs 0.023 af Primary=0.28 cfs 0.023 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.139 af Average Runoff Depth = 0.94" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 10YRX Rainfall=5.51"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=2.25" Flow Length=168' Tc=10.6 min CN=68 Runoff=0.65 cfs 0.055 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=2.42" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=2.46 cfs 0.172 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=1.46" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.03 cfs 0.003 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=1.69" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.52 cfs 0.040 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=2.08" Flow Length=141' Tc=5.6 min CN=66 Runoff=0.76 cfs 0.055 af
Link DP1: DP1	Inflow=0.65 cfs 0.055 af Primary=0.65 cfs 0.055 af
Link DP2: DP2	Inflow=2.46 cfs 0.172 af Primary=2.46 cfs 0.172 af
Link DP3: DP3	Inflow=0.03 cfs 0.003 af Primary=0.03 cfs 0.003 af
Link DP4: DP4	Inflow=0.52 cfs 0.040 af Primary=0.52 cfs 0.040 af
Link DP5: DP5	Inflow=0.76 cfs 0.055 af Primary=0.76 cfs 0.055 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.325 af Average Runoff Depth = 2.20" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 25YRX Rainfall=7.02"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=3.43" Flow Length=168' Tc=10.6 min CN=68 Runoff=1.01 cfs 0.084 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=3.64" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=3.73 cfs 0.258 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=2.42" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.06 cfs 0.005 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=2.72" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.88 cfs 0.064 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=3.22" Flow Length=141' Tc=5.6 min CN=66 Runoff=1.21 cfs 0.085 af
Link DP1: DP1	Inflow=1.01 cfs 0.084 af Primary=1.01 cfs 0.084 af
Link DP2: DP2	Inflow=3.73 cfs 0.258 af Primary=3.73 cfs 0.258 af
Link DP3: DP3	Inflow=0.06 cfs 0.005 af Primary=0.06 cfs 0.005 af
Link DP4: DP4	Inflow=0.88 cfs 0.064 af Primary=0.88 cfs 0.064 af
Link DP5: DP5	Inflow=1.21 cfs 0.085 af Primary=1.21 cfs 0.085 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.497 af Average Runoff Depth = 3.36" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 50YRX Rainfall=8.42"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=4.59" Flow Length=168' Tc=10.6 min CN=68 Runoff=1.36 cfs 0.113 af
SubcatchmentES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=4.83" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=4.96 cfs 0.343 af
SubcatchmentES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=3.42" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.09 cfs 0.007 af
SubcatchmentES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=3.77" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=1.24 cfs 0.089 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=4.35" Flow Length=141' Tc=5.6 min CN=66 Runoff=1.64 cfs 0.115 af
Link DP1: DP1	Inflow=1.36 cfs 0.113 af Primary=1.36 cfs 0.113 af
Link DP2: DP2	Inflow=4.96 cfs 0.343 af Primary=4.96 cfs 0.343 af
Link DP3: DP3	Inflow=0.09 cfs 0.007 af Primary=0.09 cfs 0.007 af
Link DP4: DP4	Inflow=1.24 cfs 0.089 af Primary=1.24 cfs 0.089 af
Link DP5: DP5	Inflow=1.64 cfs 0.115 af Primary=1.64 cfs 0.115 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.667 af Average Runoff Depth = 4.51" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac HydroCAD® 10.00 s/n 00801 © 2013 HydroCAD Software Solutions LLC

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Summary for Subcatchment ES1: Southwest Corner

0.65 cfs @ 12.15 hrs, Volume= 0.055 af, Depth= 2.25" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

A	rea (sf)	CN I	Description							
	587	98 I	Paved park	ing, HSG B	}					
	2,003	96 (96 Gravel surface, HSG B							
	8,903	61 🔅	61 >75% Grass cover, Good, HSG B							
	1,376	58	68 Woods/grass comb., Good, HSG B							
	12,869	68	Neighted A	verage						
	12,282	ę	95.44% Pei	vious Area						
	587	4	4.56% Impe	ervious Are	a					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
10.4	100	0.0147	0.16		Sheet Flow, Sheet Flow					
					Grass: Short n= 0.150 P2= 3.61"					
0.2	68	0.1191	5.56		Shallow Concentrated Flow, Shallow concentrated					
					Unpaved Kv= 16.1 fps					
10.6	168	Total								

Summary for Subcatchment ES2: Northwest Side

2.46 cfs @ 12.08 hrs, Volume= 0.172 af, Depth= 2.42" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

A	rea (sf)	CN /	Adj Desc	cription						
	4,056	98	Pave	ed parking,	HSG B					
	3,686	98	Roof	s, HSG B						
	2,732	96	Grav	avel surface, HSG B						
	6,335	61	>75%	% Grass co	ver, Good, HSG B					
	19,166	58	Woo	ds/grass co	omb., Good, HSG B					
	1,117	98	Unco	nconnected roofs, HSG B						
	37,092	71	70 Weig	hted Avera	age, UI Adjusted					
	28,233		76.1	2% Perviou	is Area					
	8,859		23.8	8% Impervi	ous Area					
	1,117		12.6	1% Unconr	nected					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
3.3	100	0.0292	0.51		Sheet Flow, Sheet flow					
					Fallow n= 0.050 P2= 3.61"					
1.9	225	0.1504	1.94		Shallow Concentrated Flow, Shallow concentrated					
					Woodland Kv= 5.0 fps					
5.2	325	Total								

Summary for Subcatchment ES3: North East Side

Runoff = 0.03 cfs @ 12.16 hrs, Volume= 0.003 af, Depth= 1.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

A	rea (sf)	CN D	escription						
	1,130	58 V	Voods/gras	ss comb., G	Good, HSG B				
	1,130	1	100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
10.0	100	0.1159	0.17	, <i>, , , , , , , , , , , , , , , , , , </i>	Sheet Flow, Sheet flow				
0.5	41	0.0834	1.44		Woods: Light underbrush n= 0.400 P2= 3.61" Shallow Concentrated Flow, Shallow Concentrated Woodland Kv= 5.0 fps				
10.5	141	Total							

Summary for Subcatchment ES4: Southeast Side

Runoff = 0.52 cfs @ 12.10 hrs, Volume= 0.040 af, Depth= 1.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

_	А	rea (sf)	CN	Adj Des	cription				
		632	98	Roc	ofs, HSG B				
		10,911	58	Wo	ods/grass co	omb., Good, HSG B			
_		782	98	Unc	connected pa	avement, HSG B			
		12,325	63	61 We	ghted Avera	age, UI Adjusted			
10,911					88.53% Pervious Área				
		1,414		11.4	11.47% Impervious Area				
		782		55.3	30% Unconr	nected			
	_								
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0	50	0.1665	0.17		Sheet Flow, Sheet flow			
						Woods: Light underbrush n= 0.400 P2= 3.61"			
	1.0	76	0.0711	1.33		Shallow Concentrated Flow, Shallow Concentrated Flow			
_						Woodland Kv= 5.0 fps			
_	6.0	126	Total						

Summary for Subcatchment ES5: South Street Side

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 0.055 af, Depth= 2.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

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A	rea (sf)	CN [Description						
	388	98 F	Paved park	ing, HSG B	}				
	220	98 F	Roofs, HSC	B					
	2,309	96 (Gravel surfa	ace, HSG E	3				
	10,772	58 V	Voods/gras	ss comb., G	Good, HSG B				
	110	98 Unconnected pavement, HSG B							
	13,799	66 V	Veighted A	verage					
	13,081	ç	94.80% Per	vious Area					
	718	5	5.20% Impe	ervious Area	а				
	110	1	5.32% Un	connected					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.3	100	0.0803	0.32		Sheet Flow, Sheet flow				
					Grass: Short n= 0.150 P2= 3.61"				
0.3	41	0.1054	2.27		Shallow Concentrated Flow, Shallow Concentrated				
					Short Grass Pasture Kv= 7.0 fps				
5.6	141	Total							

Summary for Link DP1: DP1

Inflow /	Area	=	0.295 ac,	4.56% Impervious,	Inflow Depth = 2.2	25" for 10YRX event
Inflow	:	=	0.65 cfs @	12.15 hrs, Volume	= 0.055 af	
Primary	y :	=	0.65 cfs @	12.15 hrs, Volume	= 0.055 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP2: DP2

Inflow Ar	ea =	0.852 ac, 23	3.88% Imp	ervious,	Inflow	Depth =	2.4	2" for 1	0YRX eve	ent
Inflow	=	2.46 cfs @	12.08 hrs,	Volume	=	0.172	af			
Primary	=	2.46 cfs @	12.08 hrs,	Volume	=	0.172	af,	Atten= 0%	5, Lag= 0	.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP3: DP3

Inflow A	rea =	0.026 ac,	0.00% Impervious,	Inflow Depth = 1.4	46" for 10YRX event
Inflow	=	0.03 cfs @	12.16 hrs, Volume	= 0.003 af	
Primary	=	0.03 cfs @	12.16 hrs, Volume	= 0.003 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP4: DP4

Inflow A	Area =	0.283 ac,	11.47% Impervious	, Inflow Depth = 1	1.69" for 10YRX event	
Inflow	=	0.52 cfs @	12.10 hrs, Volum	e= 0.040 a	ſ	
Primar	y =	0.52 cfs @	12.10 hrs, Volum	e= 0.040 a	f, Atten= 0%, Lag= 0.0 r	min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP5: DP5

Inflow /	Area	=	0.317 ac,	5.20% Impe	ervious,	Inflow De	pth =	2.08	3" for 10`	rRX event	
Inflow		=	0.76 cfs @	12.09 hrs,	Volume	=	0.055 a	af			
Primary	у	=	0.76 cfs @	12.09 hrs,	Volume	=	0.055 a	af, A	Atten= 0%,	Lag= 0.0 mi	n

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs



Area Listing (all nodes)

A	rea	CN	Description
(ac	res)		(subcatchment-numbers)
0.	350	61	>75% Grass cover, Good, HSG B (ES1, ES2)
0.	162	96	Gravel surface, HSG B (ES1, ES2, ES5)
0.	115	98	Paved parking, HSG B (ES1, ES2, ES5)
0.	104	98	Roofs, HSG B (ES2, ES4, ES5)
0.	020	98	Unconnected pavement, HSG B (ES4, ES5)
0.	026	98	Unconnected roofs, HSG B (ES2)
0.	995	58	Woods/grass comb., Good, HSG B (ES1, ES2, ES3, ES4, ES5)
1.	773	68	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
1.773	HSG B	ES1, ES2, ES3, ES4, ES5
0.000	HSG C	
0.000	HSG D	
0.000	Other	
1.773		TOTAL AREA

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.350	0.000	0.000	0.000	0.350	>75% Grass cover, Good	ES1,
							ES2
0.000	0.162	0.000	0.000	0.000	0.162	Gravel surface	ES1,
							ES2,
							ES5
0.000	0.115	0.000	0.000	0.000	0.115	Paved parking	ES1,
							ES2,
							ES5
0.000	0.104	0.000	0.000	0.000	0.104	Roofs	ES2,
							ES4,
							ES5
0.000	0.020	0.000	0.000	0.000	0.020	Unconnected pavement	ES4,
							ES5
0.000	0.026	0.000	0.000	0.000	0.026	Unconnected roofs	ES2
0.000	0.995	0.000	0.000	0.000	0.995	Woods/grass comb., Good	ES1,
							ES2,
							ES3,
							ES4,
							ES5
0.000	1.773	0.000	0.000	0.000	1.773	TOTAL AREA	

Ground Covers (all nodes)

Type III 24-hr 2YRX Rainfall=3.61"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=0.97" Flow Length=168' Tc=10.6 min CN=68 Runoff=0.26 cfs 0.024 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=1.08" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=1.03 cfs 0.076 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=0.50" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.01 cfs 0.001 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=0.62" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.15 cfs 0.015 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=0.86" Flow Length=141' Tc=5.6 min CN=66 Runoff=0.28 cfs 0.023 af
Link DP1: DP1	Inflow=0.26 cfs 0.024 af Primary=0.26 cfs 0.024 af
Link DP2: DP2	Inflow=1.03 cfs 0.076 af Primary=1.03 cfs 0.076 af
Link DP3: DP3	Inflow=0.01 cfs 0.001 af Primary=0.01 cfs 0.001 af
Link DP4: DP4	Inflow=0.15 cfs 0.015 af Primary=0.15 cfs 0.015 af
Link DP5: DP5	Inflow=0.28 cfs 0.023 af Primary=0.28 cfs 0.023 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.139 af Average Runoff Depth = 0.94" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 10YRX Rainfall=5.51"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=2.25" Flow Length=168' Tc=10.6 min CN=68 Runoff=0.65 cfs 0.055 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=2.42" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=2.46 cfs 0.172 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=1.46" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.03 cfs 0.003 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=1.69" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.52 cfs 0.040 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=2.08" Flow Length=141' Tc=5.6 min CN=66 Runoff=0.76 cfs 0.055 af
Link DP1: DP1	Inflow=0.65 cfs 0.055 af Primary=0.65 cfs 0.055 af
Link DP2: DP2	Inflow=2.46 cfs 0.172 af Primary=2.46 cfs 0.172 af
Link DP3: DP3	Inflow=0.03 cfs 0.003 af Primary=0.03 cfs 0.003 af
Link DP4: DP4	Inflow=0.52 cfs 0.040 af Primary=0.52 cfs 0.040 af
Link DP5: DP5	Inflow=0.76 cfs 0.055 af Primary=0.76 cfs 0.055 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.325 af Average Runoff Depth = 2.20" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 25YRX Rainfall=7.02"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=3.43" Flow Length=168' Tc=10.6 min CN=68 Runoff=1.01 cfs 0.084 af
Subcatchment ES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=3.64" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=3.73 cfs 0.258 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=2.42" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.06 cfs 0.005 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=2.72" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=0.88 cfs 0.064 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=3.22" Flow Length=141' Tc=5.6 min CN=66 Runoff=1.21 cfs 0.085 af
Link DP1: DP1	Inflow=1.01 cfs 0.084 af Primary=1.01 cfs 0.084 af
Link DP2: DP2	Inflow=3.73 cfs 0.258 af Primary=3.73 cfs 0.258 af
Link DP3: DP3	Inflow=0.06 cfs 0.005 af Primary=0.06 cfs 0.005 af
Link DP4: DP4	Inflow=0.88 cfs 0.064 af Primary=0.88 cfs 0.064 af
Link DP5: DP5	Inflow=1.21 cfs 0.085 af Primary=1.21 cfs 0.085 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.497 af Average Runoff Depth = 3.36" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Type III 24-hr 50YRX Rainfall=8.42"

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Time span=5.00-30.00 hrs, dt=0.01 hrs, 2501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES1: Southwest Corner	Runoff Area=12,869 sf 4.56% Impervious Runoff Depth=4.59" Flow Length=168' Tc=10.6 min CN=68 Runoff=1.36 cfs 0.113 af
SubcatchmentES2: Northwest Side Flow Leng	Runoff Area=37,092 sf 23.88% Impervious Runoff Depth=4.83" gth=325' Tc=5.2 min UI Adjusted CN=70 Runoff=4.96 cfs 0.343 af
Subcatchment ES3: North East Side	Runoff Area=1,130 sf 0.00% Impervious Runoff Depth=3.42" Flow Length=141' Tc=10.5 min CN=58 Runoff=0.09 cfs 0.007 af
Subcatchment ES4: Southeast Side Flow Leng	Runoff Area=12,325 sf 11.47% Impervious Runoff Depth=3.77" gth=126' Tc=6.0 min UI Adjusted CN=61 Runoff=1.24 cfs 0.089 af
Subcatchment ES5: South Street Side	Runoff Area=13,799 sf 5.20% Impervious Runoff Depth=4.35" Flow Length=141' Tc=5.6 min CN=66 Runoff=1.64 cfs 0.115 af
Link DP1: DP1	Inflow=1.36 cfs 0.113 af Primary=1.36 cfs 0.113 af
Link DP2: DP2	Inflow=4.96 cfs 0.343 af Primary=4.96 cfs 0.343 af
Link DP3: DP3	Inflow=0.09 cfs 0.007 af Primary=0.09 cfs 0.007 af
Link DP4: DP4	Inflow=1.24 cfs 0.089 af Primary=1.24 cfs 0.089 af
Link DP5: DP5	Inflow=1.64 cfs 0.115 af Primary=1.64 cfs 0.115 af

Total Runoff Area = 1.773 ac Runoff Volume = 0.667 af Average Runoff Depth = 4.51" 85.01% Pervious = 1.507 ac 14.99% Impervious = 0.266 ac

Summary for Subcatchment PS1: Southwest Corner

Runoff = 0.30 cfs @ 12.16 hrs, Volume= 0.026 af, Depth= 1.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

	A	rea (sf)	CN I	Description					
		413	98	Paved park	ing, HSG B				
		303	96	Gravel surface, HSG B					
		2,586	61 :	•75% Grass cover, Good, HSG B					
		4,200	58	Woods/grass comb., Good, HSG B					
		7,502	63	63 Weighted Average					
		7,089	9	94.49% Pervious Area					
		413	ļ	5.51% Impe	ervious Area	а			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	10.4	100	0.0147	0.16		Sheet Flow, Sheet Flow			
						Grass: Short n= 0.150 P2= 3.61"			
	0.2	68	0.1191	5.56		Shallow Concentrated Flow, Shallow concentrated			
_						Unpaved Kv= 16.1 fps			
	10.6	168	Total						

Summary for Subcatchment PS2: Northwest Side

Runoff = 1.93 cfs @ 12.08 hrs, Volume= 0.140 af, Depth= 1.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

	A	rea (sf)	CN /	Adj Desc	cription	
		685	98	Pave	ed parking,	HSG B
		3,686	98	Roof	s, HSG B	
*		686	50	Drip	Edge, HSG	B
		3,428	61	>759	% Grass co	ver, Good, HSG B
		30,068	58	Woo	ds/grass co	omb., Good, HSG B
		1,117	98	Unco	onnected ro	ofs, HSG B
		39,670	64	63 Weig	ghted Avera	ige, UI Adjusted
		34,182		86.1	7% Perviou	is Area
		5,488		13.8	3% Impervi	ous Area
		1,117		20.3	5% Unconr	nected
	-		~		o "	
	IC	Length	Slope	Velocity	Capacity	Description
(min)	(teet)	(ft/ft)	(ft/sec)	(cts)	
	3.3	100	0.0292	0.51		Sheet Flow, Sheet flow
						Fallow n= 0.050 P2= 3.61"
	1.9	225	0.1504	1.94		Shallow Concentrated Flow, Shallow concentrated
						Woodland Kv= 5.0 fps
	5.2	325	Total			

Summary for Subcatchment PS2a: Northwest Side

0.29 cfs @ 12.07 hrs, Volume= Runoff 0.023 af, Depth= 5.27" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

_	Ai	rea (sf)	CN D	escription			
_		2,250	98 L	Inconnecte	d roofs, HS	SG B	
		2,250	1	00.00% Im	pervious A	rea	
		2,250	1	00.00% Ur	iconnected		
	_				• •		
	IC	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.3	100	0.0292	0.51		Sheet Flow, Sheet flow	
						Fallow n= 0.050 P2= 3.61"	
	1.9	225	0.1504	1.94		Shallow Concentrated Flow, Shallow concentrated	
						Woodland Kv= 5.0 fps	
	5.2	325	Total				

323 Total

Summary for Subcatchment PS3: North East Side

0.03 cfs @ 12.16 hrs, Volume= Runoff =

0.003 af, Depth= 1.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

Α	rea (sf)	CN E	Description									
	1,130 58 Woods/grass comb., Good, HSG B											
1,130 100.00% Pervious Area												
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description							
10.0	100	0.1159	0.17		Sheet Flow, Sheet flow Woods: Light underbrush n= 0.400 P2= 3.61"							
0.5	41	0.0834	1.44		Shallow Concentrated Flow, Shallow Concentrated Woodland Kv= 5.0 fps							
10.5	141	Total										

Summary for Subcatchment PS4: Southeast Side

0.48 cfs @ 12.15 hrs, Volume= 0.042 af, Depth= 1.69" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

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	Area (sf)	CN	Description	l	
	632	98	Roofs, HSC	ЭB	
*	308	50	Drip Edge,	HSG B	
	509	96	Gravel surf	ace, HSG E	3
	471	61	>75% Gras	s cover, Go	bod, HSG B
	11,018	58	Woods/gra	ss comb., G	Good, HSG B
	12,938	61	Weighted A	Verage	
	12,306		95.12% Pe	rvious Area	
	632		4.88% Imp	ervious Are	а
-	Tc Length	Slop	e Velocity	Capacity	Description
(mi	in) (feet)	(ft/1	t) (ft/sec)	(cfs)	
8	3.7 100	0.166	0.19		Sheet Flow, Sheet flow
					Woods: Light underbrush n= 0.400 P2= 3.61"
1	l.0 76	0.071	1 1.33		Shallow Concentrated Flow, Shallow Concentrated Flow
					Woodland Kv= 5.0 fps
9	9.7 176	Total			

Summary for Subcatchment PS4a: Southeast Side

Runoff = 0.29 cfs @ 12.07 hrs, Volume= 0.023 af, Depth= 5.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

Α	rea (sf)	CN	Description								
	2,250	98	Roofs, HSC	βB							
	2,250		100.00% In	npervious A	rea						
Tc (min)	Length (feet)	Slop (ft/f	e Velocity) (ft/sec)	Capacity (cfs)	Description						
5.0					Direct Entry, TR55 Min						
	Summary for Subcatchment PS5: South Street Side										

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 0.044 af, Depth= 2.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YRX Rainfall=5.51"

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А	rea (sf)	CN	Adj Desc	cription								
	535	98	Pave	ed parking,	HSG B							
	220	98	Roof	s, HSG B								
*	36	50	Dripe	e Edge, HS	IG B							
	1,041	96	Grav	el surface,	HSG B							
	464	61	>75%	5% Grass cover, Good, HSG B								
	8,526	58	Woo	ods/grass comb., Good, HSG B								
	653	98	Unco	onnected pa	avement, HSG B							
	11,475	66	65 Weig	hted Avera	age, UI Adjusted							
	10,067		87.7	87.73% Pervious Area								
	1,408		12.2	7% Impervi	ious Area							
	653		46.3	8% Unconr	nected							
Tc	Length	Slope	Velocity	Capacity	Description							
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
4.9	50	0.0248	0.17		Sheet Flow, Sheet flow							
					Grass: Short n= 0.150 P2= 3.61"							
0.7	99	0.1100	2.32		Shallow Concentrated Flow, Shallow Concentrated							
					Short Grass Pasture Kv= 7.0 fps							
5.6	149	Total										

Summary for Pond I1: DRIP EDGE

Inflow Area	=	0.052 ac,10	0.00% Impervio	us, Inflow Dep	pth = 5.27"	for 10YF	RX event
Inflow	=	0.29 cfs @	12.07 hrs, Volu	ime=	0.023 af		
Outflow	=	0.26 cfs @	12.11 hrs, Volu	ime=	0.023 af, Atte	en= 10%,	Lag= 2.2 min
Discarded	=	0.26 cfs @	12.11 hrs, Volu	ime=	0.023 af		

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 55.03' @ 12.11 hrs Surf.Area= 0.088 ac Storage= 0.001 af

Plug-Flow detention time= 2.7 min calculated for 0.023 af (100% of inflow) Center-of-Mass det. time= 2.7 min (748.4 - 745.7)

Volume	Invert	Avail.Storage	Storage Description
#1	55.00'	0.212 af	24.00'W x 160.00'L x 6.00'H Prismatoid 0.529 af Overall x 40.0% Voids
Device	Routing	Invert O	utlet Devices
#1	Discarded	55.00' 6. Co	000 in/hr Exfiltration over Surface area onductivity to Groundwater Elevation = 54.00'
Discord		lov-0 EE ofo @	12 11 hrs LIM-FE 02' (Free Discharge)

Discarded OutFlow Max=0.55 cfs @ 12.11 hrs HW=55.03' (Free Discharge) **1=Exfiltration** (Controls 0.55 cfs)

Summary for Pond I2: DRIP EDGE

Inflow Are Inflow Outflow Discardeo	nflow Area =0.052 ac,100.00% Impervious, Inflow Depth =5.27" for 10YRX eventnflow =0.29 cfs @12.07 hrs, Volume=0.023 afOutflow =0.06 cfs @11.73 hrs, Volume=0.023 af, Atten= 79%, Lag= 0.0 minOiscarded =0.06 cfs @11.73 hrs, Volume=0.023 af											
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 52.35' @ 12.47 hrs Surf.Area= 0.009 ac Storage= 0.005 af												
Plug-Flov Center-of	Plug-Flow detention time= 18.5 min calculated for 0.023 af (100% of inflow) Center-of-Mass det. time= 18.5 min(764.0 - 745.5)											
Volume	Inver	<u>t Avail.Sto</u>	rage	Storage De	scription							
#1	#1 51.00' 0.022 af 2.00'W x 200.00'L x 6.00'H Prismatoid 0.055 af Overall x 40.0% Voids											
Device	Routing	Invert	t Out	let Devices								
#1	Discarded	51.00	0.06	6 cfs Exfiltr	ation at a	ll eleva	tion	S				

Discarded OutFlow Max=0.06 cfs @ 11.73 hrs HW=51.06' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Summary for Link DP1: DP1

Inflow A	Area	=	0.172 ac,	5.51% Impe	ervious,	Inflow Dep	pth =	1.84	4" for 10	/RX event
Inflow	=	=	0.30 cfs @	12.16 hrs,	Volume	= (0.026 a	af		
Primary	y =	=	0.30 cfs @	12.16 hrs,	Volume	= (0.026 a	af, <i>i</i>	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP2: DP2

Inflow /	Area =	0.962 ac,	18.46% Imp	ervious,	Inflow Depth =	1.7	4" for 10'	/RX event	
Inflow	=	1.93 cfs @	12.08 hrs,	Volume	= 0.140	af			
Primar	y =	1.93 cfs @	12.08 hrs,	Volume	= 0.140	af,	Atten= 0%,	Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP3: DP3

Inflow A	Area	=	0.026 ac,	0.00% Impe	ervious,	Inflow Depth	= 1.4	46" for 10ነ	/RX event
Inflow		=	0.03 cfs @	12.16 hrs,	Volume	= 0.0	03 af		
Primary	y :	=	0.03 cfs @	12.16 hrs,	Volume	= 0.0	03 af,	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP4: DP4

Inflow /	Area	=	0.349 ac,	18.98% Impe	ervious,	Inflow Deptl	h = 1.4	44" for 10ነ	/RX event
Inflow		=	0.48 cfs @	12.15 hrs,	Volume	= 0.	042 af		
Primar	у	=	0.48 cfs @	12.15 hrs,	Volume	= 0.	042 af,	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Link DP5: DP5

Inflow Are	a =	0.263 ac, <i>1</i>	12.27% Impe	ervious,	Inflow Dept	h= 2	2.00" for	10YF	RX event
Inflow	=	0.61 cfs @	12.09 hrs,	Volume	= 0.	044 at	f		
Primary	=	0.61 cfs @	12.09 hrs,	Volume	= 0.	044 at	f, Atten=	0%, L	_ag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

EROSION CONTROL NOTES

CONSTRUCTION SEQUENCE

DO NOT BEGIN CONSTRUCTION UNTIL ALL LOCAL, STATE, AND FEDERAL PERMITS HAVE BEEN APPLIED FOR AND RECEIVED.

INSTALL PERIMETER CONTROLS, i.e., SILT FENCING OR SILTSOXX AROUND THE LIMITS OF DISTURBANCE BEFORE ANY EARTH MOVING OPERATIONS. THE USE OF HAY BALES IS NOT ALLOWED.

PERFORM CLEARING & GRUBBING

CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE.

CUT AND GRUB ALL TREES, SHRUBS, SAPLINGS, BRUSH, VINES AND REMOVE OTHER DEBRIS AND RUBBISH AS REQUIRED.

BULLDOZE TOPSOIL INTO STOCKPILES, AND CIRCLE WITH SILT FENCING OR SILTSOXX. IF EROSION IS EXCESSIVE, THEN COVER WITH MULCH.

CONSTRUCT FILTRATION BASINS AND OUTLET, BUT DO NOT ALLOW INFLOW UNTIL ALL CONTRIBUTING THE SURFACE SHALL BE WATERED AND KEPT MOIST WITH A FINE SPRAY AS REQUIRED, WITHOUT AREAS ARE STABILIZED AND EROSION-FREE. ROUGH GRADE SITE. REMOVE AND CRUSH LEDGE, THEN BACKFILL WITH ONSITE SOILS OR GRAVEL IN 12" LIFTS, TYP. ROUGH GRADE SITE. IN LANDSCAPED AREAS OUT OF THE WAY OF SUBSEQUENT CONSTRUCTION ACTIVITY, INSTALL TOPSOIL, MULCH, SEED AND FERTILIZER. STABILIZE STEEPER SLOPES PER DETAILS.

CONSTRUCT FOUNDATIONS.

CONSTRUCT WALLS.

LAYOUT AND INSTALL ALL BURIED UTILITIES AND SERVICES TO THE PROPOSED BUILDING FOUNDATIONS. CAP AND MARK TERMINATIONS OR LOG SWING TIES.

CONSTRUCT BUILDING FRAMES.

FINISH GRADE SITE, BACKFILL DRIVEWAY & PARKING SUBBASE GRAVEL IN TWO, COMPACTED LIFTS. PROVIDE TEMPORARY EROSION PROTECTION TO DITCHES AND SWALES IN THE FORM OF MULCHING, JUTE MESH OR DITCH DAMS.

BUILDING EXTERIOR WORK: LIGHT FIXTURES

INSTALL EXTERIOR LIGHT POLE BASES, AND MAKE FINAL CONNECTIONS TO CONDUIT.

ALL PERMANENT FILTRATION BASINS, DITCHES AND SWALES SHALL BE STABILIZED PRIOR TO DIRECTING RUNOFF TO THEM.

AFTER BUILDING IS COMPLETED FINISH ALL REMAINING LANDSCAPED WORK.

CONSTRUCT ASPHALT WEARING COURSE.

REMOVE TRAPPED SEDIMENTS FROM COLLECTION DEVICES AS APPROPRIATE, AND THEN REMOVE TEMPORARY EROSION CONTROL MEASURES UPON COMPLETION OF FINAL STABILIZATION OF THE SITE

GENERAL CONSTRUCTION NOTES

THE EROSION CONTROL PROCEDURES SHALL CONFORM TO SECTION 645 OF THE "STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION" OF THE NHDOT, AND "STORM WATER MANAGEMENT AND EROSION AND SEDIMENT CONTROL HANDBOOK FOR URBAN AND DEVELOPING AREAS IN NEW HAMPSHIRE". THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.

DURING CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. NO DISTURBED AREA SHALL BE LEFT UNSTABILIZED FOR MORE THAN 45 DAYS.

ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT EROSION.

DUST CONTROL: IF TEMPORARY STABILIZATION PRACTICES, SUCH AS TEMPORARY VEGETATION AND MULCHING, DO NOT ADEQUATELY REDUCE DUST GENERATION, APPLICATION OF WATER OR CALCIUM CHLORIDE SHALL BE APPLIED IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES.

SILT FENCES AND SILTSOXX SHALL BE PERIODICALLY INSPECTED DURING THE LIFE OF THE PROJECT AND AFTER EACH STORM. ALL DAMAGED SILT FENCES AND SILTSOXX SHALL BE REPAIRED. WINTER NOTES SEDIMENT DEPOSITS SHALL PERIODICALLY BE REMOVED AND DISPOSED IN A SECURED LOCATION.

AVOID THE USE OF FUTURE OPEN SPACES (LOAM AND SEED AREAS) WHEREVER POSSIBLE DURING CONSTRUCTION. CONSTRUCTION TRAFFIC SHALL USE THE ROADBEDS OF FUTURE ACCESS DRIVES AND PARKING AREAS.

ADDITIONAL TOPSOIL REQUIRED FOR THE ESTABLISHMENT OF VEGETATION SHALL BE STOCKPILED IN NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN AMOUNTS NECESSARY TO COMPLETE FINISHED GRADING OF ALL EXPOSED AREAS -- CONSTRUCT SILT ADVANCE OF THAW OR SPRING MELT EVENTS. FENCE OR SILTSOXX AROUND TOPSOIL STOCKPILE.

AREAS TO BE FILLED SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL TO REMOVE TREES, VEGETATION, ROOTS OR OTHER OBJECTIONABLE MATERIAL, STUMPS SHALL BE DISPOSED OF IN AN APPROVED FACILITY.

ALL FILLS SHALL BE PLACED AND COMPACTED TO REDUCE EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED PROBLEMS.

ALL NON-STRUCTURAL, SITE-FILL SHALL BE PLACED AND COMPACTED TO 90% MODIFIED PROCTOR DENSITY IN LAYERS NOT EXCEEDING 18 INCHES IN THICKNESS UNLESS OTHERWISE NOTED.

FROZEN MATERIAL OR SOFT, MUCKY OR HIGHLY COMPRESSIBLE MATERIAL, TRASH. WOODY DEBRIS. LEAVES, BRUSH OR ANY DELETERIOUS MATTER SHALL NOT BE INCORPORATED INTO FILLS.

FILL MATERIAL SHALL NOT BE PLACED ON FROZEN FOUNDATION SUBGRADE.

DURING CONSTRUCTION AND UNTIL ALL DEVELOPED AREAS ARE FULLY STABILIZED, ALL EROSION CONTROL MEASURES SHALL BE INSPECTED WEEKLY AND AFTER EACH ONE HALF INCH OF RAINFALL.

THE CONTRACTOR SHALL MODIFY OR ADD EROSION CONTROL MEASURES AS NECESSARY TO ACCOMMODATE PROJECT CONSTRUCTION.

ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE, ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED: - BASE COURSE GRAVELS HAVE BEEN INSTALLED ON AREAS TO BE PAVED

- A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED - A MINIMUM OF 3 INCHES OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIPRAP HAS
- BEEN INSTALLED - EROSION CONTROL BLANKETS HAVE BEEN INSTALLED

VEGETATIVE PRACTICE

FOR PERMANENT MEASURES AND PLANTINGS:

LIMESTONE SHALL BE THOROUGHLY INCORPORATED INTO THE LOAM LAYER AT A RATE OF 2 TONS PER ACRE.

FERTILIZER SHALL BE SPREAD ON THE TOP LAYER OF LOAM AND WORKED INTO THE SURFACE. FERTILIZER APPLICATION RATE SHALL BE 500 POUNDS PER ACRE OF 10-20-20 FERTILIZER.

SEED SHALL BE SOWN AT THE RATES SHOWN IN THE TABLE BELOW. IMMEDIATELY BEFORE SEEDING, THE SOIL SHALL BE LIGHTLY RAKED. ONE HALF THE SEED SHALL BE SOWN IN ONE DIRECTION AND THE OTHER HALF AT RIGHT ANGLES TO THE ORIGINAL DIRECTION. IT SHALL BE LIGHTLY RAKED INTO THE SOIL TO A DEPTH NOT OVER 1/4 INCH AND ROLLED WITH A HAND ROLLER WEIGHING NOT OVER 100 POUNDS PER LINEAR FOOT OF WIDTH. HAY MULCH SHALL BE APPLIED IMMEDIATELY AFTER SEEDING AT A RATE OF 1.5 TO 2 TONS PER ACRE, AND SHALL BE HELD IN PLACE USING APPROPRIATE TECHNIQUES FROM THE EROSION AND SEDIMENT CONTROL HANDBOOK.

WASHING AWAY THE SOIL, UNTIL THE GRASS IS WELL ESTABLISHED, ANY AREAS WHICH ARE NOT

A GRASS SEED MIXTURE CONTAINING THE FOLLOWING SEED REQUIREMENTS SHALL BE:

GENERAL COVER	PROPORTION	SEEDING RATE	

CREEPING RED FESCUE KENTUCKY BLUEGRASS	50% 50%	100	LBS/AC	RE		
SLOPE SEED (USED ON AL	l slopes	GREATER	THAN	OR	EQUAL	T
	4.0.07					

CREEPING RED FESCUE	42%		
TALL FESCUE	42%	48 LBS/ACRE	
BIRDSFOOT TREFOIL	16%		

IN NO CASE SHALL THE WEED CONTENT EXCEED ONE PERCENT BY WEIGHT. ALL SEED SHALL COMPLY WITH APPLICABLE STATE AND FEDERAL SEED LAWS.

FOR TEMPORARY PROTECTION OF DISTURBED AREAS: MULCHING AND SEEDING SHALL BE APPLIED AT THE FOLLOWING RATES:

PERENNIAL RYE: 0.7 LBS/1,000 S.F. MULCH: 1.5 TONS/ACRE

MAINTENANCE AND PROTECTION

THE CONTRACTOR SHALL MAINTAIN ALL LOAM & SEED AREAS UNTIL FINAL ACCEPTANCE AT THE COMPLETION OF THE CONTRACT. MAINTENANCE SHALL INCLUDE WATERING, WEEDING, REMOVAL OF STONES AND OTHER FOREIGN OBJECTS OVER 1/2 INCHES IN DIAMETER WHICH MAY APPEAR AND THE FIRST TWO (2) CUTTINGS OF GRASS NO CLOSER THEN TEN (10) DAYS APART. THE FIRST CUTTING SHALL BE ACCOMPLISHED WHEN THE GRASS IS FROM 2 1/2 TO 3 INCHES HIGH. ALL BARE AND DEAD SPOTS WHICH BECOME APPARENT SHALL BE PROPERLY PREPARED. LIMED AND FERTILIZED, AND RESEEDED BY THE CONTRACTOR AT HIS EXPENSE AS MANY TIMES AS NECESSARY TO SECURE GOOD GROWTH. THE ENTIRE AREA SHALL BE MAINTAINED, WATERED AND CUT UNTIL ACCEPTANCE OF THE LAWN BY THE OWNER'S REPRESENTATIVE.

THE CONTRACTOR SHALL TAKE WHATEVER MEASURES ARE NECESSARY TO PROTECT THE GRASS WHILE IT IS DEVELOPING.

TO BE ACCEPTABLE, SEEDED AREAS SHALL CONSIST OF A UNIFORM STAND OF AT LEAST 90 PERCENT ESTABLISHED PERMANENT GRASS SPECIES, WITH UNIFORM COUNT OF AT LEAST 100 PLANTS PER SQUARE FOOT.

SEEDED AREAS WILL BE FERTILIZED AND RESEEDED AS NECESSARY TO INSURE VEGETATIVE ESTABLISHMENT.

THE SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATION IS ESTABLISHED.

THE SILT FENCE OR SILTSOXX BARRIER SHALL BE CHECKED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL.

SILT FENCING AND SILTSOXX SHALL BE REMOVED ONCE VEGETATION IS ESTABLISHED, AND DISTURBED AREAS RESULTING FROM SILT FENCE AND SILTSOXX REMOVAL SHALL BE PERMANENTLY SEEDED

ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL

ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.

AFTER NOVEMBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3.

INSPECTION AND MAINTENANCE PLAN

INTRODUCTION

SATISFACTORILY COVERED SHALL BE RESEEDED, AND ALL NOXIOUS WEEDS REMOVED.

TO 3:1)

THE INTENT OF THIS IS TO PROVIDE THE 799 SOUTH STREET LOT OWNERS OF LOTS 1, 2 AND 3. WITH A LIST OF PROCEDURES THAT DOCUMENT THE INSPECTION AND MAINTENANCE REQUIREMENTS OF THE STORMWATER MANAGEMENT SYSTEM FOR THIS DEVELOPMENT. SPECIFICALLY, THE FILTRATION BASINS AND ASSOCIATED STRUCTURES ON THE PROJECT SITE (COLLECTIVELY REFERRED TO AS THE "STORMWATER MANAGEMENT SYSTEM")

THE FOLLOWING INSPECTION AND MAINTENANCE PROGRAM IS NECESSARY TO KEEP THE STORMWATER MANAGEMENT SYSTEM FUNCTIONING PROPERLY. THESE MEASURES WILL ALSO HELP MINIMIZE POTENTIAL ENVIRONMENTAL IMPACTS. BY FOLLOWING THE ENCLOSED PROCEDURES, THE OWNER WILL BE ABLE TO MAINTAIN THE FUNCTIONAL DESIGN OF THE STORMWATER MANAGEMENT SYSTEM AND MAXIMIZED ITS ABILITY TO REMOVE SEDIMENT AND OTHER CONTAMINANTS FROM THE SITE GENERATED STORMWATER RUNOFF.

STORMWATER MANAGEMENT SYSTEM COMPONENTS THE STORMWATER MANAGEMENT SYSTEM IS DESIGNED TO MITIGATE BOTH THE QUANTITY AND QUALITY OF SITE-GENERATED RUNOFF. AS THE RESULT, THE DESIGN INCLUDES THE FOLLOWING ELEMENTS:

NON-STRUCTURAL BMP'S

NON-STRUCTURAL BEST MANAGEMENT PRACTICES (BMP'S) INCLUDE TEMPORARY AND PERMANENT MEASURES THAT TYPICALLY REQUIRE LESS LABOR AND CAPITAL INPUTS AND ARE INTENDED TO PROVIDE PROTECTION AGAINST EROSION OF SOILS. EXAMPLES OF NON-STRUCTURAL BMP'S ON THIS PROJECT INCLUDE BUT ARE NOT LIMITED TO: TEMPORARY AND PERMANENT MULCHING, TEMPORARY AND PERMANENT GRASS COVER, TREES, SHRUBS AND GROUND OVERS, MISCELLANEOUS LANDSCAPE PLANTINGS, DUST CONTROL, TREE PROTECTION, TOPSOILING, SEDIMENT BARRIERS, AND DURING CONSTRUCTION, STABILIZED CONSTRUCTION ENTRANCES.

STRUCTURAL BMP'S

STRUCTURAL BMP'S REQUIRE MORE SPECIALIZED PERSONNEL TO INSTALL. EXAMPLES ON THE PROJECT INCLUDE BUT ARE NOT LIMITED TO: STORM DRAINS, THE FILTRATION BASIN, AND ASSOCIATED OUTLET CONTROL STRUCTURES, AND INFILTRATION TRENCH DETAIL.

INSPECTION AND MAINTENANCE REQUIREMENTS

THE FOLLOWING SUMMARIZES THE INSPECTION AND MAINTENANCE REQUIREMENTS FOR THE VARIOUS BMP'S THAT MAY BE FOUND ON THIS PROJECT:

1. GRASSED AREAS: AFTER EACH RAIN EVEN OF 0.5" OR MORE DURING A 24 HOUR PERIOD, INSPECT GRASSED AREAS FOR SIGNS OF DISTURBANCE, SUCH AS EROSION. IF DAMAGED AREAS ARE DISCOVERED, IMMEDIATELY REPAIR THE DAMAGE. REPAIRS MAY INCLUDE ADDING NEW TOPSOIL, LIME, SEED, FERTILIZER AND MULCH.

2. PLANTINGS: PLANTING AND LANDSCAPING (TREES, SHRUBS) SHALL BE MONITORED BI-MONTHLY DURING THE FIRST YEAR TO INSURE VIABILITY AND VIGOROUS GROWTH. REPLACE DEAD OR DYING VEGETATION WITH NEW STOCK AND MAKE ADJUSTMENTS TO THE CONDITIONS THAT CAUSED THE DEAD OR DYING VEGETATION. DURING DRYER TIMES OF THE YEAR, PROVIDED WEEKLY WATERING OR IRRIGATION DURING THE ESTABLISHMENT PERIOD OF THE FIRST YEAR. MAKE NECESSARY ADJUSTMENTS TO ENSURE LONG-TERM HEALTH OF VEGETATED COVER, I.E. PROVIDE MORE PERMANENT MULCH OR COMPOST OR OTHER MEANS OF PROTECTION.

3. STORM DRAIN OUTLETS AND OUTLET CONTROL STRUCTURES: MONITOR DRAIN INLETS AND OUTLET APRONS FOR EXCESSIVE ACCUMULATION OF SEDIMENTS OR MISSING STONE. REMOVE SEDIMENTS AS REQUIRED TO MAINTAIN FILTERING CAPABILITIES OF THE STONE.

4. FILTRATION BASIN: AFTER ACCEPTANCE OF THE FILTRATION BASIN, PERFORM THE FOLLOWING INSPECTIONS ON A SEMI-ANNUAL BASIS OR AFTER SIGNIFICANT RAINFALL EVENTS (10 YEAR, 24 HR STORMS, OR BACK TO BACK 2 YEAR, 24 HOUR STORMS):

a. MONITOR FOR EXCESSIVE OR CONCENTRATED ACCUMULATIONS OF DEBRIS. OR EXCESSIVE EROSION. REMOVE DEBRIS AS REQUIRED. b. MONITOR THE OUTFALL STRUCTURE FOR PROBLEMS WITH CLOGGED PIPES. REPAIR OR REMOVE CLOGS AS REQUIRED, AND DETERMINE CAUSE OF CLOGGING. PIPES SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR RAINSTORM. BROKEN OR DAMAGE PIPES

SHOULD BE REPAIRED OR REPLACED AS NECESSARY. c. MONITOR SIDE SLOPES OF POND FOR DAMAGES OR EROSION - REPAIR AS

NECESSARY. d. MONITOR TURF HEALTH AND KEEP PROTECTED FROM FIRE, GRAZING, TRAFFIC AND DENSE WEED GROWTH. LIME AND FERTILIZER SHOULD BE APPLIED AS NECESSARY TO PROMOTE GOOD GROWTH AS DETERMINED BY SOIL TESTS. MOWING THE VEGETATED AREAS OF THE BASIN SHOULD BE CARRIED OUT AS NECESSARY.

e. SEDIMENT ACCUMULATION SHOULD BE CONTINUALLY CHECKED IN THE BASIN. SEDIMENT SHOULD BE REMOVED AS IT IS DISCOVERED PARTICULARLY IF IT HAS ACCUMULATED

NEAR THE OUTLET OF THE BASIN. f. THE OUTLET CONTROL STRUCTURE SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR RAINSTORM.

THE OUTLET CONTROL STRUCTURE HAS WITHIN IT A BROAD CRESTED WIER STRUCTURE FOR CONTROLLING FLOW OUT OF BASIN. ANY SEDIMENT OR DEBRIS THAT HAS BUILT UP INSIDE THE OUTLET CONTROL STRUCTURE SHOULD BE REMOVED WHEN DISCOVERED.

5. INVASIVE SPECIES

MONITOR STORMWATER MANAGEMENT SYSTEM FOR SIGNS OF INVASIVE SPECIES GROWTH. IF CAUGHT EARLIER ENOUGH, THEIR ERADICATION IS MUCH EASIER. THE MOST LIKELY PLACES WHERE INVASIONS START ARE IN WETTER, DISTURBED SOILS OR DETENTION PONDS. SPECIES SUCH AS PHRAGMITES AND PURPLE LOOSE-STRIFE ARE COMMON INVADERS IN THESE WETTER AREAS. IF THEY ARE FOUND THEN THE OWNER SHALL CONTACT A WETLAND SCIENTIST WITH EXPERIENCE IN INVASIVE SPECIES CONTROL TO IMPLEMENT A PLAN OF ACTION TO ERADICATE THE INVADERS. MEASURES THAT DO NOT REQUIRE THE APPLICATION OF CHEMICAL HERBICIDES SHOULD BE THE FIRST LINE OF DEFENSE.



- ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED PROMPTLY.
- WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.

C4/ (as needed)

ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE. 7) THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, WASHED, OR TRACKED

WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY, WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED





AMBIT ENGINEERING, INC. Civil Engineers & Land Surveyors

200 Griffin Road - Unit 3 Portsmouth, N.H. 03801-7114 Tel (603) 430-9282 Fax (603) 436-2315

NOTES:

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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.







DP2

GRAVEL

58.74

-5-05-5-05-05-0

NET \$

LAWN AREA



TEST PIT 1, ELEV.: 56.0

5/30/19 DOUG LAROSA NONE Observed Water: NONE Restrictive layer: NONE LEDGE AT 42" DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE 20% GRAVEL

TEST PIT 2, ELEV. 59.1

5/30/19 DOUG LAROSA 49" MOTTLES 5YR 7/8 Observed Water: NONE Percolation Rate: 5 MIN/INCH;12 INCH/HR Restrictive layer: NONE LEDGE AT 64" DESCRIPTION DEPTH 10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE, 15% GRAVEL 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, 20% COBBLES

TEST PIT 3, ELEV.: 62.0

5/30/19 DOUG LAROSA NONE Observed Water: NONE Percolation Rate: 5 MIN/INCH;12 INCH/HR Restrictive layer: NONE LEDGE AT 37" DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVÉ, FRIABLE, 20% GRAVEL 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, ROOTS 36"

TEST PIT 4, ELEV. 48.4

		5/30/	/19	
d by:		DOUG	LAROSA	
•		NONE		
ved Wa	ter:	NONE		
ctive la	yer:	NONE		
AL:		LEDGE	E AT 29"	
<u>PTH</u>	DESCRI	PTION		
13"	10YR 4 MASSIVE	/3 FINE E, FRIABI	SANDY LOAM, LE	
- 29"	10YR 4 GRANUL	/6 FINE AR. FRIA	SANDY LOAM, ABLE, ROOTS 26"	

TEST PIT 5, ELEV. 53.5

	5/30/19
d by:	DOUG LAROSA
•	NONE
ved Water:	NONE
ation Rate:	5 MIN/INCH;12 INCH/H
ctive layer:	NONE
AL:	LEDGE AT 25"
TH DESCR	IPTION
46" 10YR MASSIV	3/3 FINE SANDY LOAM, /E, FRIABLE
10YR 72" GRANU ROOTS	5/6 FINE SANDY LOAM, LAR, FRIABLE, 20% GRAVEL, 65"

TEST PIT 6, ELEV. 54.5

	5/30/19
d by:	DOUG LAROSA
Г:	NONE
ved Water:	NONE
lation Rate:	5 MIN/INCH;12 INCH/HR
ctive layer:	NONE
SAL:	LEDGE AT 42"
PTH DESCR	RIPTION
- 42" 10YR MASSI	3/3 FINE SANDY LOAM, VE. FRIABLE



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CLEWS SUBDIVISION 799 SOUTH STREET PORTMSMOUTH, N.H.



FB 288 PG 22



--52--

50

LOT'S



TEST PIT 1, ELEV.: 56.0

5/30/19 DOUG LAROSA NONE Observed Water: NONE Restrictive layer: NONE LEDGE AT 42" DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE 20% GRAVEL

TEST PIT 2, ELEV. 59.1

5/30/19 DOUG LAROSA 49" MOTTLES 5YR 7/8 Observed Water: NONE Percolation Rate: 5 MIN/INCH;12 INCH/HR Restrictive layer: NONE LEDGE AT 64" DESCRIPTION 10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE, 15% GRAVEL 10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, 20% COBBLES

TEST PIT 3, ELEV.: 62.0

	5/30/19
l by:	DOUG LAROSA
•	NONE
ed Wat	er: NONE
ation R	ate: 5 MIN/INCH;12 INCH/HR
tive la	ver: NONE
AL:	LEDGE AT 37"
TH	DESCRIPTION
18"	10YR 3/3 FINE SANDY LOAM, MASSIVE, FRIABLE, 20% GRAVEL
37"	10YR 5/6 FINE SANDY LOAM, GRANULAR, FRIABLE, ROOTS 36"

TEST PIT 4, ELEV. 48.4

	5/30/19
by:	DOUG LAROSA
	NONE
ed Water:	NONE
tive layer:	NONE
AL:	LEDGE AT 29"
TH DESCR	IPTION
13" 10YR MASSIN	4/3 FINE SANDY LOAM, /E, FRIABLE
29" 10YR GRANL	4/6 FINE SANDY LOAM, JLAR, FRIABLE, ROOTS 26"

TEST PIT 5, ELEV. 53.5

	5/30/19
i by:	DOUG LAROSA
•	NONE
ed Water:	NONE
ation Rate:	5 MIN/INCH;12 INCH/HF
tive layer:	NONE
AL:	LEDGE AT 25"
TH DESCR	IPTION
46" 10YR 3 MASSIV	3/3 FINE SANDY LOAM, E, FRIABLE
10YR 5 72" GRANU ROOTS	5/6 FINE SANDY LOAM, LAR, FRIABLE, 20% GRAVEL, 65"

TEST PIT 6, ELEV. 54.5

	5/30/19
d by:	DOUG LAROSA
•	NONE
ved Water:	NONE
ation Rate:	5 MIN/INCH;12 INCH/HR
tive layer:	NONE
AL:	LEDGE AT 42"
TH DESCRI	PTION
42" 10YR 3 MASSIV	3/3 FINE SANDY LOAM, E, FRIABLE



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2	REVISED DRIVES AND HOUSE LOT 1	8/1/19
1	REVISED SIDEWALK	7/9/19
0	ISSUED FOR COMMENT	6/17/19
NO.	DESCRIPTION	DATE
ta ak	REVISIONS	



POST-CONSTRUCTION

DRAINAGE PLAN

W2

1736

FB 288 PG 22





City of Portsmouth, New Hampshire

Subdivision Application Checklist

This subdivision application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all subdivision review requirements. Please refer to the Subdivision review regulations for full details.

Applicant Responsibilities (Section III.C): Applicable fees are due upon application submittal along with required number of copies of the Preliminary or final plat and supporting documents and studies. Please consult with Planning staff for submittal requirements.

Owner: <u>Noele M. Clews Revocable Trust</u>	Date Submitted: 7-9-2019
Applicant: Christopher Clews, Trustee	
Phone Number: 603-867-7801	E-mail: kit@clews.org
Site Address 1: 799 South Street	Map: 132 Lot: 24
Site Address 2:	Map: Lot: 24

	Application Requirements				
Ø	Required Items for Submittal	Item Location (e.g. Page or Plan Sheet/Note #)	Waiver Requested		
	Completed Application form. (III.C.2-3)		N/A		
	All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF) on compact disc, DVD or flash drive. (III.C.4)	Submitted	N/A		

Requirements for Preliminary/Final Plat				
	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested
	Name and address of record owner, any option holders, descriptive name of subdivision, engineer and/or surveyor or name of person who prepared the plat. (Section IV.1/V.1)	Cover Sheet, C1	☑ Preliminary Plat ☑ Final Plat	N/A

Subdivision Application Checklist/January 2018

M	Required Items for Submittal				
		(e.g. Page/line or Plan Sheet/Note #)	Preliminary / Final Plat	Requested	
	Preliminary Plat Names and addresses of all adjoining property owners. (Section IV.2) Final Plat Names and addresses of all abutting property owners, locations of buildings within one hundred (100) feet of the parcel, and any new house numbers within the subdivision. (Section V.2)	Subdivision Plan	☑ Preliminary Plat ☑ Final Plat	N/A	
	North point, date, and bar scale. (Section IV.3/V3)	Required on all Plan Sheets	☑ Preliminary Plat ☑ Final Plat	N/A	
	Zoning classification and minimum yard dimensions required. (Section IV.4/V.4)	Subdivision Plan	☑ Preliminary Plat ☑ Final Plat	N/A	
	Preliminary Plat Scale (not to be smaller than one hundred (100) feet = 1 inch) and location map (at a scale of 1" = 1000'). (Section IV.5) Final Plat Scale (not to be smaller than 1"=100'), Location map (at a scale of 1"=1,000') showing the property being subdivided and its relation to the surrounding area within a radius of 2,000 feet. Said location map shall delineate all streets and other major physical features that my either affect or be affected by the proposed development. (Section V.5)	Subdivision Plan	☑ Preliminary Plat ☑ Final Plat	N/A	
	Location and approximate dimensions of all existing and proposed property lines including the entire area proposed to be subdivided, the areas of proposed lots, and any adjacent parcels in the same ownership. (Section IV.6)	Subdivision Plan	☑ Preliminary Plat ☑ Final Plat		
	Dimensions and areas of all lots and any and all property to be dedicated or reserved for schools, parks, playgrounds, or other public purpose. Dimensions shall include radii and length of all arcs and calculated bearing for all straight lines. (Section V.6/ IV.7)	Subdivision Plan	☑ Preliminary Plat ☑ Final Plat	N/A	
\checkmark	Location, names, and present widths of all adjacent streets, with a designation as to whether public or private and approximate location of existing utilities to be used. Curbs and sidewalks shall be shown. (Section IV.8/V.7)	Subdivision Plan Utility Plan- C2	☑ Preliminary Plat ☑ Final Plat		

N	Paguired Itoms for Submittal				
		(e.g. Page/line or Plan Sheet/Note #)	Preliminary / Final Plat	Walver Requested	
	Location of significant physical features, including bodies of water, watercourses, wetlands, railroads, important vegetation, stone walls and soils types that my influence the design of the subdivision. (Section IV.9/V.8)	Existing conditions- C1	☑ Preliminary Plat ☑ Final Plat		
✓	Preliminary Plat Proposed locations, widths and other dimensions of all new streets and utilities, including water mains, storm and sanitary sewer mains, catch basins and culverts, street lights, fire hydrants, sewerage pump stations, etc. (Section IV.10) Final Plat Proposed locations and profiles of all proposed streets and utilities, including water mains, storm and sanitary sewer mains, catchbasins and culverts, together with typical cross sections. Profiles shall be drawn to a horizontal scale of 1"=50' and a vertical scale of 1"=5', showing existing centerline grade, existing left and right sideline grades, and proposed centerline grade. (Section V.9)	Utility Plan- C2 No new mains- Utility connections only	☑ Preliminary Plat ☑ Final Plat		
	When required by the Board, the plat shall be accompanied by profiles of proposed street grades, including extensions for a reasonable distance beyond the subject land; also grades and sizes of proposed utilities. (Section IV.10)	N/A	☑ Preliminary Plat ☑ Final Plat		
	Base flood elevation (BFE) for subdivisions involving greater than five (5) acres or fifty (50) lots. (Section IV.11)	N/A	☑ Preliminary Plat ☑ Final Plat		
	For subdivisions of five (5) lots or more, or at the discretion of the Board otherwise, the preliminary plat shall show contours at intervals no greater than two (2) feet. Contours shall be shown in dotted lines for existing natural surface and in solid lines for proposed final grade, together with the final grade elevations shown in figures at all lot corners. If existing grades are not to be changed, then the contours in these areas shall be solid lines.	Existing Conditions- C1	☑ Preliminary Plat ☑ Final Plat		

	Requirements for Preliminary/Final Plat				
Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Required for Preliminary / Final Plat	Waiver Requested	
	Dates and permit numbers of all necessary permits from governmental agencies from which approval is required by Federal or State law. (Section V.10)	Cover Sheet, None Required	 □ Preliminary Plat ☑ Final Plat 		
	For subdivisions involving greater than five (5) acres or fifty (50) lots, the final plat shall show hazard zones and shall include elevation data for flood hazard zones. (Section V.11)	N/A: Not in SFHA	 □ Preliminary Plat ☑ Final Plat 		
\checkmark	Location of all permanent monuments. (Section V.12)	Subdivision Plan	 Preliminary Plat Final Plat 		

N I	Bequired Itoms for Submittal		
	Required items for Submittai	(e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	 Basic Requirements: (VI.1) a. Conformity to Official Plan or Map b. Hazards c. Relation to Topography d. Planned Unit Development 	Subdivision, Existing conditions- C1	
$\overline{\mathbf{v}}$	 2. Lots: (VI.2) a. Lot Arrangement b. Lot sizes c. Commercial and Industrial Lots 	Subdivision	
	 Streets: (VI.3) a. Relation to adjoining Street System b. Street Rights-of-Way c. Access d. Parallel Service Roads e. Street Intersection Angles f. Merging Streets g. Street Deflections and Vertical Alignment h. Marginal Access Streets i. Cul-de-Sacs j. Rounding Street Corners k. Street Name Signs l. Street Names m. Block Lengths n. Block Widths o. Grade of Streets 	Driveway Plan- C3	
	4. Curbing: (VI.4)	Sheet C3	
	5. Driveways: (VI.5)	Sheet C3	
	6. Drainage Improvements: (VI.6)	Sheet C4	
	7. Municipal Water Service: (VI.7)	Sheet C2	
<u> </u>	8. Municipal Sewer Service: (VI.8)	Sheet C2	
< √ √ √ √ √ √ √ √ √ √ √ √ √	 9. Installation of Utilities: (VI.9) a. All Districts b. Indicator Tape 	Sheet C2, D3, D4	
	10. On-Site Water Supply: (VI.10)	N/A	
	11. On-Site Sewage Disposal Systems: (VI.11)	N/A	
	 12. Open Space: (VI.12) a. Natural Features b. Buffer Strips c. Parks d. Tree Planting 	N/A	
	 13. Flood Hazard Areas: (VI.13) a. Permits b. Minimization of Flood Damage c. Elevation and Flood-Proofing Records d. Alteration of Watercourses 	N/A	
1	14 Frasion and Sedimentation Control (VI 14)	Sheet C4 & D1	

Ø	Required Items for Submittal	Item Location (e.g. Page/line or Plan Sheet/Note #)	Waiver Requested
	15. Easements (VI.15) a. Utilities b. Drainage	Sidewalk, Subdivison Plan	
	16. Monuments: (VI.16)	Subdivison	
	17. Benchmarks: (VI.17)	Sheet C1	
	18. House Numbers (VI.18)	TBD	
	Design Standards		
	Required Items for Submittal	Indicate compliance and/or provide explanation as to	Waiver Requested
	 Streets have been designed according to the design standards required under Section (VII.1). a. Clearing b. Excavation c. Rough Grade and Preparation of Sub-Grade d. Base Course e. Street Paving f. Side Slopes g. Approval Specifications h. Curbing i. Sidewalks j. Inspection and Methods Storm water Sewers and Other Drainage Appurtenances have been designed according to the design standards required under Section (VII.2). a. Design 	N/A Yes- Design is conceptual	
7	 b. Standards of Construction 3. Sanitary Sewers have been designed according to the design standards required under Section (VII.3). a. Design b. Lift Stations c. Materials d. Construction Standards 	Connections/ no mains	
	 4. Water Mains and Fire Hydrants have been designed according to the design standards required under Section (VII.4). a. Connections to Lots b. Design and Construction c. Materials d. Notification Prior to Construction 	No mains	

Applicant's/Representative's Signature:____

¹ See City of Portsmouth, NH Subdivision Rules and Regulations for details. Subdivision Application Checklist/January 2018



June 5, 2019

Douglas J. LaRosa Ambit Engineering 200 Griffin Road Unit 3 Portsmouth, NH 03801

RE: Natural Gas Availability to 799 South St Portsmouth NH

Dear Doug,

Unitil's natural gas division has reviewed the requested site for natural gas service.

Unitil hereby confirms natural gas service will be available to 799 South St Portsmouth, NH. Installation is pending an authorized installation agreement with Noel M. Clews Revocable Trust and a street opening approval from the City of Portsmouth DPW. This Will Serve letter is not an agreement to install natural gas.

Let me know if you have any questions. You can email me at oliver@unitil.com. My phone number is 603-294-5174.

Sincerely,

Janet Oliver Business Development Representative



June 19,2019

1700 Lafayette Road Portsmouth, NH 03801

Michael J Busby 603-436-7708 x555-5678 michael.busby@eversource.com

Douglas J. LaRosa Ambit Engineering 200 Griffin Road, Unit 3 Portsmouth, NH 03801

Dear Douglas:

am responding to your request to confirm the availability of electric service for the proposed 799 South Street project being constructed for/by The Clews Family Trust.

The proposed project consists of an existing 2-story building with 4 residential units and two new residential lots. The proposed development will be constructed along 799 South Street.

The developer will be responsible for the installation of all underground facilities and infrastructure required to service the new building. The service will be as shown on attached marked up Utility Plan C2. The proposed building service will be fed from Poles and Underground as depicted on utility plan C2. The developer will work with Eversource to obtain all necessary easements and licenses for the proposed overhead/underground facilities listed above.

This letter serves as confirmation that Eversource has sufficient capacity in the area to provide service to this proposed development. The cost of extending service to the aforementioned location and any associated infrastructure improvements necessary to provide service will be borne by the developer unless otherwise agreed upon.

The attached drawing titled "Clews Subdivision, 799 South Street, Portsmouth, NH Utility Site Plan" dated 06/18/2019, shows transformer location to service your proposed project.

Eversource approves the locations shown; assuming the final installed locations meet all clearances, physical protection, and access requirements as outlined in Eversource's "Information & Requirements For Electric Supply" (https://www.eversource.com/content/docs/default-source/pdfs/requirements-for-electric-service-connections.pdf?sfvrsn=2).

If you require additional information or I can be of further assistance please do not hesitate to contact me at our Portsmouth Office, 603-436-7708 Ext. 555-5678

Respectfully.

NH Eastern Regional Engineering and Design Manager, Eversource

cc: (via e-mail) Michael Lee, Eastern Region Operations Manager, Eversource Mary Jo Hanson, Field Supervisor, Electric Design, Eversource



Transportation: Engineering • Planning • Design

MEMORANDUM

Ref: 1911A

To: John Chagnon, P.E., LLS Ambit Engineering

From: Stephen G. Pernaw, P.E., PTOE

Subject: Clews Subdivision Portsmouth, New Hampshire

Date: June 5, 2019

As requested, our office conducted a travel speed survey on South Street, for the Clews Subdivision in Portsmouth, New Hampshire for the purpose of evaluating the minimum safe stopping sight distance requirements for vehicles approaching the two proposed residential driveways on South Street. The plan entitled "*Driveway Plan*," Sheet C3, dated March 2019 (revised 5/9/19) that was prepared by your office indicates that the available sight distances at the Lot 1 Driveway are 226-feet looking right, and 345-feet looking left. Similarly, the sight distances at the Lot 2&3 Driveway are 437 feet looking right and 324 feet looking left (see Attachment 1). The purpose of this memorandum is to summarize the results of our survey, analysis, and findings.

<u>Vehicle Speeds</u> - The speed survey was conducted on South Street adjacent to the subject site in March 2019. The speed limit is posted at 20 mph on this section of South Street. The results of the speed survey are summarized below and show that both the average and 85th percentile speeds exceed the posted speed limit:

Traval Speed Summary March 2010

	85th Percentile Speeds		Average	Average Speeds	
Day	Eastbound	Westbound	Eastbound	Westbound	
Monday*	30.0	33.5	26.6	28.8	
Tuesday	30.7	33.7	26.8	29.1	
Wednesday	30.6	33.8	26.7	29.3	
Thursday	30.9	33.7	26.9	29.3	
Friday*	30.6	34.3	26.5	29.8	
Average	30.6	33.8	26.7	29.3	
-	mph	mph	mph	mph	

The raw speed data and other statistical summaries are attached (see Attachments 2-11).


<u>Sight Distance Evaluation</u> - The minimum safe stopping sight distance calculations contained herein are comprised of the distance traveled during the "perception-reaction time" plus the distance traveled during "braking." These computations are based on the 85th percentile approach speeds and the average approach grades on South Street (where breaking occurs). The attached calculations (see Attachments 12 & 13) demonstrate that the available stopping sight distances looking left and looking right from both residential driveways exceeds the minimum stopping sight distance requirements for the 85th percentile speed.



Attachments



Study Date: Monday, 03/18/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

	5- 14	15- 19	20- 24	25- 29	30- 34	35- 39	40- 44	45- 49	50- 54	55- 59	60- 64	65- 69	70- 74	75- 79	80- 99	Total
00:00 - 00:59	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-
01:00 - 01:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
02:00 - 02:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
03:00 - 03:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
04:00 - 04:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
05:00 - 05:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
06:00 - 06:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
07:00 - 07:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
08:00 - 08:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 - 09:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 - 10:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 - 11:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 - 12:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00 - 13:59	0	0	0	0	0	0	0	0	0	· 0	0	0	0	0	0	0
14:00 - 14:59	1	4	29	33	5	0	0	1	0	0	0	0	0	0	0	73
15:00 - 15:59	1	6	105	183	32	2	0	1	. 0	0	0	0	0	0	0	330
16:00 - 16:59	1	9	82	181	38	1	1	0	0	0	0	0	0	0	0	313
17:00 - 17:59	0	6	63	126	34	2	0	0	0	0	0	0	0	0	0	231
18:00 - 18:59	0	5	42	114	32	2	0	1	0	1	0	0	0	0	0	197
19:00 - 19:59	0	3	33	45	20	1	0	0	1	0	0	0	0	0	0	103
20:00 - 20:59	0	3	17	24	8	3	0	1	0	0	0	0	0	0	0	56
21:00 - 21:59	0	1	9	20	8	1	0	0	0	0	0	0	0	0	0	39
22:00 - 22:59	0	2	9	8	8	1	0	0	0	0	0	0	0	0	0	28
23:00 - 23:59	0	1	4	9	2	0	0	0	0	0	0	0	0	0	0	16
Totals	3	40	393	743	187	13	1	4	. 1	1	0	0	0	0	0	1386
ercent of Total	0.2	2.9	28.4	53.6	13.5	0.9	0.1	0.3	0.1	0.1	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.2	2.9	28.4	53.6	13.5	0.9	0.1	0.3	0.1	0.1	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	on:	4.3 M	PH			Ten Mil	e Pace:	20 to	29 MPH			85th F	Percentil	∋:	30.0 MPH
Me	an Spee	ed:	26.6 M	PH	Р	ercent in	Ten Mil	e Pace:		82.0%			450 7	.		00 4 1451
Medi	ian Spee	ed:	26.7 M	IPH									15th H	Percentil	.	22.1 MPH
Mo	dal Spee	ed:	27.5 M	IPH									90th F	Percentil	e:	31.8 MPH
													95th I	Percentil	e:	33.7 MPH

Study Date: Tuesday, 03/19/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

ſ	5-	15-	20-	25- 20	30-	35-	40-	45-	50-	55-	60- 64	65- 60	70- 74	75- 79	80- 00	Total
00.00 - 00.50	- 14	-19	24	29	34	39		49		39	04	03		73	 	11
01:00 - 01:59	0		0	2			0	0		0	- 0	0	0	0	0	3
02:00 - 02:59	0		0	2	0	0	0	0	0	0	0	0	0	0	0	2
03:00 - 03:59	0	1	3	- 1	1	0		0	0	0	0	0	0	0	0	7
04:00 - 04:59	0	0	10	10	4	1	0	0	0	0	0	0	0	0	0	25
05:00 - 05:59	1	1	18	19	9	2	0	0	0	0	0	0	0	0	0	50
06:00 - 06:59	0	6	29	70	12	2	0	0	0	0	0	0	0	0	0	119
07:00 - 07:59	0	13	79	133	38	4	2	0	0	1	0	0	0	0	0	270
08:00 - 08:59	1	13	116	149	35	2	1	1	0	0	0	0	0	0	1	319
09:00 - 09:59	2	2	57	104	37	2	1	0	0	0	0	0	0	0	0	205
10:00 - 10:59	0	8	41	89	30	3	0	0	0	1	0	0	0	0	0	172
11:00 - 11:59	0	9	64	96	39	2	0	0	0	0	0	0	0	0	0	210
12:00 - 12:59	0	9	62	119	41	1	0	0	0	0	0	0	1	0	0	233
13:00 - 13:59	2	5	44	123	36	3	0	0	1	0	0	0	0	0	0	214
14:00 - 14:59	0	8	64	137	36	4	0	1	0	0	0	1	0	0	1	252
15:00 - 15:59	1	14	91	156	56	1	0	0	0	0	0	0	1	0	0	320
16:00 - 16:59	0	4	75	201	37	0	2	0	0	0	0	0	0	0	0	319
17:00 - 17:59	0	8	54	128	45	4	0	0	0	0	0	0	0	0	0	239
18:00 - 18:59	0	6	47	104	26	2	0	0	0	0	0	0	0	0	0	185
19:00 - 19:59	0	3	32	51	15	1	0	0	0	0	0	0	0	0	0	102
20:00 - 20:59	1	1	18	31	12	1	0	0	0	0	0	0	0	0	0	64
21:00 - 21:59	0	0	12	25	9	1	0	0	0	0	0	0	0	0	0	47
22:00 - 22:59	1	2	8	12	5	2	0	0	0	0	0	0	0	0	0	30
23:00 - 23:59	0	1	8	7	2	0	0	0	0	0	0	0	0	0	0	18
Totals	9	115	935	1774	527	39	7	2	1	2	0	1	2	0	2	3416
ercent of Total	0.3	3.4	27.4	51.9	15.4	1.1	0.2	0.1	0.0	0.1	0.0	0.0	0.1	0.0	0.1	100
ercent of AM	0.3	3.9	30.2	48.8	14.9	1.4	0.4	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.1	100
ercent of PM	0.2	3.0	25.5	54.1	15.8	1.0	0.1	0.0	0.0		0.0	0.0	0.1	0.0	0.0	
Standard	Deviatio	on:	4.8 M	PH	_			e Pace:	20 to	29 MPH			8510 1	Percentile	ə:	30.7 MPH
Me	an Spee	ed:	26.8 M	PH	Р	ercent in	I en Mil	e Pace:		79.3%			15th	Percentile	<u>.</u>	22.1 MPH
Medi	ian Spee	ed:	26.8 M	IPH									QOth I	Percentil		32 3 MPH
Mo	dal Spee	ed:	27.5 N	IPH									95th	Percentile	ə:	33.9 MPH

Study Date: Wednesday, 03/20/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

	5- 14	15- 19	20- 24	25- 29	30- 34	35- 39	40- 44	45- 49	50- 54	55- 59	60- 64	65- 69	70- 74	75- 79	80- 99	Total
00:00 - 00:59	0	0		4	1	2	0	0	0	0	0	0		0	0	10
01:00 - 01:59	o	0	1	1	1	0	0	0	0	0	0	0	0	0	0	3
02:00 - 02:59	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
03:00 - 03:59	0	2	3	1	0	1	0	0	0	0	0	0	0	0	0	7
04:00 - 04:59	0	1	6	7	3	2	0	0	0	0	0	0	0	0	0	19
05:00 - 05:59	0	4	13	22	6	1	0	0	0	0	0	0	0	0	0	46
06:00 - 06:59	0	7	28	60	25	1	0	0	0	0	0	0	0	0	0	121
07:00 - 07:59	1	6	67	146	47	3	0	0	0	0	0	0	0	1	0	271
08:00 - 08:59	0	11	105	163	39	2	2	0	0	0	0	0	0	0	0	322
09:00 - 09:59	1	10	65	103	30	4	0	1	0	0	0	0	0	0	0	214
10:00 - 10:59	0	8	56	101	32	2	0	0	0	0	0	0	0	0	0	199
11:00 - 11:59	0	4	36	137	51	3	0	0	1	0	0	0	0	0	0	232
12:00 - 12:59	0	10	75	134	41	1	0	0	0	0	1	1	0	0	0	263
13:00 - 13:59	1	5	67	111	32	2	0	1	0	0	0	0	0	0	0	219
14:00 - 14:59	1	6	78	125	32	2	0	0	0	0	.0	0	1	0	1	246
15:00 - 15:59	1	12	107	171	32	1	0	0	0	0	0	0	0	0	0	324
16:00 - 16:59	0	7	70	149	61	1	0	0	0	0	0	0	0	0	0	288
17:00 - 17:59	0	7	78	140	36	2	0	0	0	0	0	0	0	0	0	263
18:00 - 18:59	0	7	48	89	26	4	0	0	0	0	0	0	0	0	0	174
19:00 - 19:59	0	8	62	62	25	0	0	0	1	1	0	0	0	0	0	159
20:00 - 20:59	0	1	21	46	9	0	0	0	0	0	0	0	0	0	0	77
21:00 - 21:59	0	3	14	24	7	0	0	0	0	0	0	0	0	0	0	48
22:00 - 22:59	0	0	4	10	10	1	1	0	1	0	0	0	0	0	0	27
23:00 - 23:59	0	1	7	13	6	1	0	0	1	0	0	0	0	0	0	29
Totals	5	120	1014	1820	552	37	3	2	4	1	1	1	1	1	1	3563
ercent of Total	0.1	3.4	28.5	51.1	15.5	1.0	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.1	3.7	26.5	51.6	16.3	1.5	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.1	0.0	100
ercent of PM	0.1	3.2	29.8	50.7	15.0	0.7	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	in:	4.7 M	PH			Ten Mil	e Pace:	20 to :	29 MPH			85th F	Percentile	e:	30.6 MPH
Ме	an Spee	ed:	26.7 M	PH	P	ercent in	Ten Mil	e Pace:		79.5%			454 5			
Medi	an Spee	ed:	26.8 M	PH									15th H		e:	22.0 MPH
Mod	dal Spee	ed:	27.5 M	PH									90th F	Percentile	e :	32.2 MPH
													95th F	Percentile	e :	33.9 MPH

Study Date: Thursday, 03/21/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

ļ	5-	15-	20-	25-	30-	35-	40-	45-	50-	55-	60-	65-	70-	75-	80-	
	14	19	24	29	34	39	44	49	54	59	64	69	74	79	99	Total
00:00 - 00:59	0	0	4	2	2	2	1	0	0	0	0	0	0	0	0	11
01:00 - 01:59	0	0	2	2	1	1	0	0	0	0	0	0	0	0	0	6
02:00 - 02:59	0	0	0	3	1	0	0	0	0	0	0	0	0	0	0	4
03:00 - 03:59	0	1	1	3	2	1	0	0	0	0	0	0	0	0	0	8
04:00 - 04:59	0	0	3	4	3	2	0	0	0	0	0	0	0	0	0	12
05:00 - 05:59	0	1	17	34	9	0	0	0	0	0	0	0	0	0	0	61
06:00 - 06:59	0	1	30	54	23	3	0	0	0	0	0	0	0	0	0	111
07:00 - 07:59	0	4	65	153	41	3	0	0	0	0	0	0	0	0	0	266
08:00 - 08:59	0	9	104	190	47	1	0	0	0	0	0	0	0	0	0	351
09:00 - 09:59	0	10	59	117	36	1	0	0	1	0	0	0	0	0	0	224
10:00 - 10:59	1	9	51	121	48	2	1	0	0	0	0	0	0	0	0	233
11:00 - 11:59	1	5	32	115	35	3	0	0	0	0	0	0	0	0	0	191
12:00 - 12:59	0	5	52	125	35	2	0	0	0	0	0	0	0	0	0	219
13:00 - 13:59	1	6	53	126	30	4	0	0	1	0	0	0	0	0	1	222
14:00 - 14:59	2	8	83	121	40	2	0	1	0	0	0	0	0	0	0	257
15:00 - 15:59	0	9	91	160	43	3	0	0	0	0	0	0	0	0	0	306
16:00 - 16:59	1	6	70	144	53	2	0	0	1	0	0	0	0	0	0	277
17:00 - 17:59	1	6	58	132	45	4	0	0	1	0	0	0	Ó	0	0	247
18:00 - 18:59	0	3	53	124	35	3	1	0	0	0	0	0	0	0	0	219
19:00 - 19:59	0	1	66	58	18	2	0	0	0	0	0	0	0	0	0	145
20:00 - 20:59	0	0	29	35	16	2	0	0	0	0	0	0	0	0	0	82
21:00 - 21:59	0	3	17	24	12	1	0	0	0	0	0	0	0	0	0	57
22:00 - 22:59	0	2	7	16	8	0	0	0	0	0	0	0	0	0	0	33
23:00 - 23:59	1	3	6	19	2	1	0	0	0	0	0	0	0	0	0	32
Totals	8	92	953	1882	585	45	3	1	4	0	0	0	0	0	1	3574
ercent of Total	0.2	2.6	26.7	52.7	16.4	1.3	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.1	2.7	24.9	54.0	16.8	1.3	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.3	2.5	27.9	51.7	16.1	1.2	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	n:	4.4 M	PH			Ten Mil	e Pace:	20 to	29 MPH			85th F	Percentile	e:	30.9 MPH
Me	an Spee	ed:	26.9 M	PH	Р	ercent in	Ten Mil	e Pace:		79.3%						
Medi	an Spee	ed:	26.9 M	РН									15th i	Percentile	e:	22.3 MPH
Mo	dal Spee	ed:	27.5 M	РН									90th I	Percentile	e:	32.4 MPH
				-									95th I	Percentile	e:	33.9 MPH

Study Date: Friday, 03/22/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

	5- 14	15- 19	20- 24	25- 29	30- 34	35- 39	40- 44	45- 49	50- 54	55- 59	60- 64	65- 69	70- 74	75- 79	80- 99	Total
00:00 - 00:59	0	1	2	7	1	0	0	0	0	0	0	0	0	0	0	11
01:00 - 01:59	0	0	3	2	Ö	0	0	0	0	0	0	0	0	0	0	5
02:00 - 02:59	0	0	1	1	2	0	0	0	0	0	0	0	0	0	0	4
03:00 - 03:59	0	1	0	0	1	1	0	0	0	0	0	0	0	0	0	3
04:00 - 04:59	0	0	2	11	2	2	0	0	0	0	0	0	0	0	0	17
05:00 - 05:59	0	2	12	20	7	1	Ó	0	0	0	0	0	0	0	0	42
06:00 - 06:59	0	6	32	56	13	1	0	0	0	0	0	0	0	0	0	108
07:00 - 07:59	0	9	62	108	32	4	0	0	0	0	0	0	0	0	0	215
08:00 - 08:59	1	6	25	47	18	0	0	0	0	0	0	0	0	0	0	97
09:00 - 09:59	-	-	-	-	1	-	-	-	-	-	-	-	-	-	-	-
10:00 - 10:59	-	-	-	-	-	1	-	-	-	-	-	-	-	-	-	-
11:00 - 11:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 - 12:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13:00 - 13:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14:00 - 14:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15:00 - 15:59	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-
16:00 - 16:59	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-
17:00 - 17:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-
18:00 - 18:59	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-
19:00 - 19:59	-		-		-	-		-	-	-	-	-		-	-	-
20:00 - 20:59	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-
21:00 - 21:59	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-
22:00 - 22:59	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-
23:00 - 23:59	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-
Totals	1	25	139	252	76	9	0	0	0	0	0	0	0	0	0	502
ercent of Total	0.2	5.0	27.7	50.2	15.1	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.2	5.0	27.7	50.2	15.1	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	n:	4.4 M	PH			Ten Mil	e Pace:	20 to	29 MPH			85th F	Percentile	e:	30.6 MPH
Me	an Spee	ed:	26.5 M	PH	Р	ercent in	Ten Mil	e Pace:		77.9%			450.0			
Medi	ian Spee	ed:	26.7 M	PH									15th H	-ercentile) .	21.8 MPH
Mo	dal Spee	ed:	27.5 M	PH									90th I	Percentile	e :	32.3 MPH
													95th I	Percentile	e:	33.9 MPH

Study Date: Monday, 03/18/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

	5- 14	15- 19	20- 24	25- 29	30- 34	35- 39	40- 44	45- 49	50- 54	55- 59	60- 64	65- 69	70- 74	75- 79	80- 99	Total
00:00 - 00:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
01:00 - 01:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
02:00 - 02:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
03:00 - 03:59	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-
04:00 - 04:59	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-
05:00 - 05:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
06:00 - 06:59	-	-	-	-	-	-	-	-	_	-	-	-	-	-	-	-
07:00 - 07:59	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-
08:00 - 08:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 - 09:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 - 10:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 - 11:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 - 12:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00 - 13:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:00 - 14:59	0	6	8	25	31	3	0	0	· 0	0	0	0	0	0	0	73
15:00 - 15:59	0	4	39	133	94	13	1	1	0	0	0	0	0	0	0	285
16:00 - 16:59	· 0	5	33	127	109	13	1	0	0	0	0	0	0	0	0	288
17:00 - 17:59	1	2	42	184	94	11	0	0	0	0	0	0	0	0	0	334
18:00 - 18:59	0	5	29	116	92	11	2	0	0	0	0	0	0	0	0	255
19:00 - 19:59	0	4	16	65	54	11	0	0	0	0	0	0	0	0	0	150
20:00 - 20:59	0	2	18	49	29	2	0	0	0	0	0	0	0	0	0	100
21:00 - 21:59	0	0	10	39	16	1	0	0	0	0	0	0	0	0	0	66
22:00 - 22:59	0	0	9	13	12	2	0	0	0	0	0	0	0	0	0	36
23:00 - 23:59	0	0	3	• 7	6	2	1	0	0	0	0	0	0	0	0	19
Totals	1	28	207	758	537	69	5	1	0	0	0	0	0	0	0	1606
ercent of Total	0.1	1.7	12.9	47.2	33.4	4.3	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.1	1.7	12.9	47.2	33.4	4.3	0.3	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	on:	4.4 M	PH			Ten Mil	e Pace:	25 to	34 MPH			85th I	Percentil	e:	33.5 MPH
Me	an Spee	ed:	28.8 M	PH	Р	ercent in	Ten Mil	e Pace:		80.6%			1 = 11- 1	Dereentil	. .	
Medi	ian Spee	ed:	28.7 M	IPH									1500	ercentil	е.	23.0 MPH
Mo	dal Spee	ed:	27.5 M	IPH									90th I	Percentil	e:	34.2 MPH
													95th I	Percentil	e:	34.9 MPH

Study Date: Tuesday, 03/19/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

[5-	15-	20-	25-	30-	35-	40-	45-	50-	55-	60-	65-	70-	75-	80-	Total
00.00 00.50	14	19	24		34	39	44	49	- 54	29	04	69	14	19	99	10(8)
00:00 - 00:59			2	5		1	0	0	0	0		0	- 0	0	0	- 10
01:00 - 01:59		0	1	2		1	0	0	0	0	- 0		0	0	0	- 10
02:00 - 02:59	0	0		<u> </u>		2	0	0	0	0		0	0	0	0	
04:00 - 04:59	0	0		5	2	1	0	0	0	0		0	0	0	0	10
05:00 - 05:59	0	1	2	5		2	0	0	0	0	0	0	0	0	0	12
06:00 - 06:59	0			36	33	5	0	0	0	0	0	0	0	0	0	87
07:00 - 07:59	0	2	26	89	78	17	3	0	0	0	0	0	0	Ō	0	215
08:00 - 08:59	5	15	41	132	55	9	0	0	1	0	0	1	0	0	0	259
09:00 - 09:59	0	1	14	79	80	8	2	0	0	0	0	0	0	0	0	184
10:00 - 10:59	0	2	14	87	75	17	0	0	0	0	0	0	0	0	0	195
11:00 - 11:59	1	1	25	96	67	7	0	1	0	0	0	0	0	0	0	198
12:00 - 12:59	0	1	14	106	68	12	1	0	0	0	0	0	0	0	0	202
13:00 - 13:59	0	5	20	92	89	10	0	0	0	0	0	0	0	0	1	217
14:00 - 14:59	0	4	16	134	104	11	.0	0	0	0	0	0	0	0	0	269
15:00 - 15:59	3	7	27	126	79	21	1	0	0	0	0	0	0	0	1	265
16:00 - 16:59	0	5	35	137	101	9	2	0	1	1	0	0	0	0	0	291
17:00 - 17:59	0	2	27	158	102	12	1	0	0	0	0	0	0	0	0	302
18:00 - 18:59	1	2	15	84	82	17	1	0	0	0	0	0	0	0	0	202
19:00 - 19:59	0	1	21	84	54	6	0	0	0	0	0	0	0	0	0	166
20:00 - 20:59	2	1	22	62	31	2	0	0	0	0	0	0	0	0	0	120
21:00 - 21:59	1	0	9	45	13	4	1	0	0	0	0	0	0	0	0	73
22:00 - 22:59	0	0	4	14	13	2	0	0	0	0	0	0	0	0	0	33
23:00 - 23:59	0	0	4	5	6	3	1	0	0	0	0	0	0	0	0	19
Totals	13	51	354	1589	1145	178	13	1	2	1	0	1	0	0	2	3350
ercent of Total	0.4	1.5	10.6	47.4	34.2	5.3	0.4	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.1	100
ercent of AM	0.5	1.9	11.8	45.5	33.8	5.8	0.4	0.1	0.1	0.0	0.0	0.1	0.0	0.0	0.0	100
ercent of PM	0.3	1.3	9.9	48.5	34.4	5.0	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	100
Standard	Deviatio	m:	4.8 M	PH			Ten Mil	e Pace:	25 to	34 MPH			85th I	Percentil	e:	33.7 MP
Me	an Spee	ed:	29.1 M	PH	P	ercent in	Ten Mil	e Pace:		81.6%			450	.		05 0 1 15
Medi	ian Spee	ed:	29.0 M	PH									15th I	Percentil	e:	25.3 MP
Mo	dal Spee	ed:	27.5 M	PH									90th	Percentil	e:	34.4 MP
													95th I	Percentil	e:	35.8 MP

Study Date: Wednesday, 03/20/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

ſ	5-	15-	20-	25-	30-	35-	40-	45-	50-	55-	60-	65-	70-	75-	80-	
	14	19	24	29	34	39	44	49	54	59	64	69	74	79	99	Total
00:00 - 00:59	0	0	0	7	1	0	0	0	0	0	0	0	0	0	0	8
01:00 - 01:59	0	0	0	3	1	0	0	0	0	0	0	0	0	0	0	4
02:00 - 02:59	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00 - 03:59	0	0	1	1	2	0	0	0	0	0	0	0	0	0	0	4
04:00 - 04:59	0	0	1	3	3	3	0	0	0	0	0	0	0	0	0	10
05:00 - 05:59	0	1	2	9	2	5	0	0	0	0	0	0	0	0	0	19
06:00 - 06:59	0	1	7	25	45	10	1	0	0	00	0	· 0	0	0	0	89
07:00 - 07:59	0	3	13	92	80	14	1	1	0	0	0	0	0	0	0	204
08:00 - 08:59	0	4	28	125	78	15	0	1	0	0	0	0	0	0	0	251
09:00 - 09:59	0	1	17	74	77	20	2	0	0	0	0	1	0	0	0	192
10:00 - 10:59	0	1	8	82	70	6	2	0	0	0	0	0	0	0	0	169
11:00 - 11:59	0	1	13	101	77	18	1	0	0	0	0	0	0	0	0	211
12:00 - 12:59	0	3	25	87	67	16	0	0	0	0	0	0	0	0	0	198
13:00 - 13:59	1	3	27	115	79	7	0	0	0	0	0	0	0	0	0	232
14:00 - 14:59	0	2	16	130	104	10	1	0	0	0	0	0	0	0	0	263
15:00 - 15:59	1	5	33	137	97	9	2	0	0	0	0	0	0	0	0	284
16:00 - 16:59	0	2	36	118	111	15	1	0	0	0	0	0	0	0	0	283
17:00 - 17:59	0	4	36	195	105	16	1	0	0	0	0	0	0	0	0	357
18:00 - 18:59	່ 1	2	19	92	107	12	1	0	0	0	0	0	0	· 0	0	234
19:00 - 19:59	3	3	22	61	35	4	2	0	0	0	0	0	0	0	0	130
20:00 - 20:59	0	8	22	64	30	1	0	0	0	0	0	0	0	0	0	125
21:00 - 21:59	0	2	19	29	36	3	0	0	0	0	0	0	0	0	0	89
22:00 - 22:59	0	0	3	22	10	5	0	0	0	0	0	0	0	0	0	40
23:00 - 23:59	0	0	4	14	14	3	0	0	0	0	0	0	0	0	0	35
Totals	6	46	352	1587	1231	192	15	2	0	0	0	1	0	0	0	3432
ercent of Total	0.2	1.3	10.3	46.2	35.9	5.6	0.4	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.0	1.0	7.7	45.0	37.5	7.8	0.6	0.2	0.0	0.0	0.0	0.1	0.0	0.0	0.0	100
ercent of PM	0.3	1.5	11.5	46.9	35.0	4.4	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	n:	4.4 M	PH			Ten Mil	e Pace:	25 to	34 MPH			85th F	Percentile	: :	33.8 MPH
Me	ean Spee	d:	29.3 M	PH	Р	ercent in	Ten Mil	e Pace:		82.1%			=			
Med	ian Spee	d:	29.1 M	РН									15th F	Percentile	e:	25.3 MPH
Мо	dal Spee	d:	27.5 M	PH									90th F	Percentile):	34.5 MPH
													95th F	Percentile	e:	36.0 MPH

Study Date: Thursday, 03/21/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

ĺ	5-	15-	20-	25-	30-	35-	40-	45-	50-	55-	60-	65-	70-	75-	80-	Total
00.00 00.50	14	19	24	29	34	39	44	49	54	59	64	69	/4	/9	99	10(a)
00:00 - 00:59			0	<u> </u>	2		- 0	0	0	0	0	- 0	- 0			0
01:00 - 01:59			0		2	1	2	- 0	0	0	0	0	0		0	
02:00 - 02:59	0	0		2	1	1	0	0	0	0	0	0	0			
03.00 - 03.59	0		1	1	1	0	0	0	- 0	0	0	0	0		0	6
04:00 - 04:59	0	0	2		7	0	0	0	- 0	0	0	0	0	0	0	14
05:00 - 05:59	1	- 0	2	40	21	11	0	0	0	0	0		0	0	0	02
07:00 - 07:59	<u>'</u>	0	10	92	69	17	0	0		0	0	0	0	0	0	188
08:00 - 08:59	0	2	15	107	113	9	0	0	0	0	0	0	0	0	0	246
09:00 - 09:59	0	2	21	84	72	13	0	0	0	0	0	0	0	0	0	192
10:00 - 10:59	1	5	26	87	65	9	1	0	0	0	0	0	0	0	0	194
11:00 - 11:59	0		27	95	87	8	0	0	0	0	0	0	0	0	0	218
12:00 - 12:59	0	8	14	93	87	20	1	0	0	0	0	0	0	0	0	223
13:00 - 13:59	1	1	27	106	83	8	0	0	0	0	0	0	0	0	0	226
14:00 - 14:59	0	2	15	101	106	10	1	0	.0	0	0	0	0	0	0	235
15:00 - 15:59	1	6	33	136	101	11	2	0	Ö	0	0	0	0	0	0	290
16:00 - 16:59	0	6	28	150	106	13	0	0	1	0	0	0	0	0	0	304
17:00 - 17:59	0	3	29	147	131	20	1	0	0	0	0	0	0	0	0	331
18:00 - 18:59	1	3	8	113	105	10	1	0	0	0	1	0	0	0	0	242
19:00 - 19:59	0	1	30	106	39	2	0	0	0	0	0	0	0	0	0	178
20:00 - 20:59	0	2	16	58	32	4	0	0	0	0	0	0	0	0	0	112
21:00 - 21:59	1	3	8	33	24	1	0	1	0	0	0	0	0	0	0	71
22:00 - 22:59	0	0	6	21	14	3	2	0	0	0	0	0	0	0	0	46
23:00 - 23:59	0	1	7	17	14	1	0	0	0	0	0	0	0	0	0	40
Totals	6	47	333	1606	1293	172	11	1	1	0	1	0	0	0	0	3471
ercent of Total	0.2	1.4	9.6	46.3	37.3	5.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.2	0.9	9.5	44.8	38.4	5.9	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.2	1.6	9.6	47.0	36.6	4.5	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	n:	4.3 M	PH			Ten Mil	e Pace:	25 to	34 MPH			85th F	Percentile	:	33.7 MPH
Me	an Spee	ed:	29.3 M	PH	P	ercent in	Ten Mil	e Pace:		83.5%			154-1	Danaantii		
Medi	ian Spee	ed:	29.2 M	PH												
Mo	dal Spee	ed:	27.5 M	PH									90th H	ercentile	2	34.4 MPH
													95th I	Percentile	:	35.3 MPH

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Study Date: Friday, 03/22/2019

Unit ID: SGP12

Location: South Street, Portsmouth, NH

	5-	15-	20-	25-	30-	35-	40-	45-	50-	55-	60-	65-	70-	75-	80-	
	14	19	24	29	34	39	44	49	54	59	64	69	74	79	99	Total
00:00 - 00:59	0	0	2	5	3	2	0	0	0	0	0	0	0	0	0	12
01:00 - 01:59	0	1	0	1	3	0	0	0	0	0	0	0	0	0	0	5
02:00 - 02:59	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	4
03:00 - 03:59	0	0	1	0	2	1	1	0	0	0	0	0	0	0	0	5
04:00 - 04:59	0	0	2	5	2	0	0	0	0	0	0	0	0	0	0	9
05:00 - 05:59	0	0	1	10	3	1	1	0	0	0	0	0	0	0	0	16
06:00 - 06:59	0	1	4	31	41	5	0	0	· 0	0	0	0	0	0	0	82
07:00 - 07:59	0	3	16	68	52	13	2	0	0	0	0	0	0	0	0	154
08:00 - 08:59	0	1	6	26	24	8	0	0	0	0	0	0	0	0	0	65
09:00 - 09:59	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-
10:00 - 10:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 - 11:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 - 12:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13:00 - 13:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14:00 - 14:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15:00 - 15:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16:00 - 16:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17:00 - 17:59	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-
18:00 - 18:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
19:00 - 19:59	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	-
20:00 - 20:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
21:00 - 21:59	. –	1	-	-	-	-	-	-	-	-	-	-	-	-	-	-
22:00 - 22:59	· -	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
23:00 - 23:59	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Totals	0	6	32	148	132	30	4	0	0	0	0	0	0	0	0	352
ercent of Total	0.0	1.7	9.1	42.0	37.5	8.5	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of AM	0.0	1.7	9.1	42.0	37.5	8.5	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
ercent of PM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100
Standard	Deviatio	n:	4.6 M	PH ·			Ten Mile	e Pace:	25 to	34 MPH			85th F	Percentile	e:	34.3 MPH
Me	an Spee	ed:	29.8 M	РН	P	ercent in	Ten Mile	e Pace:		79.5%						
Medi	ian Spee	ed:	29.6 M	РН									15th F	Percentile	e:	25.5 MPH
Mo	dal Spee	ed:	27.5 M	РН									90th F	Percentile	: :	34.9 MPH
				-									95th F	Percentile	e:	37.7 MPH



Sight Distance Calculations - South Street / Proposed Lot 1 Driveway May 30, 2019

Given:

A. 85th percentile EB speed = 30.6 mph (Pernaw & Co., Inc. speed survey) B. 85th percentile WB speed = 33.8 mph (Pernaw & Co., Inc. speed survey) C. Available sight distance "looking right" = 226 feet (Ambit "Driveway Plan," Sheet C3, dated March 2019, revised 5/9/19.) D. Available sight distance "looking left" = 345 feet (Ambit "Driveway Plan," Sheet C3, dated March 2019, revised 5/9/19.)

Calculate distance traveled during perception-reaction time

EB = 1.47 X 30.6 X 2.5 = 112.5 feet (113' rounded) WB = 1.47 X 33.8 X 2.5 = 124.2 feet (125' rounded)

Calculate average grade during braking distance

EB average roadway grade = (62 - 56) / 104' = -0.058%WB average roadway grade = (54 - 46) / 90' = +0.089%

Calculate braking distance

 $EB = \frac{(30.6)^2}{30 (11.2 / 32.2 - .058)} = 107.7 \text{ feet (108' rounded)}$

$$WB = \frac{(33.8)^2}{30 (11.2 / 32.2 + .089)} = 87.2 \text{ feet (88' rounded)}$$

Calculate required stopping sight distance:

EB:	Distance during perception-reaction time =	113 feet
EB:	Braking distance =	<u>108</u> feet
EB:	Total SSD =	221 feet
WB:	Distance during perception-reaction time =	125 feet
WB:	Braking distance =	<u>88</u> feet
WB:	Total SSD =	213 feet

Conclusions:

1. The available stopping sight distance looking <u>right</u> from the Lot 1 Driveway (226 feet) exceeds the minimum stopping sight distance for 85th percentile speed (221 feet) and is therefore adequate.

2. The available stopping sight distance looking <u>left</u> from the Lot 1 Driveway (345 feet) exceeds the minimum stopping sight distance for 85th percentile speed (213 feet) and is therefore adequate.



Sight Distance Calculations - South Street / Proposed Lot 2&3 Driveway May 30, 2019

Given:

A. 85th percentile EB speed = 30.6 mph (Pernaw & Co., Inc. speed survey) B. 85th percentile WB speed = 33.8 mph (Pernaw & Co., Inc. speed survey) C. Available sight distance "looking right" = 437 feet (Ambit "Driveway Plan," Sheet C3, dated March 2019, revised 5/9/19.) D. Available sight distance "looking left" = 324 feet (Ambit "Driveway Plan," Sheet C3, dated March 2019, revised 5/9/19.)

Calculate distance traveled during perception-reaction time

EB = 1.47 X 30.6 X 2.5 = 112.5 feet (113' rounded) WB = 1.47 X 33.8 X 2.5 = 124.2 feet (125' rounded)

Calculate average grade during braking distance

EB average roadway grade = 0.0%WB average roadway grade = (61 - 52) / 120' = +0.075%

Calculate braking distance

 $EB = \frac{(30.6)^2}{30 (11.2 / 32.2 - .000)} = 89.7 \text{ feet (90' rounded)}$

$$WB = \frac{(33.8)^2}{30 (11.2 / 32.2 + .075)} = 90.1 \text{ feet (91' rounded)}$$

Calculate required stopping sight distance:

EB:	Distance during perception-reaction time =	113 feet
EB:	Braking distance =	<u>90</u> feet
EB:	Total SSD =	203 feet
WB:	Distance during perception-reaction time =	125 feet
WB:	Braking distance =	<u>91</u> feet
WB:	Total SSD =	216 feet

Conclusions:

1. The available stopping sight distance looking <u>right</u> from the Lot 2&3 Driveway (437 feet) exceeds the minimum stopping sight distance for 85th percentile speed (203 feet) and is therefore adequate.

2. The available stopping sight distance looking <u>left</u> from the Lot 2&3 Driveway (324 feet) exceeds the minimum stopping sight distance for 85th percentile speed (216 feet) and is therefore adequate.