



August 11, 2020

Portsmouth Planning Board  
Portsmouth City Hall  
1 Junkins Avenue  
Portsmouth, NH 03801

**Re: Proposed Chase Bank – 1574 Woodbury Avenue - Site Plan Review**

Dear Members of the Board,

We are in receipt of comments provided by the Technical Advisory Committee (TAC) that were discussed during the TAC meeting held on August 4, 2020. We have addressed these comments and provide this response letter. For simplicity, we have copied the comment herein and provided a response immediately following the item in blue italicized text.

1. Move notes 1-3 on Cover Sheet to sheet C-2 – these are required to be included on the site plan sheet that will be recorded.  
*Relocated as requested.*
2. Move notes 4-6 on Cover Sheet to LP-1  
*Relocated as requested.*
3. Applicant should be prepared to address the requirements stated in section 10.835.40 of the Zoning Ordinance related to traffic capacity and safety for Planning Board approval of the conditional use permit. In addition, the Drive-Through Facilities requirements for the Site Plan review regulations require that a Traffic Impact Study must be completed (see Section 3.6.3). If you do not intend to submit a Traffic Impact Study, please submit a waiver request explaining why the traffic generation memo should suffice.  
*ITE Traffic Generation rates were used to compare the existing restaurant use with the proposed bank. Significant reductions in Total Weekday, Saturday trip counts were reduced as well as in am and pm peak hour estimates. A waiver request accompanies this comment response for consideration by the Board.*
4. Your drive-through ATM does not provide stacking lanes and spaces as required by Section 3.6.2. If you do not intend to provide these, please submit a waiver request explaining why this is not necessary.  
*Stacking Spaces have been added at 10'X20' each to signify a stacking length for 7 cars including the lead vehicle at the ATM.*
5. On Sheet C-2, please add a note that a separate sign permit will be required for any proposed signage listed in the "Signage Table"

*Added as requested.*

6. Please provide a detail of the dumpster screening consistent with the requirements of Section 10.1132 of the Zoning Ordinance  
*Added as requested. Please refer to drawing sheet CD-5.*
7. Your proposed dumpster location is required to be a minimum of 10' from the abutting lot line per the zoning ordinance and a minimum of 20' from the abutting lot line per the site plan review regulations. If it is less than 10', it will require a variance from the Zoning Board of Adjustment, if it is less than 20', it will require a waiver from the Planning Board.  
*The requested dimensions have been provided as requested. The trash enclosure is more than 10-ft. from the property line, but less than 20-ft. A Waiver Request accompanies this response letter for the proposed relief.*
8. What are the dimensions of the proposed bike rack pad and how far is it from the building?  
*Dimensions shown on the updated plan. Please refer to Sheet C-4.*
9. Bike rack detail on CD-1 for a wave rack is not consistent with the City's preferred standard – see <http://planportsmouth.com/citybikerackguidelines.pdf>.  
*Added to CD-1.*
10. Penalty sign not needed under handicap parking sign.  
*Eliminated as requested (Sheet CD-1).*
11. Directional signs at driveways should be placed no closer than 15 feet to plaza drive aisle, to avoid blocking sight lines for drivers exiting bank driveways into plaza drive aisle.  
*Revised as requested.*
12. Proposed CDS Stormwater Unit needs to be a separate stand-alone unit that is not located over the existing drainage pipe.  
*Configuration is revised on the current drawings to be in-line and connected to the existing drain via a "Dog-House" style Manhole.*
13. Provide site specific details in the plan set (Sht. CD-2) for the CDS unit including site specific data requirements, model #, dimensions. Also, the plans should indicate the responsible party for maintenance and maintenance requirements.  
*Details are shown on Drawing Sheet CD-2 including specifications and Model No. O & M responsibility shall be JPM Chase (or their property manager).*
14. Indicate where the "doghouse manhole" is intended to be located.  
*With the current revision, this structure will be required to facilitate a drainage connection for the drain network for this redevelopment. This manhole is located within the landscaped area between the access driveway and the exit lane from the Drive-Thru.*
15. Indicate where "pipe connection to existing manhole detail" shown on Sht. CD-2 is proposed.  
*No longer needed, so this reference is removed.*
16. Building sewer lateral should be 6 inch diameter pipe.  
*Corrected as requested.*
17. General Comment: It appears that the proposed building sewer lateral could be installed with fewer cleanouts (i.e.: changes in direction). Request engineer to review with DPW.  
*Direction changes are required to provide sewer service from the proposed building to the existing service lateral on-site. This shall be accomplished with as few bends as possible (45-degree bends with Clean-Outs).*

August 11, 2020

I trust these comments have been adequately addressed, and if further comments remain, these items could be addressed as a condition as part of your Decision. If any questions or comments remain, please feel free to contact me directly @ 857-262-0191.

Respectfully Submitted,  
**CORE STATES GROUP**

A handwritten signature in blue ink, appearing to read "Alan D. Roscoe". The signature is fluid and cursive, with a large initial "A" and "R".

**Alan D. Roscoe, P.E.**

August 12, 2020

Portsmouth Planning Board  
Portsmouth City Hall  
1 Junkins Avenue  
Portsmouth, NH 03801

**RE: Waiver Requests for Site Plan Review – 1574 Woodbury Avenue  
Proposed Chase Bank (at former Ruby Tuesday's)**

Dear Members of the Board,

As allowed under Portsmouth Planning Board regulations, we hereby request a waiver from portions of the Land Use Code. Specifically, we request a waiver from Section 3.6.3 of the regulations, for relief from providing a full Traffic Impact Study as the proposed use is a less traffic intensive than the existing. We have prepared and submitted a Traffic Generation Estimate based on Institute of Traffic Engineering (ITE) databases and formulas to compare the proposed bank with drive-through use with the restaurant use. The result of this comparison concludes that the proposed bank will generate significantly fewer traffic trips to this location than the existing restaurant. This is true across Total Weekday; Saturday; and on Weekday am and pm peak hours. In our opinion, a full traffic impact study would reach the same conclusion, and therefore may not be necessary to evaluate this aspect of this project. The intent of this requirement is satisfied with the submittal of the Traffic Generation Memorandum.

Additionally, we request a waiver from site plan regulations pertaining to the proposed location of our trash enclosure. We comply with the Zoning Ordinance standard of 10-ft. (min.) to abutting property lines, but desire a waiver from site plan regulations to be located less than 20-ft. from the property line. The proposed trash enclosure is to be located 12.89-ft. from the northerly property boundary. Given other site plan regulations for other elements of the proposed use (drive-through and ATMs), we have placed the trash receptacle in an accessible location for ease of access by trash removal contractors and complying with the intent of the requirement.

The intent of the requirements is met with these positive attributes described above and will improve overall conditions on and off-site. Granting the requested relief will not substantially derogate from these standards.

We appreciate your consideration of this request.

Respectfully Submitted,  
**CORE STATES GROUP**

Alan D. Roscoe  
Project Manager

**DESIGN AND DEVELOPMENT CONTACTS:**

**APPLICANT** J.P. MORGAN CHASE BANK  
1450 BRICKELL AVENUE 3RD FLOOR  
MIAMI, FL 33131  
CONTACT: CHRIS FOIT  
(786) 473-1769

**OWNER** RICHARD FUSEGNI  
201 KEARSARGE WAY  
PORTSMOUTH, NH 03801  
CONTACT: SCOTT MITCHELL  
(603) 475-377

**CIVIL ENGINEER** CORE STATES INC.  
9 GALEN STREET, SUITE 117  
WATERTOWN, MA 02472  
CONTACT: ALAN D. ROSCOE, P.E.  
(857) 500-4702

**ARCHITECT** CORE STATES INC.  
201 S. MAPLE AVE  
AMBLER, PA 19002  
CONTACT: KEN MACKENZIE  
(267) 464-8048

**SURVEYOR** ALLEN & MAJOR ASSOCIATES, INC.  
400 HARVEY ROAD  
MANCHESTER, NH 03103  
CONTACT: JAMES P. SMITH NH LLS  
(603) 627-5500

**GOVERNING AGENCIES CONTACTS:**

**PLANNING** PLANNING DEPARTMENT  
1 JUNKINS AVENUE, 3RD FLOOR  
PORTSMOUTH, NH 03801  
CONTACT: JULIET WALKER, PLANNING DIRECTOR  
(603) 610-7216

**BUILDING** INSPECTION DEPARTMENT  
1 JUNKINS AVENUE  
PORTSMOUTH, NH 03801  
CONTACT: ROBERT MARSILIA, CHIEF BUILDING INSPECTOR  
(603) 610-7243

**FIRE AUTHORITY** FIRE DEPARTMENT  
170 COURT STREET  
PORTSMOUTH, NH 03801  
CONTACT: TODD GERMAIN, INTERIM FIRE CHIEF  
(603) 427-1515

**UTILITY CONTACTS:**

**GAS** UNITIL  
6 LIBERTY LANE WEST  
HAMPTON, NH 03842  
(866) 933-3820

**ELECTRIC AND FIRE ALARM** EVERSOURCE  
55 BEARFOOT ROAD  
NORTHBOROUGH, MA 01532  
(800) 322-3223

**WATER** PORTSMOUTH PUBLIC WORKS DEPARTMENT  
680 PEVERLY HILL ROAD  
PORTSMOUTH, NH 03801  
(603) 427-1530

**SEWER** PORTSMOUTH PUBLIC WORKS DEPARTMENT  
680 PEVERLY HILL ROAD  
PORTSMOUTH, NH 03801  
(603) 427-1530

**TELEPHONE** VERIZON  
185 FRANKLIN STREET  
BOSTON, MA 02107  
(800) 870-9999

**CABLE** COMCAST  
179 W MAIN STREET  
AYER, MA 01432  
(800) 266-2278

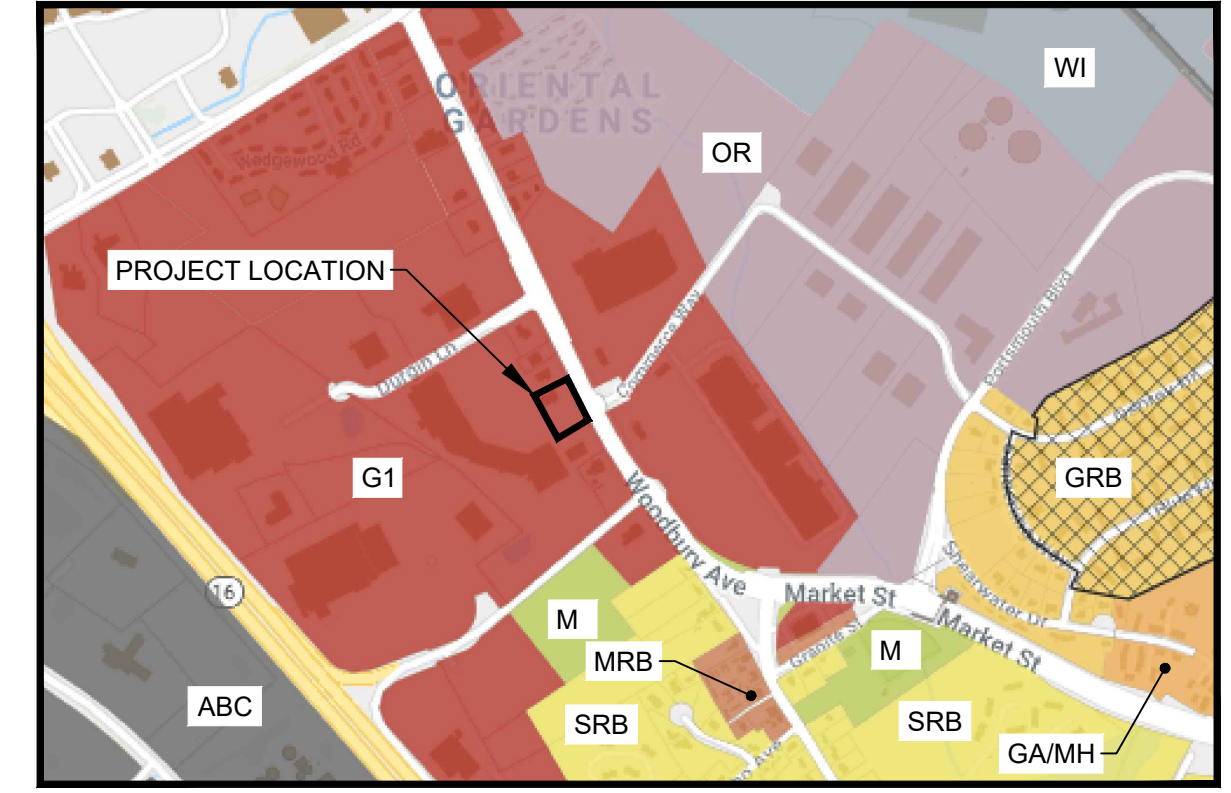
# SITE PLAN APPROVAL FOR



## PROPOSED CHASE BANK

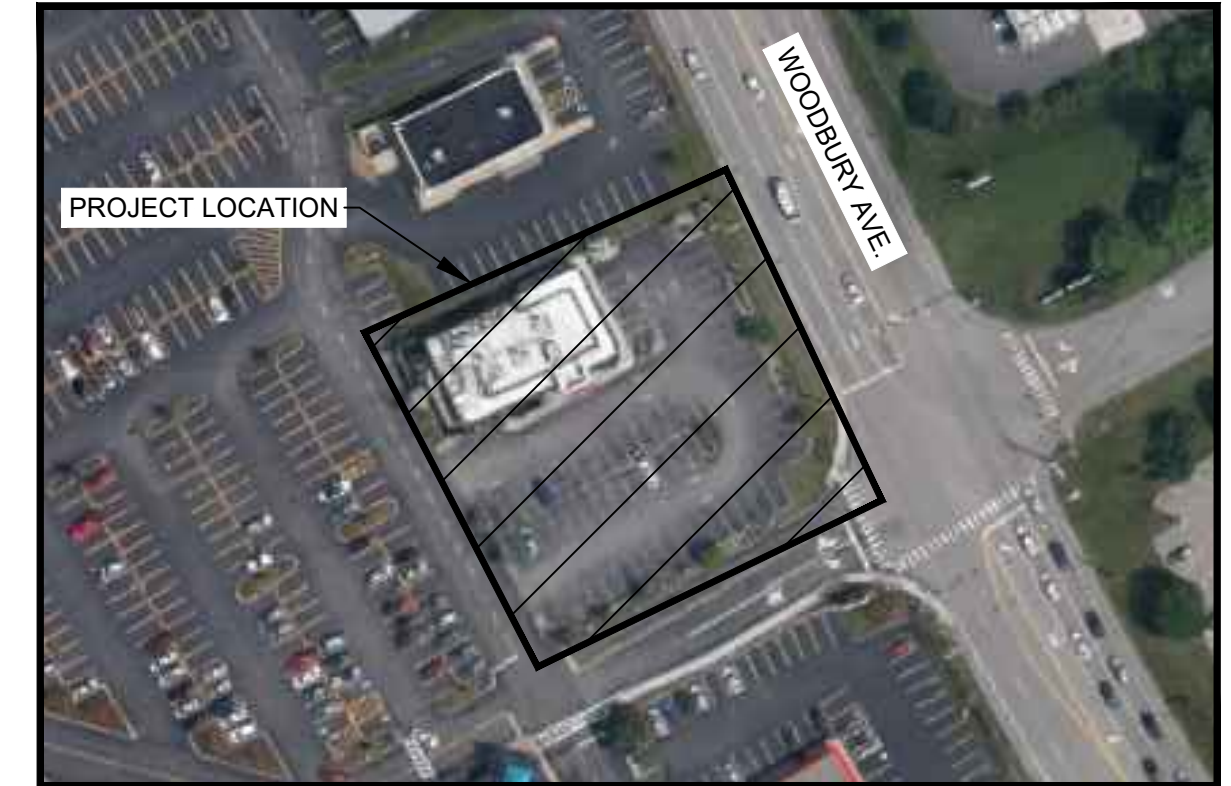
1574 WOODBURY AVENUE  
CITY OF PORTSMOUTH  
ACCESSORS MAP 238, LOT 17  
ROCKINGHAM COUNTY, NEW HAMPSHIRE 03801

| SHEET INDEX        |   |                                |                 |
|--------------------|---|--------------------------------|-----------------|
| PAGE NUMBER        | DESCRIPTION                             | PREPARED BY                    | REVISION NUMBER |
| <b>CIVIL PLANS</b> |   |                                |                 |
| CV-1               | COVER SHEET                             | CORE STATES                    | △               |
| CV-2               | GENERAL NOTES                           | CORE STATES                    |                 |
| C-1                | DEMOLITION PLAN                         | CORE STATES                    |                 |
| C-2                | SITE PLAN                               | CORE STATES                    | △               |
| C-3                | GRADING PLAN                            | CORE STATES                    |                 |
| C-4                | UTILITY PLAN                            | CORE STATES                    | △               |
| C-5                | SOIL EROSION & SEDIMENT CONTROL PLAN    | CORE STATES                    |                 |
| C-6                | DRAINAGE AREA MAPS                      | CORE STATES                    |                 |
| C-7                | LIGHTING PLAN                           | CORE STATES                    |                 |
| C-8                | LIGHTING DETAILS                        | CORE STATES                    |                 |
| C-9                | SOIL EROSION & SEDIMENT CONTROL DETAILS | CORE STATES                    |                 |
| CD-1 - CD-5        | CONSTRUCTION DETAILS                    | CORE STATES                    | △               |
| LP-1               | LANDSCAPE PLANTING                      | EVERGREEN DESIGN GROUP         | △               |
| LP-2               | PLANTING DETAILS & SPECIFICATIONS       | EVERGREEN DESIGN GROUP         |                 |
| 1 OF 1             | EXISTING CONDITIONS SURVEY              | ALLEN & MAJOR ASSOCIATES, INC. |                 |



**ZONING MAP**  
1" = 1,000'

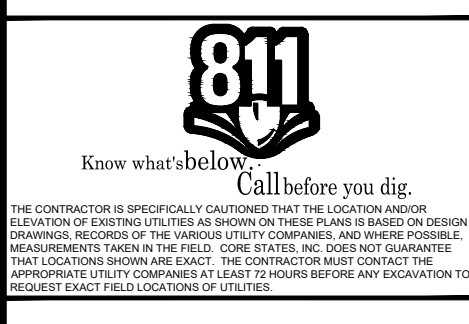
**G1 - GATEWAY CORRIDOR DISTRICT**  
**M - MUNICIPAL**  
**SRB - SINGLE RESIDENCE B**  
**GRB - GENERAL RESIDENCE B**  
**MRB - MIXED RESIDENTIAL BUSINESS**  
**OR - OFFICE RESEARCH**  
**ABC - AIRPORT BUSINESS COMMERCIAL**  
**GA/MH - GARDEN APARTMENT/ MOBILE HOME PARK**  
**WI - WATERFRONT INDUSTRIAL**



**VICINITY MAP**  
1" = 100'



DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK. IT IS USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED. USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.

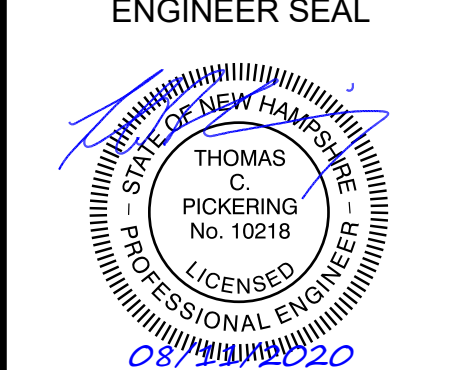


| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |
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**DOCUMENT**

**SITE PLAN APPROVAL FOR CHASE BANK**

**SITE LOCATION**  
1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801



**SHEET TITLE**  
COVER SHEET

**JOB #:** JPM 27086  
**DATE:** 07/01/2020  
**SCALE:** AS NOTED  
**DRAWN BY:** MAL  
**CHECKED BY:** KGF

**SHEET NO.**  
**CV-1**

STANDARD ABBREVIATIONS table with columns for symbol and description. Includes entries for ACRES, ARCHITECTURAL, BUILDING, CONCRETE, etc.

GENERAL SITE NOTES: 1. ALL CONSTRUCTION MATERIALS AND TECHNIQUES OF INSTALLATION SHALL MEET PERFORMANCE VALUES... 2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT THIS PROJECT IS CONSTRUCTED IN ACCORDANCE WITH THESE DOCUMENTS...

REVISIONS. 30. SAFETY NOTICE TO CONTRACTOR: IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE...

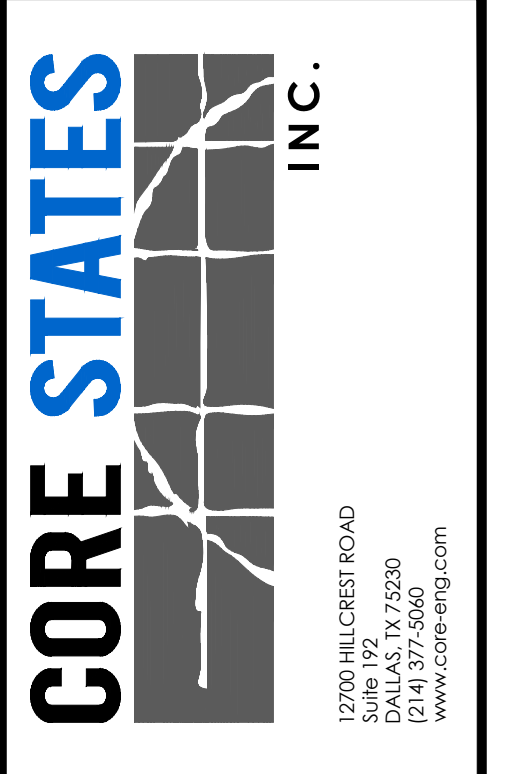
SOIL EROSION AND SEDIMENT CONTROL NOTES: 1. ALL APPLICABLE EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE IN PLACE PRIOR TO ANY GRADING OPERATION AND/OR INSTALLATION OF PROPOSED STRUCTURES OR UTILITIES.

AS THE EXACT LOCATION, OR AS THE ONLY OBSTACLES THAT MAY OCCUR ON THE SITE. VERIFY EXISTING CONDITIONS AND PRIOR WITH CAUTION... 13. ELECTRICAL, TELEPHONE, CABLE, WATER, FIBER OPTIC CABLE AND/OR GAS LINES NEEDING TO BE REMOVED OR RELOCATED SHALL BE COORDINATED WITH THE AFFECTED UTILITY COMPANY...

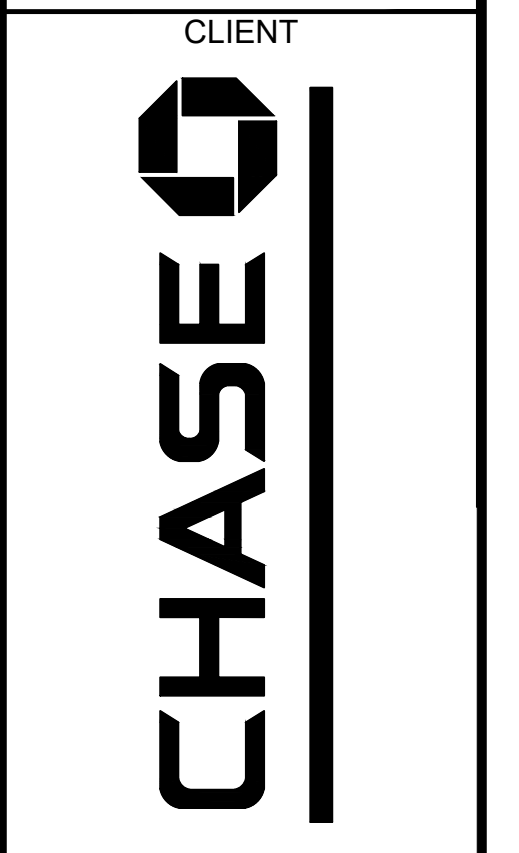
GENERAL UTILITY NOTES: 1. CONTRACTOR SHALL COORDINATE ANY DISRUPTIONS TO EXISTING UTILITY SERVICES WITH ADJACENT PROPERTY OWNERS. 2. ALL ELECTRIC, TELEPHONE AND GAS EXTENSIONS INCLUDING SERVICE LINES SHALL BE CONSTRUCTED TO THE APPROPRIATE UTILITY COMPANY SPECIFICATIONS...

EXCEPTIONS TO THIS MUST BE APPROVED IN WRITING BY THE AUTHORITY HAVING JURISDICTION. 19. A MINIMUM HORIZONTAL DISTANCE OF 3 FEET, UNLESS OTHERWISE NOTED IN THE PLANS, SHALL BE MAINTAINED BETWEEN WATER LINES AND OTHER UNDERGROUND OF A NONSANITARY NATURE (GAS, ELECTRIC, ETC.)... 21. TREES SHALL BE PLACED SO AS TO AVOID BURIED UTILITIES.

GENERAL PAVING AND GRADING NOTES: 1. ALL PAVING AND GRADING CONSTRUCTION MATERIALS AND METHODS SHALL MEET THE STANDARD SPECIFICATIONS AND REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION. 2. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF VARIOUS UTILITY COMPANIES...



DOCUMENTS PREPARED BY CORE STATES, INC., INCLUDING THIS DOCUMENT, ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK.



CLIENT Know what's before you dig. Call before you dig. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF VARIOUS UTILITY COMPANIES AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. CORE STATES, INC. DOES NOT GUARANTEE THE LOCATION AND/OR ELEVATION OF UTILITIES AS SHOWN ON THESE PLANS.

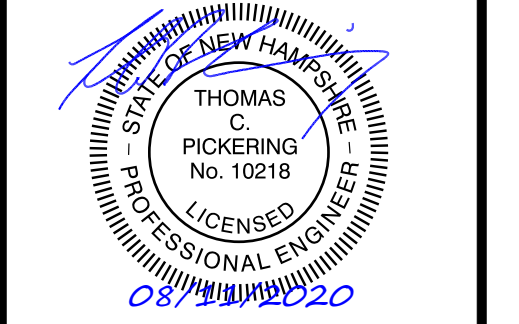
Table with columns: REV, DATE, COMMENT, BY. Row 1: 1, 8/10/2020, PER TAC COMMENTS, MAL

DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

SITE LOCATION 1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801

ENGINEER SEAL



SHEET TITLE

GENERAL NOTES

JOB #: JPM 27086 DATE: 07/01/2020 SCALE: NO SCALE DRAWN BY: MAL CHECKED BY: KGF

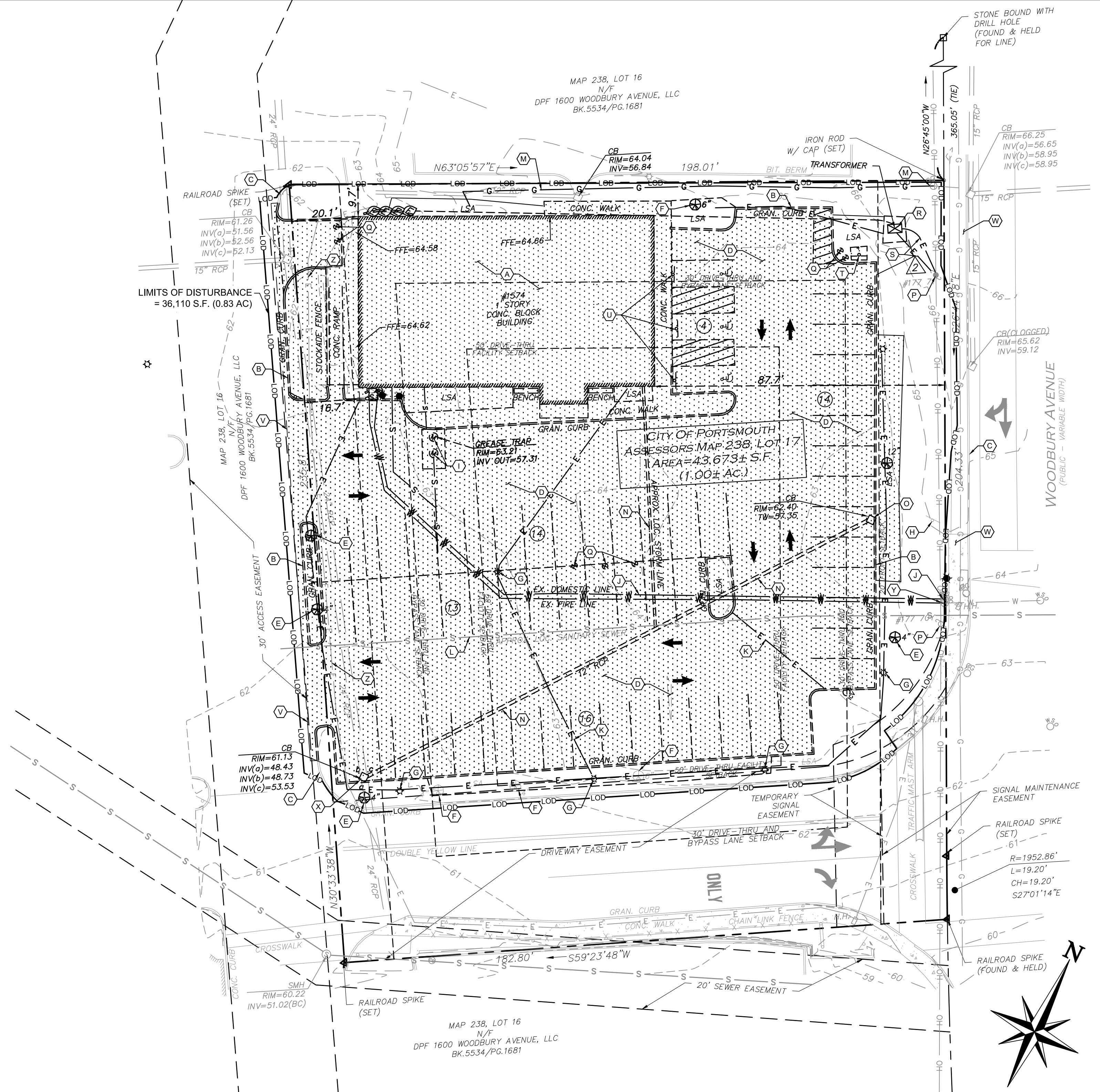
SHEET NO. CV-2

**DEMOLITION LEGEND**

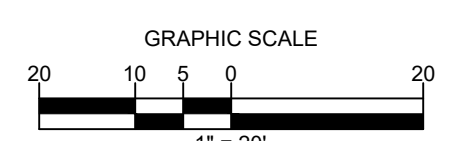
- PROPERTY BOUNDARY LINE
- CENTER LINE OF EXISTING ROADWAY
- ADJOINING PROPERTY LINE
- - - EXISTING EDGE OF PAVEMENT
- - - EXISTING FENCE
- - - EXISTING BLOCK NUMBER
- - - EXISTING LOT NUMBER
- - - EXISTING 5' INTERVAL CONTOUR LINE
- - - EXISTING 1' INTERVAL CONTOUR LINE
- EXISTING TREE
- EXISTING TREE LINE
- EXISTING GAS MAIN
- EXISTING UTILITY POLE
- EXISTING STORM STRUCTURES
- EXISTING SANITARY STRUCTURES
- EXISTING WATER MAIN
- EXISTING FIRE HYDRANT
- EXISTING WATER VALVE
- EXISTING UNDERGROUND ELECTRIC
- EXISTING TELEPHONE
- EXISTING OVERHEAD WIRES
- EXISTING SANITARY
- EXISTING STORM
- EXISTING BUILDING

**E & S LEGEND**

- SOX --- SOX --- ○ PROPOSED SILT SOXX
- PROPOSED INLET PROTECTION



**DEMO PLAN**  
SCALE 1" = 20'



**DEMOLITION KEY NOTES:**

- A. EXISTING BUILDING AND FOUNDATIONS TO BE REMOVED. CONTRACTOR TO BACKFILL AS NECESSARY AND PROVIDE COMPACTION PER GEO-TECHNICAL REPORT RECOMMENDATIONS.
- B. EXISTING CURB TO BE REMOVED.
- C. EXISTING CURB TO REMAIN. CONTRACTOR TO PROTECT IN PLACE.
- D. EXISTING HARDSCAPE TO BE REMOVED.
- E. EXISTING TREE TO BE REMOVED.
- F. EXISTING TREE TO REMAIN. CONTRACTOR TO PROTECT IN PLACE.
- G. EXISTING LIGHT POLE TO BE REMOVED.
- H. EXISTING SIGN TO REMAIN. CONTRACTOR TO PROTECT IN PLACE.
- I. EXISTING GREASE TRAP TO REMOVED.
- J. EXISTING DOMESTIC WATER LINE TO BE REMOVED AND CAPPED AT EXISTING WATER VALVE TO BE REUSED FOR PROPOSED WATER SERVICE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY AND LOCAL JURISDICTION PRIOR TO COMMENCING WORK.
- K. DE-ENERGIZE LIGHTING PRIOR TO COMMENCING WORK OF EXISTING LIGHT POLE ELECTRICAL LINES. LINES TO BE CUT, CAPPED AND ABANDONED IN PLACE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING WORK.
- L. EXISTING SEWER TO BE CUT AND CAPPED AT EXISTING TAP. CONTRACTOR TO PROVIDE LOCATION, SIZE AND INVERT TO THE ENGINEER OF RECORD PRIOR TO COMMENCING OF PROPOSED WORK.
- M. EXISTING GAS SERVICE LINE TO BE CUT AND CAPPED AT EXISTING GAS VALVE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO BEGINNING OF ANY WORK.
- N. EXISTING STORM LINE TO BE REMOVED.
- O. EXISTING CATCH BASIN TO BE REMOVED.
- P. EXISTING UTILITY POLE TO REMAIN AND TO BE PROTECTED IN PLACE.
- Q. EXISTING BOLLARDS TO BE REMOVED.
- R. EXISTING TRANSFORMER AND CONCRETE PAD TO BE RELOCATED. REFER TO SITE PLANS FOR PROPOSED LOCATION. CONTRACTOR TO VERIFY IF THE EXISTING TRANSFORMER IS REQUIRED FROM THE UTILITY COMPANY AND ARCHITECTURAL DRAWINGS. IF A NEW SERVICE IS REQUIRED, CONTRACTOR TO COORDINATE WITH UTILITY COMPANY TO OBTAIN PROPER SERVICE.
- S. EXISTING ELECTRICAL CONDUITS AND METERS TO BE REMOVED AS NECESSARY.
- T. EXISTING PYLON SIGN AND FOOTINGS TO BE REMOVED.
- U. EXISTING HANDICAP SIGNS TO BE REMOVED.
- V. PROPOSED SAWDUST LINE.
- W. EXISTING PUBLIC RIGHT-OF-WAY SIDEWALK TO REMAIN. CONTRACTOR TO PROTECT IN PLACE.
- X. EXISTING 24" RCP TO BE REMOVED FOR PROPOSED WATER QUALITY CDS UNIT BY CONTECH PER PROPOSED UTILITY DESIGN. CONTRACTOR TO REMOVE/REPLACE EXISTING PIPE AS NECESSARY TO INSTALL UNIT. ANY DAMAGED TO EXISTING STRUCTURES OR PIPES ARE TO BE REPLACED.
- Y. EXISTING FIRE LINE TO BE REMOVED AT EXISTING WATER VALVE FOR THE INSTALLATION OF PROPOSED FIRE HYDRANT. CONTRACTOR TO COORDINATE WITH AUTHORITY JURISDICTION AND UTILITY COMPANY PRIOR TO COMMENCING WORK.
- Z. EXISTING ELECTRICAL CONDUIT TO REMAIN AND PROTECTED IN PLACE. CONTRACTOR TO VERIFY FINAL TERMINATION OF ELECTRICAL BEFORE BEGINNING OF WORK.

**DEMOLITION NOTES:**

1. THE TOTAL LIMITS OF DISTURBANCE FOR THIS PROJECT IS 36,110 S.F. (0.83 AC).
2. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION AND FOR CALLING THE APPROPRIATE ONE-CALL CENTER AT LEAST 72 HOURS IN ADVANCE OF ANY EXCAVATION.

**MAINTENANCE NOTES:**

ALL MEASURES STATED ON THIS PLAN SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR A COMPLETED PHASE OF WORK OR FINAL STABILIZATION OF THE SITE. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR THE APPLICABLE PERMIT, WHICHEVER IS MORE STRINGENT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:

1. INLET PROTECTION SHALL BE REPAIRED TO THEIR ORIGINAL CONDITION IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE INLET PROTECTION WHEN CLOGGING BECOMES APPARENT.
2. SILT SOXX SHALL BE REPAIRED OR REPLACED TO THEIR ORIGINAL CONDITION IF DAMAGED.

**ALERT TO CONTRACTOR:**

PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

**CORE STATES INC.**  
12700 HILLCREST ROAD  
DALLAS, TX 75250  
(214) 377-5960  
www.core-eng.com

DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK. IT IS USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED. USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.

CLIENT  
**CHASE**

811  
Know what's below. Call before you dig.

| REVISIONS |           |                  |     |
|-----------|-----------|------------------|-----|
| REV       | DATE      | COMMENT          | BY  |
| 1         | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT  
**SITE PLAN APPROVAL FOR CHASE BANK**  
SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

ENGINEER SEAL  
THOMAS C. PICKERING  
No. 10218  
LICENSED PROFESSIONAL ENGINEER  
08/10/2020

SHEET TITLE  
**DEMOLITION PLAN**  
JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF  
SHEET NO.  
**C-1**



**GENERAL NOTES:**

**THIS DRAWING REFERENCES:** 1574 WOODBURY AVENUE PORTSMOUTH, NH TOPOGRAPHIC PLAN OF LAND PREPARED BY ALLEN & MAJOR ASSOCIATES, INC. 400 HARVEY ROAD MANCHESTER, NH 03103 CONTACT: JAMES P. SMITH NH LLS TEL: (603) 627-5500 DATED: 06/23/2020

**PROPERTY OWNER:** RICHARD P. FUSEGNI 201 KEARSARGE WAY PORTSMOUTH, NH 03801 CONTACT: SCOTT MITCHELL (603) 475-377

**APPLICANT:** J.P. MORGAN CHASE BANK 1450 BRICKELL AVENUE 3RD FLOOR MIAMI, FL 33131 CONTACT: CHRIS FOIT TEL: (786) 473-1769

1) **SITE ADDRESS:** 1574 WOODBURY AVENUE PORTSMOUTH, NH 03801 COUNTY OF ROCKINGHAM

2) **ZONING DATA:** ZONED: G-1 GATEWAY CORRIDOR DISTRICT EXISTING USE: RESTAURANT - RUBY TUESDAYS (PERMITTED) PROPOSED USE: BANK (PERMITTED BY RIGHT) : DRIVE THROUGH (CONDITIONAL USE PERMIT REQUIRED)

| \$10.5B34.60 (SMALL COMMERCIAL BUILDING) | REQUIRED       | EXISTING            | PROPOSED            |
|--|----------------|---------------------|---------------------|
| MIN. LOT AREA, SF:                       | N/A            | 43,673 S.F.         | NO CHANGE           |
| MIN. LOT FRONTAGE, FT:                   | 50 FT.         | 204.32 FT.          | NO CHANGE           |
| FRONT YARD SETBACK, FT:                  | 0 FT. - 20 FT. | 87.7 FT.            | 20 FT.              |
| MIN. SIDE YARD SETBACK, FT:              | 10 FT.         | 9.7 FT.             | 19.94 FT.           |
| MIN. REAR YARD SETBACK, FT:              | 15 FT.         | 16.7 FT.            | 93.6 FT.            |
| MAX. HEIGHT, FT:                         | 40 FT.         | ± 20 FT.            | 21.5 FT.            |
| MAX. HEIGHT, STORIES:                    | 3              | 1                   | 1                   |
| MIN. STREET FACADE HEIGHT:               | 18 FT.         | ± 20 FT.            | 21.5 FT.            |
| MIN. OPEN SPACE COVERAGE:                | 10%            | 17.79% (7,770 S.F.) | 18.18% (7,942 S.F.) |
| MAX. BUILDING COVERAGE:                  | 70%            | 10.53%              | 7.55%               |
| MAX. BUILDING FOOTPRINT:                 | 10,000 S.F.    | 4,600 S.F.          | 3,325 S.F.          |
| MIN. STREET FACING FACADE GLAZING:       | 50%            | "-"                 | 52%                 |

3) **PARKING REQUIREMENTS:** \$10.1112.30 OFF-STREET PARKING REQUIREMENTS PER THE CITY OF PORTSMOUTH ZONING ORDINANCE: FOR PROFESSIONAL, BUSINESS AND FINANCIAL SERVICES: 1 SPACE PER 350 SQUARE FEET OF GROSS FLOOR GROSS AREA

GROSS FLOOR GROSS AREA = 3,325 S.F. (MAIN BUILDING) CALCULATION: 1 SPACE X (3,325 S.F. / 350 S.F.) REQUIRED = 10 SPACES

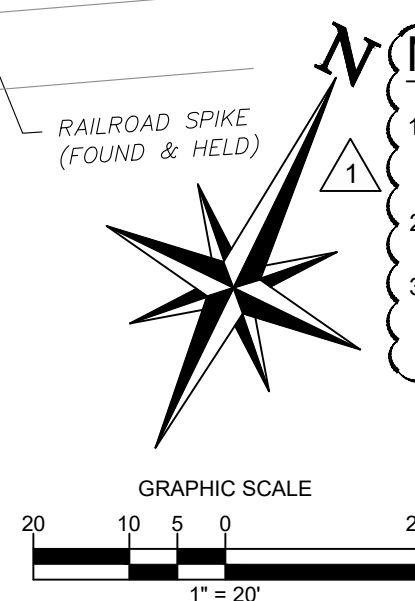
EXISTING PARKING SPACES: 61 SPACES (INCLUDING 3 ACCESSIBLE SPACES) PROPOSED PARKING SPACES: 31 SPACES (INCLUDING 2 ACCESSIBLE SPACES)

**PARKING DIMENSIONS**  
EXISTING: VARIES  
REQUIRED: 8.5' X 19'  
PROPOSED: 9' X 19'

4) ALL EXISTING FEATURES ARE TO REMAIN UNLESS OTHERWISE NOTED.  
5) ALL PAVEMENT MARKINGS SHALL BE LONG LIFE EPOXY.  
6) PRIOR TO STARTING CONSTRUCTION, THE CONTRACTOR SHALL BE RESPONSIBLE TO MAKE SURE THAT ALL REQUIRED PERMITS AND APPROVALS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED AND THOROUGHLY REVIEWED ALL PLANS AND OTHER DOCUMENTS BY ALL OF THE PERMITTING AUTHORITIES.  
7) ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS AND THE REQUIREMENTS AND STANDARDS OF THE LOCAL GOVERNING AUTHORITY.  
8) ALL DIMENSIONS SHOWN ON THE PLANS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER IF ANY DISCREPANCIES EXIST PRIOR TO PROCEEDING WITH CONSTRUCTION FOR NECESSARY PLAN CHANGES. NO EXTRA COMPENSATION SHALL BE PAID TO THE CONTRACTOR FOR WORK HAVING TO BE REDONE DUE TO DIMENSIONS OR GRADES SHOWN INCORRECTLY ON THESE PLANS IF SUCH NOTIFICATION HAS NOT BEEN GIVEN.  
9) SOLID WASTE TO BE DISPOSED OF BY CONTRACTOR IN ACCORDANCE WITH ALL LOCAL, STATE AND FEDERAL REGULATIONS.  
10) ALL EXCAVATED UNSUITABLE MATERIAL MUST BE TRANSPORTED TO AN APPROVED DISPOSAL LOCATION. CONTRACTOR IS RESPONSIBLE FOR ALL SHORING REQUIRED DURING EXCAVATION AND SHALL BE PERFORMED IN ACCORDANCE WITH CURRENT OSHA STANDARDS, AS WELL AS ADDITIONAL PROVISIONS TO ASSURE STABILITY OF CONTIGUOUS STRUCTURES, AS FIELD CONDITIONS DICTATE.

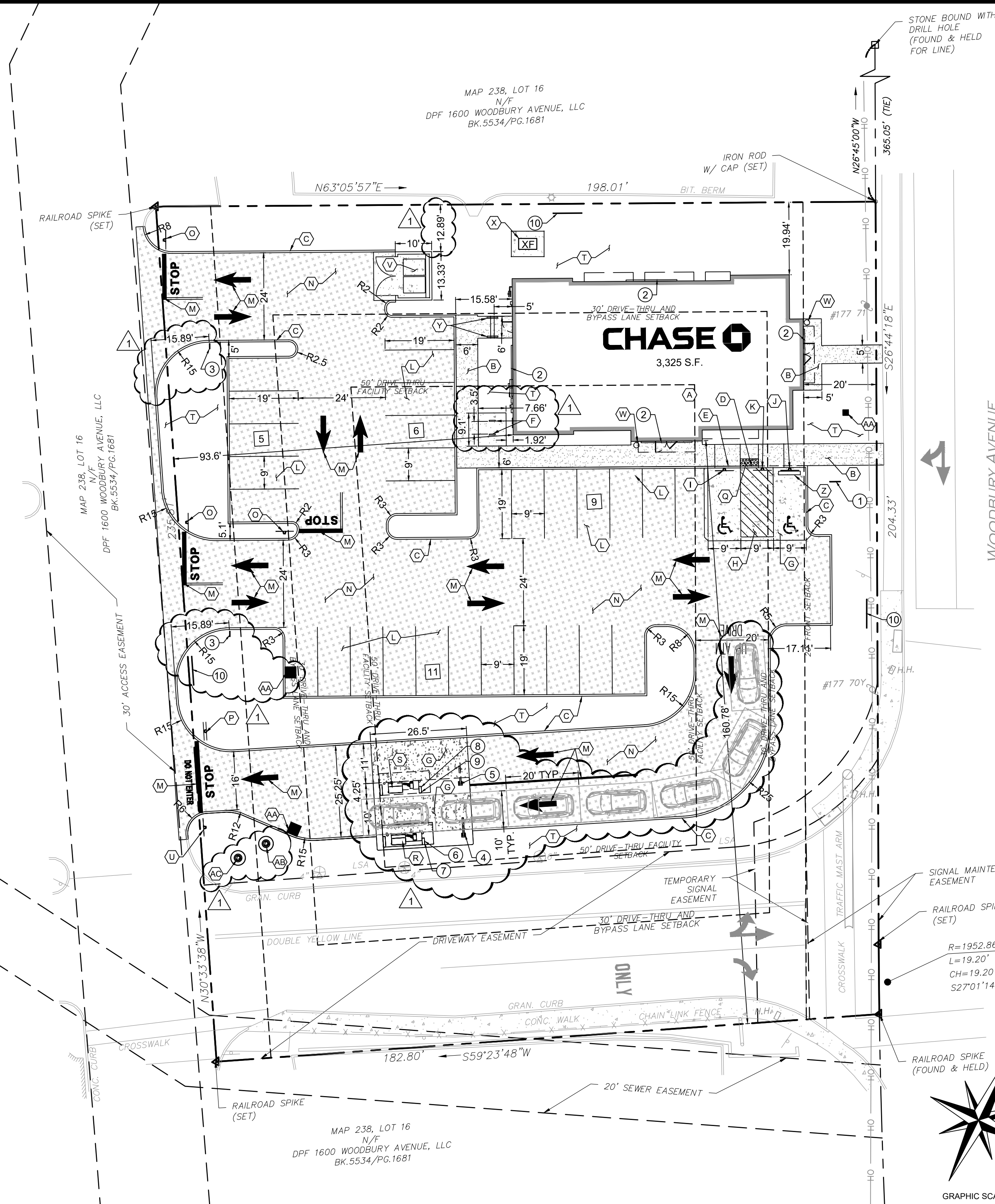
**NOTES:**

- ALL CONDITIONS ON THIS PLAN SHALL REMAIN IN EFFECT PERPETUITY PURSUANT TO THE REQUIREMENTS TO THE SITE PLAN REVIEW REGULATIONS.
- THIS SITE PLAN SHALL BE RECORDED IN THE ROCKINGHAM COUNTY REGISTRY OF DEEDS.
- ALL IMPROVEMENTS SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE PLAN BY THE OWNER AND ALL FUTURE OWNERS. NO CHANGES SHALL BE MADE TO THIS SITE PLAN WITHOUT THE EXPRESS APPROVAL FROM THE PORTSMOUTH PLANNING DIRECTOR.



| SIGNAGE TABLE (SIGN DISTRICT 5) |  |   |                                   |
|---------------------------------|--|---|-----------------------------------|
| <b>TEMPORARY SIGNS</b>          |  |   |                                   |
| PROVISION                       | REQUIRED   | PROPOSED  | COMMENT                           |
| MAXIMUM SIGN AREA               | 64 SF  | 60 SF   | COMPLIANT                         |
| MAXIMUM SIGN HEIGHT             | 12 FEET  | <12 FT (MOUNTED ON CONSTRUCTION FENCE)  | COMPLIANT                         |
| <b>FREESTANDING SIGN</b>        |  |   |                                   |
| MAXIMUM ALLOWED PER LOT         | 1  | 1 MONUMENT SIGN<br>1 ATM SIGN<br>1 FUTURE ATM SIGN  | COMPLIANT<br>VARIANCE<br>VARIANCE |
| MAXIMUM SIGN AREA               | 100 SF   | 56.2 SF<br>9.5 SF<br>9.5 SF   | COMPLIANT                         |
| MAXIMUM SETBACK FRONT LOT LINE  | 10 FT  | 10 FT   | COMPLIANT                         |
| <b>WALL SIGN</b>                |  |   |                                   |
| MAXIMUM ALLOWED                 | ONE PER STREET FRONTAGE AND/OR AT MAIN ENTRANCE                        | 1 EAST ELEVATION (WOODBURY AVE)<br>1 SOUTH ELEVATION<br>1 NORTH ELEVATION<br>1 WEST ELEVATION | COMPLIANT<br>VARIANCE<br>VARIANCE |
| MAXIMUM SIGN AREA               | 100 SF   | 36.9 SF   | COMPLIANT                         |
| <b>TOTAL AGGREGATE SIGNS</b>    |  |   |                                   |
| MAXIMUM AGGREGATE SIGN AREA     | 1.5 SF PER LINEAR FEET OF BUILDING FRONTAGE = 1.5 X 42.5 LF = 63.75 SF | 147.6 SF  | VARIANCE                          |

NOTE: A SEPARATE SIGN PERMIT WILL BE REQUIRED FOR ANY PROPOSED SIGNAGE LISTED IN THE SIGNAGE TABLE.



**SITE PLAN**  
SCALE 1" = 20'

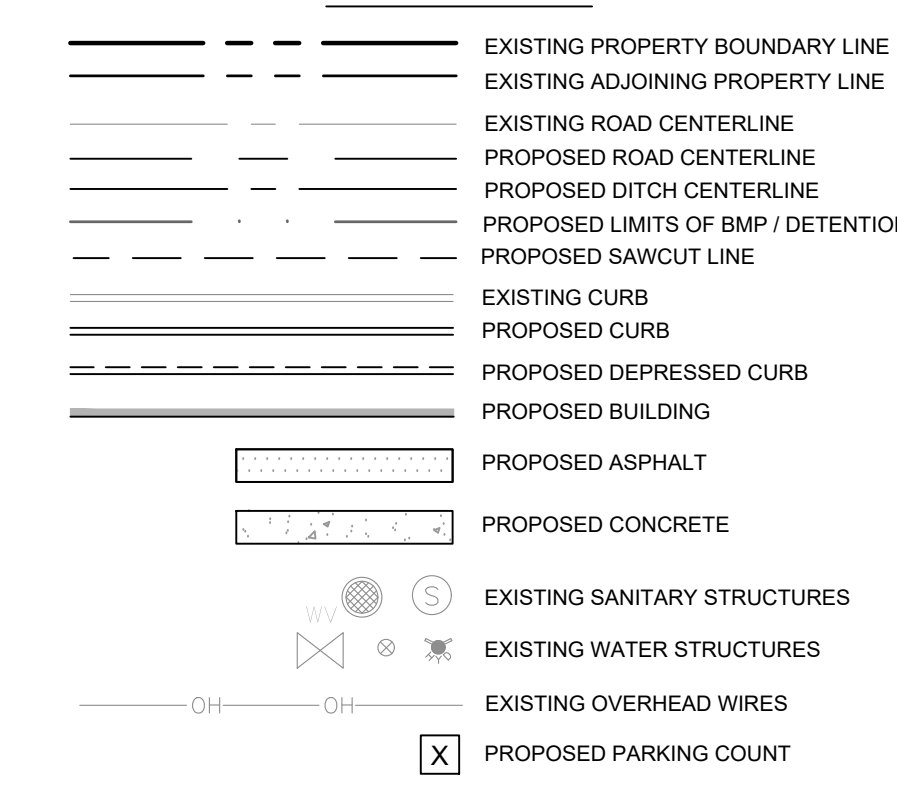
**SIGN KEY NOTES:**

- PROPOSED CHASE PROTOTYPICAL PYLON SIGN (56.2 S.F.).
- PROPOSED WHITE W/BLUE OCTAGON ILLUMINATED CHANNEL "CHASE" LOGO SIGN WALL SIGN (36.9 S.F.).
- PROPOSED DOUBLE FACED NON-ILLUMINATED DIRECTIONAL SIGN (2.3 S.F.).
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR.
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR FOR FUTURE DRIVE-UP ATM.
- PROPOSED SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- PROPOSED SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- FUTURE SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- FUTURE SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- TEMPORARY "COMING SOON" SIGN MOUNTED ON CONSTRUCTION FENCE (60S.F.).

**SITE KEY NOTES:**

- PROPOSED 3,325 S.F. JPMORGAN CHASE BANK BUILDING. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
- PROPOSED CONCRETE SIDEWALK. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED CONCRETE CURB. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED DEPRESSIONED CONCRETE CURB. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED TRANSITION CURB SECTION. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED BIKE RACK ON CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED 9' X 19' ACCESSIBLE PARKING SPACE AND AISLE WITH SYMBOLS OF ACCESSIBILITY. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED VAN ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED NO PARKING ANYTIME SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED 9' X 19' STANDARD PARKING SPACE.
- PROPOSED SITE MARKINGS. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED ASPHALT PAVEMENT. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED STOP SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED STOP & DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED DETECTABLE WARNING SURFACE. REFER TO CONSTRUCTION DETAILS SHEET CD-2.
- PROPOSED DRIVE-UP SIGNATURE ATM CANOPY. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
- PROPOSED "FUTURE" DRIVE-UP SIGNATURE ATM CANOPY. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
- PROPOSED LANDSCAPE AREA. REFER TO LANDSCAPE PLAN FOR DETAILS.
- PROPOSED DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED TRASH ENCLOSURE. REFER TO CONSTRUCTION DETAILS SHEET CD-5.
- PROPOSED TRASH BIN. REFER TO ARCHITECT PLANS FOR DETAIL.
- RELOCATED ELECTRIC TRANSFORMER AND CONCRETE PAD. REFER TO LIGHTING PLANS FOR DETAIL.
- PROPOSED HANDRAIL. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED WHEEL STOP. REFER TO CONSTRUCTION DETAILS SHEET CD-1.
- PROPOSED STORM DRAIN INLET. REFER TO CONSTRUCTION DETAILS SHEET CD-3.
- PROPOSED WATER QUALITY CDS UNIT BY CONTECH. REFER TO CONSTRUCTION DETAILS SHEET CD-2.
- PROPOSED STORM DOGHOUSE MANHOLE. REFER TO CONSTRUCTION DETAILS SHEET CD-2.

**SITE LEGEND**



**ALERT TO CONTRACTOR:**  
PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

**CORE STATES INC.**  
12700 HILLCREST ROAD DALLAS, TX 75220 (214) 377-5960 www.core-states.com

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**REVISIONS**

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

**DOCUMENT**  
**SITE PLAN APPROVAL FOR CHASE BANK**

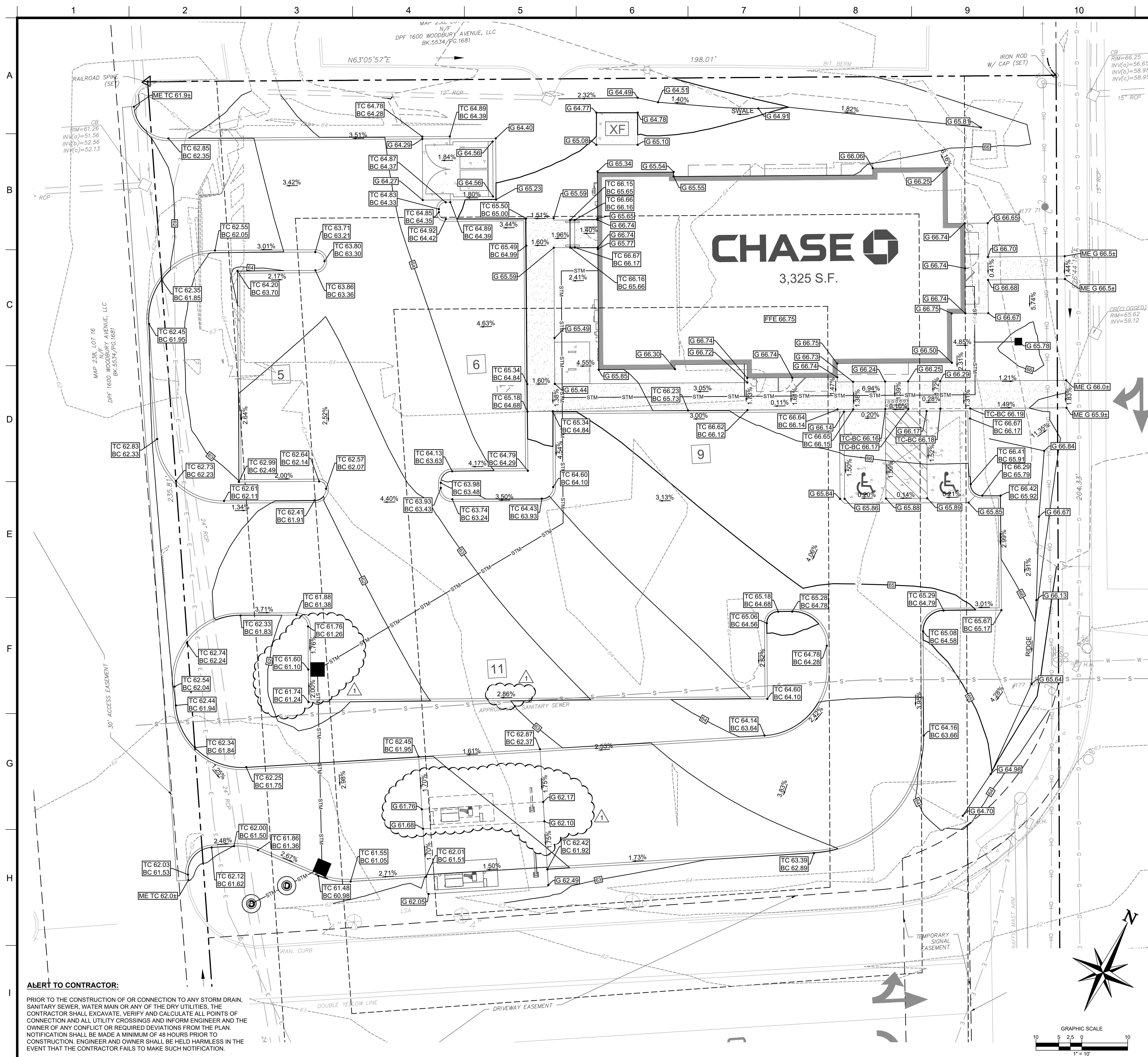
**SITE LOCATION**  
1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801

**ENGINEER SEAL**  
THOMAS C. PICKERING  
No. 10218  
PROFESSIONAL ENGINEER  
08/2019-2020

**SHEET TITLE**  
**SITE PLAN**

**JOB #:** JPM 27086  
**DATE:** 07/01/2020  
**SCALE:** AS NOTED  
**DRAWN BY:** MAL  
**CHECKED BY:** KGF

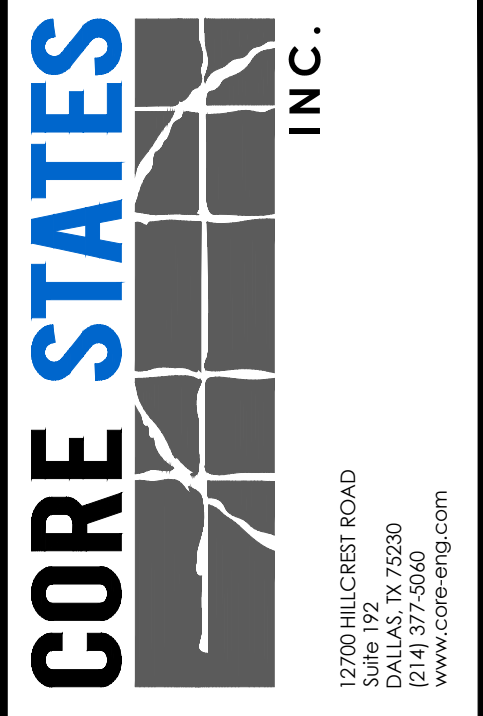
**SHEET NO.**  
**C-2**



**GENERAL NOTES:**

- THIS PROJECT REFERENCES A SURVEY PREPARED BY:  
EXISTING CONDITIONS SURVEY  
CHASE BANK SITE  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH  
ALLEN & MAJOR ASSOCIATES, INC.  
DATED 06/23/2020
- GRADING NOTES:**  
1. ALL PAVING AND GRADING CONSTRUCTION MATERIALS AND METHODS SHALL MEET THE STANDARD SPECIFICATIONS AND REQUIREMENTS OF THE TOWNSHIP.  
2. CONTRACTOR IS RESPONSIBLE FOR DEMOLITION OF EXISTING STRUCTURES INCLUDING REMOVAL OF ANY EXISTING UTILITIES SERVING THE STRUCTURE. UTILITIES ARE TO BE REMOVED TO THE RIGHT-OF-WAY.  
3. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.  
4. ALL CUT OR FILL SLOPES SHALL BE 4:1 OR FLATTER UNLESS OTHERWISE NOTED.  
5. PRECAST STRUCTURES ARE REQUIRED.  
6. STORM PIPES TO BE CLEANED OUT TO REMOVE ALL SILT AND DEBRIS. PRIOR TO FINAL INSPECTION.  
7. EXISTING CONTOUR INTERVALS SHOWN AT 1.0 FOOT.  
8. PROPOSED CONTOUR INTERVALS SHOWN AT 1.0 FOOT.  
9. IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO REPAIR AND/OR REPLACE THE EXISTING STRUCTURE AS NECESSARY TO RETURN IT TO EXISTING CONDITIONS OR BETTER.  
10. ALL STORM PIPE ENTERING STRUCTURES SHALL BE GROUTED TO ASSURE CONNECTION AT STRUCTURE IS WATERTIGHT.  
11. ALL STORM SEWER MANHOLES IN PAVED AREAS SHALL BE FLUSH WITH PAVEMENT, AND SHALL HAVE TRAFFIC BEARING RING & COVERS. MANHOLES IN UNPAVED AREAS SHALL BE 2" ABOVE FINISH GRADE. LIDS SHALL BE LABELED "STORM SEWER".  
12. THE CONTRACTOR SHALL ADHERE TO ALL TERMS & CONDITIONS AS OUTLINED IN THE APPLICABLE STATE GENERAL PERMIT FOR STORMWATER DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTIVITIES.  
13. CONTRACTOR SHALL ADJUST AND/OR CUT EXISTING PAVEMENT AS NECESSARY TO ASSURE A SMOOTH FIT AND CONTINUOUS GRADE.  
14. CONTRACTOR SHALL ASSURE POSITIVE DRAINAGE AWAY FROM BUILDINGS FOR ALL NATURAL AND PAVED AREAS.  
15. TOPOGRAPHIC INFORMATION IS TAKEN FROM A TOPOGRAPHIC SURVEY BY LAND SURVEYORS. IF THE CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, THEN THE CONTRACTOR SHALL SUPPLY, AT THEIR EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED LAND SURVEYOR TO THE OWNER FOR REVIEW.  
16. ALL UNSURFACED AREAS DISTURBED BY GRADING OPERATION SHALL RECEIVE 4 INCHES OF TOPSOIL. CONTRACTOR SHALL APPLY STABILIZATION FABRIC TO ALL SLOPES 3H:1V OR STEEPER. CONTRACTOR SHALL STABILIZE DISTURBED AREAS IN ACCORDANCE WITH GOVERNING SPECIFICATIONS UNTIL A HEALTHY STAND OF VEGETATION IS OBTAINED.  
17. CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE GOVERNING CODES AND BE CONSTRUCTED TO SAME.  
18. ALL STORM STRUCTURES SHALL HAVE A SMOOTH UNIFORM POURED MORTAR INVERT FROM INVERT IN TO INVERT OUT, UNLESS OTHERWISE NOTED.  
19. CONTRACTOR TO MAINTAIN 1.5% MAXIMUM CROSS-SLOPE ON ALL SIDEWALKS AND CROSSWALKS. CONTRACTOR TO MODIFY PAVEMENT GRADES AS NECESSARY TO MAINTAIN MAXIMUM CROSS-SLOPE IN CROSSWALKS.  
20. CONTRACTOR TO PROVIDE POSITIVE DRAINAGE AWAY FROM ALL STRUCTURES WITH 2% MINIMUM SLOPE IN PAVEMENT AREAS AND 1.5% MINIMUM SLOPE IN UNPAVED AREAS UNLESS OTHERWISE NOTED.  
21. FOLLOWING DEMOLITION, CLEARING/GRUBBING OPERATIONS, DESIGN CUTS, AND ADDITIONAL CUTTING TO REMOVE UNSUITABLE CONDITIONS AND PRIOR TO FILL PLACEMENT AND NEW CONSTRUCTION, THE ENTIRE SITE SHOULD BE PROOF-COMPACTED WITH MULTIPLE PERPENDICULAR PASSES OF A LARGE (15-TON STATIC WEIGHT) VIBRATORY ROLLER TO COMPACT LOOSE, NEAR SURFACE SOILS. HOWEVER, DEPENDING ON THE GROUNDWATER LEVEL AT THE TIME OF CONSTRUCTION, IT MAY BE NECESSARY TO OPERATE THE ROLLER IN STATIC MODE IN ORDER TO MINIMIZE DRAWING WATER UP THROUGH THESE TYPICALLY GRANULAR SOIL TYPES. LOOSE, SOFT, WET OR UNSTABLE SOILS IDENTIFIED DURING THE PROOF-COMPACTATION SHOULD BE EXCAVATED TO AN ACCEPTABLE BEARING STRATUM AS DETERMINED BY A PSI REPRESENTATIVE. IF PRESENT, ABANDONED FOUNDATIONS, SLABS AND UTILITIES SHOULD BE REMOVED ENTIRELY BELOW THE PROPOSED BUILDING AND PAVEMENT AREAS. ANY REQUIRED BACKFILL OR NEW FILL SHOULD COMPLY WITH SECTION 3.3 STRUCTURAL FILL MATERIAL AND PLACEMENT. THE PLACEMENT OF A GEOTEXTILE AND/OR COARSE GRADED STONE MAY BE REQUIRED TO STABILIZE THE UNDERCUT  
22. CONTRACTOR TO ADD WATERPROOFING AT ALL LOCATIONS WHERE THE EXTERIOR GRADE IS PROPOSED ABOVE THE FINISHED FLOOR ELEVATION. CONTRACTOR TO COORDINATE WITH ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

**ALERT TO CONTRACTOR:**  
PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.



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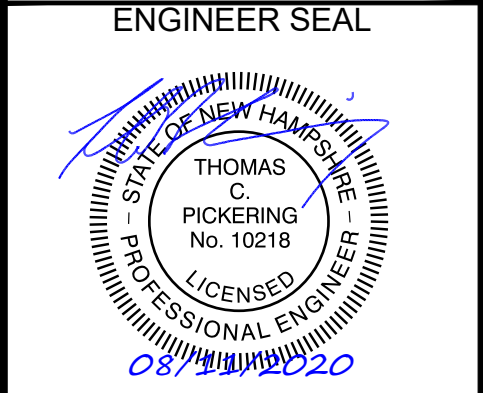
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**REVISIONS**

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

**DOCUMENT**

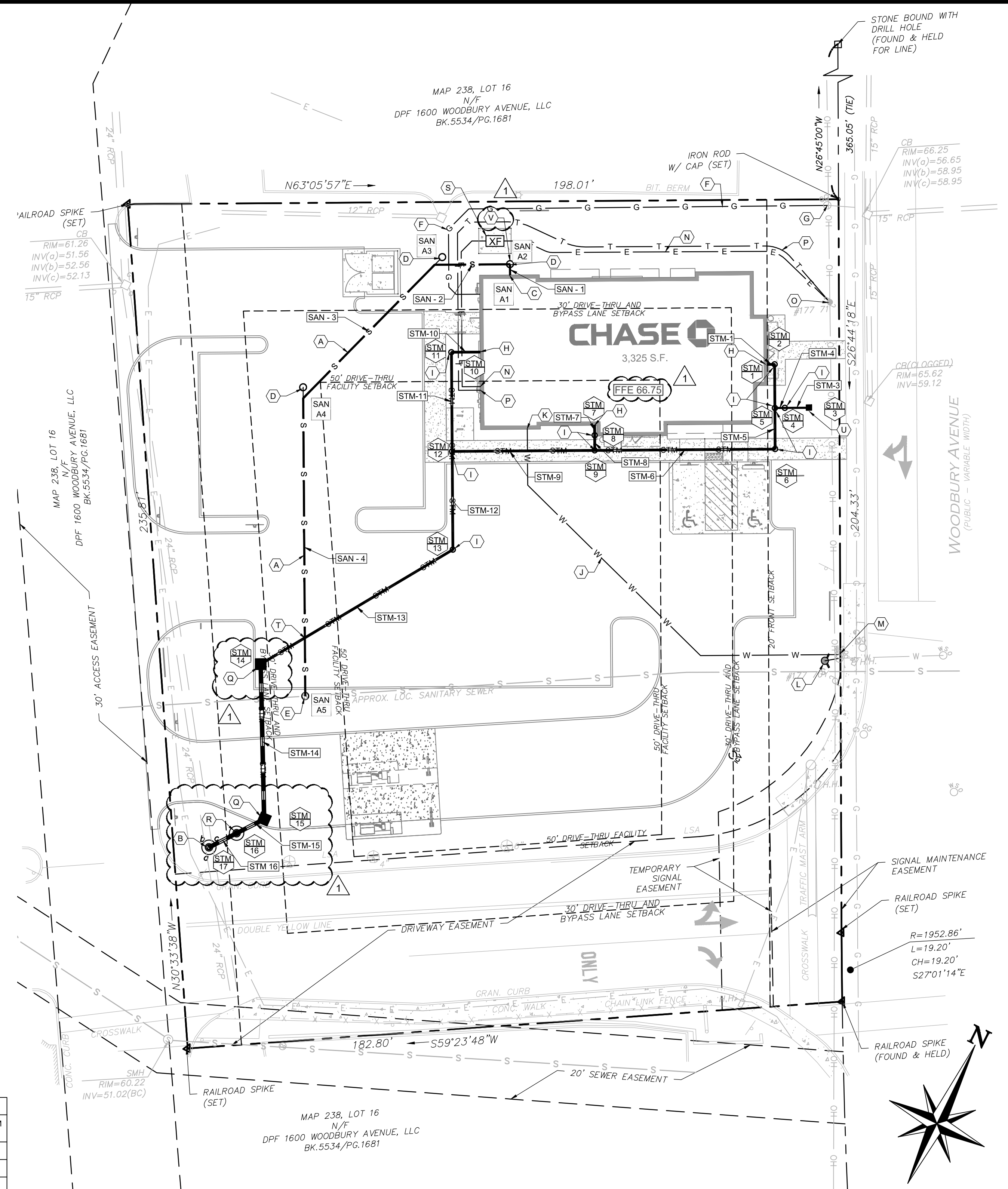
**SITE PLAN APPROVAL FOR CHASE BANK**  
  
**SITE LOCATION**  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801



**SHEET TITLE**  
**GRADING PLAN**

JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF

SHEET NO.  
**C-3**



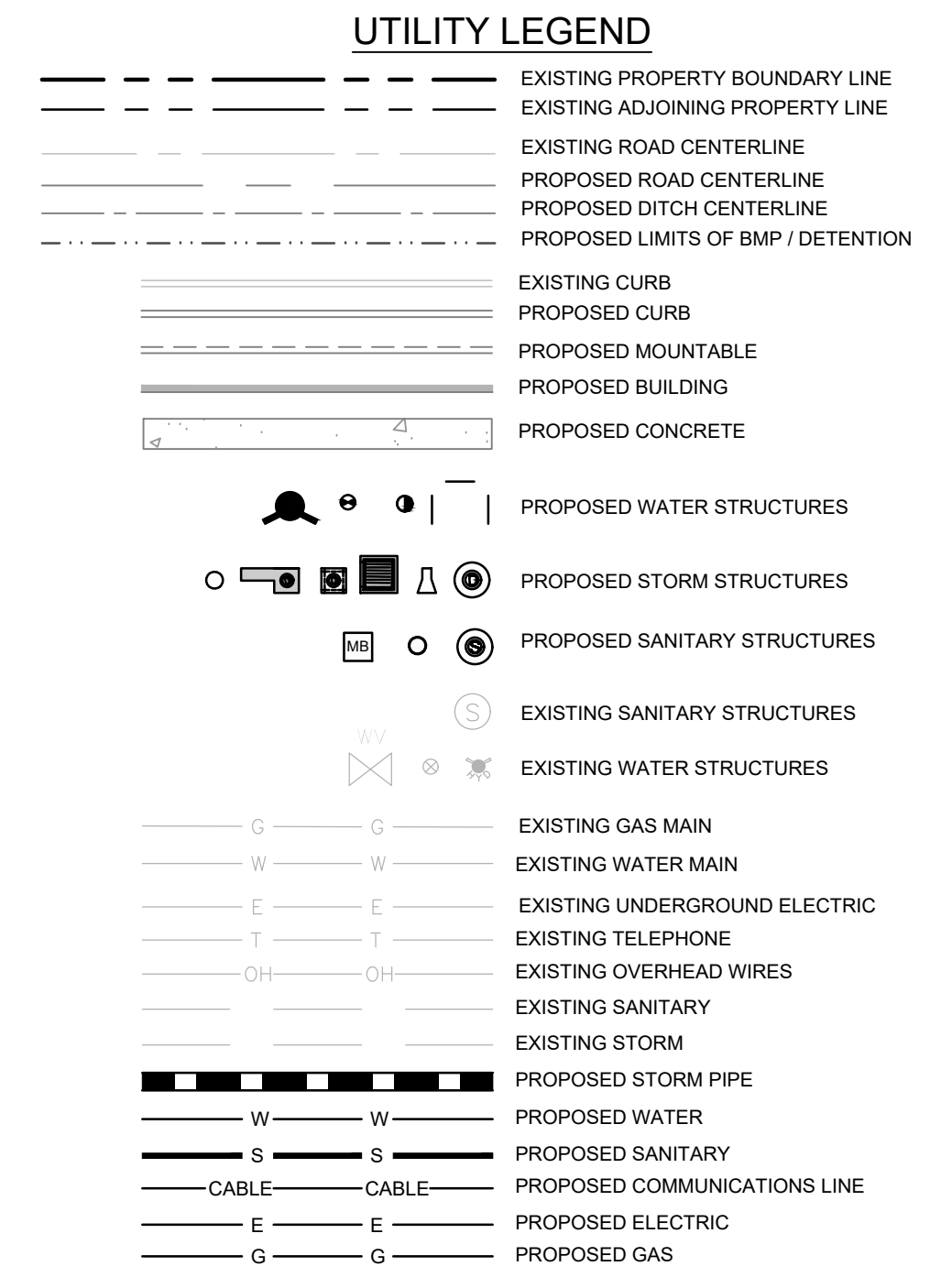
- GENERAL NOTES:**
- THIS PROJECT REFERENCES A SURVEY PREPARED BY:  
EXISTING CONDITIONS SURVEY  
CHASE BANK SITE  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH  
ALLEN & MAJOR ASSOCIATES, INC.  
DATED 06/23/2020
- UTILITY KEY NOTES:**
- PROPOSED 6" SDR 26 PVC SANITARY SEWER LINE AT A MINIMUM OF 2.0% SLOPE.
  - PROPOSED STORM DOGHOUSE MANHOLE. REFER TO CONSTRUCTION DETAILS SHEET CD-2.
  - PROPOSED SANITARY LATERAL CONNECTION TO PROPOSED BUILDING. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
  - PROPOSED SANITARY SEWER CLEANOUT. REFER TO CONSTRUCTION DETAILS SHEET CD-2.
  - PROPOSED WYE CONNECTION TO EXISTING SEWER MAIN. CONTRACTOR TO VERIFY LOCATION PRIOR TO COMMENCING OF PROPOSED WORK.
  - PROPOSED GAS SERVICE LINE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO INSTALLATION.
  - PROPOSED GAS CONNECTION TO EXISTING GAS VALVE. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY TO VERIFY IF EXISTING CONNECTION IS ADEQUATE.
  - PROPOSED 6" PVC UNDERGROUND ROOF DRAIN.
  - PROPOSED STORM DRAIN CLEAN OUT.
  - PROPOSED 1-1/2" WATER SERVICE LINE.
  - PROPOSED WATER SERVICE CONNECTION TO BUILDING.
  - PROPOSED FIRE HYDRANT TO BE INSTALLED PER TOWNSHIP REQUEST.
  - PROPOSED WATER SERVICE CONNECTION TO EXISTING WATER VALVE. CONTRACTOR TO DETERMINE SIZE OF EXISTING SERVICE AND COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING OF WORK.
  - PROPOSED ELECTRICAL CONDUITS.
  - PROPOSED ELECTRICAL POINT OF CONNECTION AT EXISTING POWER POLE. CONTRACTOR TO VERIFY IF SERVICE PROVIDED AT POWER POLE IS ADEQUATE FOR THE PROPOSED BUILDING. CONTRACTOR TO COORDINATE WITH UTILITY COMPANY PRIOR TO COMMENCING ANY WORK.
  - PROPOSED DATA CONDUITS.
  - PROPOSED STORM DRAIN INLET. REFER TO CONSTRUCTION DETAILS SHEET CD-3.
  - PROPOSED WATER QUALITY CDS UNIT BY CONTECH. REFER TO CONSTRUCTION DETAILS SHEET CD-2.
  - PROPOSED ELECTRICAL TRANSFORMER AND CONCRETE PAD. REFER TO LIGHTING PLANS FOR DETAIL.
  - PROPOSED SANITARY AND STORM CROSSING.  
TOP OF SANITARY PIPE = 57.34'  
BOTTOM OF STORM PIPE = 58.97'
  - PROPOSED 12-INCH YARD DRAIN. REFER TO CONSTRUCTION DETAILS SHEET CD-3.
  - PROPOSED SANITARY VERTICAL DROP CONNECTION TO BUILDING. SEE DETAIL ON SHEET CD-2.

| SANITARY STRUCTURE TABLE |  |       |                    |                   |                      |                     |                        |                               |                       |
|--------------------------|--|-------|--------------------|-------------------|----------------------|---------------------|------------------------|-------------------------------|-----------------------|
| STRUCTURE NAME           | STRUCTURE TYPE                           | RIM   | UPSTREAM PIPE NAME | UPSTREAM PIPE INV | DOWNSTREAM PIPE NAME | DOWNSTREAM PIPE INV | DOWNSTREAM PIPE LENGTH | DOWNSTREAM PIPE SIZE AND TYPE | DOWNSTREAM PIPE SLOPE |
| A1                       | BUILDING STUB                            | 65.45 |                    |                   | SAN - 1              | 63.75'              | 3.00'                  | 6" SDR 26 PVC                 | 2.00%                 |
| A2                       | SANITARY VERTICAL DROP CONNECTION DETAIL | 65.27 | SAN - 1            | 63.69'            | SAN - 2              | 59.62'              | 20.98'                 | 6" SDR 26 PVC                 | 2.00%                 |
| A3                       | 45° BEND                                 | 64.87 | SAN - 2            | 59.20'            | SAN - 3              | 59.20'              | 52.17'                 | 6" SDR 26 PVC                 | 2.00%                 |
| A4                       | 45° BEND                                 | 63.37 | SAN - 3            | 58.16'            | SAN - 4              | 58.16'              | 83.03'                 | 6" SDR 26 PVC                 | 2.00%                 |
| A5                       | EXISTING LATERAL TIE-IN                  | 62.19 | SAN - 4            | 56.50'            |                      |                     |                        |                               |                       |

| STORM STRUCTURE TABLE |                       |       |                    |                   |                      |                     |                        |                               |                       |
|-----------------------|-----------------------|-------|--------------------|-------------------|----------------------|---------------------|------------------------|-------------------------------|-----------------------|
| STRUCTURE NAME        | STRUCTURE TYPE        | RIM   | UPSTREAM PIPE NAME | UPSTREAM PIPE INV | DOWNSTREAM PIPE NAME | DOWNSTREAM PIPE INV | DOWNSTREAM PIPE LENGTH | DOWNSTREAM PIPE SIZE AND TYPE | DOWNSTREAM PIPE SLOPE |
| 1                     | BUILDING STUB         | 66.74 |                    |                   | STM-1                | 63.54'              | 2.02'                  | 6" PVC                        | 8.64%                 |
| 2                     | CO                    | 66.72 | STM-1              | 63.37'            | STM-2                | 63.37'              | 12.15'                 | 6" PVC                        | 0.89%                 |
| 3                     | PROPOSED AREA DRAIN   | 65.78 |                    |                   | STM-3                | 64.50'              | 6.85'                  | 4" PVC                        | 1.00%                 |
| 4                     | CLEANOUT              | 66.22 | STM-3              | 64.43'            | STM-4                | 64.43'              | 2.79'                  | 4" PVC                        | 42.19%                |
| 5                     | WYE BEND              | 66.38 | STM-2              | 63.26'            | STM-5                | 63.26'              | 11.46'                 | 6" PVC                        | 2.96%                 |
| 6                     | CO                    | 66.21 | STM-5              | 62.92'            | STM-6                | 62.92'              | 50.25'                 | 6" PVC                        | 2.50%                 |
| 7                     | BUILDING STUB         | 66.74 |                    |                   | STM-7                | 63.22'              | 2.82'                  | 6" PVC                        | 2.50%                 |
| 8                     | CO                    | 66.72 | STM-7              | 63.15'            | STM-8                | 63.15'              | 4.21'                  | 6" PVC                        | 35.45%                |
| 9                     | CO                    | 66.65 | STM-8              | 61.66'            | STM-9                | 61.66'              | 39.74'                 | 6" PVC                        | 2.50%                 |
| 10                    | BUILDING STUB         | 65.78 |                    |                   | STM-10               | 62.49'              | 7.99'                  | 6" PVC                        | 2.50%                 |
| 11                    | CO                    | 65.60 | STM-10             | 62.29'            | STM-11               | 62.29'              | 27.61'                 | 6" PVC                        | 5.86%                 |
| 12                    | CO                    | 65.45 | STM-9              | 60.67'            | STM-12               | 60.67'              | 27.38'                 | 6" PVC                        | 2.50%                 |
| 13                    | CO                    | 63.92 | STM-12             | 59.98'            | STM-13               | 59.98'              | 62.32'                 | 6" PVC                        | 2.09%                 |
| 14                    | PROP CURB INLET       | 61.21 | STM-13             | 58.68'            | STM-14               | 57.68'              | 43.18'                 | 12" HDPE                      | 1.32%                 |
| 15                    | PROP CURB INLET       | 61.07 | STM-14             | 57.11'            | STM-15               | 57.11'              | 8.85'                  | 12" HDPE                      | 1.50%                 |
| 16                    | PROP CDS UNIT         | 61.58 | STM-15             | 56.98'            | STM-16               | 56.98'              | 8.85'                  | 12" HDPE                      | 1.50%                 |
| 17                    | PROP DOGHOUSE MANHOLE | 61.13 | STM-16             | 56.85'            |                      |                     |                        |                               |                       |

**UTILITY PLAN**  
SCALE 1" = 20'

- UTILITY NOTES:**
- THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND / OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED UPON AS BEING COMPLETE OR EXACT. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.



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DALLAS, TX 75250  
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CLIENT

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| REVISIONS |           |                  |     |
|-----------|-----------|------------------|-----|
| REV       | DATE      | COMMENT          | BY  |
| 1         | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT

SITE PLAN  
APPROVAL FOR  
CHASE BANK

SITE LOCATION

1574 WOODBURY  
AVENUE,  
PORTSMOUTH, NH  
03801

ENGINEER SEAL

SHEET TITLE

UTILITY PLAN

JOB #:

JPM 27086

DATE:

07/01/2020

SCALE:

AS NOTED

DRAWN BY:

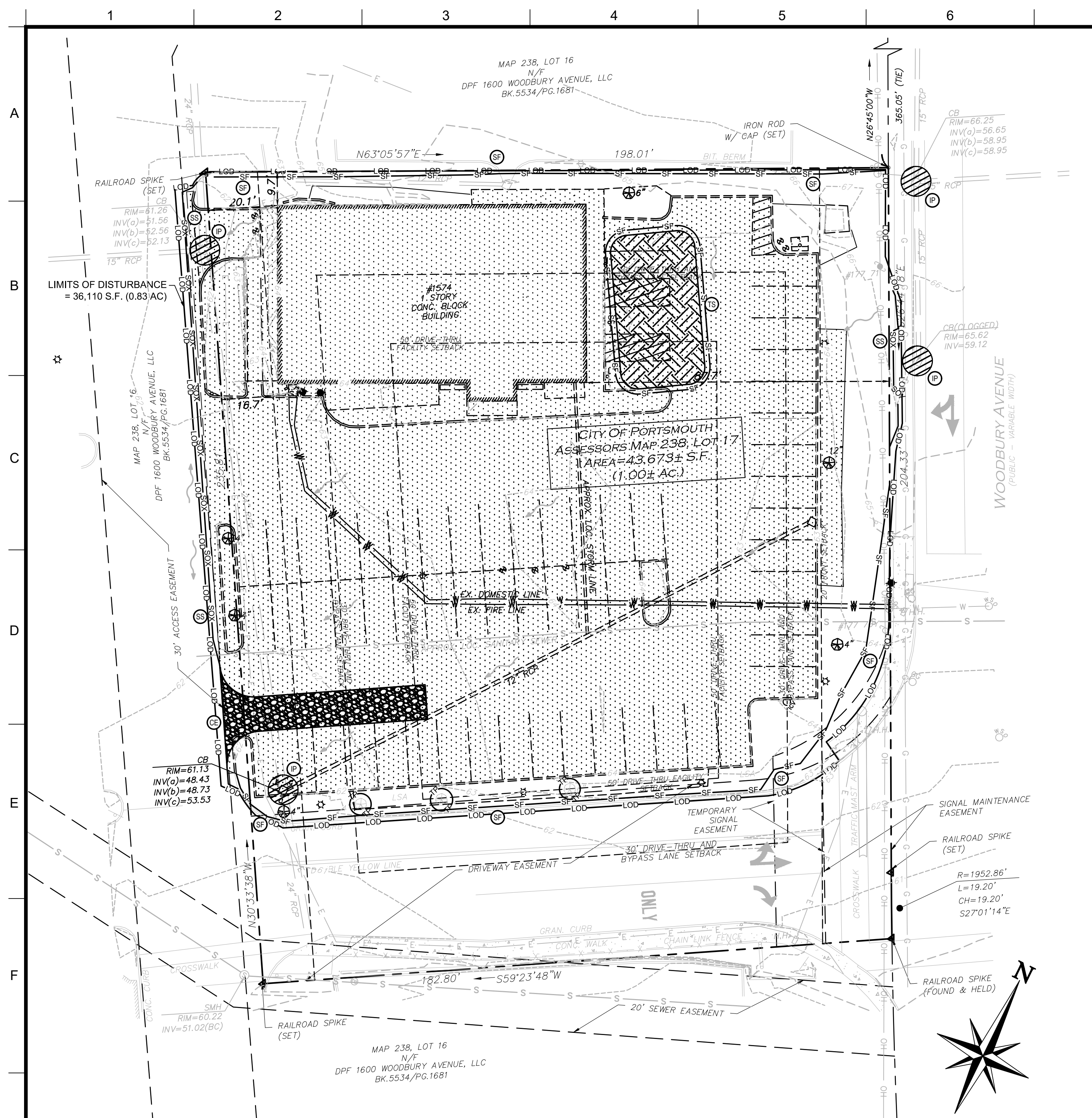
MAL

CHECKED BY:

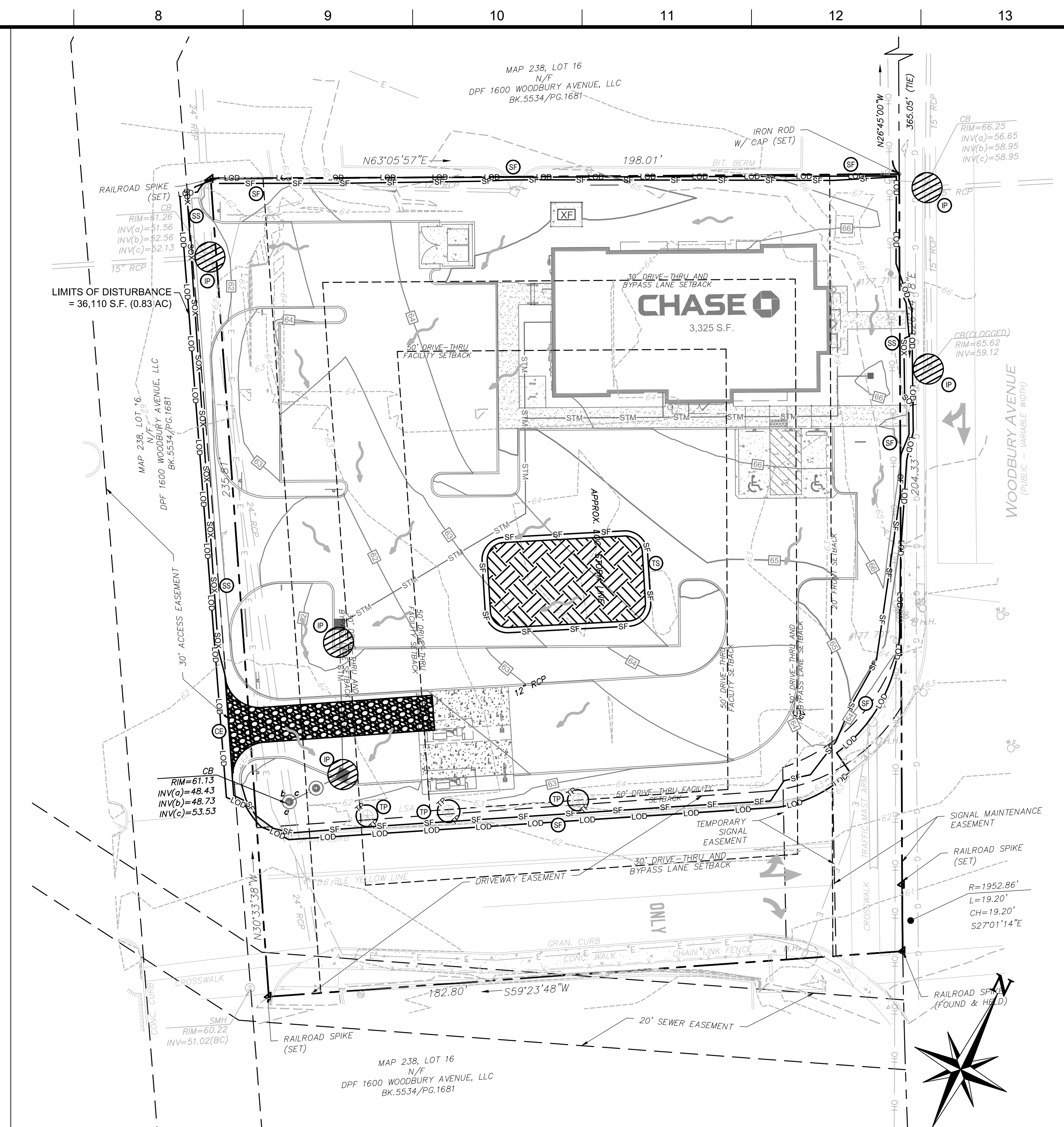
KGF

SHEET NO.

C-4

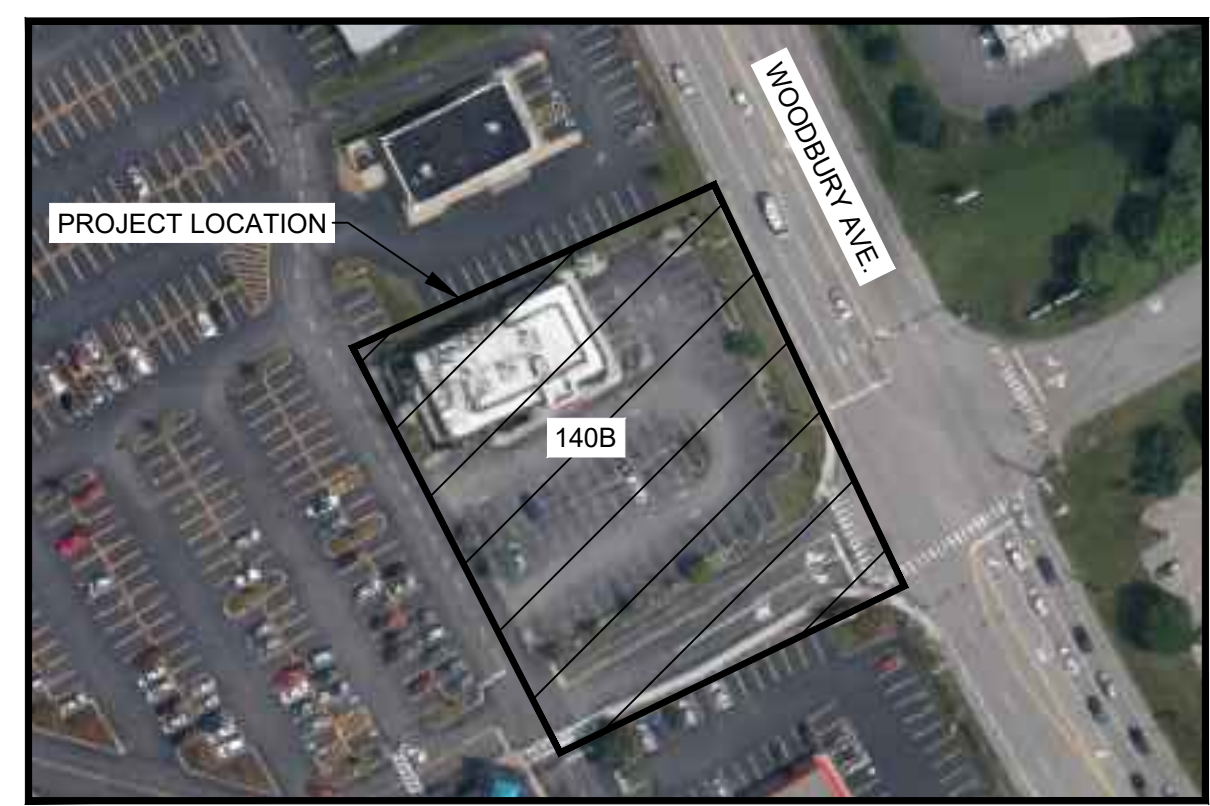


**EROSION & SEDIMENT CONTROL PLAN PHASE I**  
SCALE 1" = 20'



**EROSION AND SEDIMENT CONTROL PLAN PHASE II**  
SCALE 1" = 20'

| SOIL SUITABILITY AND CHARACTERISTICS |   |                       |   |
|--------------------------------------|---|-----------------------|---|
| DESIGNATION                          | SOIL  | HYDROLOGIC SOIL GROUP | DESCRIPTION   |
| 140B                                 | CHATFIELD-HOLLIS-CANTON COMPLEX, 0 TO 8 PERCENT SLOPES, ROCKY | B                     | CHATFIELD, VERY STONY, AND SIMILAR SOILS: 35 PERCENT HOLLIS, VERY STONY, AND SIMILAR SOILS: 25 PERCENT CANTON, VERY STONY, AND SIMILAR SOILS: 25 PERCENT MINOR COMPONENTS: 15 PERCENT |



**SOILS MAP**  
1" = 100'

**GENERAL NOTES:**  
1. THIS PROJECT REFERENCES A SURVEY PREPARED BY:  
EXISTING CONDITIONS SURVEY  
CHASE BANK SITE  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH  
ALLEN & MAJOR ASSOCIATES, INC.  
DATED 06/23/2020

**ALERT TO CONTRACTOR:**  
PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

**SEQUENCE OF CONSTRUCTION:**

- PHASE I**
- INSTALL TEMPORARY CONSTRUCTION ENTRANCE, TEMPORARY INLET PROTECTION(S), SILT SOXX AND SILT FENCE.
  - DEMOLISH EXISTING SITE FEATURES TO INSTALL ROCK CONSTRUCTION ENTRANCE.
  - BEGIN REMOVAL OF EXISTING BUILDING AND ON-SITE FOUNDATIONS.
  - BEGIN DEMOLITION ACTIVITIES HARDSCAPE REMOVAL AND EXISTING UTILITIES DEMOLITION.
  - BEGIN CONSTRUCTION OF NEW CHASE BANK BUILDING.
- PHASE II**
- TEMPORARILY SEED OR MULCH, THROUGHOUT CONSTRUCTION, DENUDED AREAS THAT WILL BE INACTIVE FOR 7 DAYS OR MORE.
  - INSTALL SUBSURFACE UTILITIES PROPOSED TO SERVE NEW CHASE BANK BUILDING AND PROPOSED STORM DRAINAGE.
  - CONTINUE WITH BALANCE OF EARTHWORK ACTIVITIES AND COMPLETE ROUGH GRADING TO ACHIEVE GRADES PROPOSED ON PLANS.
  - INSTALL CURB AND PAVEMENT SUBBASE.
  - PERMANENTLY STABILIZE AREAS TO BE VEGETATED AS THEY ARE BROUGHT TO FINAL GRADE.
  - PREPARE SITE FOR PAVING.
  - PAVE SITE AND INSTALL SIGNAGE.
  - COMPLETE GRADING AND INSTALLATION OF PERMANENT STABILIZATION OVER ALL AREAS.
  - CALL ENGINEER OF RECORD AFTER THE SITE APPEARS TO BE FULLY STABILIZED FOR AN INSPECTION.
  - REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES AFTER APPROVAL OF THE ENGINEER OF RECORD AND STABILIZE ANY AREAS DISTURBED BY THE REMOVAL OF THE BMP.
  - CONTINUE DAILY INSPECTION REPORTS UNTIL THE FINAL DAILY INSPECTION IS SIGNED OFF BY THE OWNER THAT THE SITE IS FULLY STABILIZED AND THE PERMIT MAY BE TERMINATED.

NOTE: THE GENERAL CONTRACTOR MAY COMPLETE CONSTRUCTION-RELATED ACTIVITIES CONCURRENTLY ONLY IF ALL PRECEDING BMPS HAVE BEEN COMPLETELY INSTALLED.

**MAINTENANCE NOTES:**

- ALL MEASURES STATED ON THIS PLAN SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR A COMPLETED PHASE OF WORK OR FINAL STABILIZATION OF THE SITE. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR THE APPLICABLE PERMIT, WHICHEVER IS MORE STRINGENT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:
- INLET PROTECTION SHALL BE REPAIRED TO THEIR ORIGINAL CONDITION IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE INLET PROTECTION WHEN CLOGGING BECOMES APPARENT.
  - SILT SOXX AND SILT FENCE SHALL BE REPAIRED OR REPLACED TO THEIR ORIGINAL CONDITION IF DAMAGED.
  - ALL CONSTRUCTION VEHICLES SHALL ENTER AND EXIT THROUGH THE CONSTRUCTION ENTRANCE TO BE INSTALLED FOR THE DURATION OF CONSTRUCTION ACTIVITIES.

**E&S LEGEND**

- SOX SILT SOXX
- SF SILT FENCE
- TP TREE PROTECTION
- IP INLET PROTECTION
- TS TOPSOIL STOCKPILE
- CE CONSTRUCTION ENTRANCE
- LOD LIMITS OF DISTURBANCE

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12700 HILLCREST ROAD  
DALLAS, TX 75220  
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CLIENT  
**CHASE**

811  
Know what's below. Call before you dig.  
THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND DEPTH OF EXISTING UTILITIES SHOWN ON THESE PLANS IS BASED ON RECORD DRAWINGS AND FIELD SURVEY. CORE STATES, INC. DOES NOT GUARANTEE THE LOCATION AND DEPTH OF UTILITIES. THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF UTILITIES PRIOR TO CONSTRUCTION OF ANY STRUCTURE OR INSTALLATION OF ANY EQUIPMENT.

**REVISIONS**

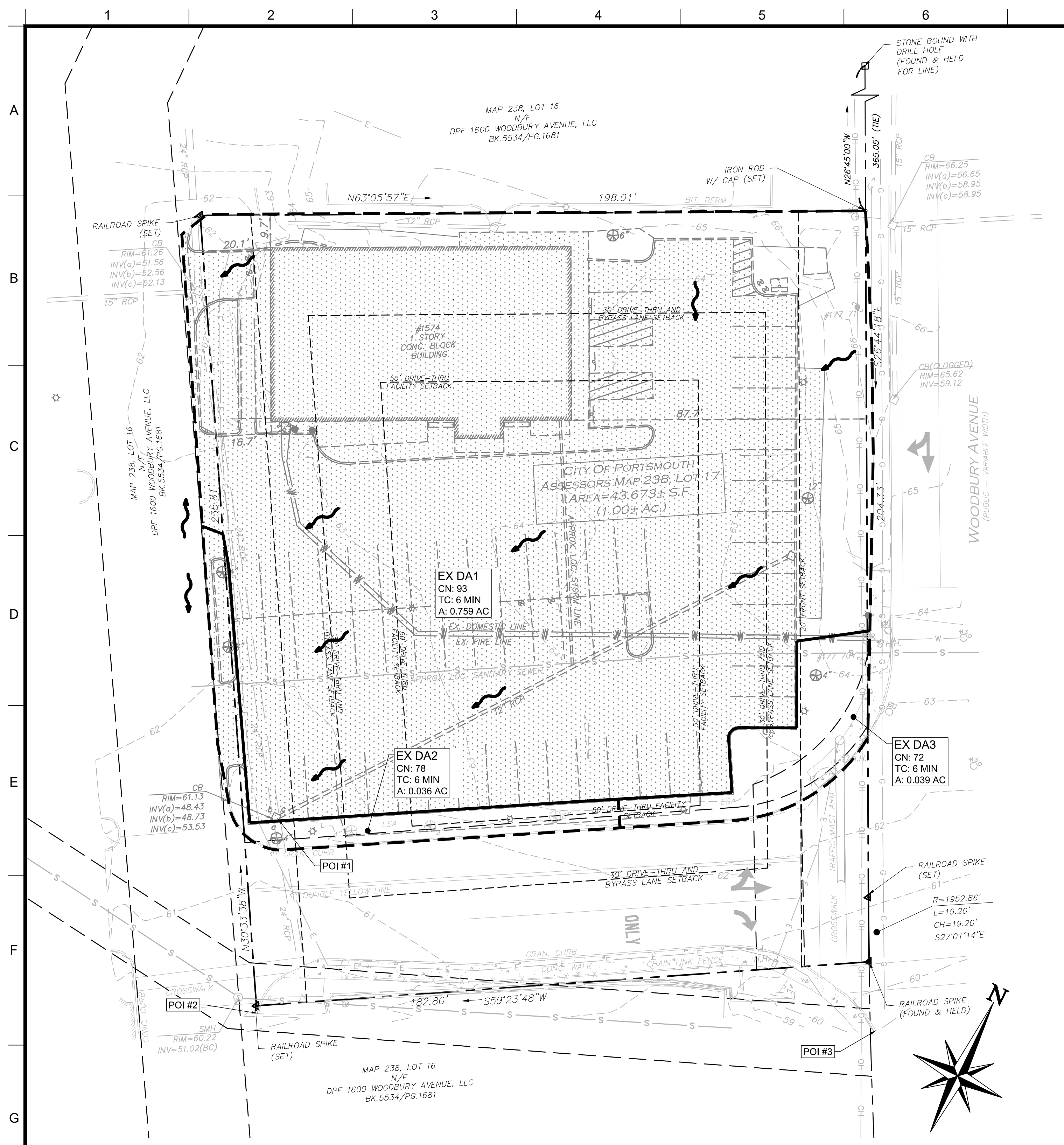
| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT  
**SITE PLAN APPROVAL FOR CHASE BANK**

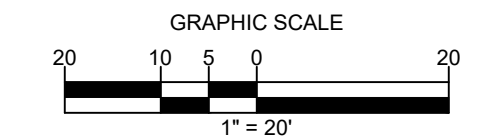
**SITE LOCATION**  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

ENGINEER SEAL  
THOMAS C. PICKERING  
No. 10218  
PROFESSIONAL ENGINEER  
08/10/2020

SHEET TITLE  
**SOIL EROSION AND SEDIMENT CONTROL**  
JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF  
SHEET NO.  
**C-5**



**PRE-DEVELOPMENT DRAINAGE AREA MAP**  
SCALE 1" = 20'



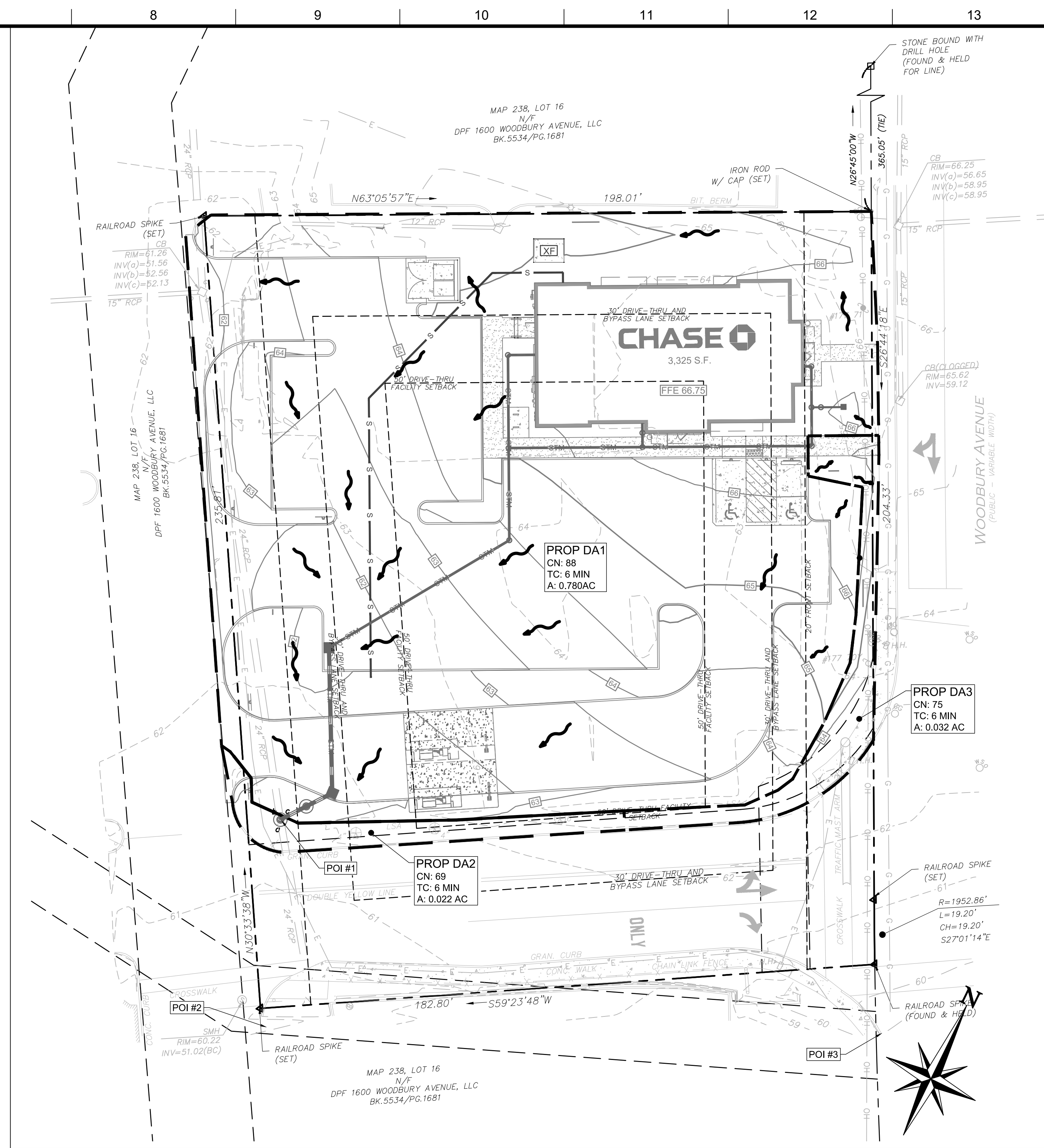
|                              |  |
|------------------------------|--|
| <b>EX-DA 1</b>               | IMPERVIOUS (CN 98) = 27,143 SQUARE FEET<br>PERVIOUS (CN 69) = 5,919 SQUARE FEET<br>TOTAL (CN 93) = 33,062 SQUARE FEET (0.759 AC) |
| <b>EX-DA 2 (BYPASS)</b>      | IMPERVIOUS (CN 98) = 495 SQUARE FEET<br>PERVIOUS (CN 69) = 1,064 SQUARE FEET<br>TOTAL (CN 78) = 1,559 SQUARE FEET (0.036 AC)     |
| <b>EX-DA 3 (BYPASS PROW)</b> | IMPERVIOUS (CN 98) = 171 SQUARE FEET<br>PERVIOUS (CN 69) = 1,515 SQUARE FEET<br>TOTAL (CN 72) = 1,686 SQUARE FEET (0.039 AC)     |

**GENERAL NOTES:**

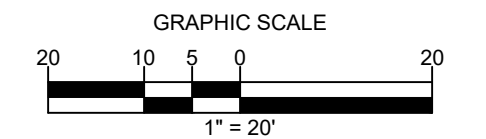
- THIS PROJECT REFERENCES A SURVEY PREPARED BY:  
EXISTING CONDITIONS SURVEY  
CHASE BANK SITE  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH  
ALLEN & MAJOR ASSOCIATES, INC.  
DATED 8/28/19

**ALERT TO CONTRACTOR:**

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**POST-DEVELOPMENT DRAINAGE AREA MAP**  
SCALE 1" = 20'



|                                |   |
|--------------------------------|---|
| <b>PROP-DA 1</b>               | IMPERVIOUS (CN 98) = 22,846 SQUARE FEET<br>PERVIOUS (CN 69) = 11,114 SQUARE FEET<br>TOTAL (CN 88) = 33,960 SQUARE FEET (0.780 AC) |
| <b>PROP-DA 2 (BYPASS)</b>      | IMPERVIOUS (CN 98) = 0 SQUARE FEET<br>PERVIOUS (CN 69) = 919 SQUARE FEET<br>TOTAL (CN 69) = 919 SQUARE FEET (0.022 AC)            |
| <b>PROP-DA 3 (BYPASS PROW)</b> | IMPERVIOUS (CN 98) = 297 SQUARE FEET<br>PERVIOUS (CN 69) = 1,097 SQUARE FEET<br>TOTAL (CN 75) = 1,394 SQUARE FEET (0.032 AC)      |

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REVISIONS

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

ENGINEER SEAL

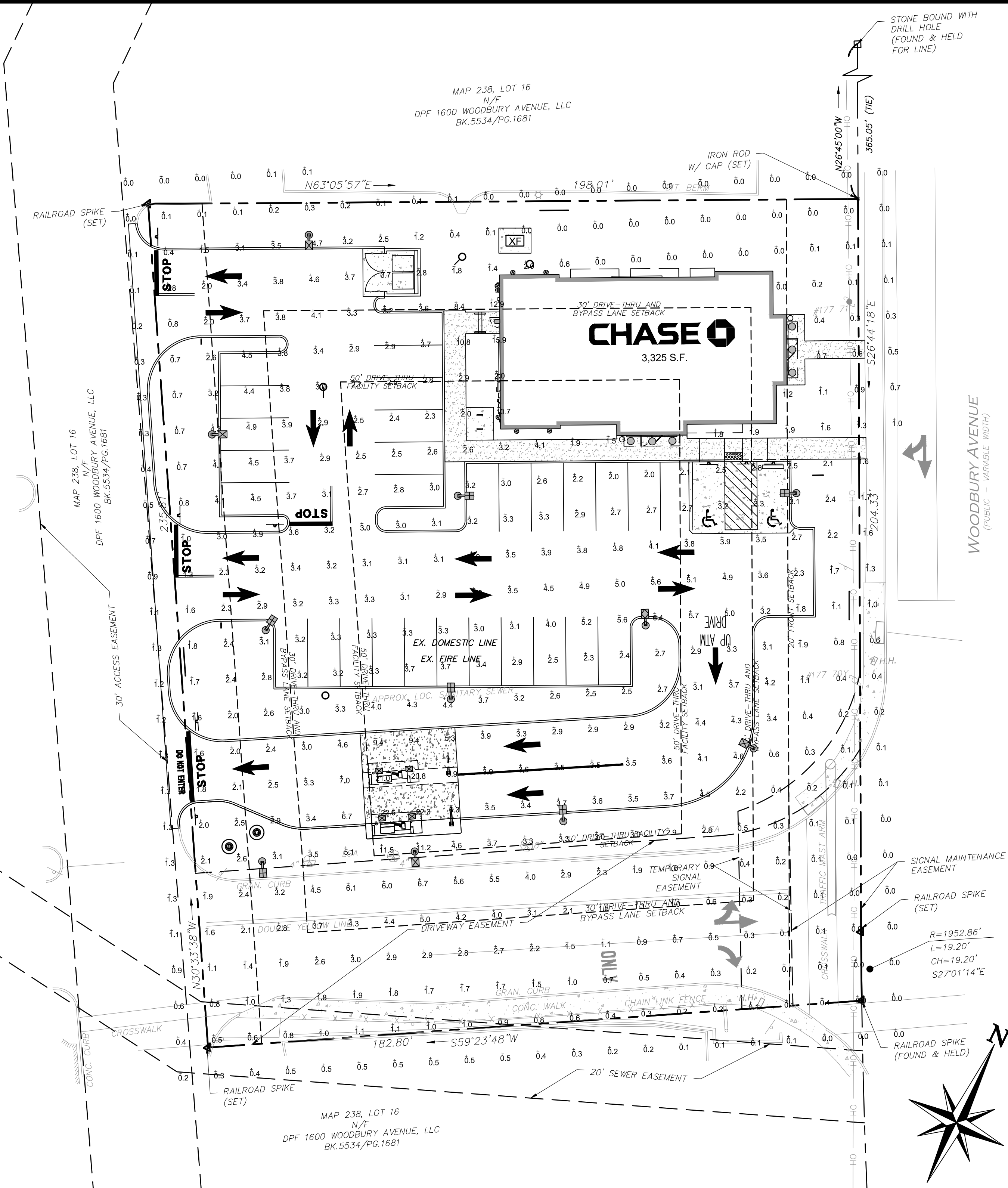
SHEET TITLE

**DRAINAGE AREA MAP**

|             |            |
|-------------|------------|
| JOB #:      | JPM 27086  |
| DATE:       | 07/01/2020 |
| SCALE:      | AS NOTED   |
| DRAWN BY:   | MAL        |
| CHECKED BY: | KGF        |

SHEET NO.

**C-6**



LIGHTING PLAN MEASURED AT 3-FT  
SCALE 1" = 20'

### LUMINAIRE SCHEDULE

| SYMBOL | MANUFACTURER    | MODEL              | CATALOG                                  | QTY | DISTRIBUTION | MOUNT HT. |
|--------|-----------------|--------------------|--|-----|--------------|-----------|
| ☉      | COOPER LIGHTING | GLEON GALLEON LED  | GLEON-AF-02-LED-E1-5WQ-7030              | 6   | TYPE 5       | 20'-0"    |
| ☒      | COOPER LIGHTING | GLEON GALLEON LED  | GLEON-AF-02-E1-SL4-7030                  | 4   | TYPE 4       | 20'-0"    |
| ☒      | COOPER LIGHTING | GLEON GALLEON LED  | GLEON-AF-02-E1-SL4-7030-HSS              | 1   | TYPE 4       | 20'-0"    |
| ☒      | CREE LIGHTING   | LED SQUARE CANOPY  | C-CP-A-SQ-49L-50K-DB                     | 4   | TYPE 5       | 9'-10"    |
| ☉      | AMERLUX         | HORNET HP          | HDL-HP-R-NC-A17-T-16-120-0-10V/HDL-HP-RL | 6   | TYPE 5       | 9'-10"    |
| ☉      | EATON           | LANTERNA 9004      | 9004-W2-RW-LED-3090-W-BK-L1-UNV          | 6   | TYPE 5       | 9'-10"    |
| ☒      | LUMARK          | XTOR CROSSTOUR LED | XTOR6B-W-BZ-MS/DIM-L20-CBP               | 1   | TYPE 4       | 12'-0"    |

### CALCULATION SUMMARY

| AREA                              | AVERAGE | MAX     | MIN    |
|-----------------------------------|---------|---------|--------|
| PROPERTY ANALYSIS MEASURED @ 3 FT | 2.79 fc | 22.4 fc | 0.0 fc |

- #### LIGHTING NOTES
- LIGHT ANALYSIS CONDUCTED AT 3-FEET ABOVE FINISHED GRADE.
  - MOUNTING HEIGHT OF THE SITE LIGHT FIXTURES ARE MEASURED FROM FINISH ASPHALT GRADE.
  - ALL PROPOSED FIXTURES ON TIMER TO OPERATE DURING NIGHT TIME HOURS, 30 MINUTES AFTER SUNSET AND 30 MINUTES BEFORE SUNRISE.
  - CONTRACTOR TO INSTALL FIXTURE ON COPPER LIGHTING POLE RSS-6-A-XX-S-Y-N-XX. MOUNT HEIGHTS ARE BASED ON HEIGHT ABOVE FINISHED ASPHALT GRADE.

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#### LIGHTING LEGEND

|     |                                  |
|-----|----------------------------------|
| --- | EXISTING PROPERTY BOUNDARY LINE  |
| --- | EXISTING ADJOINING PROPERTY LINE |
| --- | EXISTING ROAD CENTERLINE         |
| --- | EXISTING CURB                    |
| --- | PROPOSED CURB                    |
| ○   | FOOTCANDLE CALCULATION POINT     |
| ☒   | PROPOSED SITE LIGHT FIXTURE      |
| ☒   | EXISTING LIGHT FIXTURES          |

**CORE STATES**  
INC.

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DALLAS, TX 75220  
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CLIENT

**CHASE**

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#### REVISIONS

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH 03801

ENGINEER SEAL

THOMAS C. PICKERING  
No. 10218  
LICENSED PROFESSIONAL ENGINEER  
08/2019-08/2020

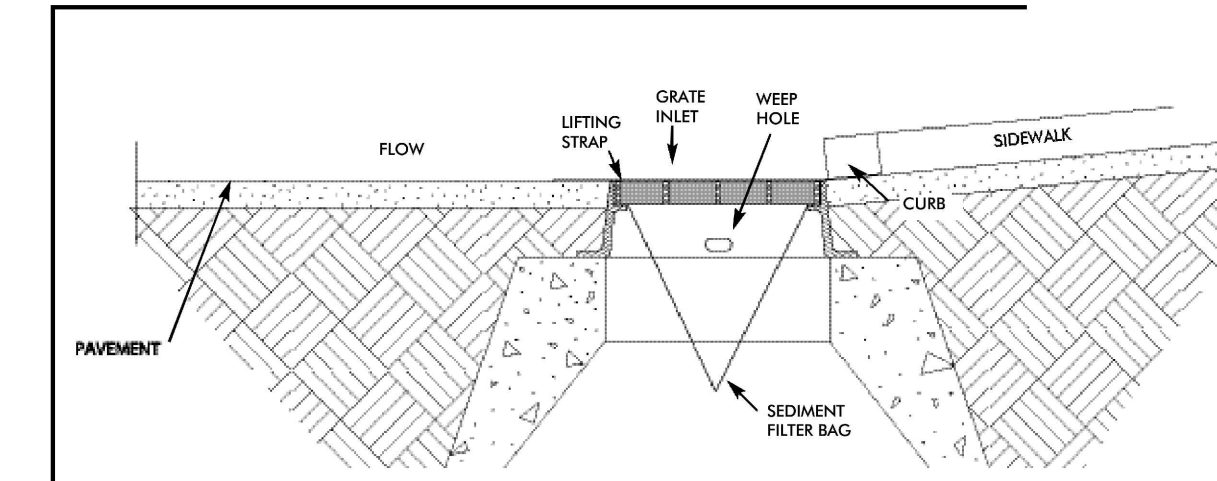
SHEET TITLE  
LIGHTING PLAN

|             |            |
|-------------|------------|
| JOB #:      | JPM 27086  |
| DATE:       | 07/01/2020 |
| SCALE:      | AS NOTED   |
| DRAWN BY:   | MAL        |
| CHECKED BY: | KGF        |

SHEET NO.  
**C-7**

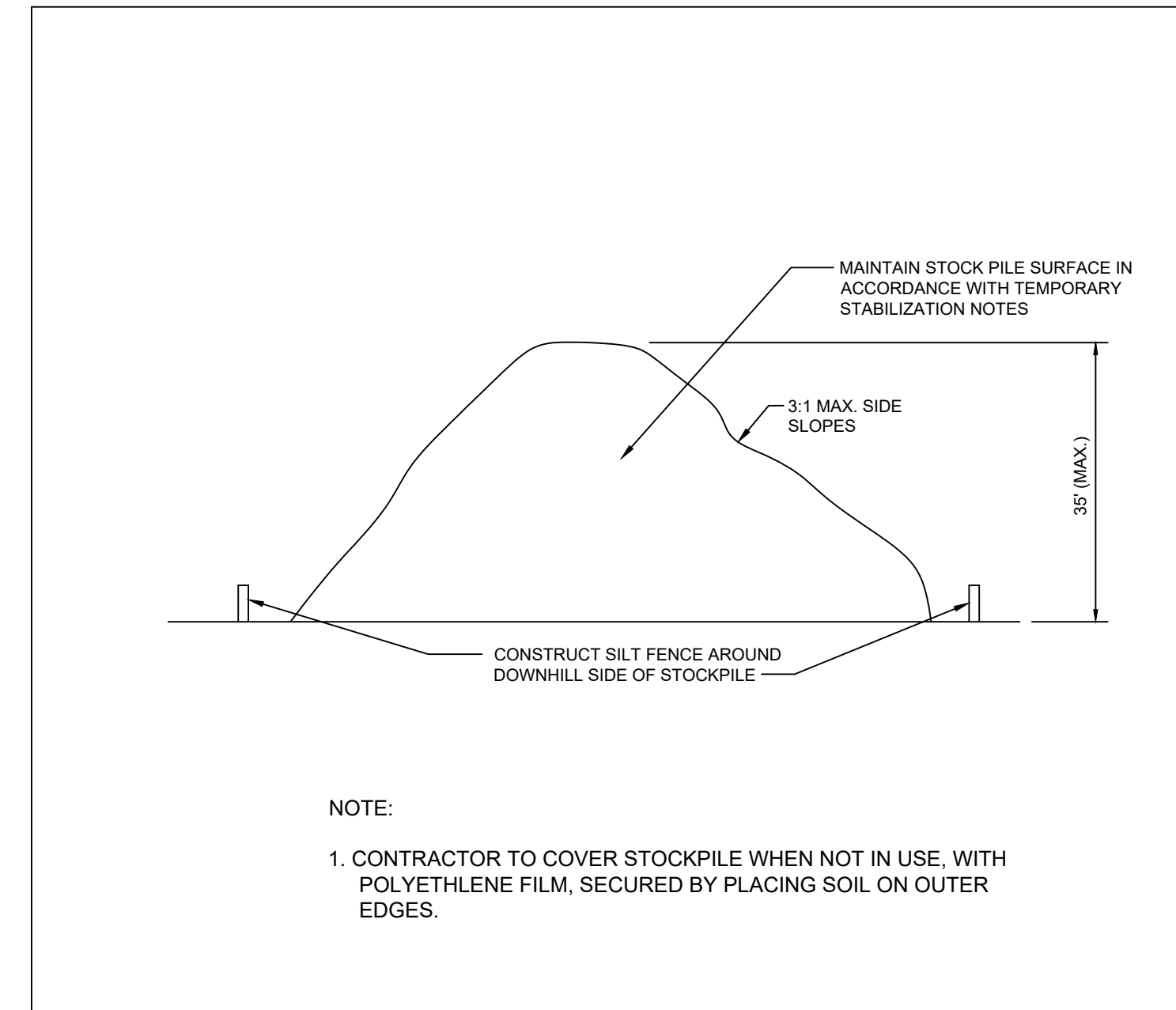


**SEDIMENT FILTER BAG GRATE INLET PROTECTION DETAILS**



**SECTION**

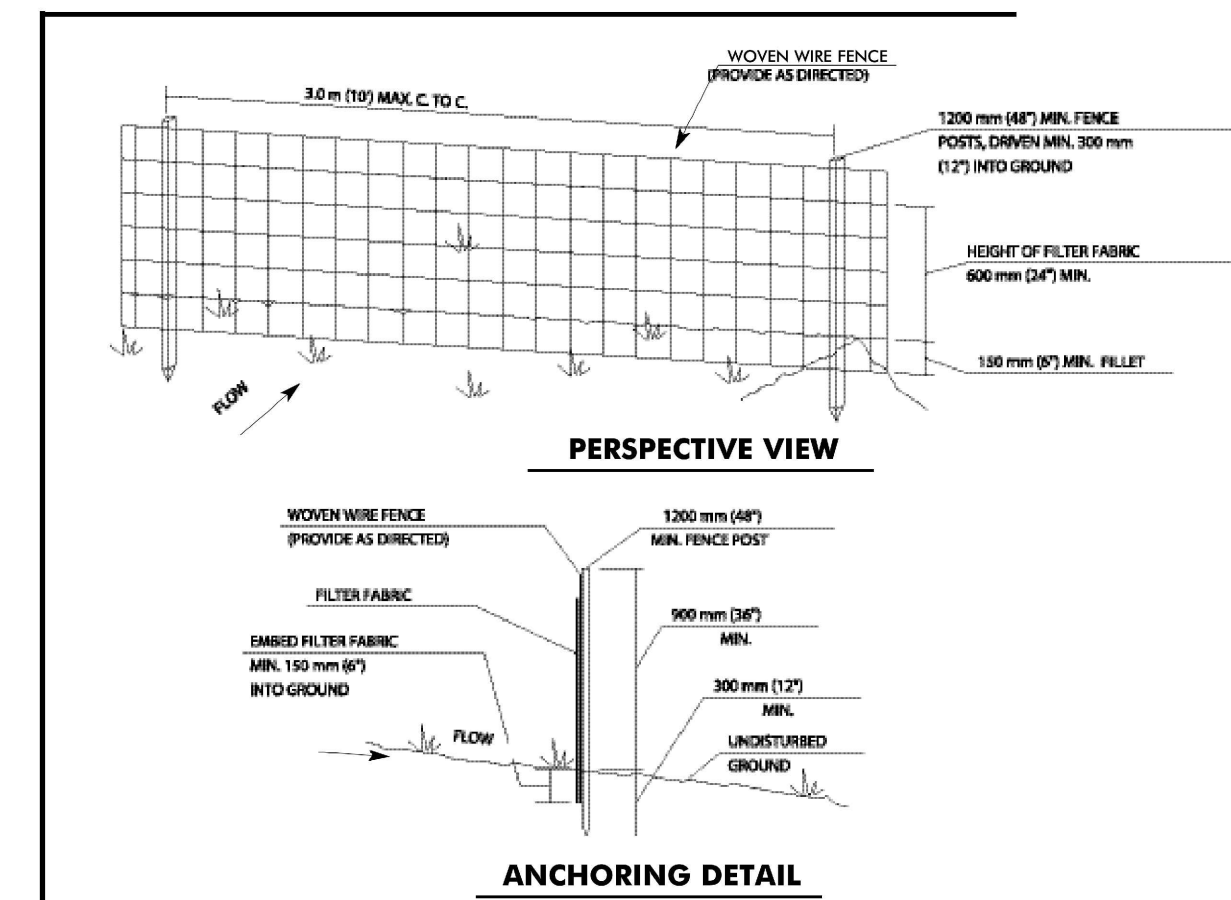
- CONSTRUCTION REQUIREMENTS**
1. Remove drainage inlet grate and place sediment filter bag around the frame, replace grate and sediment filter bag in position or follow manufacturer's recommendation. Lifting straps shall be exposed and ready for maintenance procedures.
  2. Inspect sediment filter bag weekly and after every rainfall event.
  3. Replace, clean or remove sediment filter bag as directed.



- NOTE:**
1. CONTRACTOR TO COVER STOCKPILE WHEN NOT IN USE, WITH POLYETHYLENE FILM, SECURED BY PLACING SOIL ON OUTER EDGES.

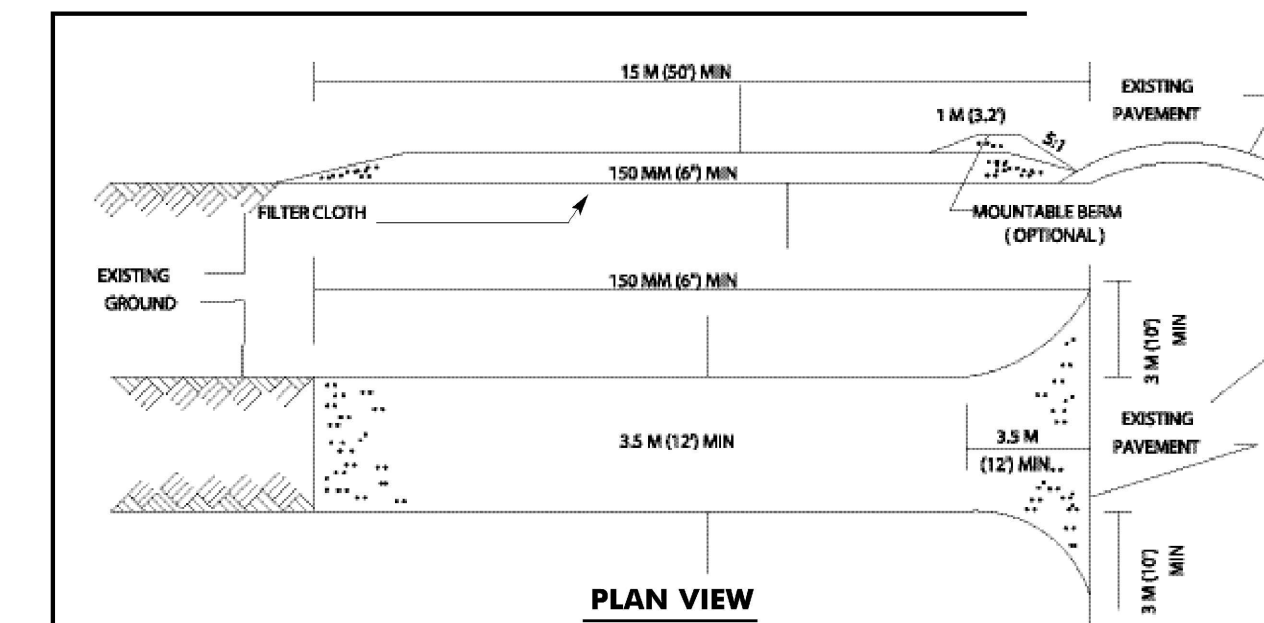
**TEMPORARY STOCKPILE DETAIL**  
N.T.S.

**SILT FENCE PERIMETER BARRIER DETAILS**

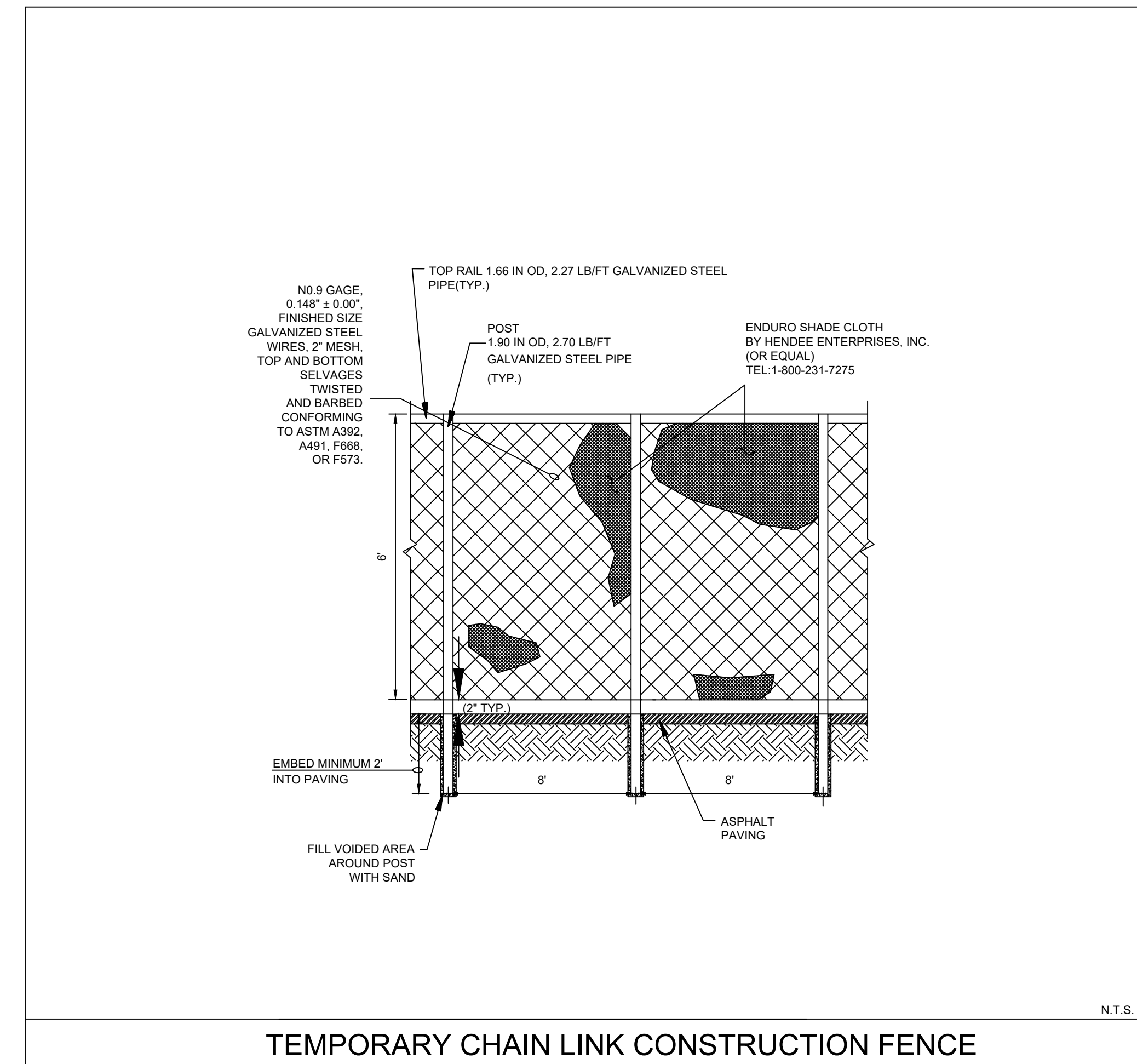


- CONSTRUCTION REQUIREMENTS**
1. Securely fasten filter fabric and woven wire fence (if provided) to fence posts with wire ties, staples, or other approved methods.
  2. Securely fasten filter fabric to the woven wire fence with ties spaced every 600mm (24 in.) at the top, midsection and bottom.
  3. When two sections of filter fabric adjoin each other, overlap the sections by 150mm (6 in.), fold, and staple at a post. Securely splice woven wire fence at a post.
  4. Place silt fence 1500 mm (5 ft.) beyond the toe of slope or on the contour. At the end of silt fence runs, flare uphill.
  5. Provide woven wire fence and/or closer fence post spacing in areas where high runoff volumes are anticipated, or in low spots where sediment will be collected.
  6. Remove silt fence, as directed, when no longer needed. Before the silt fence is removed, stabilize with vegetation any sediment which is permitted to remain in place.

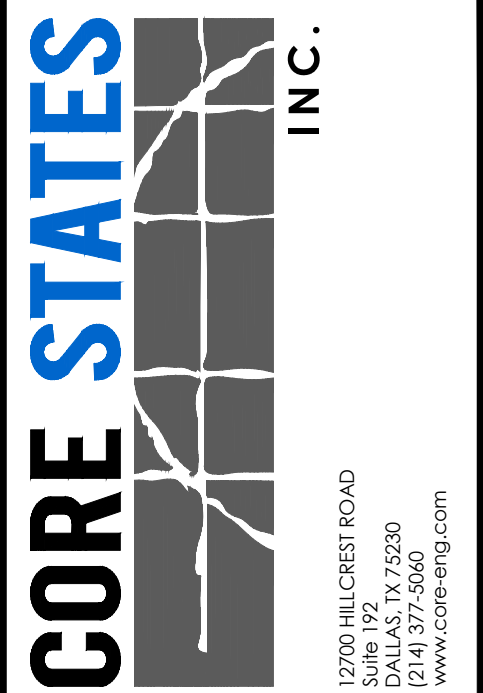
**STABILIZED CONSTRUCTION ENTRANCE DETAILS**



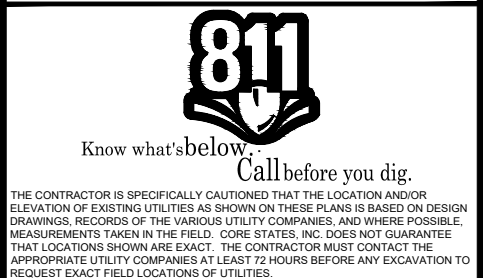
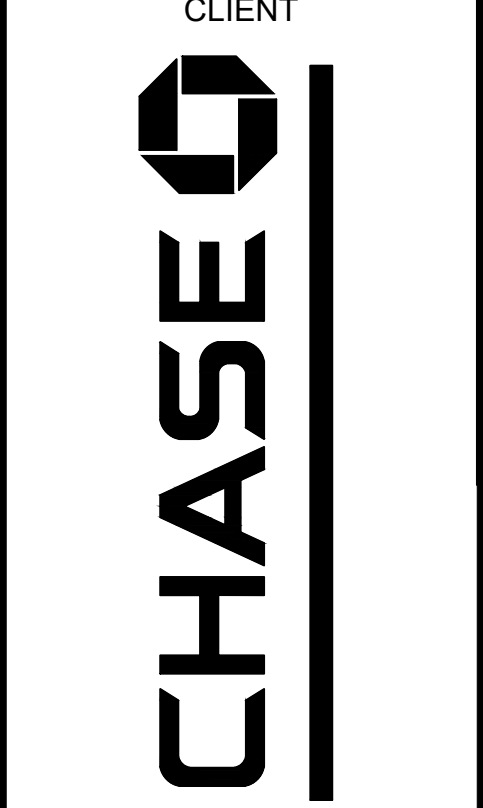
- CONSTRUCTION REQUIREMENTS**
1. Stone Size - Use 37 mm (1 1/2 in.) stone.
  2. Length - Not less than 15m (50 ft.) (Except on a single residence lot where a 9m (30 ft.) minimum length would apply).
  3. Thickness - Not less than 150mm (6 in.).
  4. Width - 3.5 meter (twelve (12) ft.) minimum, but not less than the full width at points where ingress or egress occurs. 7 meters (twenty-four (24) ft.) if single entrance to site.
  5. Filter Cloth - Will be placed over the entire area prior to placing of stone.
  6. Surface Water - All surface water flowing or diverted toward construction entrances shall be piped across the entrance. If piping is impractical, a mountable berm with 5:1 slopes will be permitted.
  7. Maintenance - The entrance shall be maintained in a condition which will prevent tracking or flowing of sediment onto public rights-of-way. All sediment spilled, dropped, washed or tracked onto public rights-of-way must be removed immediately.
  8. When washing is required, it shall be done on an area stabilized with stone and which drains into an approved sediment trapping device.
  9. Periodic inspection and needed maintenance shall be provided after each rain.



**TEMPORARY CHAIN LINK CONSTRUCTION FENCE**  
N.T.S.



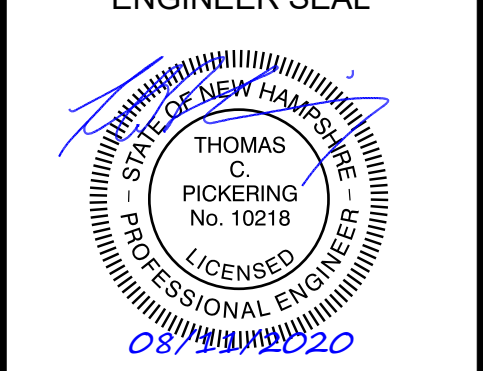
DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT, ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK. IF USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED, USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.



| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

**DOCUMENT**  
SITE PLAN APPROVAL FOR CHASE BANK

**SITE LOCATION**  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

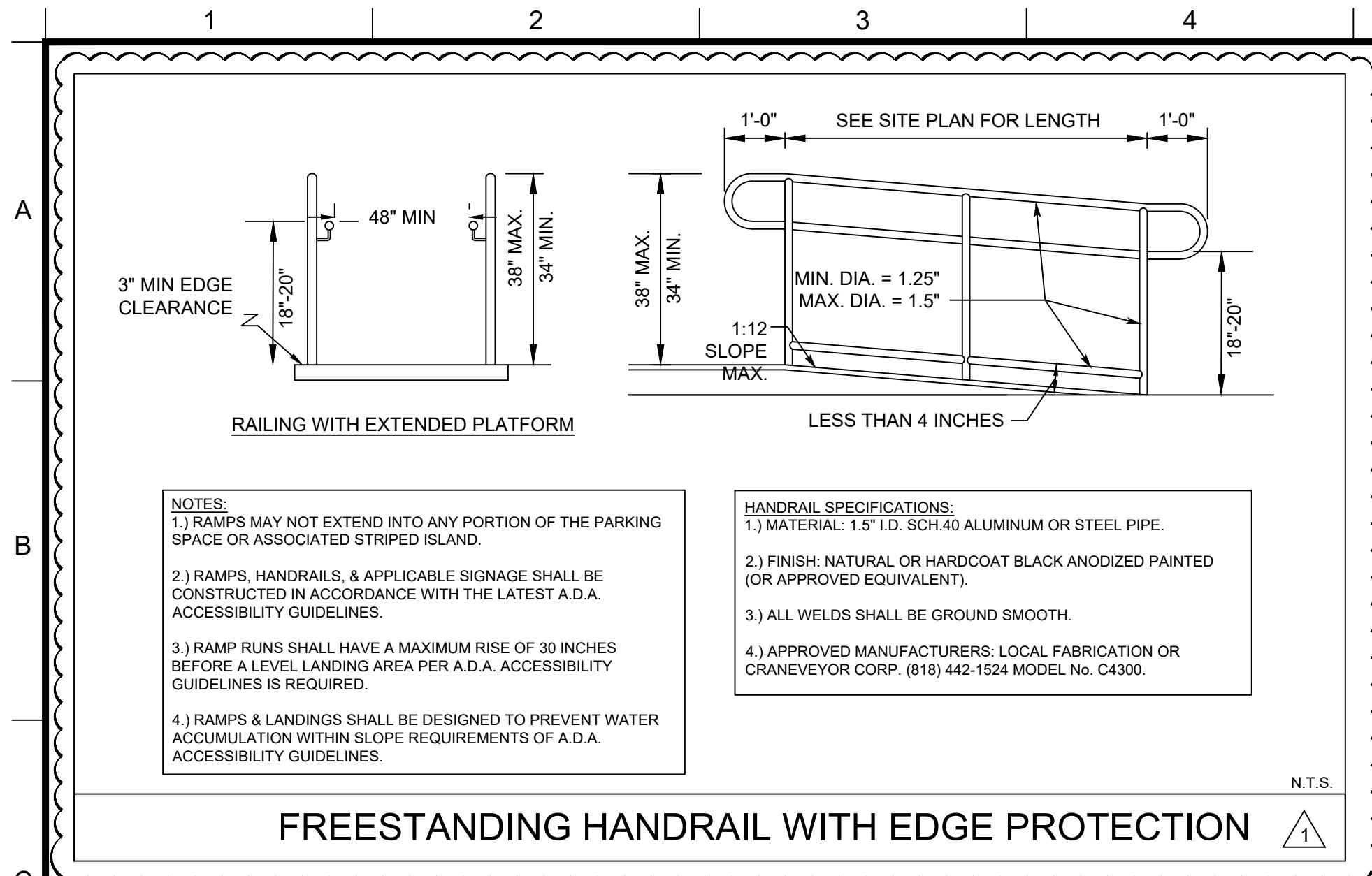


**SHEET TITLE**  
SOIL EROSION AND SEDIMENT CONTROL DETAILS

|             |            |
|-------------|------------|
| JOB #:      | JPM 27086  |
| DATE:       | 07/01/2020 |
| SCALE:      | AS NOTED   |
| DRAWN BY:   | MAL        |
| CHECKED BY: | KGF        |

SHEET NO.  
**C-9**





- NOTES:**
- 1) RAMP MAY NOT EXTEND INTO ANY PORTION OF THE PARKING SPACE OR ASSOCIATED STRIPED ISLAND.
  - 2) RAMP, HANDRAILS, & APPLICABLE SIGNAGE SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST A.D.A. ACCESSIBILITY GUIDELINES.
  - 3) RAMP RUNS SHALL HAVE A MAXIMUM RISE OF 30 INCHES BEFORE A LEVEL LANDING AREA PER A.D.A. ACCESSIBILITY GUIDELINES IS REQUIRED.
  - 4) RAMP & LANDINGS SHALL BE DESIGNED TO PREVENT WATER ACCUMULATION WITHIN SLOPE REQUIREMENTS OF A.D.A. ACCESSIBILITY GUIDELINES.
- HANDRAIL SPECIFICATIONS:**
- 1) MATERIAL: 1.5" I.D. SCH. 40 ALUMINUM OR STEEL PIPE.
  - 2) FINISH: NATURAL OR HARD COAT BLACK ANODIZED PAINTED (OR APPROVED EQUIVALENT).
  - 3) ALL WELDS SHALL BE GROUND SMOOTH.
  - 4) APPROVED MANUFACTURERS: LOCAL FABRICATION OR CRANEVEYOR CORP. (618) 442-1524 MODEL NO. C4300.

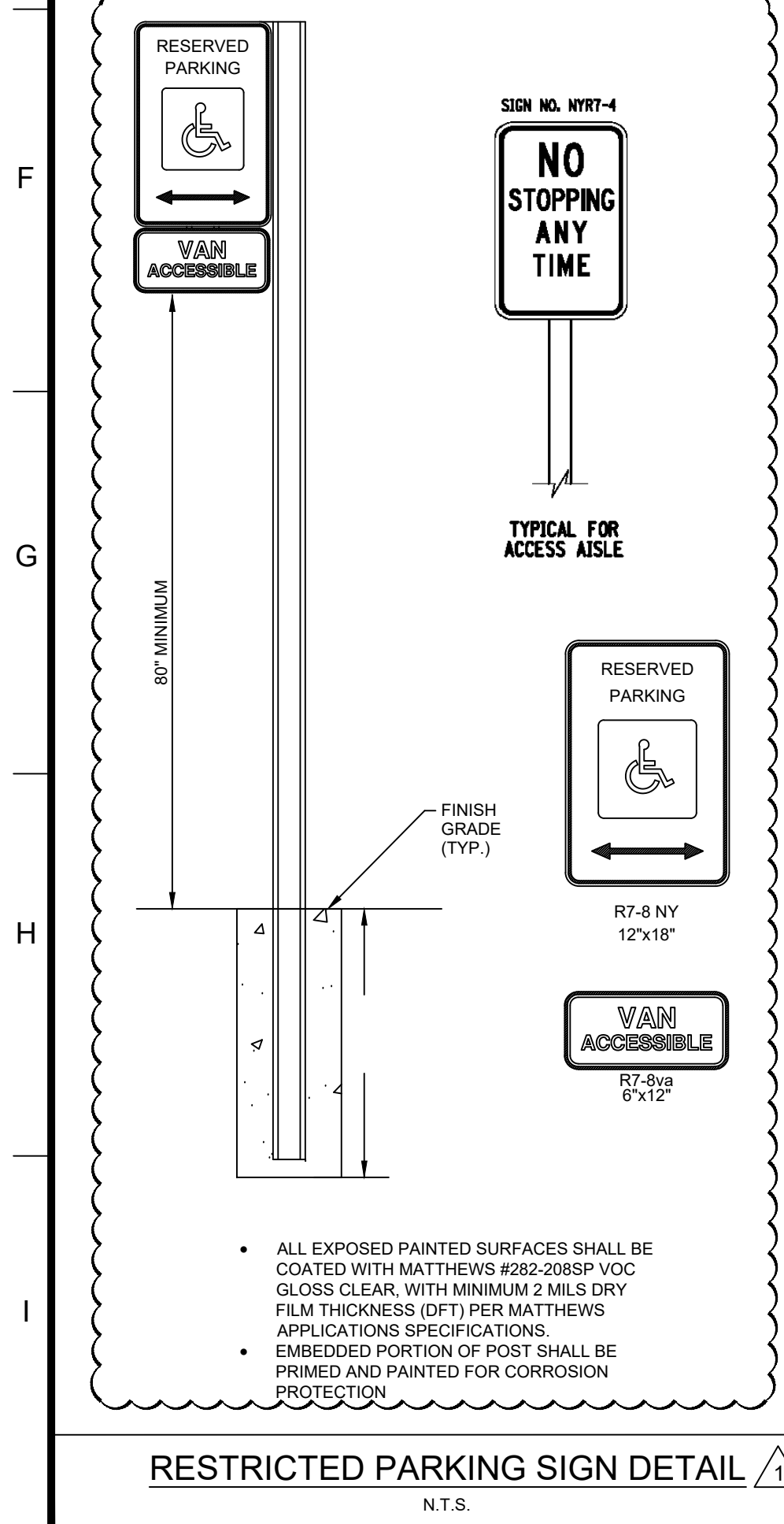
**FREESTANDING HANDRAIL WITH EDGE PROTECTION**

N.T.S.



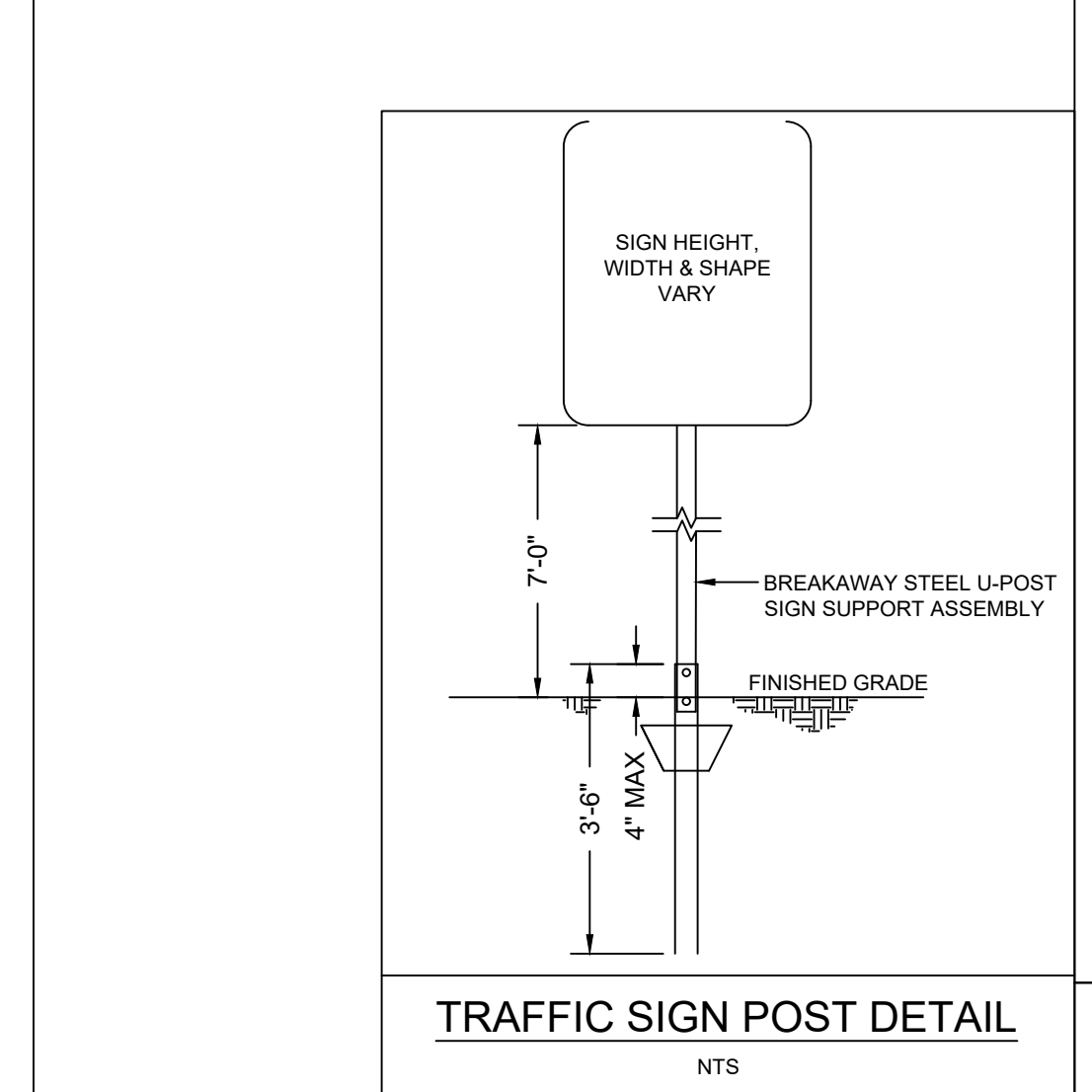
**ACCESSIBLE PARKING STALL**

N.T.S.



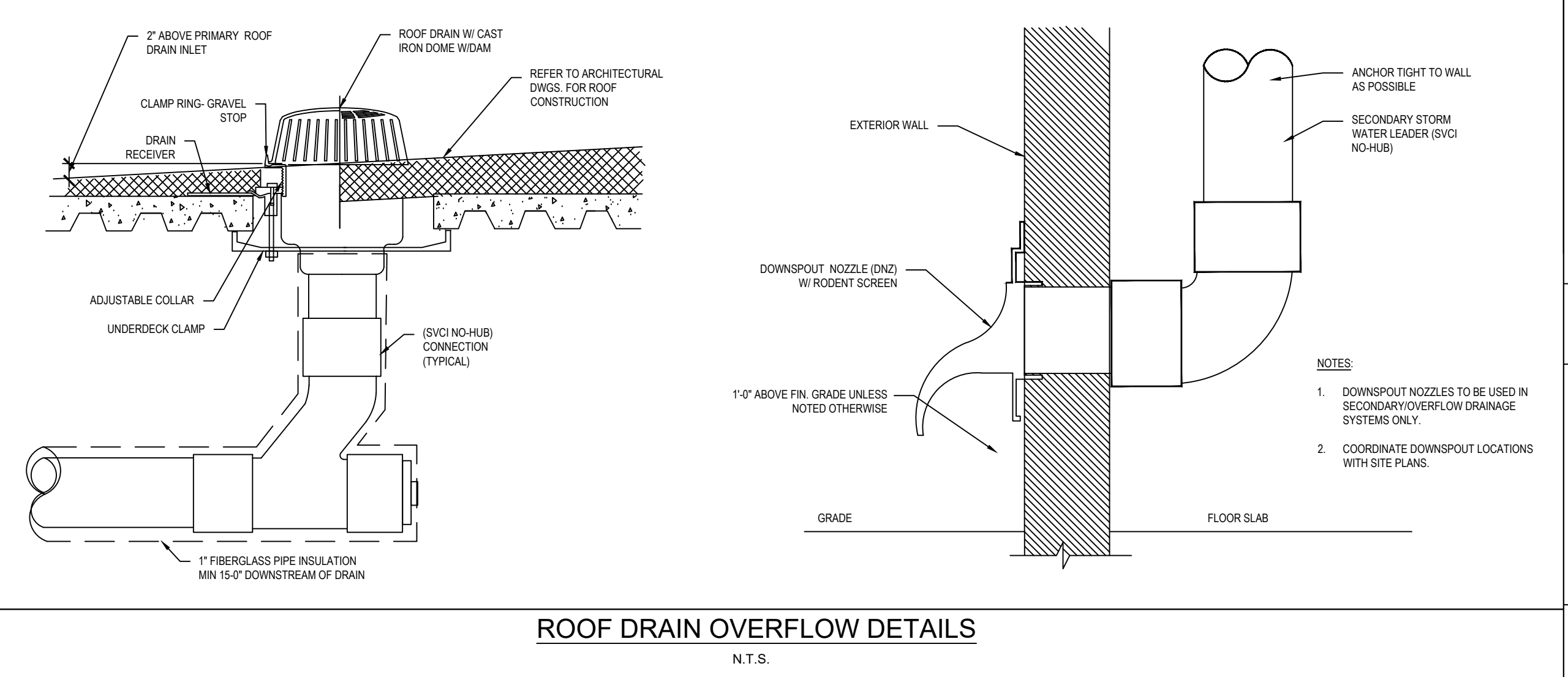
**RESTRICTED PARKING SIGN DETAIL**

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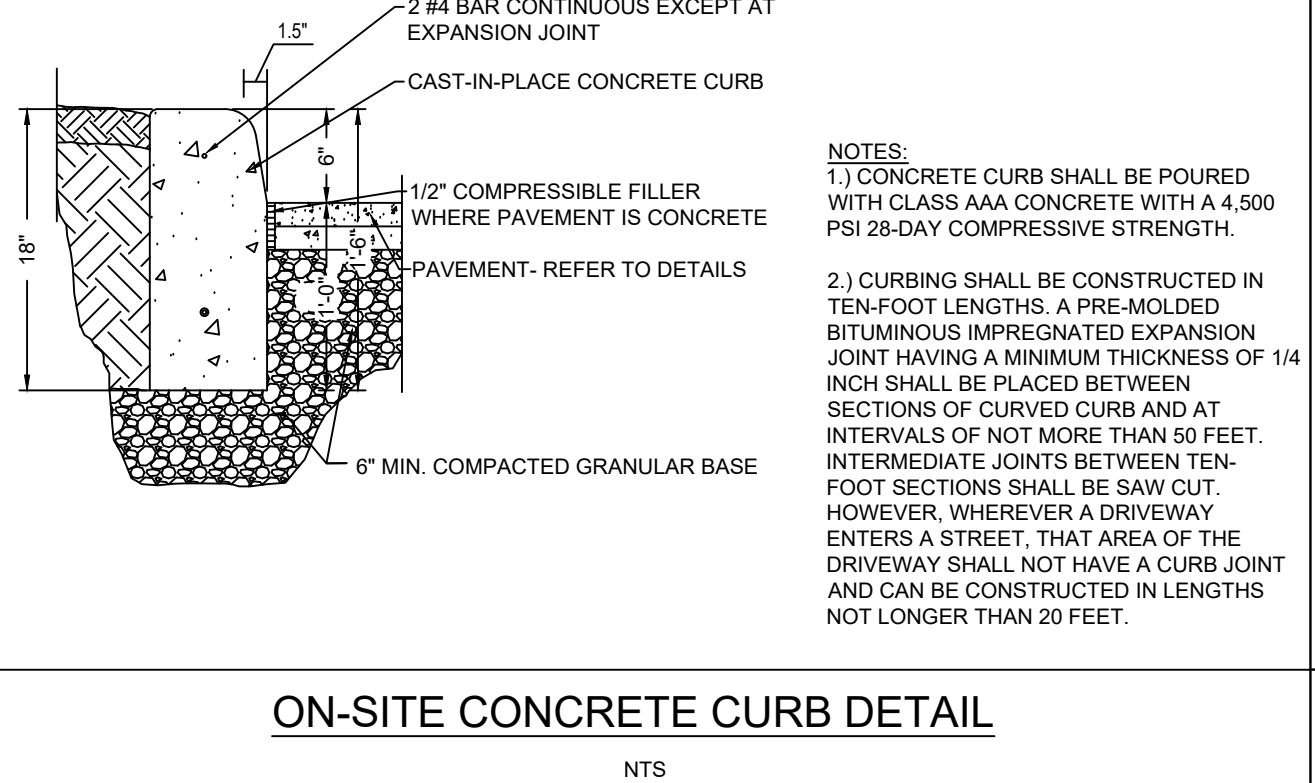
**TRAFFIC SIGN POST DETAIL**

N.T.S.



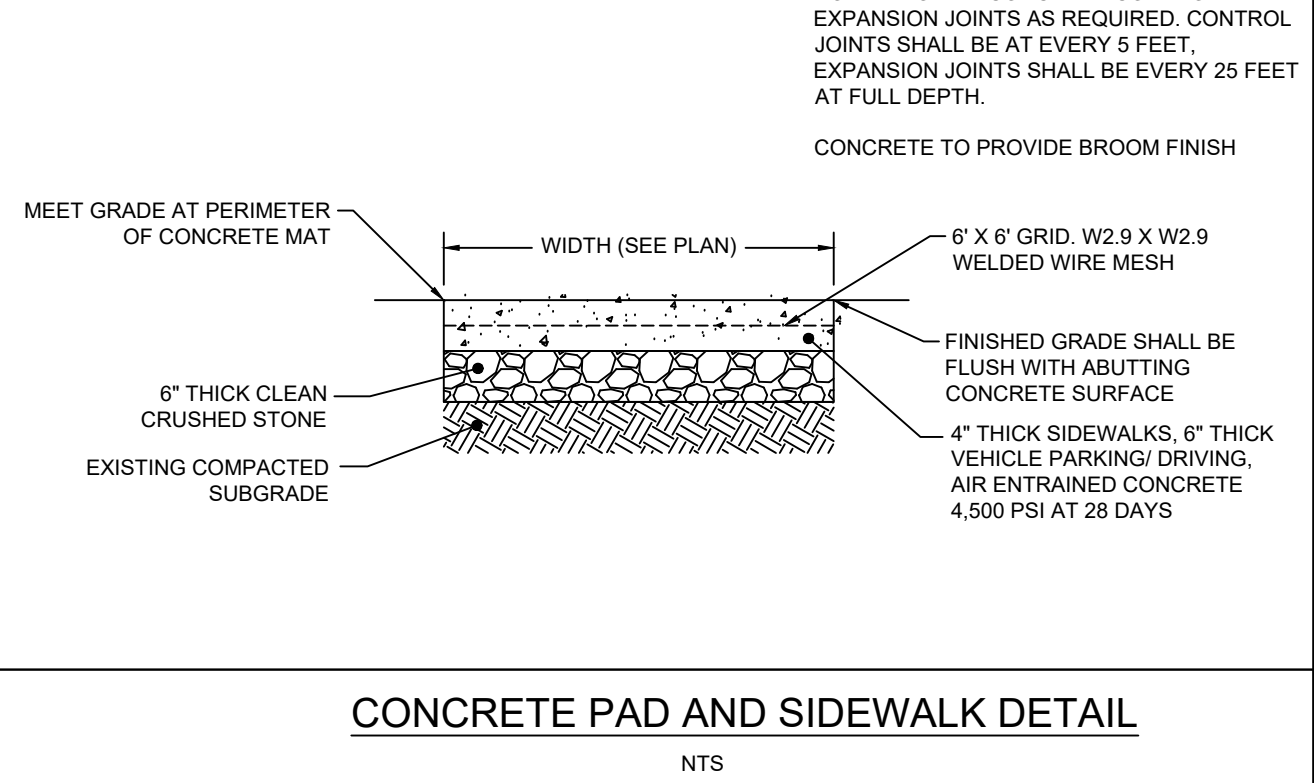
**ROOF DRAIN OVERFLOW DETAILS**

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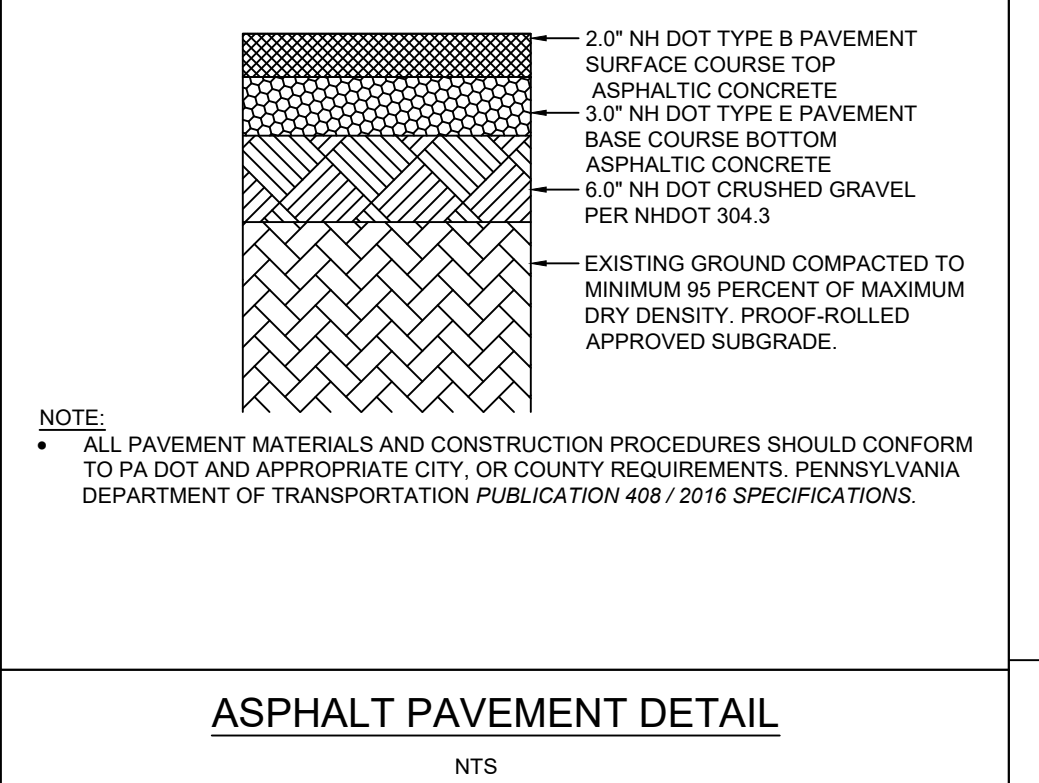
**ON-SITE CONCRETE CURB DETAIL**

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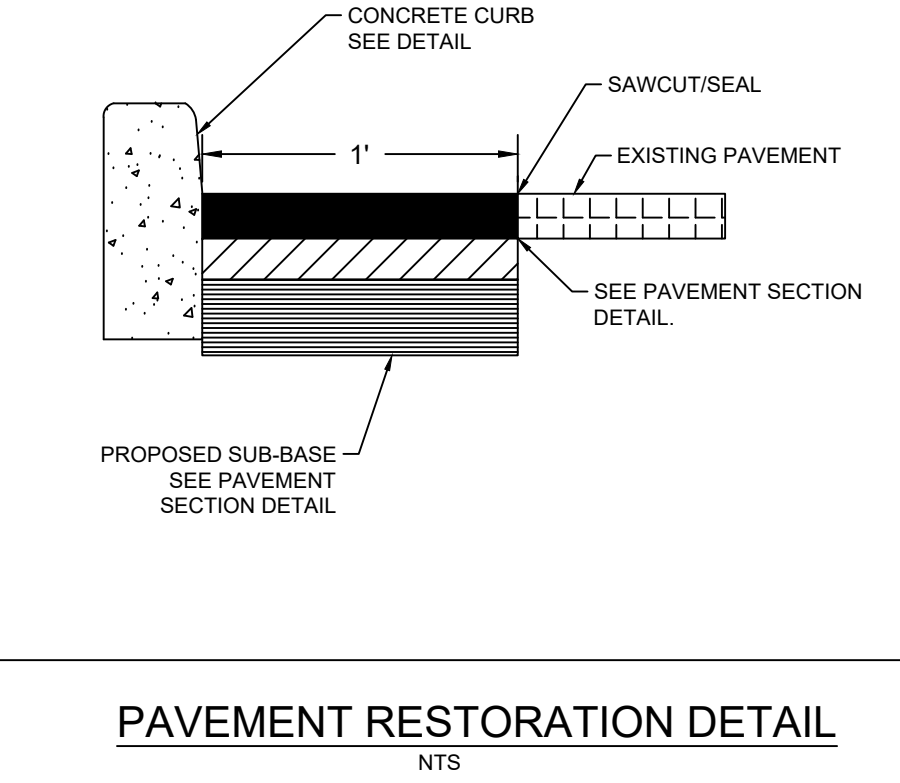
**CONCRETE PAD AND SIDEWALK DETAIL**

N.T.S.



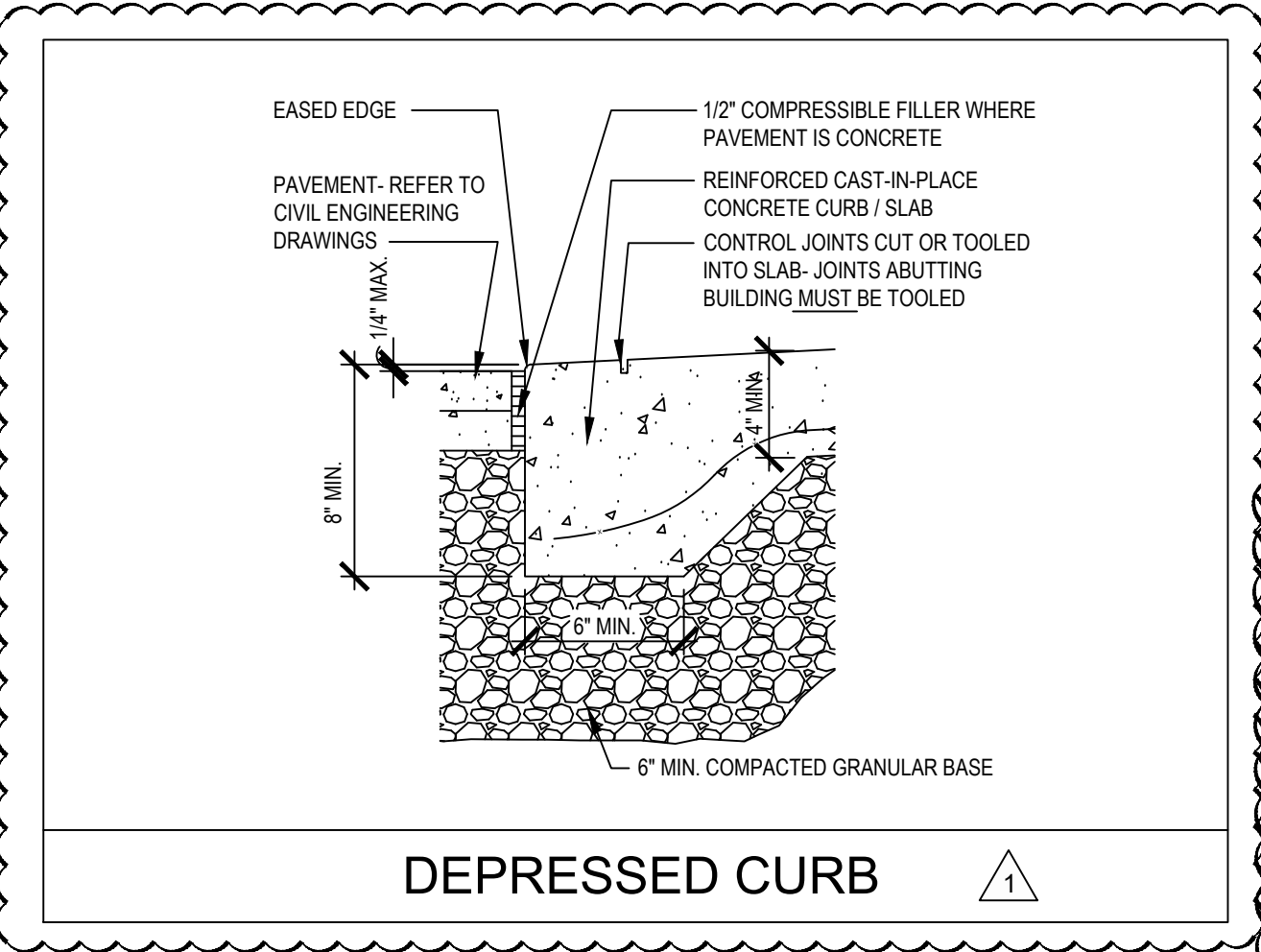
**ASPHALT PAVEMENT DETAIL**

N.T.S.



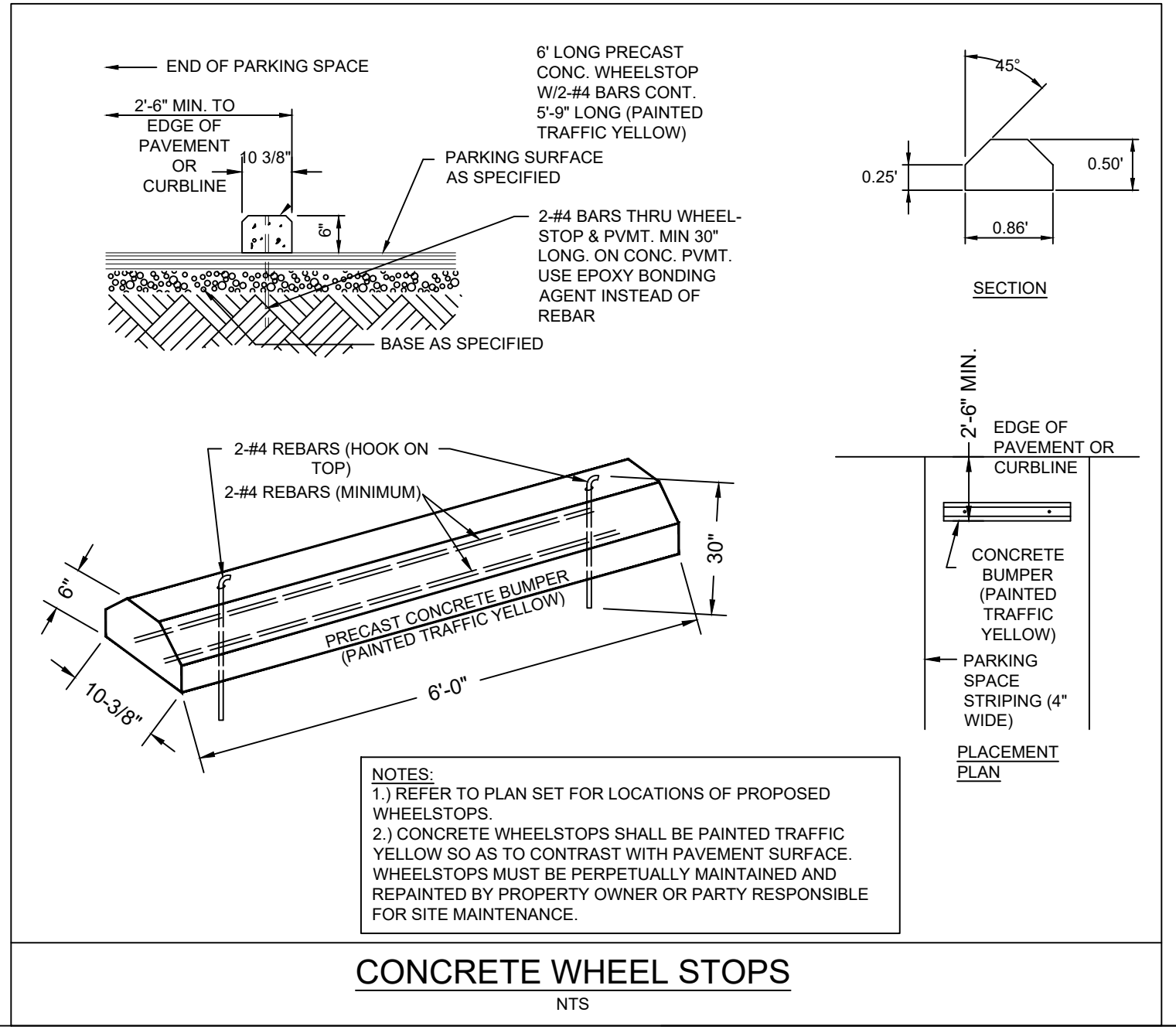
**PAVEMENT RESTORATION DETAIL**

N.T.S.



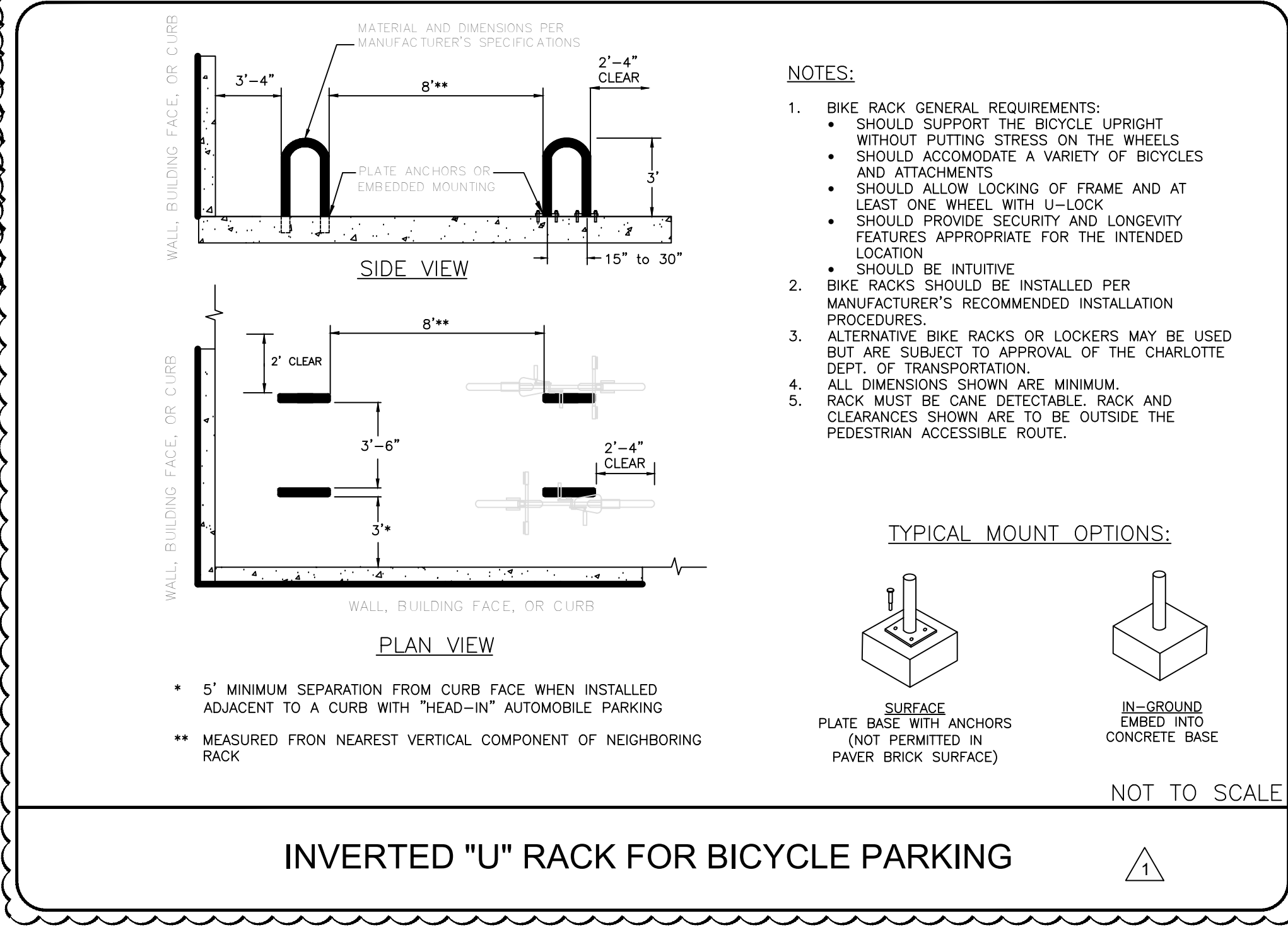
**DEPRESSED CURB**

N.T.S.



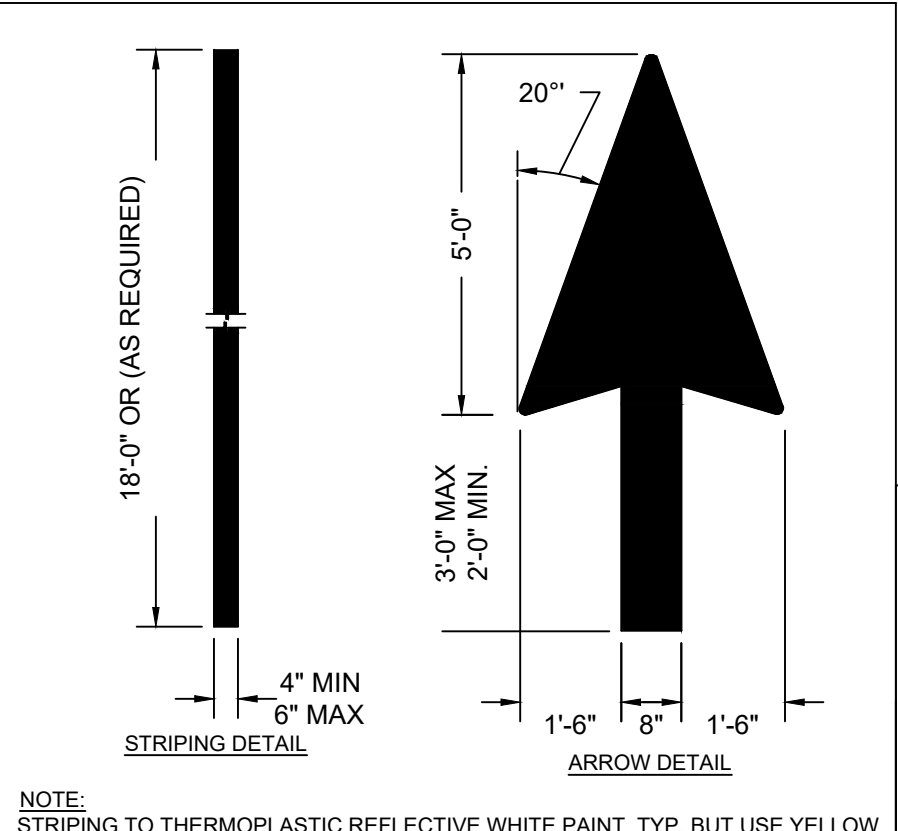
**CONCRETE WHEEL STOPS**

N.T.S.



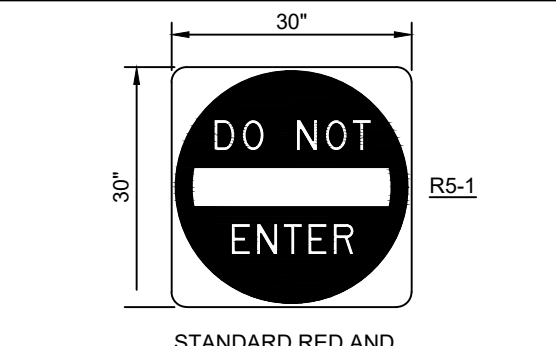
**INVERTED "U" RACK FOR BICYCLE PARKING**

N.T.S.



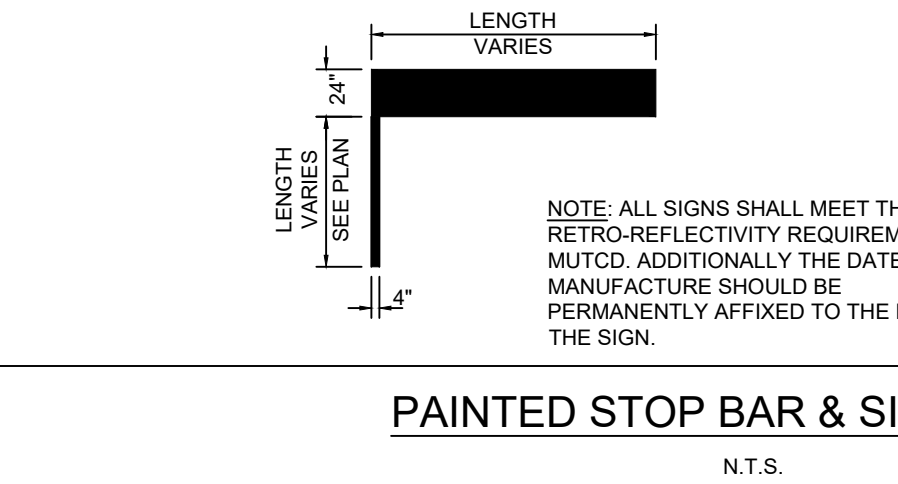
**PAVEMENT STRIPING DETAIL**

N.T.S.



**"DO NOT ENTER" SIGN**

N.T.S.



**PAINTED STOP BAR & SIGN DETAIL**

N.T.S.

**CORE STATES INC.**

12700 HILLCREST ROAD  
DALLAS, TX 75220  
(214) 377-5860  
www.core-states.com

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**CLIENT**

**CHASE**

**811**

Know what's below. Call before you dig.

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

**DOCUMENT**

**SITE PLAN APPROVAL FOR CHASE BANK**

**SITE LOCATION**  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

**ENGINEER SEAL**

THOMAS C. PICKERING  
No. 10218  
LICENSED PROFESSIONAL ENGINEER  
08/01/2015-08/01/2020

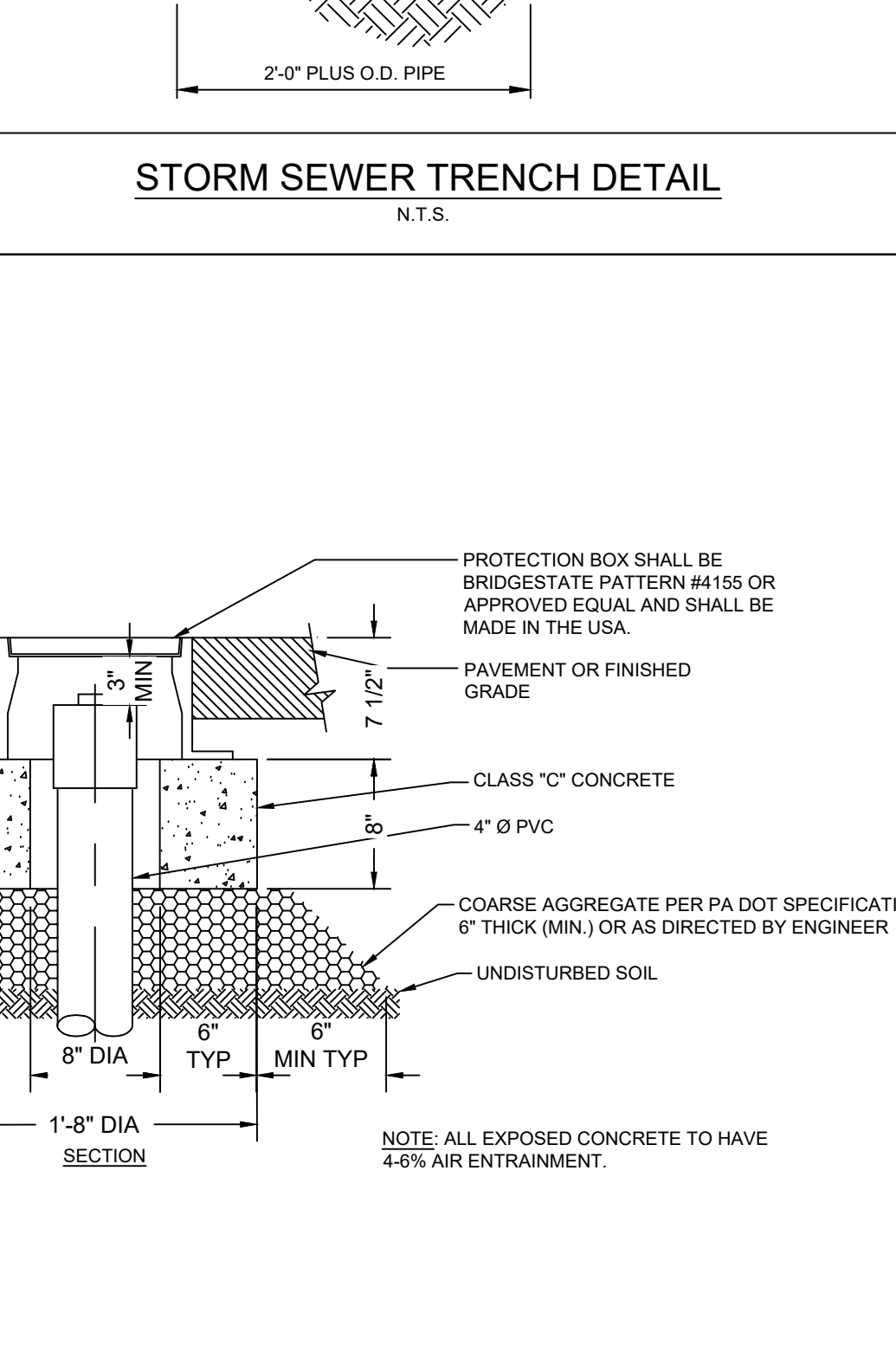
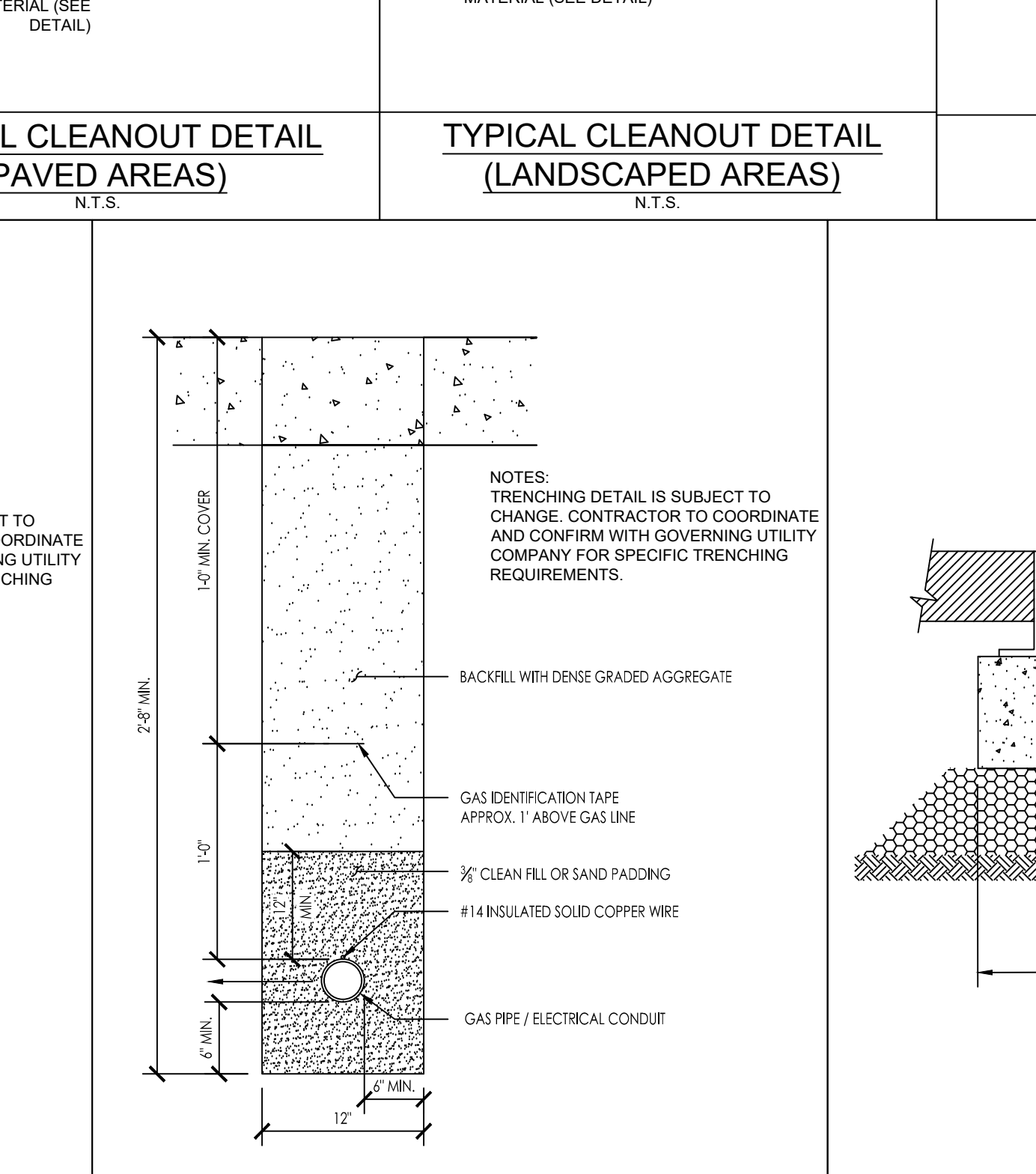
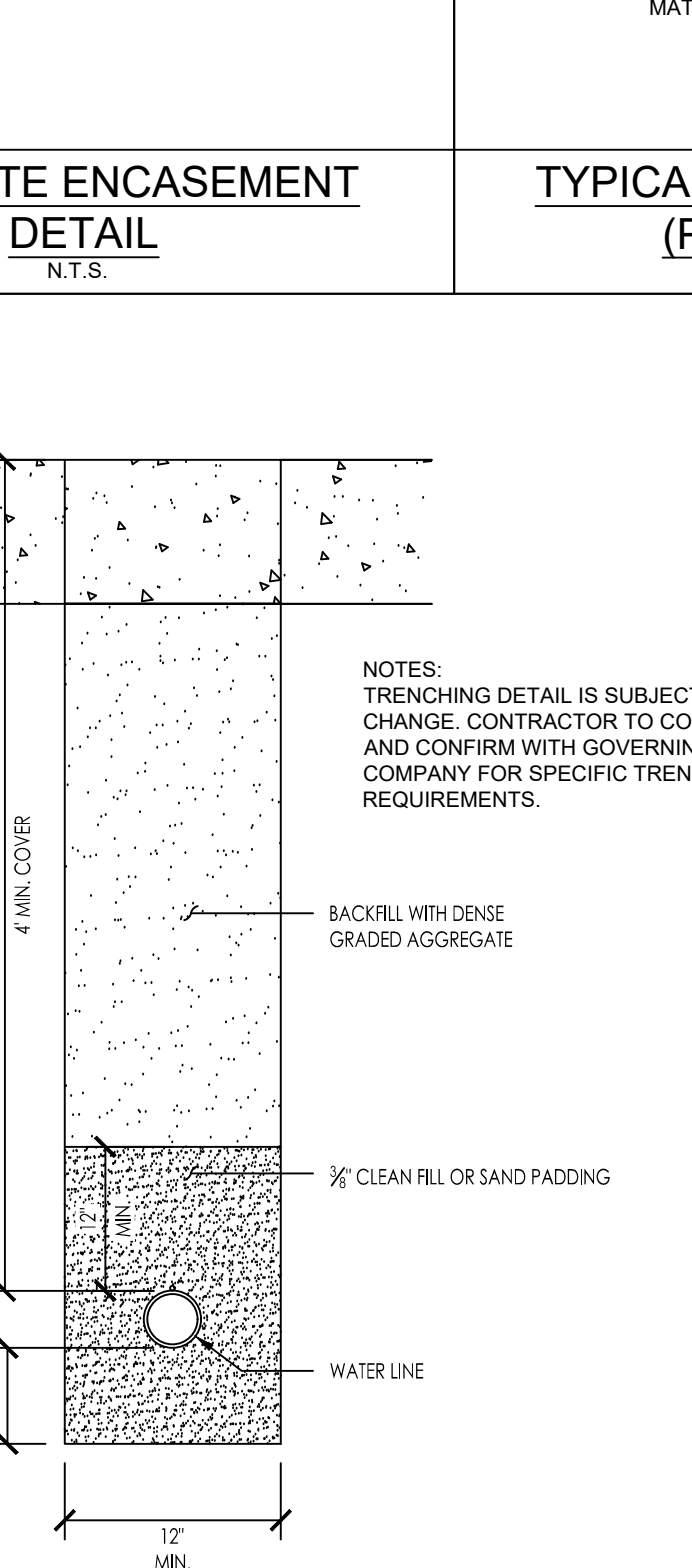
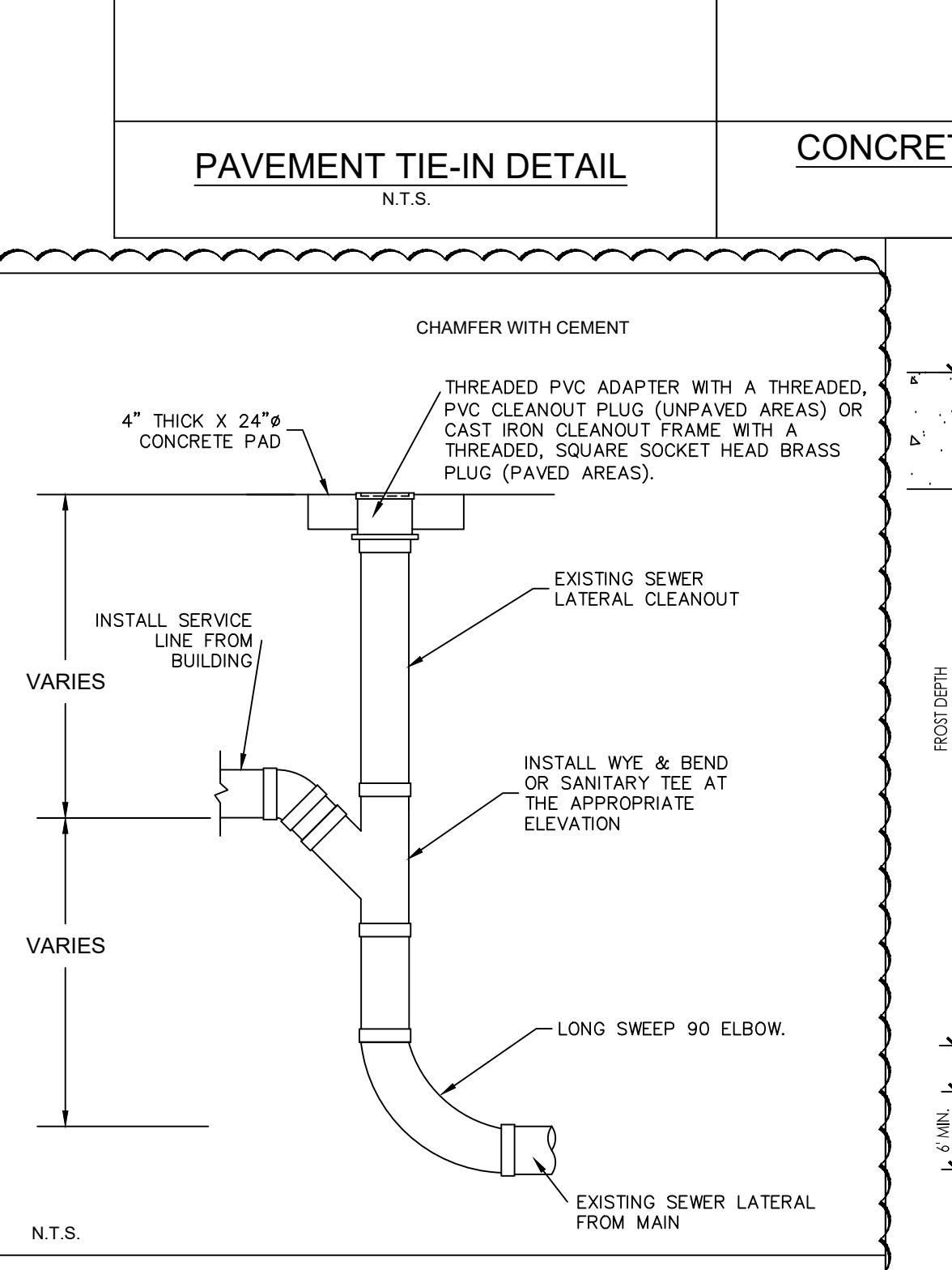
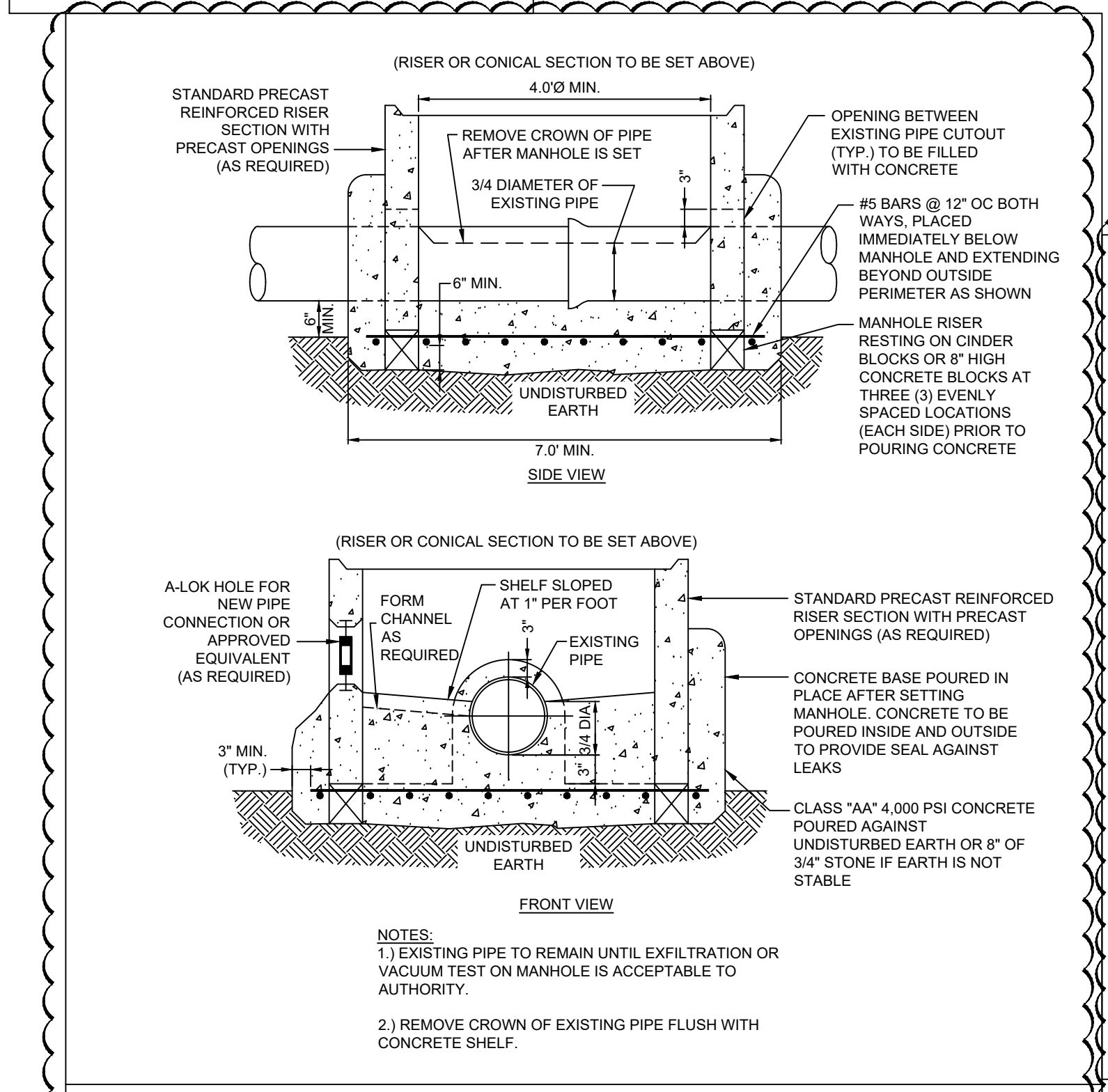
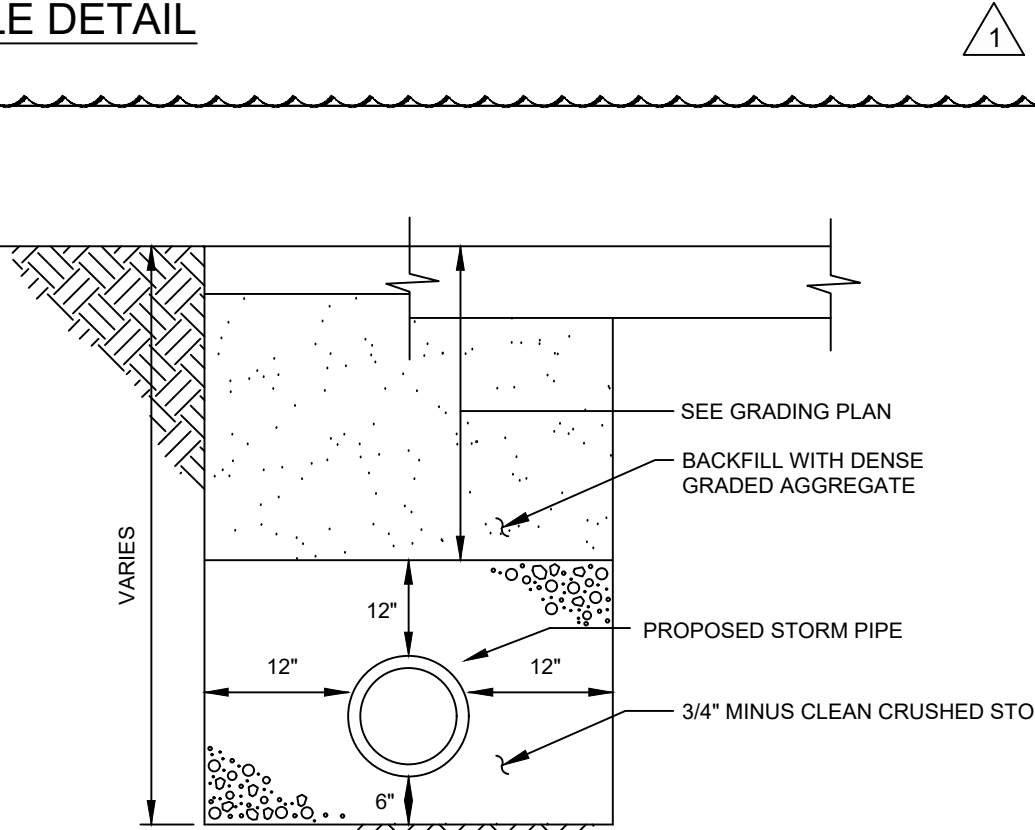
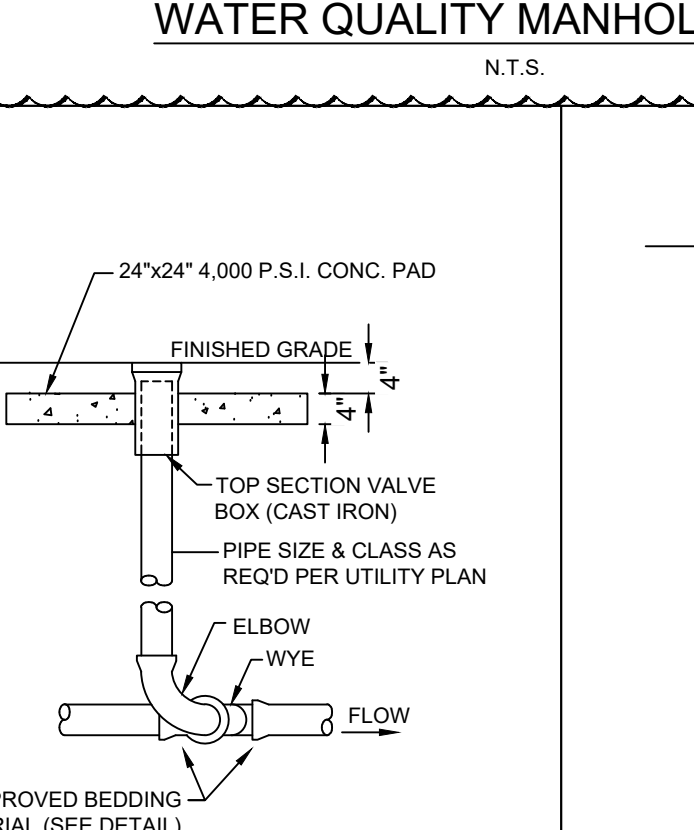
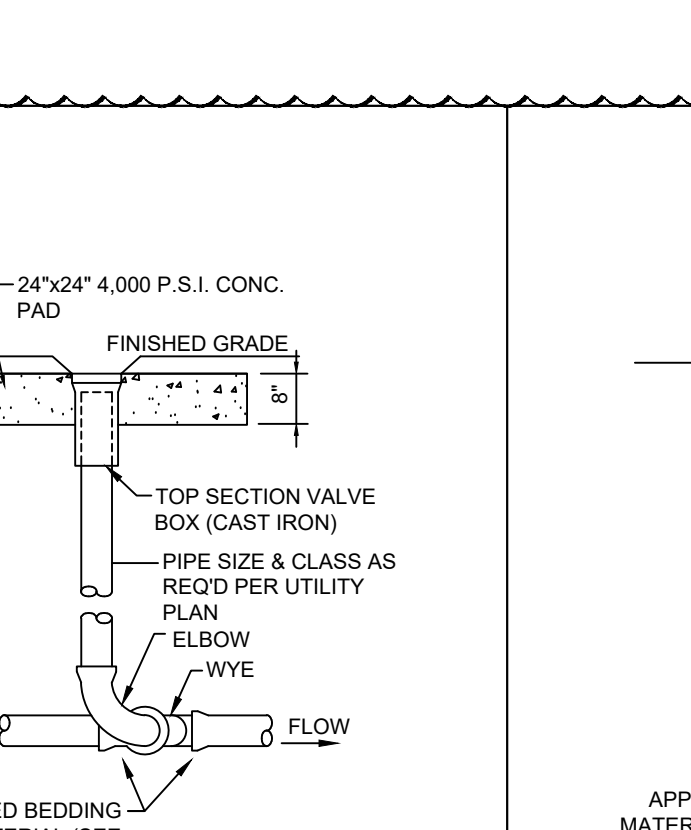
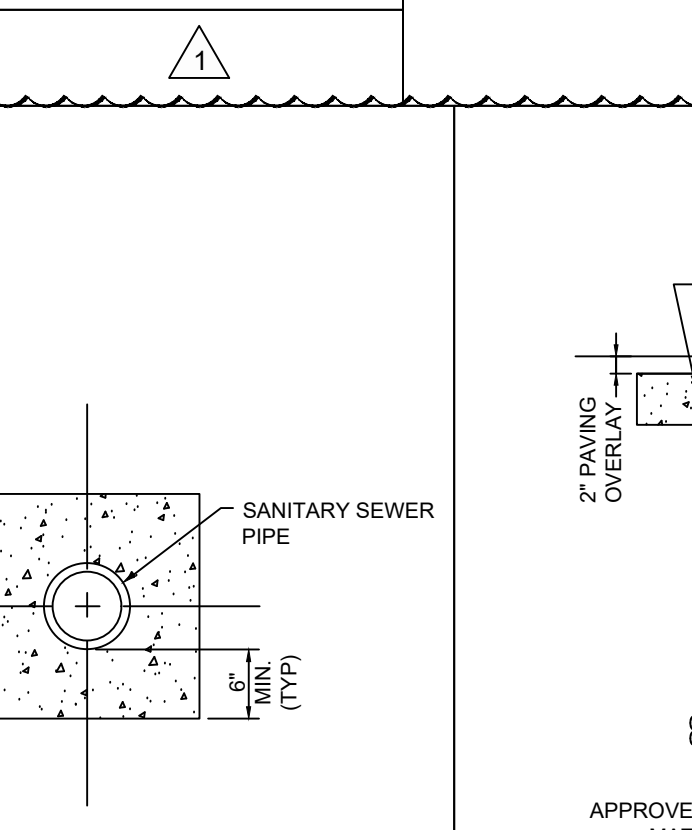
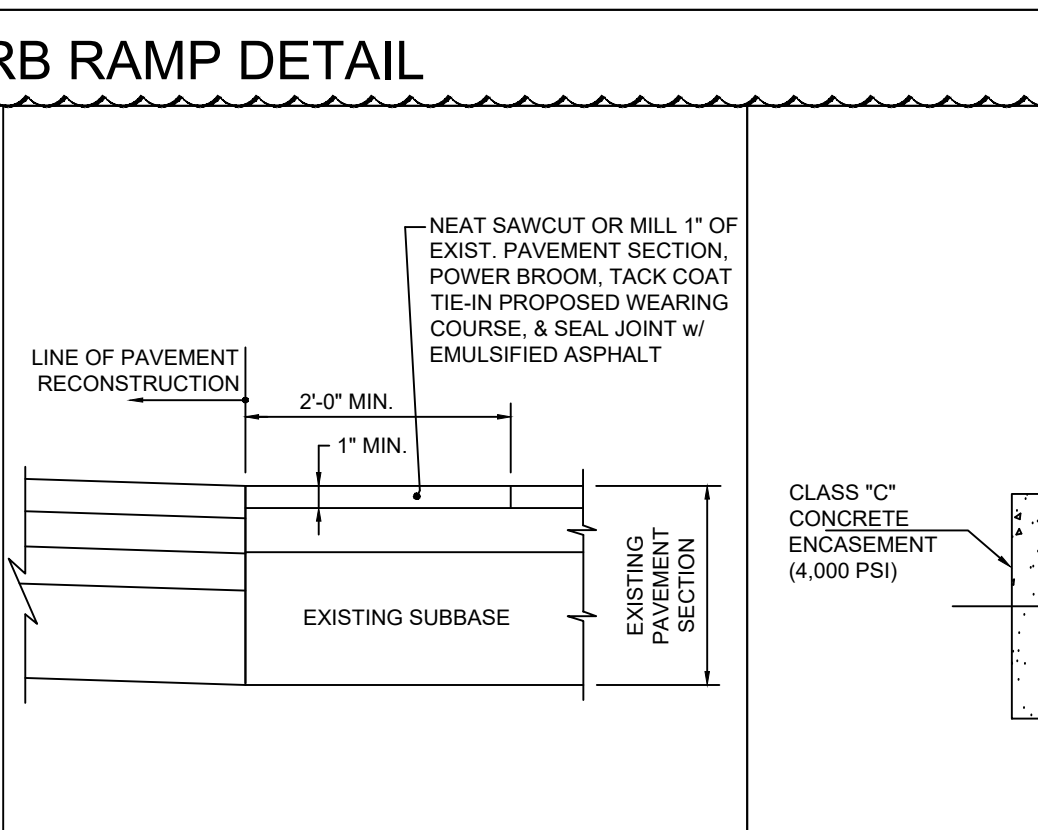
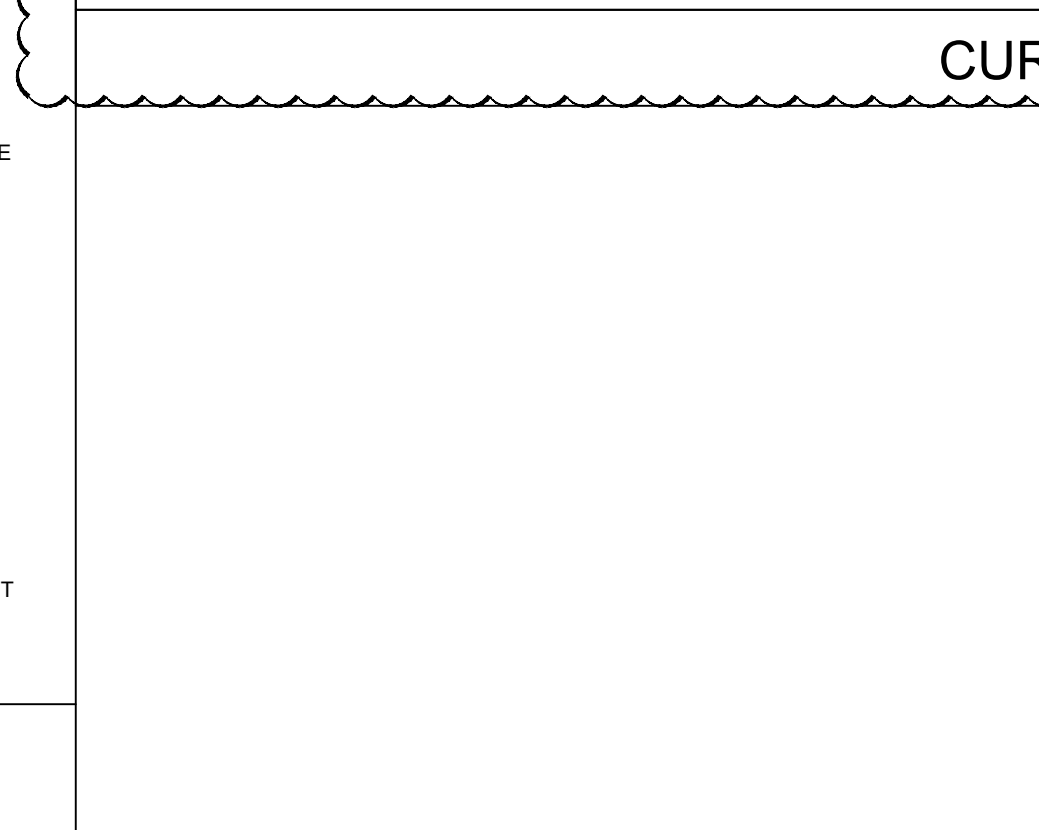
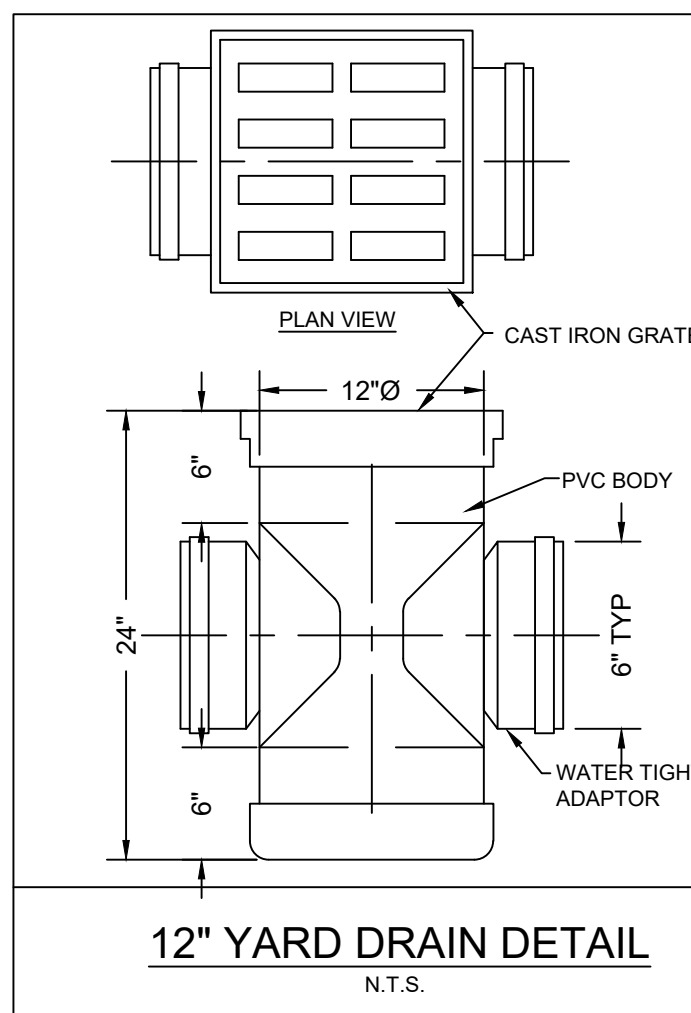
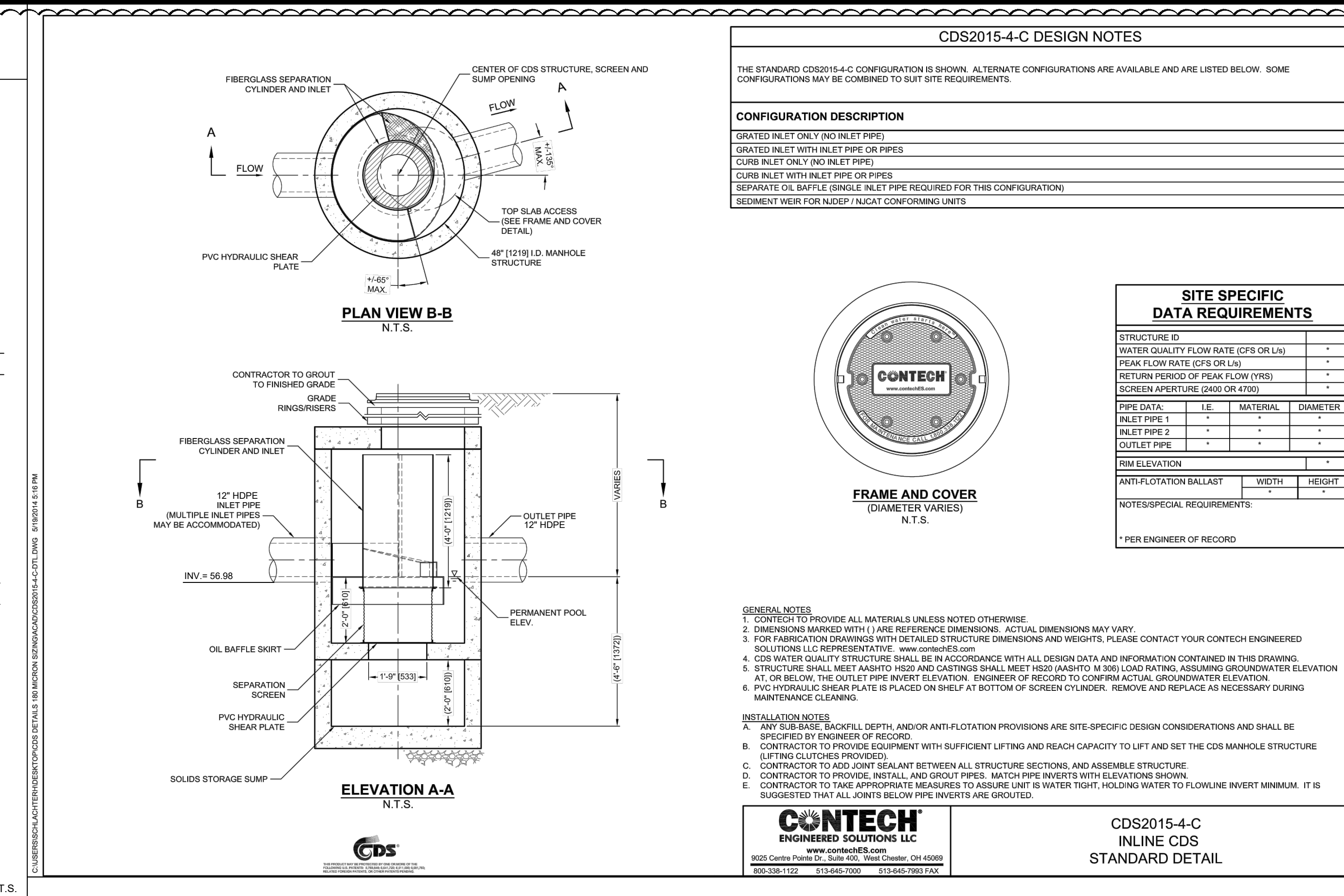
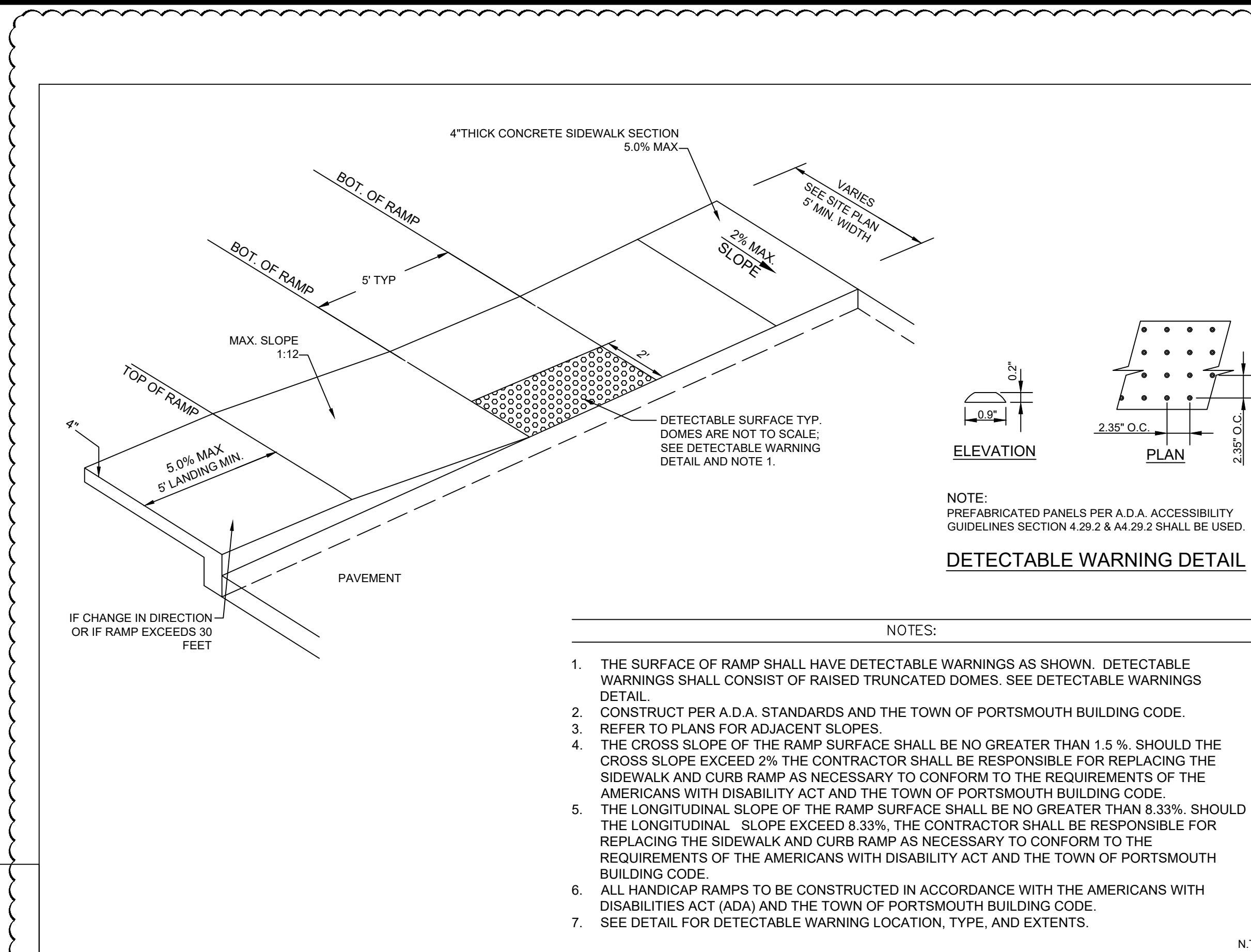
**SHEET TITLE**

**CONSTRUCTION DETAILS**

JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF

**SHEET NO.**

**CD-1**



**CORE STATES INC.**  
 12700 HILLCREST ROAD  
 DALLAS, TX 75220  
 (214) 377-5660  
[www.core-eng.com](http://www.core-eng.com)

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**CHASE**

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| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

**DOCUMENT**

**SITE PLAN APPROVAL FOR CHASE BANK**

**SITE LOCATION**  
 1574 WOODBURY AVENUE,  
 PORTSMOUTH, NH  
 03801

**ENGINEER SEAL**

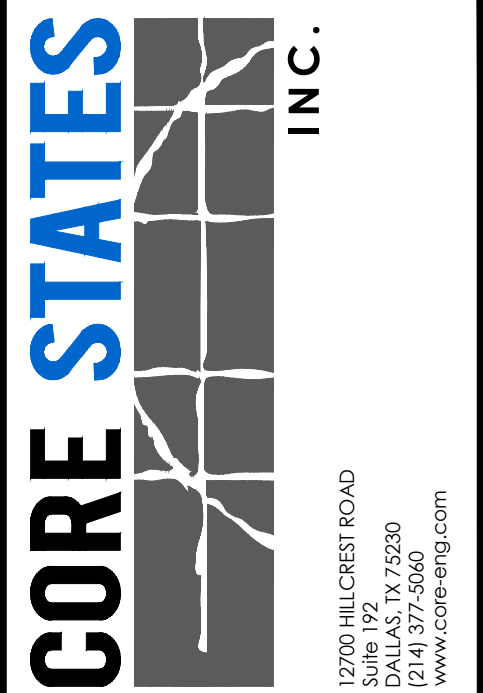
THOMAS C. PICKERING  
 No. 10218  
 LICENSED PROFESSIONAL ENGINEER  
 08/10/2020

**SHEET TITLE**  
 CONSTRUCTION DETAILS

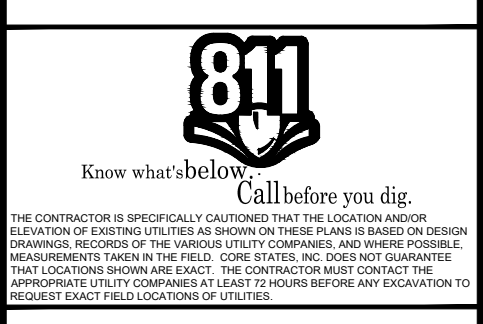
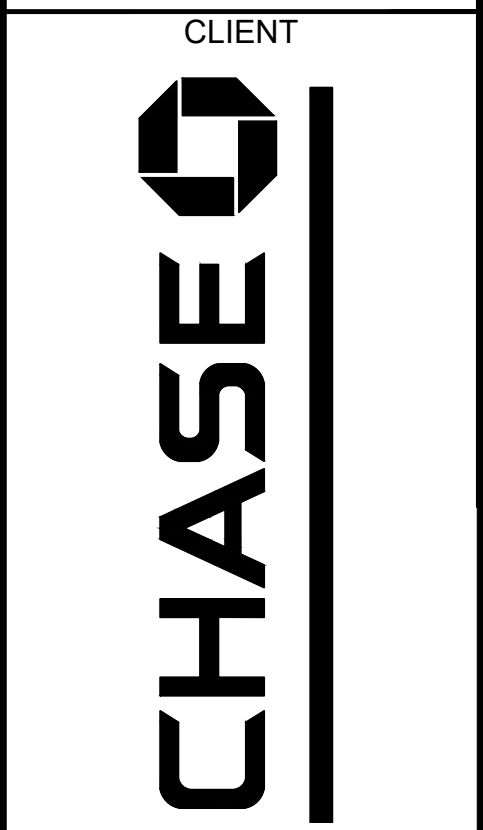
JOB #: JPM 27086  
 DATE: 07/01/2020  
 SCALE: AS NOTED  
 DRAWN BY: MAL  
 CHECKED BY: KGF

SHEET NO.  
**CD-2**





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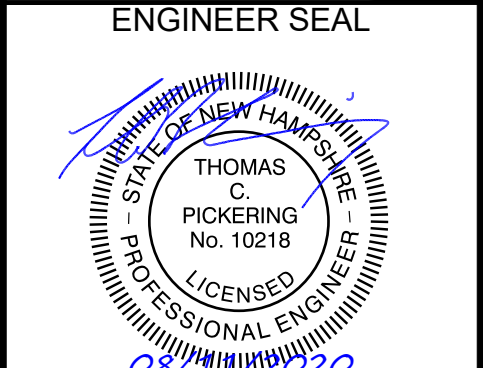


| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |
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DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

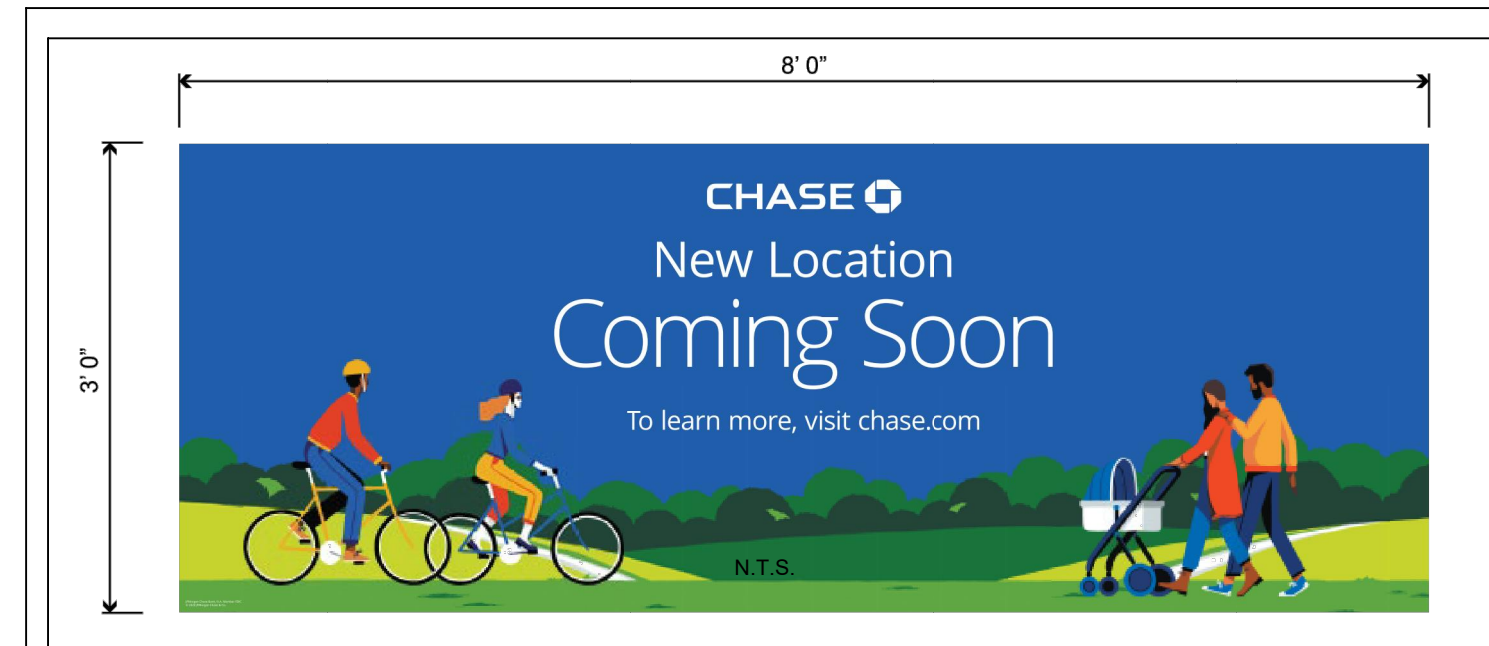
SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801



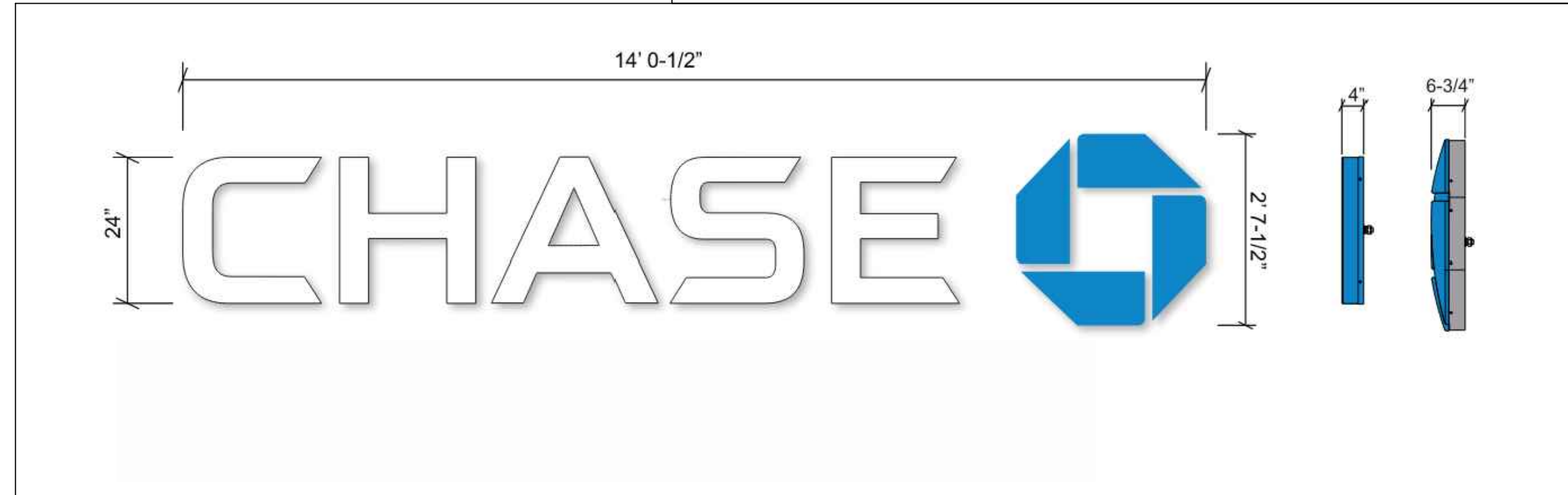
SHEET TITLE  
CONSTRUCTION DETAILS

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| JOB #:      | JPM 27086  |
| DATE:       | 07/12/2020 |
| SCALE:      | AS NOTED   |
| DRAWN BY:   | MAL        |
| CHECKED BY: | KGf        |

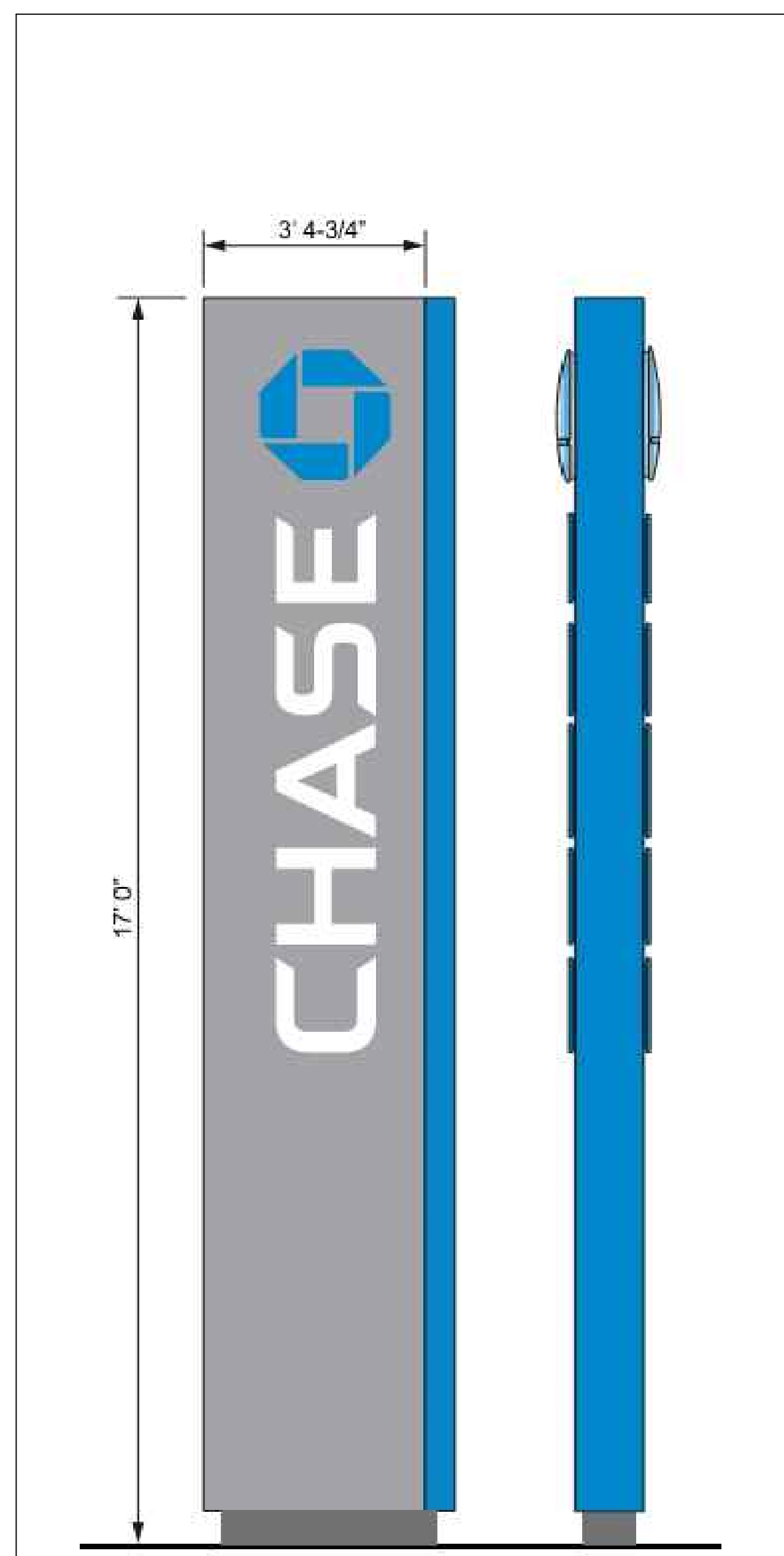
SHEET NO.  
**CD-4**



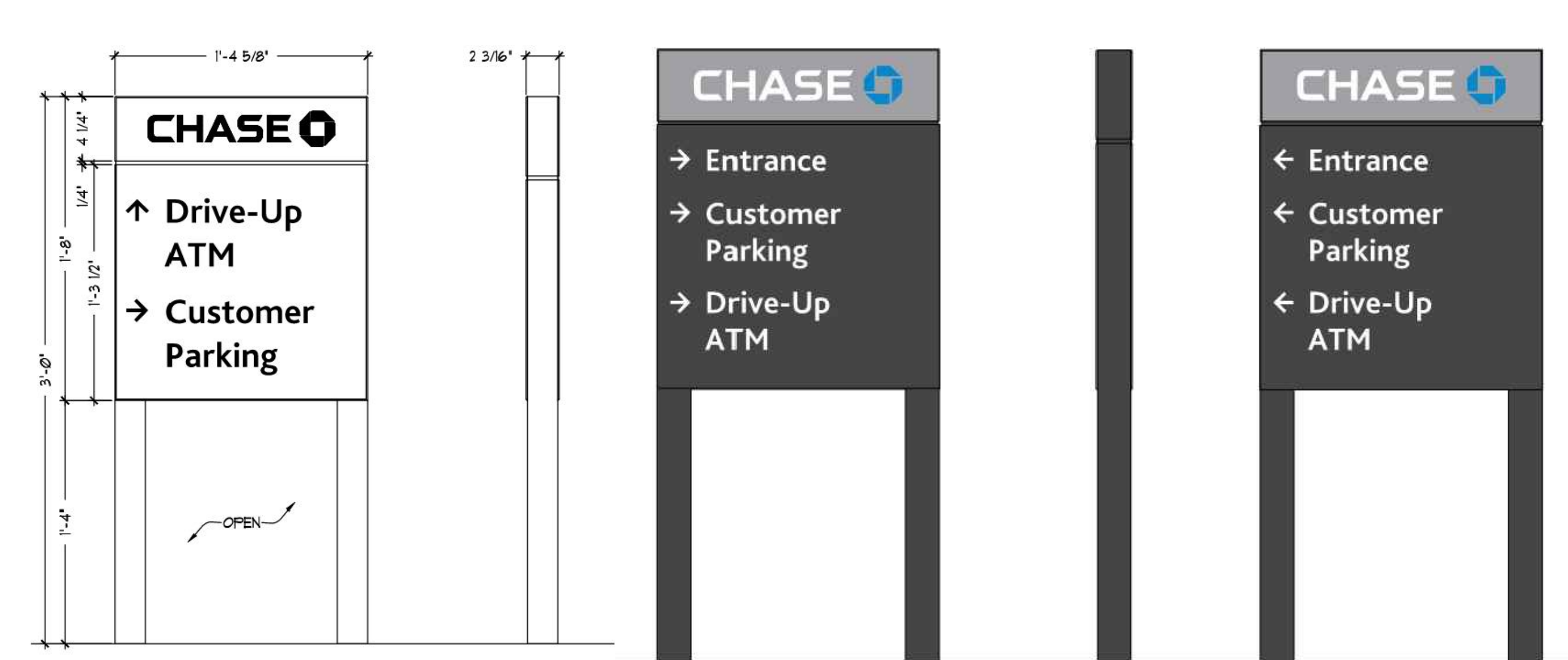
CHASE "COMING SOON" SIGN - 24 SF



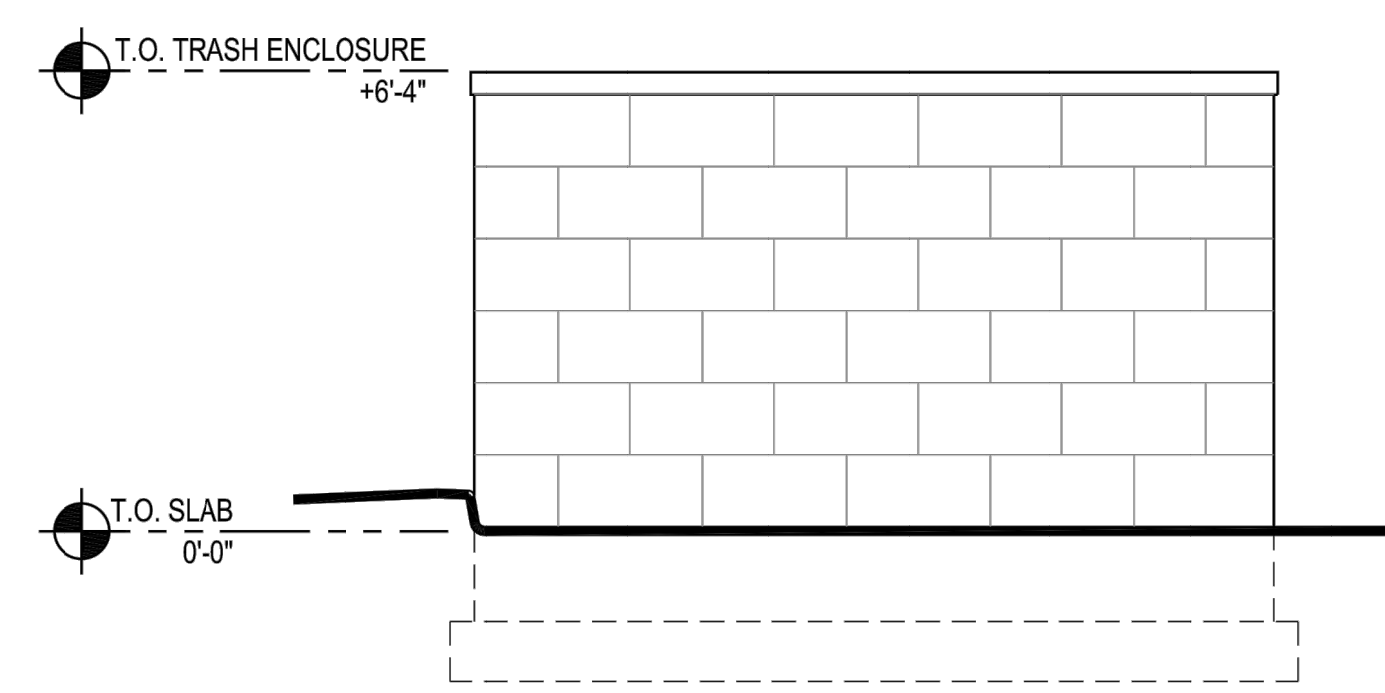
CHASE WALL SIGN - 36.9 SF  
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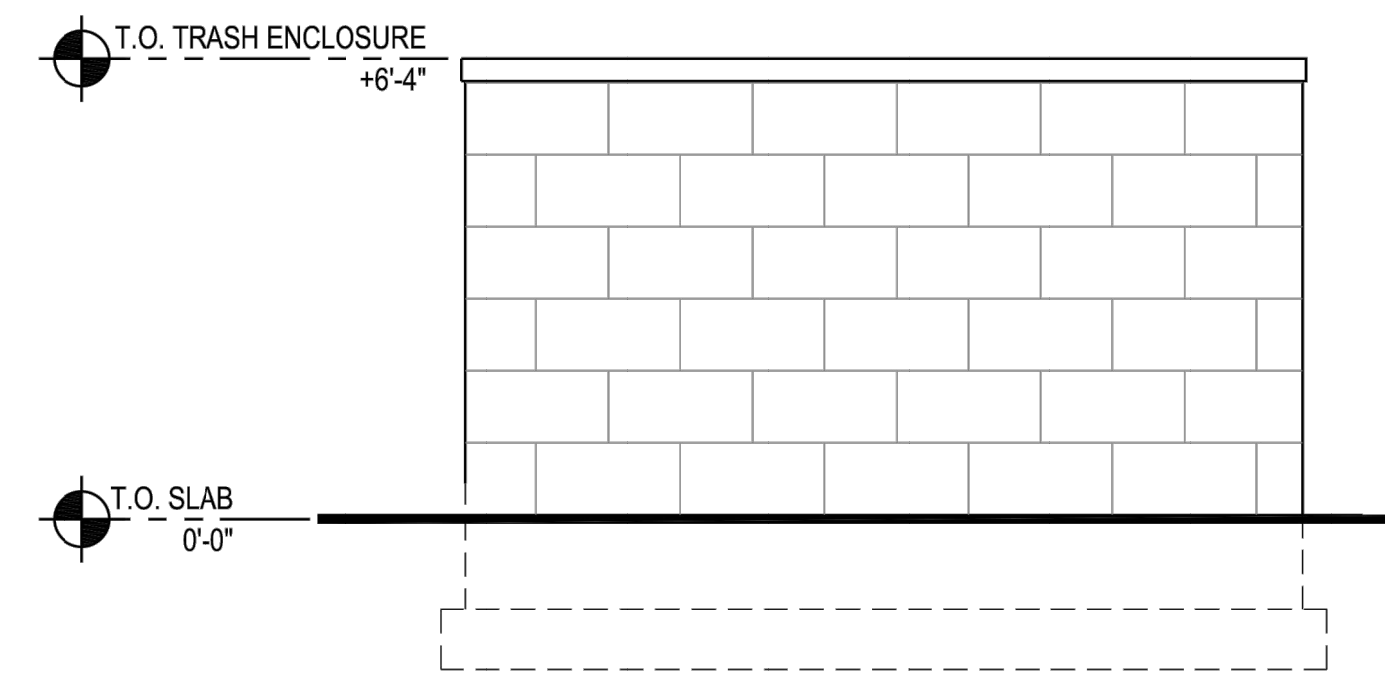
CHASE PYLON SIGN - 57.73 SF  
N.T.S.



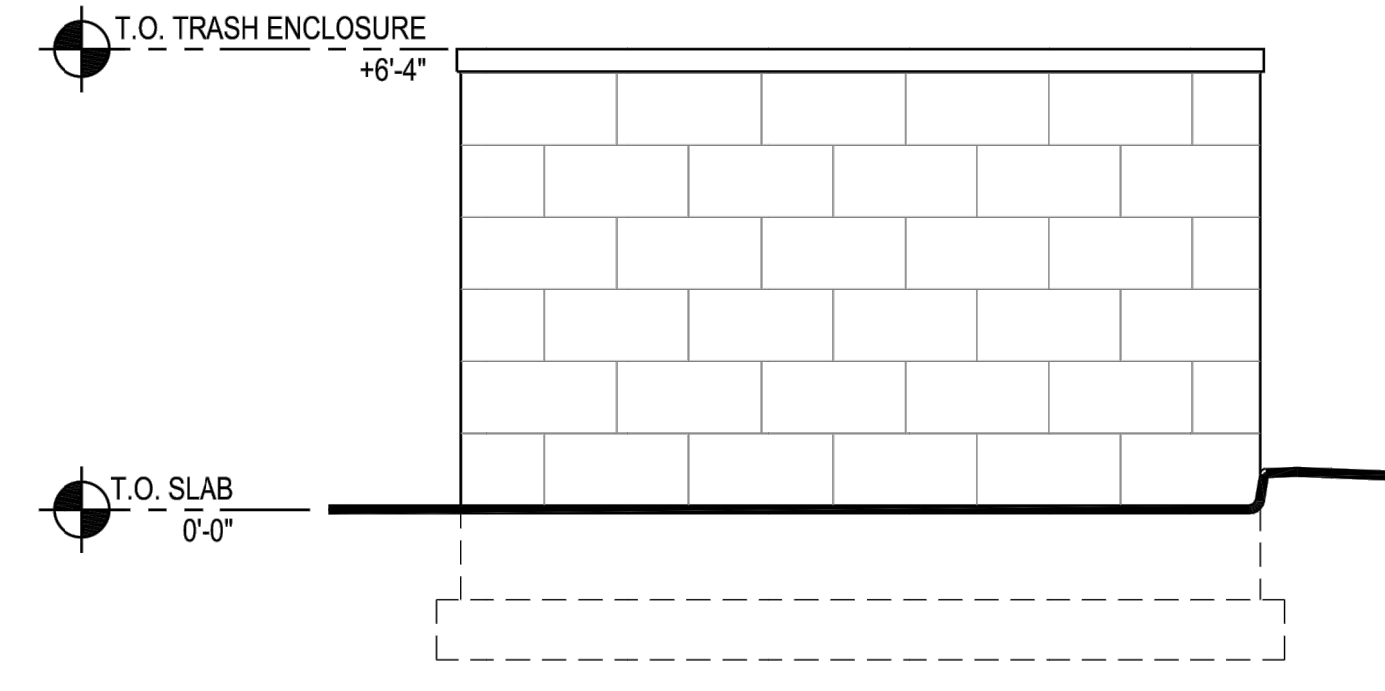
CHASE DIRECTIONAL SIGN - 2.3 SF  
N.T.S.



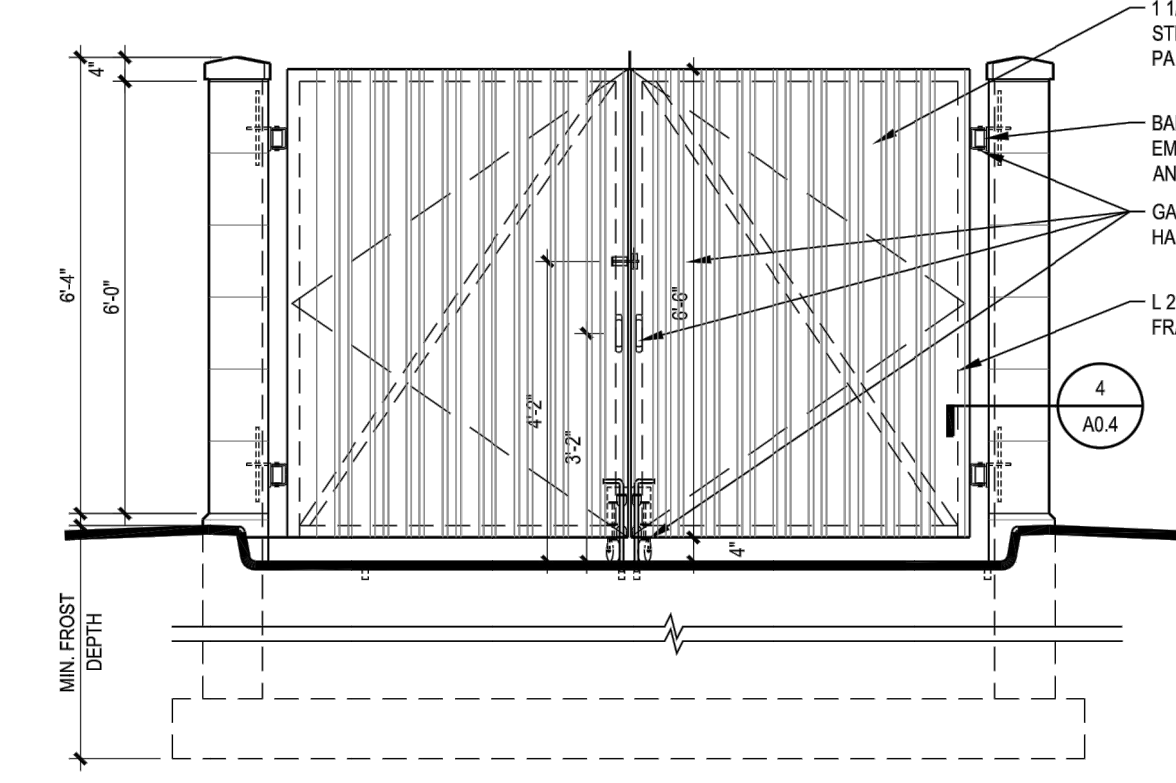
5 TRASH ENCLOSURE - ELEVATION  
A0.6 3/8" = 1'-0"



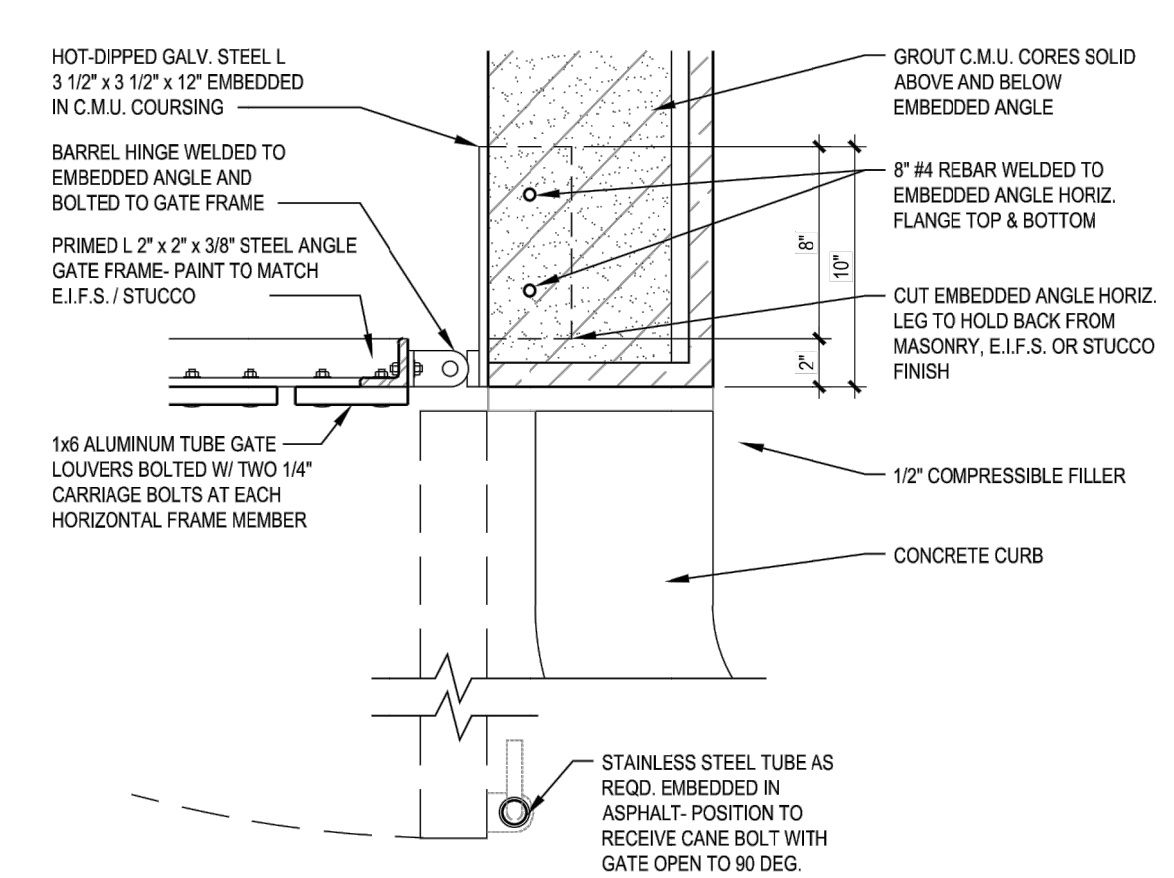
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A0.6 3/8" = 1'-0"



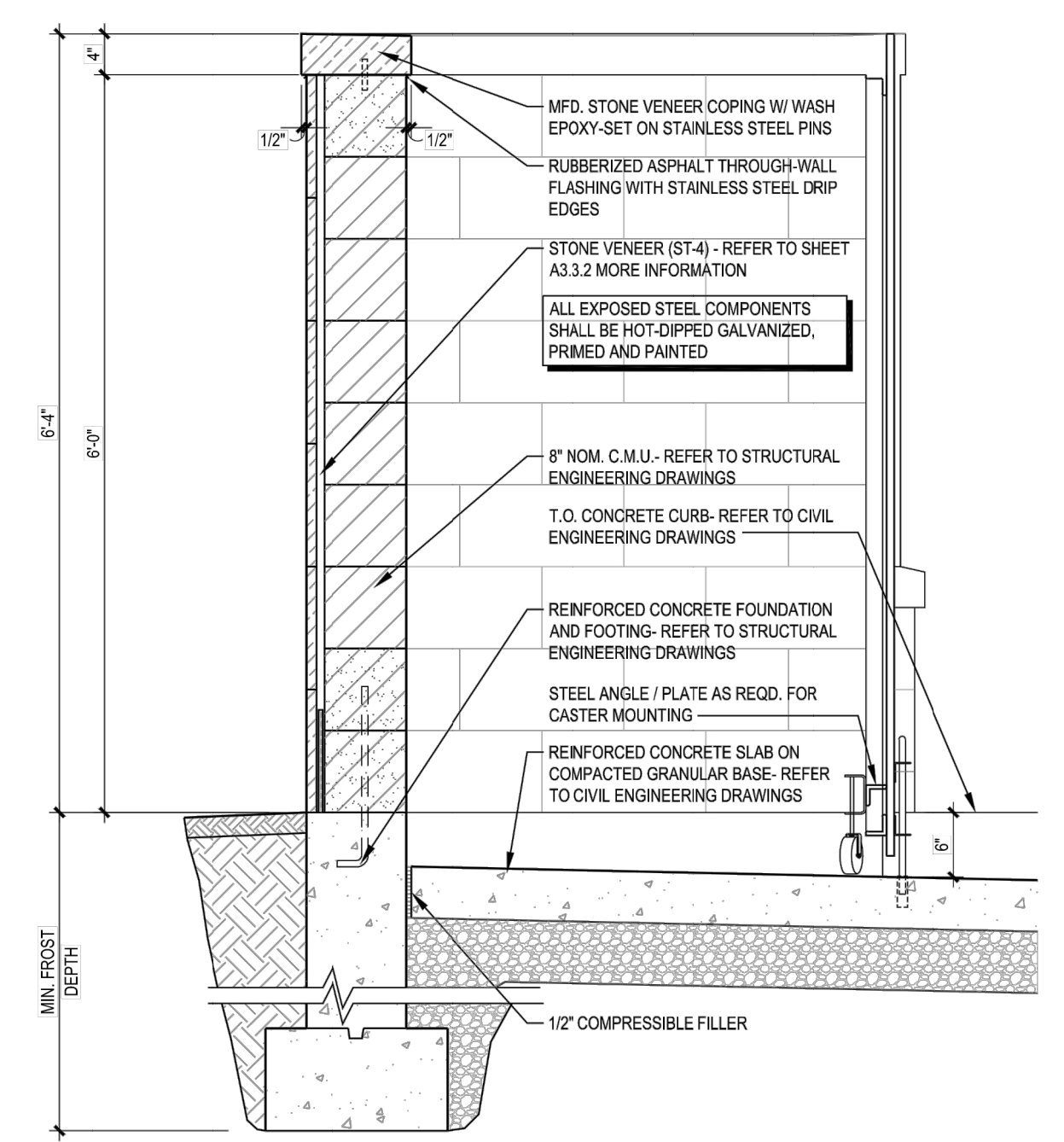
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A0.6 3/8" = 1'-0"



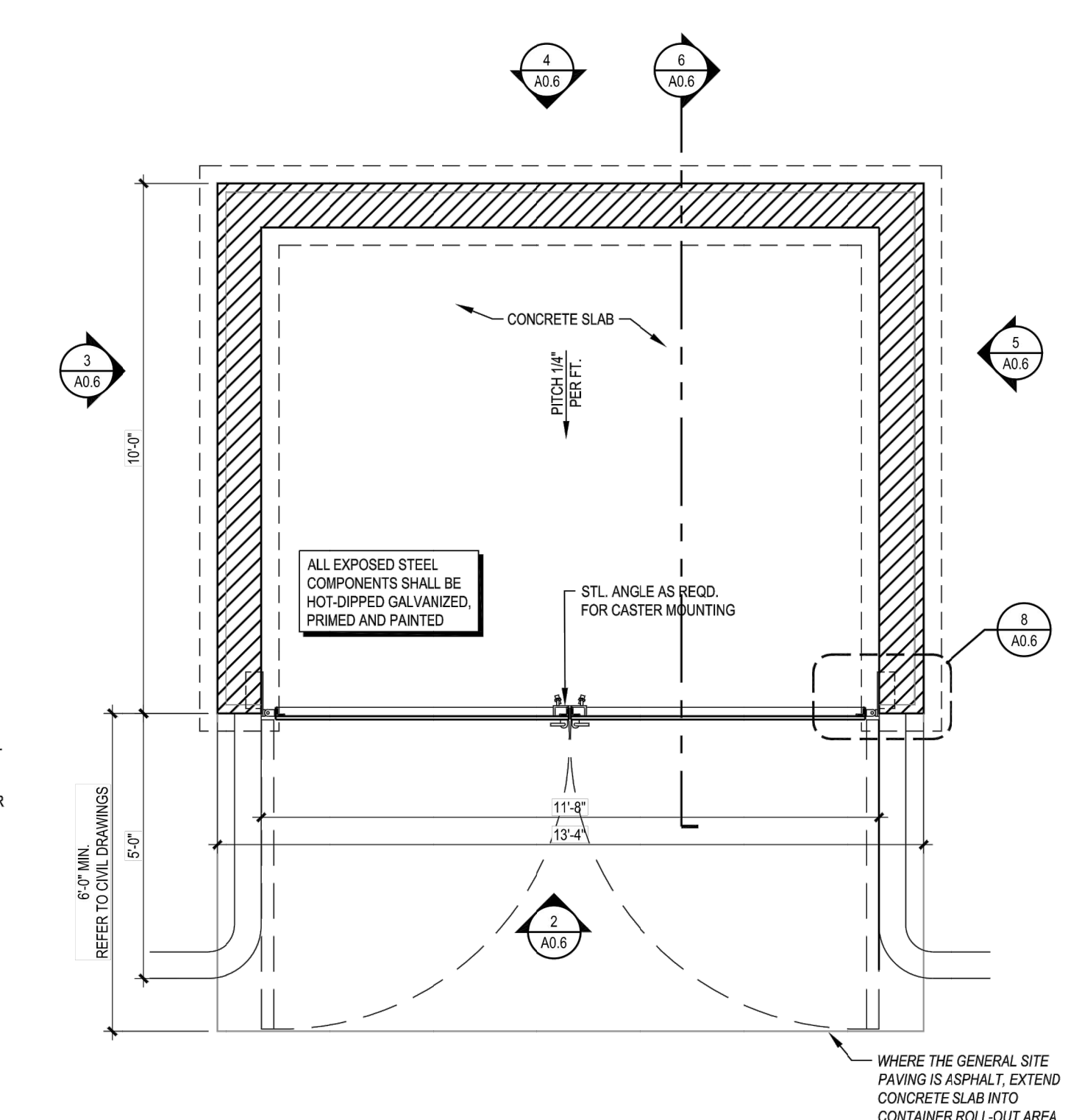
2 TRASH ENCLOSURE - ELEVATION  
A0.6 3/8" = 1'-0"



8 TRASH ENCLOSURE - CORNER DETAIL  
A0.6 1 1/2" = 1'-0"



6 TRASH ENCLOSURE - SECTION  
A0.6 3/4" = 1'-0"



1 TRASH ENCLOSURE - PLAN  
A0.6 3/8" = 1'-0"

**CORE STATES** INC.  
12700 HILLCREST ROAD  
DALLAS, TX 75220  
(214) 377-5860  
www.core-eng.com

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CLIENT  
**CHASE**

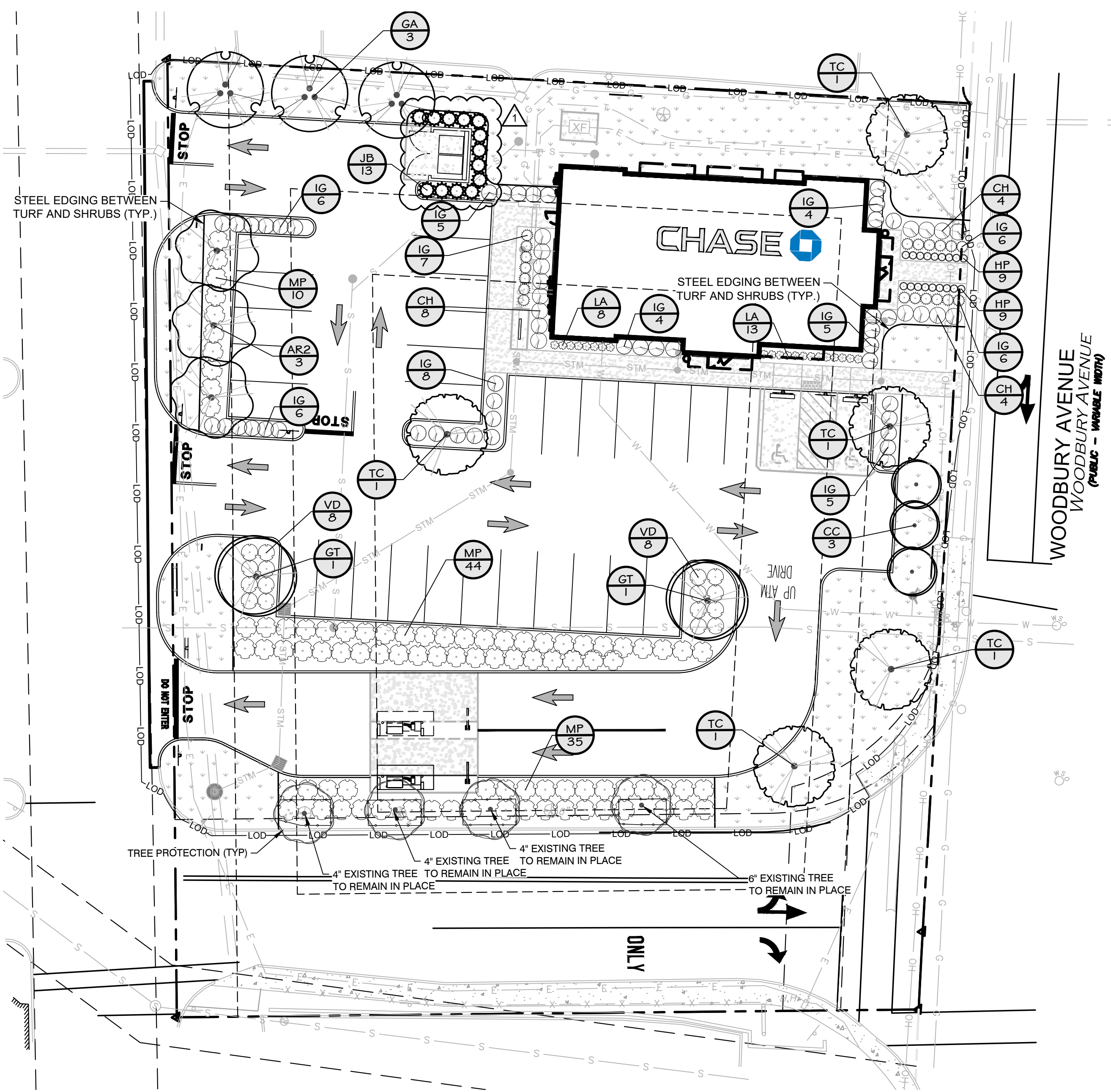
811  
Know what's below. Call before you dig.  
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| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TAC COMMENTS | MAL |

DOCUMENT  
SITE PLAN APPROVAL FOR CHASE BANK  
SITE LOCATION  
1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801

ENGINEER SEAL  
THOMAS C. PICKERING  
No. 102218  
PROFESSIONAL ENGINEER  
LICENSED  
08/11/2019-08/11/2020

SHEET TITLE  
CONSTRUCTION DETAILS  
JOB #: JPM 27086  
DATE: 07/1/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF  
**CD-5**



PLANT SCHEDULE

| TREES         | CODE | BOTANICAL / COMMON NAME  | CAL       | CONT. | SIZE    | QTY      |
|---------------|------|--|-----------|-------|---------|----------|
|               | AR2  | Acer rubrum 'Red Sunset'<br>Red Sunset Maple                                 | 2" Cal.   | B&B   | 8'-10'  | 3        |
|               | CC   | Cercis canadensis<br>Eastern Redbud Multi-trunk - 3-5 canes                  | 1.5" Cal. | B&B   | 6'-8'   | 3        |
|               | GA   | Ginkgo biloba 'Autumn Gold' TM<br>Autumn Gold Maidenhair Tree                | 2" Cal.   | B&B   | 8'-10'  | 3        |
|               | GT   | Gleditsia triacanthos inermis 'Skycole' TM<br>Skyline Thornless Honey Locust | 2" Cal.   | B&B   | 8'-10'  | 2        |
|               | TC   | Tilia cordata<br>Littleaf Linden   | 2" Cal.   | B&B   | 8'-10'  | 5        |
| SHRUBS        | CODE | BOTANICAL / COMMON NAME  | CONTAINER | SIZE  | SPACING | QTY      |
|               | CH   | Clethra alnifolia 'Hummingbird'<br>Summersweet                               | 5 gal.    |       |         | 16       |
|               | HP   | Hypericum prolificum<br>Broombrush   | 3 gal.    |       |         | 18       |
|               | IG   | Ilex glabra 'Shamrock'<br>Inkberry   | 5 gal.    |       |         | 62       |
|               | JB   | Juniperus chinensis 'Blue Point'<br>Blue Point Juniper                       | 7 gal.    | 4'    |         | 13       |
|               | LA   | Lavandula angustifolia<br>English Lavender                                   | 3 gal.    |       |         | 21       |
|               | MP   | Myrica pensylvanica 'Bobzam'<br>Bobee Northern Bayberry                      | 5 gal.    |       |         | 89       |
|               | VD   | Viburnum dentatum 'Arrowwood'<br>Arrowwood Viburnum                          | 5 gal.    |       |         | 16       |
| GROUND COVERS | CODE | BOTANICAL / COMMON NAME  | CONT.     | SIZE  | SPACING | QTY      |
|               | TURF | Poa pratensis<br>Kentucky Bluegrass  | sod       |       |         | 7,722 sf |

**NOTES:**

THE PROPERTY OWNER AND ALL FUTURE PROPERTY OWNERS SHALL BE RESPONSIBLE FOR THE MAINTENANCE, REPAIR, AND REPLACEMENT OF ALL SCREENING AND LANDSCAPE MATERIALS.

ALL PLANT MATERIAL SHALL BE TENDED AND MAINTAINED IN A HEALTHY GROWING CONDITION, REPLACED WHEN NECESSARY, AND KEPT FREE OF REFUSE AND DEBRIS. ALL REQUIRED FENCES AND WALLS SHALL BE MAINTAINED IN GOOD REPAIR.

THE PROPERTY OWNER SHALL BE RESPONSIBLE TO REMOVE AND REPLACE DEAD OR DISEASED PLANT MATERIALS IMMEDIATELY WITH THE SAME TYPE, SIZE, AND QUANTITY OF PLANT MATERIALS ORIGINALLY INSTALLED, UNLESS ALTERNATIVE PLANTINGS ARE REQUESTED, JUSTIFIED AND APPROVED BY THE PLANNING BOARD OR PLANNING DIRECTOR.

**ROOT BARRIERS**

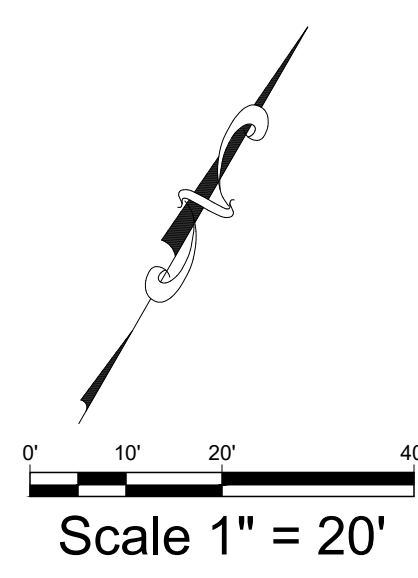
THE CONTRACTOR SHALL INSTALL ROOT BARRIERS NEAR ALL NEWLY-PLANTED TREES THAT ARE LOCATED WITHIN FIVE (5) FEET OF PAVING OR CURBS. ROOT BARRIERS SHALL BE "CENTURY" OR "DEEP-ROOT 24" DEEP PANELS (OR EQUAL). BARRIERS SHALL BE LOCATED IMMEDIATELY ADJACENT TO HARDSCAPE. INSTALL PANELS PER MANUFACTURER'S RECOMMENDATIONS. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR USE ROOT BARRIERS OF A TYPE THAT COMPLETELY ENCIRCLE THE ROOTBALL.

**MULCHES**

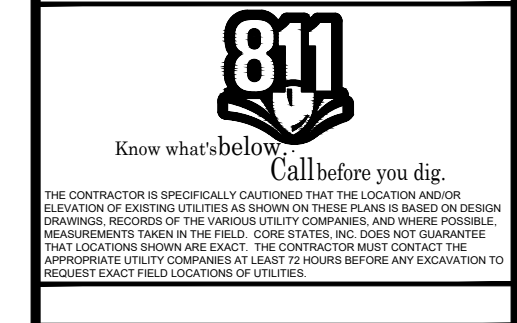
AFTER ALL PLANTING IS COMPLETE, CONTRACTOR SHALL INSTALL 3" THICK LAYER OF 1-1/2" SHREDDED WOOD MULCH, NATURAL (UNDYED), OVER LANDSCAPE FABRIC IN ALL PLANTING AREAS (EXCEPT FOR TURF AND SEEDED AREAS). CONTRACTOR SHALL SUBMIT SAMPLES OF ALL MULCHES TO LANDSCAPE ARCHITECT AND OWNER FOR APPROVAL PRIOR TO CONSTRUCTION. ABSOLUTELY NO EXPOSED GROUND SHALL BE LEFT SHOWING ANYWHERE ON THE PROJECT AFTER MULCH HAS BEEN INSTALLED (SUBJECT TO THE CONDITIONS AND REQUIREMENTS OF THE "GENERAL GRADING AND PLANTING NOTES" AND SPECIFICATIONS).

GENERAL GRADING AND PLANTING NOTES

- BY SUBMITTING A PROPOSAL FOR THE LANDSCAPE PLANTING SCOPE OF WORK, THE CONTRACTOR CONFIRMS THAT HE HAS READ, AND WILL COMPLY WITH, THE ASSOCIATED NOTES, SPECIFICATIONS, AND DETAILS WITH THIS PROJECT.
- THE GENERAL CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL EXISTING VEGETATION (EXCEPT WHERE NOTED TO REMAIN).
- IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, "FINISH GRADE" REFERS TO THE FINAL ELEVATION OF THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE GRADING PLANS.
  - BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE ROUGH GRADES OF ALL LANDSCAPE AREAS ARE WITHIN +0.1' OF FINISH GRADE. SEE SPECIFICATIONS FOR MORE DETAILED INSTRUCTION ON TURF AREA AND PLANTING BED PREPARATION.
  - CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON GRADING PLANS, AND CONSTRUCT AND MAINTAIN SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT. ALL LANDSCAPE AREAS SHALL HAVE POSITIVE DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM SLOPE SPECIFIED IN THE REPORT AND ON THE GRADING PLANS, AND AREAS OF POTENTIAL PONDING SHALL BE REGRADED TO BLEND IN WITH THE SURROUNDING GRADES AND ELIMINATE PONDING POTENTIAL.
  - THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHER OR NOT THE EXPORT OF ANY SOIL WILL BE NEEDED, TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE AMOUNT OF SOIL AMENDMENTS TO BE ADDED (BASED ON A SOIL TEST PER SPECIFICATIONS), AND THE FINISH GRADES TO BE ESTABLISHED.
  - ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BELOW THE ADJACENT FINISH SURFACE. IN ORDER TO ALLOW FOR PROPER MULCH DEPTH, TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS.
  - ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BELOW THE FINISH SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS.
- SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BETWEEN THE GRADING PLANS, GEOTECHNICAL REPORT, THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS TO THE ATTENTION OF THE LANDSCAPE ARCHITECT, GENERAL CONTRACTOR, AND OWNER.
- ALL PLANT LOCATIONS ARE DIAGRAMMATIC. ACTUAL LOCATIONS SHALL BE VERIFIED WITH THE LANDSCAPE ARCHITECT OR DESIGNER PRIOR TO PLANTING. THE LANDSCAPE CONTRACTOR SHALL ENSURE THAT ALL REQUIREMENTS OF THE PERMITTING AUTHORITY ARE MET (I.E., MINIMUM PLANT QUANTITIES, PLANTING METHODS, TREE PROTECTION METHODS, ETC.).
  - THE LANDSCAPE CONTRACTOR IS RESPONSIBLE FOR DETERMINING PLANT QUANTITIES; PLANT QUANTITIES SHOWN ON LEGENDS AND CALLOUTS ARE FOR GENERAL INFORMATION ONLY. IN THE EVENT OF A DISCREPANCY BETWEEN THE PLAN AND THE PLANT LEGEND, THE PLANT QUANTITY AS SHOWN ON THE PLAN (FOR INDIVIDUAL SYMBOLS) OR CALLOUT (FOR GROUNDCOVER PATTERNS) SHALL TAKE PRECEDENCE.
  - NO SUBSTITUTIONS OF PLANT MATERIALS SHALL BE ALLOWED WITHOUT THE WRITTEN PERMISSION OF THE LANDSCAPE ARCHITECT. IF SOME OF THE PLANTS ARE NOT AVAILABLE, THE LANDSCAPE CONTRACTOR SHALL NOTIFY THE LANDSCAPE ARCHITECT IN WRITING (VIA PROPER CHANNELS).
  - THE CONTRACTOR SHALL, AT A MINIMUM, PROVIDE REPRESENTATIVE PHOTOS OF ALL PLANTS PROPOSED FOR THE PROJECT. THE CONTRACTOR SHALL ALLOW THE LANDSCAPE ARCHITECT AND THE OWNER/OWNER'S REPRESENTATIVE TO INSPECT AND APPROVE OR REJECT ALL PLANTS DELIVERED TO THE JOBSITE. REFER TO SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SUBMITTALS.
- THE CONTRACTOR SHALL MAINTAIN THE LANDSCAPE IN A HEALTHY CONDITION FOR 90 DAYS AFTER ACCEPTANCE BY THE OWNER. REFER TO SPECIFICATIONS FOR CONDITIONS OF ACCEPTANCE FOR THE START OF THE MAINTENANCE PERIOD, AND FOR FINAL ACCEPTANCE AT THE END OF THE MAINTENANCE PERIOD.
- SEE SPECIFICATIONS AND DETAILS FOR FURTHER REQUIREMENTS.



DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK. IT IS USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED. USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.

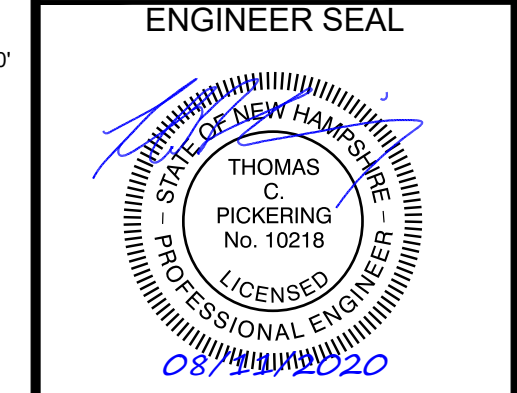


| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TRC COMMENTS | MAL |

DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801



SHEET TITLE  
LANDSCAPE PLANTING

JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE:  
DRAWN BY: MAL  
CHECKED BY: KGF  
SHEET NO.  
LP-1

**PLANTING SPECIFICATIONS**

**GENERAL**

- A. QUALIFICATIONS OF LANDSCAPE CONTRACTOR**
- ALL LANDSCAPE WORK SHOWN ON THESE PLANS SHALL BE PERFORMED BY A SINGLE FIRM SPECIALIZING IN LANDSCAPE PLANTING.
  - A LIST OF SUCCESSFULLY COMPLETED PROJECTS OF THIS TYPE, SIZE AND NATURE MAY BE REQUESTED BY THE OWNER FOR FURTHER QUALIFICATION MEASURES.
  - THE LANDSCAPE CONTRACTOR SHALL HOLD A VALID NURSERY AND FLORAL CERTIFICATE ISSUED BY THE TEXAS DEPARTMENT OF AGRICULTURE, AS WELL AS OPERATE UNDER A COMMERCIAL PESTICIDE APPLICATOR LICENSE ISSUED BY EITHER THE TEXAS DEPARTMENT OF AGRICULTURE OR THE TEXAS STRUCTURAL PEST CONTROL BOARD.
- B. SCOPE OF WORK**
- WORK COVERED BY THESE SECTIONS INCLUDES THE FURNISHING AND PAYMENT OF ALL MATERIALS, LABOR, SERVICES, EQUIPMENT, LICENSES, TAXES AND ANY OTHER ITEMS THAT ARE NECESSARY FOR THE EXECUTION, INSTALLATION AND COMPLETION OF ALL WORK, SPECIFIED HEREIN AND / OR SHOWN ON THE LANDSCAPE PLANS, NOTES, AND DETAILS.
  - ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LAWS, CODES AND REGULATIONS REQUIRED BY AUTHORITIES HAVING JURISDICTION OVER SUCH WORK, INCLUDING ALL INSPECTIONS AND PERMITS REQUIRED BY FEDERAL, STATE AND LOCAL AUTHORITIES IN SUPPLY, TRANSPORTATION AND INSTALLATION OF MATERIALS.
  - THE LANDSCAPE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UNDERGROUND UTILITY LINES (WATER, SEWER, ELECTRICAL, TELEPHONE, GAS, CABLE, TELEVISION, ETC.) PRIOR TO THE START OF ANY WORK.

**PRODUCTS**

- A. ALL MANUFACTURED PRODUCTS SHALL BE NEW.**
- B. CONTAINER AND BALLED-AND-BURLAPPED PLANTS:**
- FURNISH NURSERY-GROWN PLANTS COMPLYING WITH ANSI Z60.1-2014. PROVIDE WELL-SHAPED, FULLY BRANCHED, HEALTHY, VIGOROUS STOCK FREE OF DISEASE, INSECTS, EGGS, LARVAE, AND DEFECTS SUCH AS KNOTS, SUN SCALD, INJURIES, ABRASIONS, AND DISFIGUREMENT. ALL PLANTS WITHIN A SPECIES SHALL HAVE SIMILAR SIZE, AND SHALL BE OF A FORM TYPICAL FOR THE SPECIES. ALL TREES SHALL BE OBTAINED FROM SOURCES WITHIN 200 MILES OF THE PROJECT SITE, AND WITH SIMILAR CLIMATIC CONDITIONS.
  - ROOT SYSTEMS SHALL BE HEALTHY, DENSELY BRANCHED ROOT SYSTEMS, NON-POT-BOUND, FREE FROM ENCIRCLING AND/OR GIRDLING ROOTS, AND FREE FROM ANY OTHER ROOT DEFECTS (SUCH AS S-HAPED ROOTS).
  - TREES MAY BE PLANTED FROM CONTAINERS OR BALLED-AND-BURLAPPED (BAB), UNLESS SPECIFIED ON THE PLANTING LEGEND. BARE-ROOT TREES ARE NOT ACCEPTABLE.
  - ANY PLANT DEEMED UNACCEPTABLE BY THE LANDSCAPE ARCHITECT OR OWNER SHALL BE IMMEDIATELY REMOVED FROM THE SITE AND SHALL BE REPLACED WITH AN ACCEPTABLE PLANT OF LIKE TYPE AND SIZE AT THE CONTRACTOR'S OWN EXPENSE. ANY PLANTS APPEARING TO BE UNHEALTHY, EITHER IF DETERMINED TO STILL BE ALIVE, SHALL NOT BE ACCEPTED. THE LANDSCAPE ARCHITECT AND OWNER SHALL BE THE SOLE JUDGES AS TO THE ACCEPTABILITY OF PLANT MATERIAL.
  - ALL TREES SHALL BE STANDARD IN FORM, UNLESS OTHERWISE SPECIFIED. TREES WITH CENTRAL LEADERS WILL BE ACCEPTED IF LEADER IS DAMAGED OR REMOVED. PRUNE ALL DAMAGED TWIGS AFTER PLANTING.
  - CALIPER MEASUREMENTS FOR STANDARD (SINGLE TRUNK) TREES SHALL BE AS FOLLOWS: SIX INCHES ABOVE THE ROOT FLARE FOR TREES UP TO AND INCLUDING FOUR INCHES IN CALIPER, AND TWELVE INCHES ABOVE THE ROOT FLARE FOR TREES EXCEEDING FOUR INCHES IN CALIPER.
  - MULTI-TRUNK TREES SHALL BE MEASURED BY THEIR OVERALL HEIGHT, MEASURED FROM THE TOP OF THE ROOT BALL, WHERE CALIPER MEASUREMENTS ARE USED. THE CALIPER SHALL BE CALCULATED AS ONE-HALF OF THE SUM OF THE CALIPER OF THE THREE LARGEST TRUNKS.
  - ANY TREE OR SHRUB SHOWN TO HAVE EXCESS SOIL PLACED ON TOP OF THE ROOT BALL, SO THAT THE ROOT FLARE HAS BEEN COMPLETELY COVERED, SHALL BE REJECTED.
- C. SOD: PROVIDE WELL-ROOTED SOD OF THE VARIETY NOTED ON THE PLANS. SOD SHALL BE CUT FROM HEALTHY, MATURE TURF WITH SOIL THICKNESS OF 3/4" TO 1". EACH PALLET OF SOD SHALL BE ACCOMPANIED BY A CERTIFICATE FROM SUPPLIER STATING THE COMPOSITION OF THE SOD.**
- D. TOPSOIL: SANDY TO CLAY LOAM TOPSOIL, FREE OF STONES LARGER THAN 1/2" INCH, FOREIGN MATTER, PLANTS, ROOTS, AND SEEDS.**
- E. COMPOST: WELL-COMPOSTED, STABLE, AND WEED-FREE ORGANIC MATTER, pH RANGE OF 5.5 TO 8; MOISTURE CONTENT 35 TO 55 PERCENT BY WEIGHT; 100 PERCENT PASSING THROUGH 3/4-INCH SIEVE; SOLUBLE SALT CONTENT OF 5 TO 10 DECISEMENS/M; NOT EXCEEDING 0.5 PERCENT INERT CONTAMINANTS AND FREE OF SUBSTANCES TOXIC TO PLANTINGS. NO MANURE OR ANIMAL-BASED PRODUCTS SHALL BE USED.**
- F. FERTILIZER: GRANULAR FERTILIZER CONSISTING OF NITROGEN, PHOSPHORUS, POTASSIUM, AND OTHER NUTRIENTS IN PROPORTIONS, AMOUNTS, AND RELEASE RATES RECOMMENDED IN A SOIL REPORT FROM A QUALIFIED SOIL-TESTING AGENCY (SEE BELOW).**
- G. MULCH: SIZE AND TYPE AS INDICATED ON PLANS, FREE FROM DELETERIOUS MATERIALS AND SUITABLE AS TOP DRESSING OF TREES AND SHRUBS.**
- H. TREE STAKING AND GUYING**
- STAKES: 6" LONG GREEN METAL T-POSTS.
  - GUY AND TIE WIRE: ASTM A 641, CLASS 1, GALVANIZED-STEEL WIRE, 2-STRAND, TWISTED, 0.106 INCH DIAMETER.
  - STRAP CHAFING GUARD: REINFORCED NYLON OR CANVAS AT LEAST 1-1/2 INCH WIDE, WITH GROMMETS TO PREVENT DAMAGE.
- I. STEEL EDGING: PROFESSIONAL STEEL EDGING, 14 GAUGE THICK X 4 INCHES WIDE, FACTORY PAINTED DARK GREEN. ACCEPTABLE MANUFACTURERS INCLUDE COL-MET OR APPROVED EQUAL.**
- M. PRE-EMERGENT GRANULAR, NON-STAINING PRE-EMERGENT HERBICIDE THAT IS LABELED FOR THE SPECIFIC ORNAMENTALS OR TURF ON WHICH IT WILL BE USED. PRE-EMERGENT HERBICIDES SHALL BE APPLIED PER THE MANUFACTURER'S LABELED RATES.**

**METHODS**

- A. SOIL PREPARATION**
- BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE GRADE OF ALL LANDSCAPE AREAS ARE WITHIN +0.1' OF FINISH GRADE. IF THE CONTRACTOR SHALL NOTIFY THE OWNER IMMEDIATELY SHOULD ANY DISCREPANCIES EXIST.
  - SOIL TESTING:
    - AFTER FINISH GRADES HAVE BEEN ESTABLISHED, CONTRACTOR SHALL HAVE SOIL SAMPLES FROM THE PROJECT'S LANDSCAPE AREAS TESTED BY AN ESTABLISHED SOIL TESTING LABORATORY. EACH SAMPLE SUBMITTED TO THE LAB SHALL CONTAIN NO LESS THAN ONE QUART OF SOIL, TAKEN FROM BETWEEN THE SOIL SURFACE AND 6" DEPTH. IF NO SAMPLE LOCATIONS ARE INDICATED ON THE PLANS, THE CONTRACTOR SHALL TAKE A MINIMUM OF THREE SAMPLES FROM VARIOUS REPRESENTATIVE LOCATIONS FOR TESTING.
    - THE CONTRACTOR SHALL HAVE THE SOIL TESTING LABORATORY PROVIDE RESULTS FOR THE FOLLOWING: SOIL TEXTURAL CLASS, GENERAL SOIL FERTILITY, pH, ORGANIC MATTER CONTENT, SALT (CEC), LIME, SODIUM ADSORPTION RATIO (SAR) AND BORON CONTENT.
    - THE CONTRACTOR SHALL ALSO SUBMIT THE PROJECT'S PLANT LIST TO THE LABORATORY ALONG WITH THE SOIL SAMPLES.
    - THE SOIL REPORT PRODUCED BY THE LABORATORY SHALL CONTAIN RECOMMENDATIONS FOR THE FOLLOWING (AS APPROPRIATE): SEPARATE SOIL PREPARATION AND BACKFILL MIX RECOMMENDATIONS FOR GENERAL ORNAMENTAL PLANTS, SERIES PLANTS, TURF, AND NATIVE SEED, AS WELL AS PRE-PLANT FERTILIZER APPLICATIONS AND RECOMMENDATIONS FOR ANY OTHER SOIL RELATED ISSUES. THE REPORT SHALL ALSO PROVIDE A FERTILIZER PROGRAM FOR THE ESTABLISHMENT PERIOD AND FOR LONG-TERM MAINTENANCE.
  - THE CONTRACTOR SHALL INSTALL SOIL AMENDMENTS AND FERTILIZERS PER THE SOILS REPORT RECOMMENDATIONS. ANY CHANGE IN COST DUE TO THE SOILS REPORT RECOMMENDATIONS, EITHER INCREASE OR DECREASE, SHALL BE SUBMITTED TO THE OWNER WITH THE REPORT.
  - FOR BIDDING PURPOSES ONLY, THE SOIL PREPARATION SHALL CONSIST OF THE FOLLOWING:
    - TURF: INCORPORATE THE FOLLOWING AMENDMENTS INTO THE TOP 8" OF SOIL BY MEANS OF ROTOTILLING AFTER CROSS-RIPING:
      - NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F.
      - PREPLANT TURF FERTILIZER (10-20-10 OR SIMILAR, SLOW RELEASE, ORGANIC) - 15 LBS PER 1,000 S.F.
    - "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE
    - TREES, SHRUBS, AND PERENNIALS: INCORPORATE THE FOLLOWING AMENDMENTS INTO THE TOP 8" OF SOIL BY MEANS OF ROTOTILLING AFTER CROSS-RIPING:
      - NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F.
      - 12-12-12 FERTILIZER (OR SIMILAR, ORGANIC, SLOW RELEASE) - 10 LBS. PER CU. YD.
      - "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE
      - IRON SULPHATE - 2 LBS. PER CU. YD.
- IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, "FINISH GRADE" REFERS TO THE FINAL ELEVATION OF THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE GRADING PLANS.**
- BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE ROUGH GRADES OF ALL LANDSCAPE AREAS ARE WITHIN +0.1' OF FINISH GRADE. SEE SPECIFICATIONS FOR MORE DETAILED INSTRUCTION ON TURF AREA AND PLANTING BED PREPARATION.
  - CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON GRADING PLANS, AND CONSTRUCT AND MAINTAIN SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT. ALL LANDSCAPE AREAS SHALL HAVE POSITIVE DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM SLOPE SPECIFIED IN THE REPORT AND ON THE GRADING PLANS, AND AREAS OF POTENTIAL PONDING SHALL BE REGRADED TO BLEND IN WITH THE SURROUNDING GRADES AND ELIMINATE PONDING POTENTIAL.
  - THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHER OR NOT THE EXPORT OF ANY SOIL WILL BE NEEDED, TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE AMOUNT OF SOIL AMENDMENTS TO BE ADDED (BASED ON A SOIL TEST, PER SPECIFICATIONS), AND THE FINISH GRADES TO BE ESTABLISHED.
  - ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BELOW THE ADJACENT FINISH SURFACE, IN ORDER TO ALLOW FOR PROPER MULCH DEPTH. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS.
  - ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BELOW THE FINISH SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS.
  - SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BETWEEN THE GRADING PLANS, GEOTECHNICAL REPORT, THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS TO THE ATTENTION OF THE LANDSCAPE ARCHITECT, GENERAL CONTRACTOR, AND OWNER.
- ONCE SOIL PREPARATION IS COMPLETE, THE LANDSCAPE CONTRACTOR SHALL ENSURE THAT THERE ARE NO DEBRIS, TRASH, OR STONES LARGER THAN 1" REMAINING IN THE TOP 6" OF SOIL.**

**B. SUBMITTALS**

- THE CONTRACTOR SHALL PROVIDE SUBMITTALS AND SAMPLES, IF REQUIRED, TO THE LANDSCAPE ARCHITECT, AND RECEIVE APPROVAL IN WRITING FOR SUCH SUBMITTALS BEFORE WORK COMMENCES.
- SUBMITTALS SHALL INCLUDE PHOTOS OF PLANTS WITH A RULER OR MEASURING STICK FOR SCALE, PHOTOS OR SAMPLES OF ANY REQUIRED MULCHES, AND SOIL TEST RESULTS AND PREPARATION RECOMMENDATIONS FROM THE TESTING LAB (INCLUDING COMPOST AND FERTILIZER RATES AND TYPES, AND OTHER AMENDMENTS FOR TREE/SHRUB, TURF, AND SEED AREAS AS MAY BE APPROPRIATE).
- SUBMITTALS SHALL ALSO INCLUDE MANUFACTURER CUT SHEETS FOR PLANTING ACCESSORIES SUCH AS TREE STAKES AND TIES, EDGING, AND LANDSCAPE FABRICS (IF ANY).
- WHERE MULTIPLE ITEMS ARE SHOWN ON A PAGE, THE CONTRACTOR SHALL CLEARLY INDICATE THE ITEM BEING CONSIDERED.

**C. GENERAL PLANTING**

- REMOVE ALL NURSERY TAGS AND STAKES FROM PLANTS.
- EXCEPT IN AREAS TO BE PLANTED WITH ORNAMENTAL GRASSES, APPLY PRE-EMERGENT HERBICIDES AT THE MANUFACTURER'S RECOMMENDED RATE.
- TRENCHING NEAR PLANTING:
  - CONTRACTOR SHALL NOT DISTURB ROOTS 1-1/2" AND LARGER IN DIAMETER WITHIN THE CRITICAL ROOT ZONE (CRZ) OF EXISTING TREES, AND SHALL EXERCISE ALL POSSIBLE CARE AND PRECAUTIONS TO AVOID INJURY TO TREE ROOTS, TRUNKS, AND BRANCHES. THE CRZ IS DEFINED AS A CIRCULAR AREA EXTENDING OUTWARD FROM THE TREE TRUNK, WITH A RADIUS EQUAL TO 1" FOR EVERY 1" OF TRUNK DIAMETER-AT-BREAST-HEIGHT (4.5' ABOVE THE AVERAGE GRADE AT THE TROT IN).
  - ALL EXCAVATION WITHIN THE CRZ SHALL BE PERFORMED USING HAND TOOLS. NO MACHINE EXCAVATION OR TRENCHING OF ANY KIND SHALL BE ALLOWED WITHIN THE CRZ.
  - ALTER ALIGNMENT OF PIPE TO AVOID TREE ROOTS 1-1/2" AND LARGER IN DIAMETER. WHERE TREE ROOTS 1-1/2" AND LARGER IN DIAMETER ARE ENCOUNTERED IN THE FIELD, TUNNEL UNDER SUCH ROOTS. WRAP EXPOSED ROOTS WITH SEVERAL LAYERS OF BURLAP AND KEEP MOIST. CLOSE ALL TRENCHES WITHIN THE CANOPY EXPLINES WITHIN 24 HOURS AND SIDES OF TRENCH.
  - ALL SEVERED ROOTS SHALL BE HAND PRUNED WITH SHARP TOOLS AND ALLOWED TO AIR-DRY. DO NOT USE ANY SORT OF SEALERS OR WOUND PAINTS.

**D. TREE PLANTING**

- TREE PLANTING HOLES SHALL BE EXCAVATED TO MINIMUM WIDTH OF TWO TIMES THE WIDTH OF THE ROOTBALL, AND TO A DEPTH EQUAL TO THE DEPTH OF THE ROOTBALL LESS TWO TO FOUR INCHES. SCARIFY THE SIDES AND BOTTOM OF THE PLANTING HOLE PRIOR TO THE PLACEMENT OF THE TREE. REMOVE ANY GLAZING THAT MAY HAVE BEEN CAUSED DURING THE EXCAVATION OF THE HOLE.
- FOR CONTAINER AND BOX TREES, TO REMOVE ANY POTENTIALLY GIRDLING ROOTS AND OTHER ROOT DEFECTS, THE CONTRACTOR SHALL SHAVE A 1" LAYER OFF OF THE SIDES AND BOTTOM OF THE ROOTBALL OF ALL TREES, JUST BEFORE PLACING INTO THE PLANTING PIT. DO NOT "TEASE" ROOTS OUT FROM THE ROOTBALL.
- INSTALL THE TREE ON UNDISTURBED SUBGRADE SO THAT THE TOP OF THE ROOTBALL IS TWO TO FOUR INCHES ABOVE THE SURROUNDING GRADE.
- BACKFILL THE TREE HOLE UTILIZING THE EXISTING TOPSOIL FROM ON-SITE. ROCKS LARGER THAN 1" DIA. AND ALL OTHER DEBRIS SHALL BE REMOVED FROM THE SOIL PRIOR TO THE BACKFILL. SHOULD ADDITIONAL SOIL BE REQUIRED TO ACCOMPLISH THIS TASK, USE STORED TOPSOIL FROM ON-SITE OR IMPORT ADDITIONAL TOPSOIL FROM OFF-SITE AT NO ADDITIONAL COST TO THE OWNER. IMPORTED TOPSOIL SHALL BE OF SIMILAR TEXTURAL CLASS AND COMPOSITION IN THE ON-SITE SOIL. TREES SHALL NOT BE STAKED UNLESS LOCAL CONDITIONS (SUCH AS HEAVY WINDS) REQUIRE STAKES TO KEEP TREES UPRIGHT. SHOULD STAKING BE REQUIRED, THE TOTAL NUMBER OF TREE STAKES (BEYOND THE MINIMUMS LISTED BELOW) WILL BE LEFT TO THE LANDSCAPE CONTRACTOR'S DISCRETION. SHOULD ANY TREES FALL OFF OF THE STAKES, THE CONTRACTOR SHALL STRAIGHTEN THE TREE, OR REPLACE IT SHOULD IT BECOME DAMAGED. TREE STAKING SHALL ADHERE TO THE FOLLOWING GUIDELINES:
  - 1" - 2" TREES: TWO STAKES PER TREE
  - 2 1/2" - 4" TREES: THREE STAKES PER TREE
  - TREES OVER 4" CALIPER: GUY AS NEEDED
  - MULTI TRUNK TREES: THREE STAKES PER TREE MINIMUM. QUANTITY AND POSITIONS AS NEEDED TO STABILIZE THE TREE
  - MULTI-TRUNK TREES: THREE STAKES PER TREE MINIMUM. QUANTITY AND POSITIONS AS NEEDED TO STABILIZE THE TREE
- UPON COMPLETION OF PLANTING, CONSTRUCT AN EARTH WATERING BASIN AROUND THE TREE. COVER THE INTERIOR OF THE TREE RING WITH THE WEED BARRIER CLOTH AND TOPDRESS WITH MULCH (TYPE AND DEPTH PER PLANS).

**D. SHRUB, PERENNIAL, AND GROUNDCOVER PLANTING**

- DIG THE PLANTING HOLES TWICE AS WIDE AND 2" LESS DEEP THAN EACH PLANT'S ROOTBALL. INSTALL THE PLANT IN THE HOLE WITHIN 24 HOURS AFTER PLANTING WITH SOIL AMENDED PER SOIL TEST RECOMMENDATIONS.
- INSTALL THE WEED BARRIER CLOTH, OVERLAPPING IT AT THE ENDS. UTILIZE STEEL STAPLES TO KEEP THE WEED BARRIER CLOTH IN PLACE.
- WHEN PLANTING IS COMPLETE, INSTALL MULCH (TYPE AND DEPTH PER PLANS) OVER ALL PLANTING BEDS, COVERING THE ENTIRE PLANTING AREA.

**E. SODDING**

- SOD VARIETY TO BE AS SPECIFIED ON THE LANDSCAPE PLAN.
- LAY SOD WITHIN 24 HOURS FROM THE TIME OF STRIPPING. DO NOT LAY IF THE GROUND IS FROZEN. LAY THE SOD TO FORM A TIGHTLY FITTED JOINTS. BUTT ENDS WITH TIGHTLY FITTED JOINTS. ENDS OF SOD STRIPS - DO NOT OVERLAP. STAGGER STRIPS TO OFFSET JOINTS IN ADJACENT COURSES.
- ROLL THE SOD TO ENSURE GOOD CONTACT OF THE SOD'S ROOT SYSTEM WITH THE SOIL UNDERNEATH.
- WATER THE SOD THOROUGHLY WITH A FINE SPRAY IMMEDIATELY AFTER PLANTING TO OBTAIN AT LEAST SIX INCHES OF PENETRATION INTO THE SOIL BELOW THE SOD.

**F. MULCHING**

- INSTALL MULCH TOPDRESSING, TYPE AND DEPTH PER MULCH NOTE, IN ALL PLANTING AREAS AND TREE RINGS.
- DO NOT INSTALL MULCH WITHIN 6" OF TREE ROOT FLARE AND WITHIN 24" OF HABITABLE STRUCTURES, EXCEPT AS MAY BE NOTED ON THESE PLANS. MULCH COVER WITHIN 6" OF CONCRETE WALKS AND CURBS SHALL NOT PROTRUDE ABOVE THE FINISH SURFACE OF THE WALKS AND CURBS. MULCH COVER WITHIN 12" OF WALLS SHALL BE AT LEAST 3" LOWER THAN THE TOP OF WALL.

**G. CLEAN UP**

- DURING LANDSCAPE PREPARATION AND PLANTING, KEEP ALL PAVEMENT CLEAN AND ALL WORK AREAS IN A NEAT, ORDERLY CONDITION.
- DISPOSED LEGALLY OF ALL EXCAVATED MATERIALS OFF THE PROJECT SITE.

**H. INSPECTION AND ACCEPTANCE**

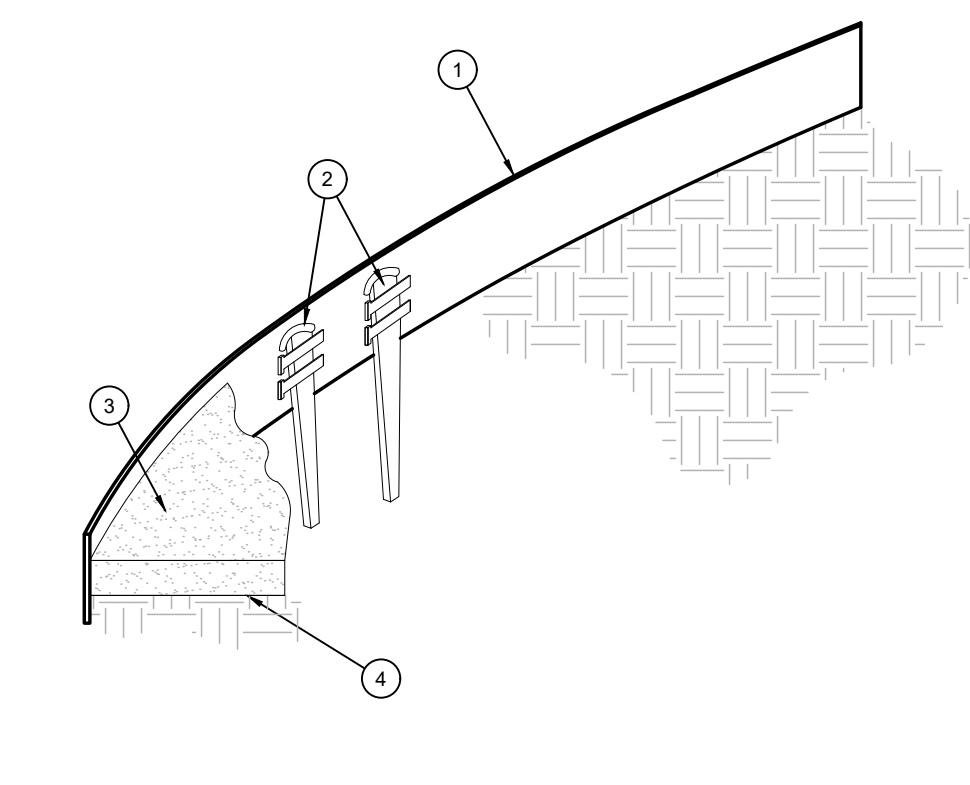
- UPON COMPLETION OF THE WORK, THE LANDSCAPE CONTRACTOR SHALL PROVIDE THE SITE CLEAN, FREE OF DEBRIS AND TRASH, AND SUITABLE FOR USE AS INTENDED. THE LANDSCAPE CONTRACTOR SHALL THEN REQUEST AN INSPECTION BY THE OWNER TO DETERMINE FINAL ACCEPTABILITY.
- WHEN THE INSPECTED PLANTING WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, THE LANDSCAPE CONTRACTOR SHALL REPLACE AND/OR REPAIR THE REJECTED WORK TO THE OWNER'S SATISFACTION WITHIN 24 HOURS.
- THE LANDSCAPE MAINTENANCE PERIOD WILL NOT COMMENCE UNTIL THE LANDSCAPE WORK HAS BEEN REINSPECTED BY THE OWNER AND FOUND TO BE ACCEPTABLE. AT THAT TIME, A WRITTEN NOTICE OF FINAL ACCEPTANCE WILL BE ISSUED BY THE OWNER, AND THE MAINTENANCE AND GUARANTEE PERIODS WILL COMMENCE.

**I. LANDSCAPE MAINTENANCE**

- THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL WORK SHOWN ON THESE PLANS FOR 90 DAYS BEYOND FINAL ACCEPTANCE OF ALL LANDSCAPE WORK BY THE OWNER. LANDSCAPE MAINTENANCE SHALL INCLUDE WEEKLY SITE VISITS FOR THE FOLLOWING ACTIONS (AS APPROPRIATE): PROPER PRUNING, RESTAKING OF TREES, RESETTling OF PLANTS THAT HAVE SETTLED, MOVING AND AERATION OF LAWNS, WEEDING, TREATING FOR INSECTS AND DISEASES, REPLACEMENT OF MULCH, REMOVAL OF LITTER, REPAIRS TO THE IRRIGATION SYSTEM DUE TO FAULTY PARTS AND/OR WORKMANSHIP, AND THE APPROPRIATE WATERING OF ALL PLANTINGS.
- THE LANDSCAPE CONTRACTOR SHALL MAINTAIN THE IRRIGATION SYSTEM IN PROPER WORKING ORDER, WITH SCHEDULING ADJUSTMENTS BY SEASON TO MAXIMIZE WATER CONSERVATION. SHOULD SEEDED AND/OR SODDED AREAS NOT BE COVERED BY AN AUTOMATIC IRRIGATION SYSTEM, THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR WATERING THESE AREAS AND OBTAINING A FULL, HEALTHY STAND OF PLANTS AT NO ADDITIONAL COST TO THE OWNER.
- TO ACHIEVE FINAL ACCEPTANCE AT THE END OF THE MAINTENANCE PERIOD, ALL OF THE FOLLOWING CONDITIONS MUST OCCUR:
  - THE LANDSCAPE SHALL SHOW ACTIVE, HEALTHY GROWTH (WITH EXCEPTIONS MADE FOR SEASONAL DORMANCY). ALL PLANTS NOT MEETING THIS CONDITION SHALL BE REJECTED AND REPLACED BY HEALTHY PLANT MATERIAL PRIOR TO FINAL ACCEPTANCE.
  - ALL HARDSCAPE SHALL BE CLEANED PRIOR TO FINAL ACCEPTANCE.
  - SODDED AREAS MUST BE ACTIVELY GROWING AND MUST REACH A MINIMUM HEIGHT OF 1 1/2 INCHES BEFORE FIRST MOWING. BARE AREAS LARGER THAN TWELVE SQUARE INCHES MUST BE RESODDED (AS APPROPRIATE) PRIOR TO FINAL ACCEPTANCE. ALL SODDED TURF SHALL BE NEATLY MOWED.

**J. WARRANTY PERIOD, PLANT GUARANTEE AND REPLACEMENTS**

- THE LANDSCAPE CONTRACTOR SHALL GUARANTEE ALL TREES, SHRUBS, PERENNIALS, SOD, AND IRRIGATION SYSTEMS FOR A PERIOD OF ONE YEAR FROM THE DATE OF THE OWNER'S FINAL ACCEPTANCE (90 DAYS FOR ANNUAL PLANTS). THE CONTRACTOR SHALL REPLACE, AT HIS OWN EXPENSE AND TO THE SATISFACTION OF THE OWNER, ANY PLANTS WHICH DIE IN THAT TIME, OR REPAIR ANY PORTIONS OF THE IRRIGATION SYSTEM WHICH OPERATE IMPROPERLY.
  - AFTER THE INITIAL MAINTENANCE PERIOD AND DURING THE GUARANTEE PERIOD, THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR REPLACEMENT OF PLANTS WHEN PLANT DEATH CANNOT BE ATTRIBUTED DIRECTLY TO OVERWATERING OR OTHER DAMAGE BY HUMAN ACTIONS.
- PROVIDE A MINIMUM OF (2) COPIES OF RECORD DRAWINGS TO THE OWNER UPON COMPLETION OF WORK. A RECORD DRAWING IS A RECORD OF ALL CHANGES THAT OCCURRED IN THE FIELD AND THAT ARE DOCUMENTED THROUGH CHANGE ORDERS, ADDENDA, OR CONTRACTOR CONSULTANT DRAWING MARKUPS.

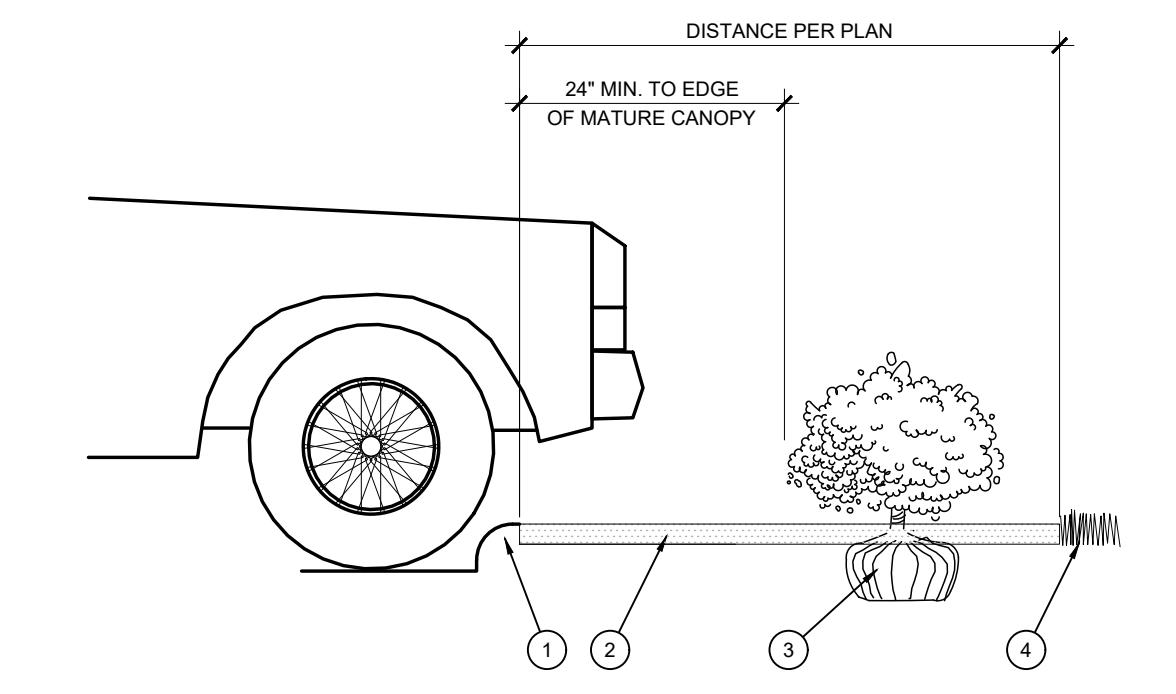


- ROLLED-TOP STEEL EDGING PER PLANS.
- TAPERED STEEL STAKES.
- MULCH, TYPE AND DEPTH PER PLANS.
- FINISH GRADE.

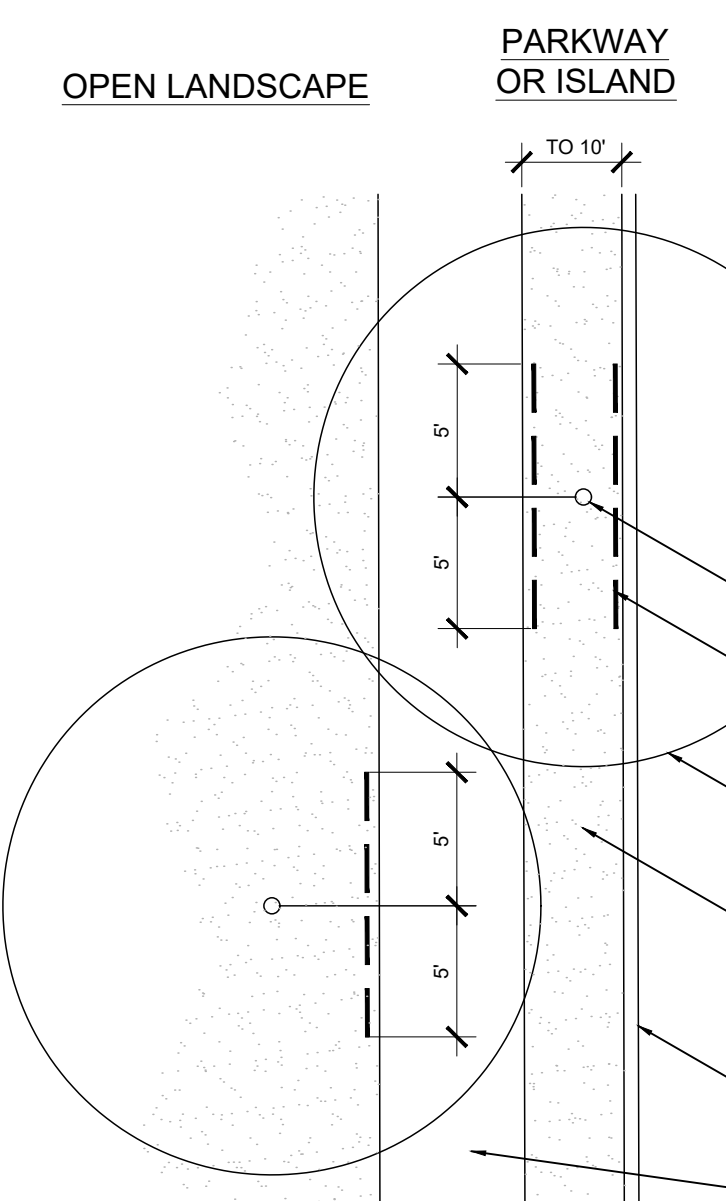
**NOTES:**

- INSTALL EDGING SO THAT STAKES WILL BE ON INSIDE OF PLANTING BED.
- BOTTOM OF EDGING SHALL BE BURIED A MINIMUM OF 1" BELOW FINISH GRADE.
- TOP OF MULCH SHALL BE 1" LOWER THAN TOP OF EDGING.

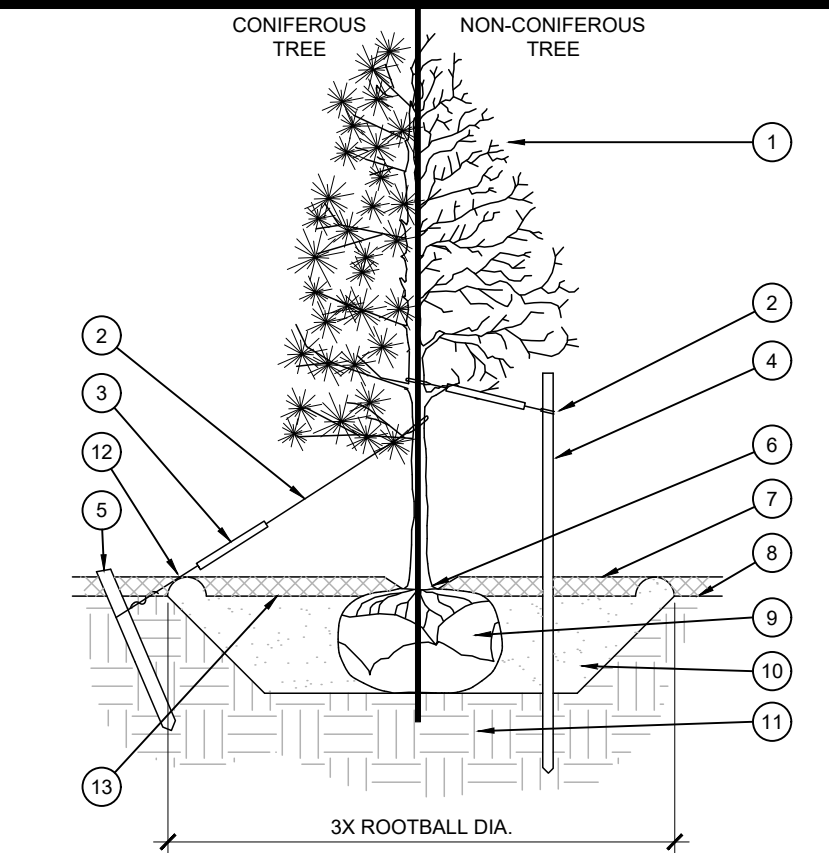
**D STEEL EDGING**  
SCALE: NOT TO SCALE



**E PLANTING AT PARKING AREA**  
SCALE: NOT TO SCALE



**F ROOT BARRIER - PLAN VIEW**  
SCALE: NOT TO SCALE

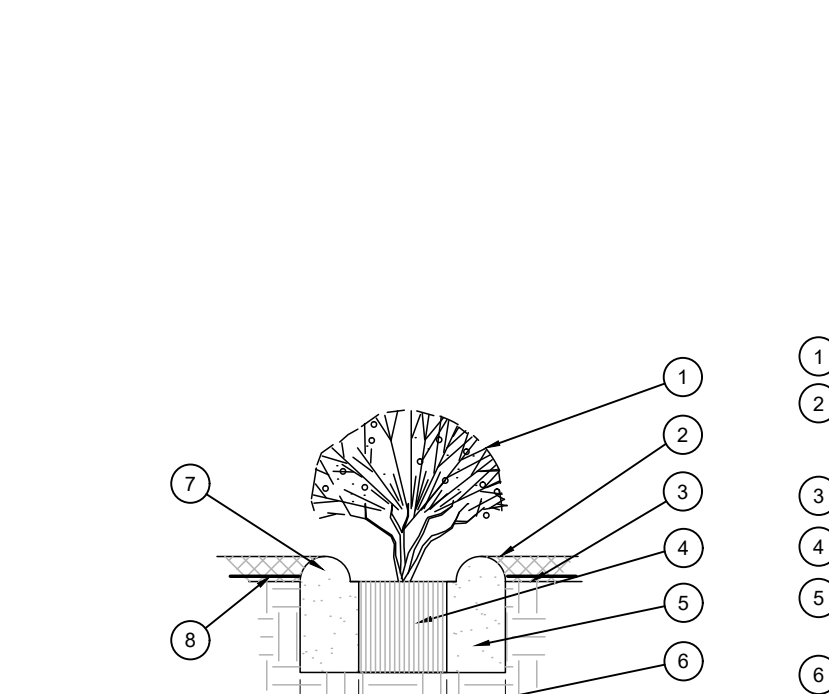


- ROLLED-TOP STEEL EDGING PER PLANS.
- TAPERED STEEL STAKES.
- MULCH, TYPE AND DEPTH PER PLANS.
- FINISH GRADE.

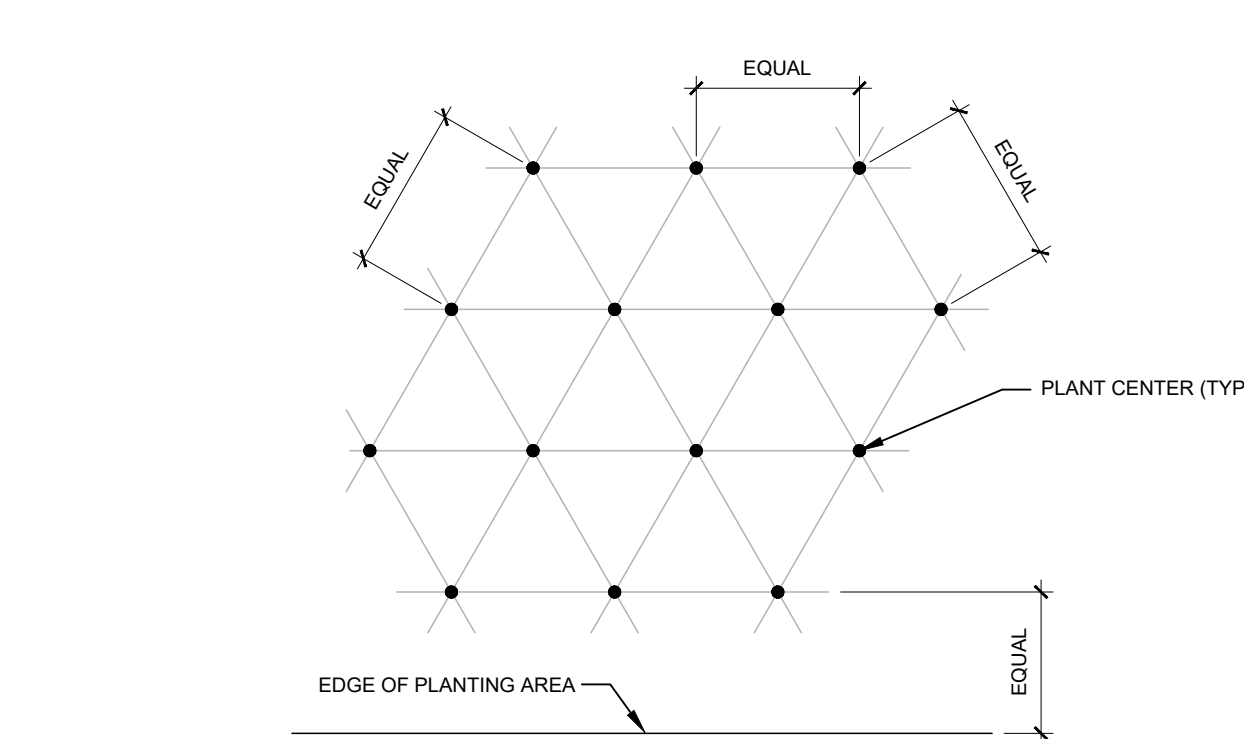
**NOTES:**

- INSTALL EDGING SO THAT STAKES WILL BE ON INSIDE OF PLANTING BED.
- BOTTOM OF EDGING SHALL BE BURIED A MINIMUM OF 1" BELOW FINISH GRADE.
- TOP OF MULCH SHALL BE 1" LOWER THAN TOP OF EDGING.

**A TREE PLANTING**  
SCALE: NOT TO SCALE



**B SHRUB AND PERENNIAL PLANTING**  
SCALE: NTS



NOTE: ALL PLANTS SHALL BE PLANTED AT EQUAL TRIANGULAR SPACING (EXCEPT WHERE SHOWN ON PLANS AS INFORMAL GROUPINGS). REFER TO PLANT LEGEND FOR SPACING DISTANCE BETWEEN PLANTS.

1) STEP 1: DETERMINE TOTAL PLANTS FOR THE AREA WITH THE FOLLOWING FORMULA:  
TOTAL AREA / AREA DIVIDER = TOTAL PLANTS

| PLANT SPACING | AREA DIVIDER | PLANT SPACING | AREA DIVIDER |
|---------------|--------------|---------------|--------------|
| 6"            | 0.22         | 18"           | 1.95         |
| 8"            | 0.30         | 24"           | 5.41         |
| 10"           | 0.60         | 30"           | 5.41         |
| 12"           | 0.87         | 36"           | 7.79         |
| 15"           | 1.35         |               |              |

2) STEP 2: SUBTRACT THE ROW (S) OF PLANTS THAT WOULD OCCUR AT THE EDGE OF THE PLANTED AREA WITH THE FOLLOWING FORMULA: TOTAL PERIMETER LENGTH / PLANT SPACING = TOTAL PLANT SUBTRACTION

EXAMPLE: PLANTS AT 18" O.C. IN 100 SF PLANTING AREA, 40 LF PERIMETER  
STEP 1: 100 SF / 1.95 = 51 PLANTS  
STEP 2: 40 LF / 18" = 2.22 = 2 PLANTS  
51 PLANTS - (2 PLANTS) = 49 PLANTS TOTAL

**C PLANT SPACING**  
SCALE: NTS

**CORE STATES INC.**

1770 HILLCREST ROAD  
DALLAS, TX 75220  
(214) 377-5660  
www.core-states-inc.com

DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT, ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECT, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S SOLE RISK. IT IS USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED. USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.

**CLIENT**

**CHASE**

**811**

Know what's below. Call before you dig.

THE CONTRACTOR SPECIFICALLY WARRANTS THAT THE LOCATION AND DEPTH OF EXISTING UTILITIES SHOWN ON THESE PLANS ARE ACCURATE. MANUFACTURERS MARKED IN THE FIELD. CORE STATES, INC. DOES NOT GUARANTEE THE LOCATION, DEPTH, OR CONTENT OF UTILITIES SHOWN ON THESE PLANS. REQUESTER ACCEPTS ALL CONSEQUENCES OF UTILITIES.

| REV | DATE      | COMMENT          | BY  |
|-----|-----------|------------------|-----|
| 1   | 8/10/2020 | PER TRC COMMENTS | MAL |

**DOCUMENT**

**SITE PLAN APPROVAL FOR CHASE BANK**

**SITE LOCATION**  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

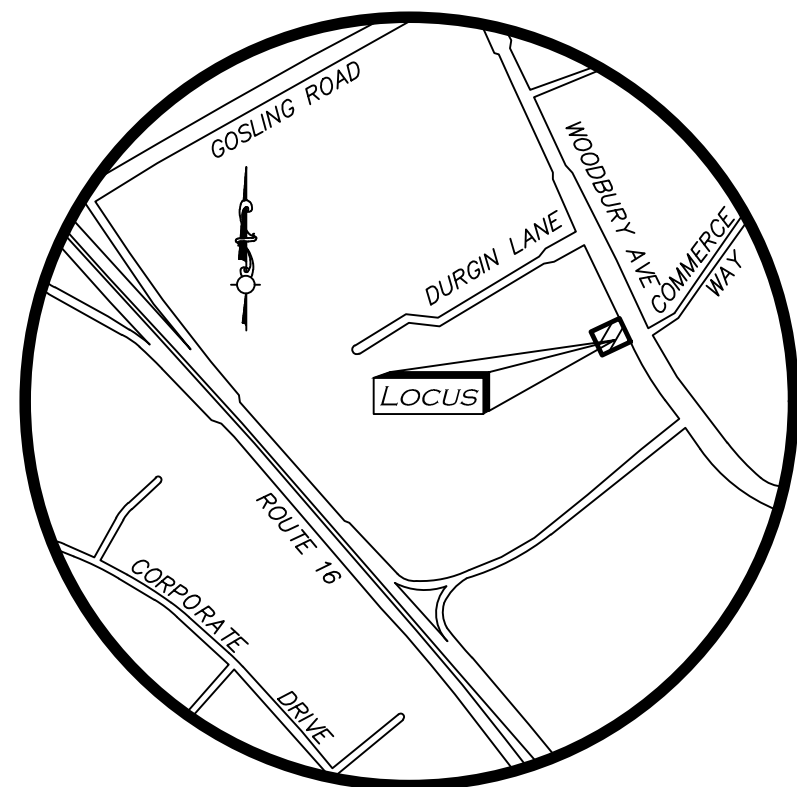
**ENGINEER SEAL**

THOMAS C. PICKERING  
No. 10218  
LICENSED PROFESSIONAL ENGINEER  
08/11/2020

**SHEET TITLE**  
PLANTING DETAILS & SPECIFICATIONS

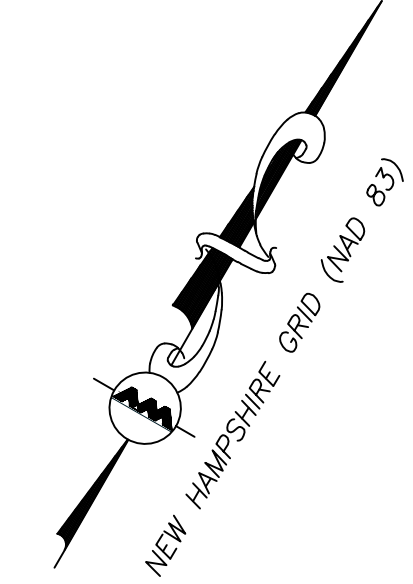
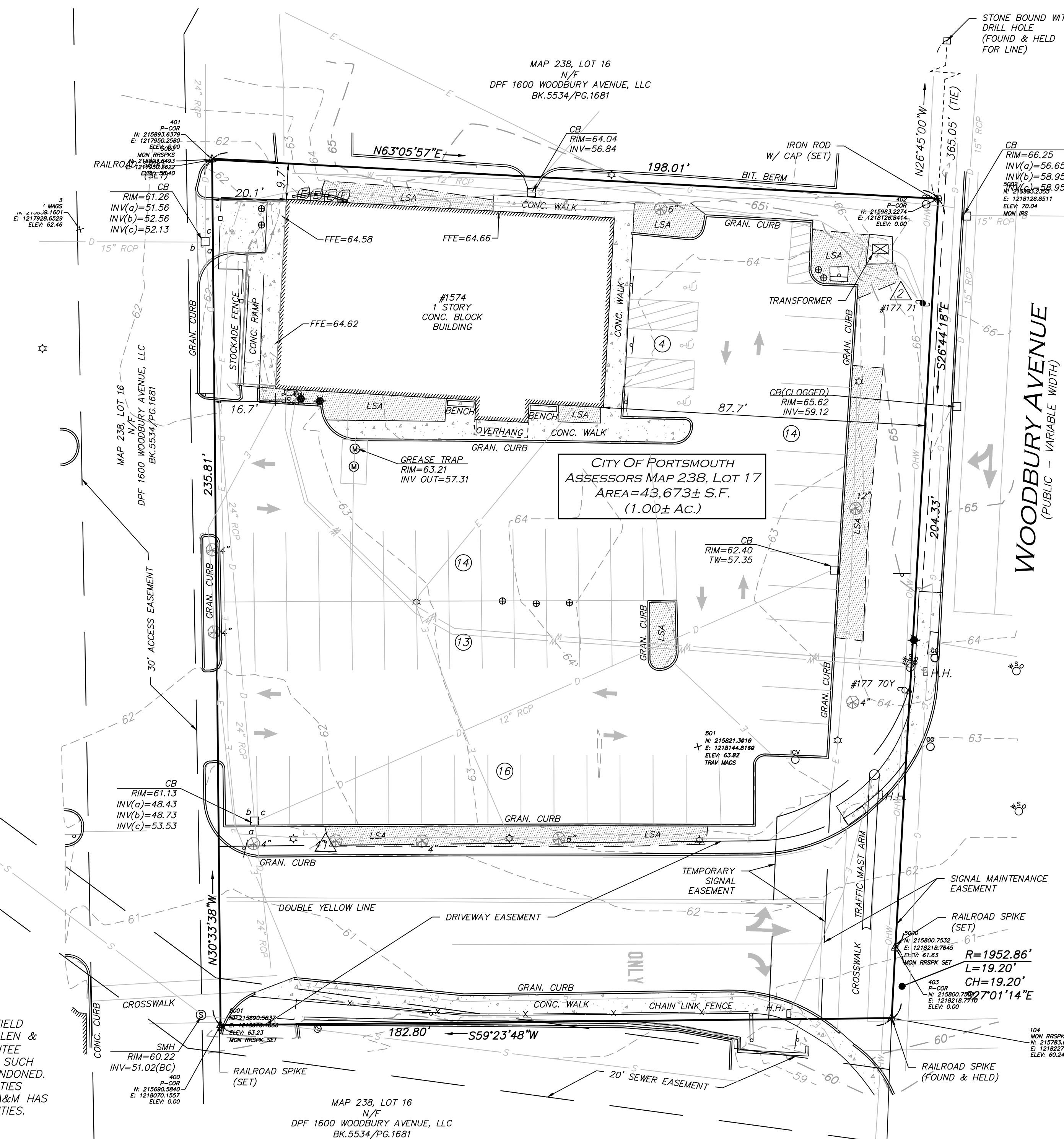
JOB #: JPM 27086  
DATE: 07/01/2020  
SCALE:  
DRAWN BY: MAL  
CHECKED BY: KGF

SHEET NO.  
**LP-2**



**LOCUS MAP**  
(NOT TO SCALE)

| PARKING SUMMARY     |           |
|---------------------|-----------|
| STANDARD STALLS     | 58        |
| HANDICAPPED STALLS  | 3         |
| <b>TOTAL STALLS</b> | <b>61</b> |



| LEGEND                    |           |
|---------------------------|-----------|
| IRON ROD (IR)             | ○         |
| PK NAIL                   | △         |
| SEWER MANHOLE (SMH)       | ⊙         |
| MISC. MANHOLE (MH)        | ⊕         |
| CATCH BASIN (CB)          | ⊞         |
| UTILITY POLE              | ⊠         |
| UTILITY POLE W/ RISER     | ⊡         |
| WATER GATE                | ⊞         |
| GAS GATE                  | ⊞         |
| BOLLARD                   | ⊙         |
| LIGHT                     | ⊙         |
| PEDESTRIAN LIGHT          | ⊙         |
| TREE                      | ⊙         |
| SIGN                      | ⊙         |
| SIGN                      | ⊙         |
| TRANSFORMER               | ⊙         |
| ELECTRIC METER            | ⊙         |
| HANDICAPPED PARKING SPACE | ⊙         |
| TRAFFIC MAST              | ⊙         |
| PAINTED ARROW             | ⊙         |
| PARKING SPACE COUNT       | ⊙         |
| FIRE STANDPIPE            | ⊙         |
| CONCRETE                  | ▒         |
| LANDSCAPED AREA (LSA)     | ▒         |
| BUILDING                  | ▒         |
| BUILDING OVERHANG         | ▒         |
| EASEMENT LINE             | ---       |
| 1' CONTOUR                | ---53---  |
| 5' CONTOUR                | ---55---  |
| PROPERTY LINE             | ---       |
| ABUTTERS LINE             | ---       |
| SEWER LINE                | ---       |
| DRAIN LINE                | ---       |
| ELECTRIC LINE             | ---       |
| WATER LINE                | ---       |
| OVERHEAD WIRES            | ---OHW--- |
| FINISHED FLOOR ELEVATION  | FFE       |
| BITUMINOUS                | BIT.      |
| CONCRETE                  | CONC.     |
| GRANITE                   | GRAN.     |
| BOTTOM CENTER             | (BC)      |
| REINFORCED CONCRETE PIPE  | RCP       |
| POLYVINYL CHLORIDE PIPE   | PVC       |
| NOW OR FORMERLY           | N/F       |
| BOOK                      | BK.       |
| PAGE                      | PG.       |

THIS PLAN IS THE RESULT OF AN ACTUAL ON THE GROUND SURVEY PERFORMED ON OR BETWEEN JUNE 19, 2019 AND JULY 8, 2019 AND HAD AN ERROR OF CLOSURE OF NO GREATER THAN 1/10,000.

ALLEN & MAJOR ASSOCIATES, INC.

**ISSUED FOR REVIEW**  
JUNE 23, 2020

JAMES P. SMITH NH LLS #908 DATE

| REV | DATE | DESCRIPTION |
|-----|------|-------------|
|     |      |             |
|     |      |             |
|     |      |             |

APPLICANT/OWNER:  
**CORE STATES GROUP**  
9 GALEN STREET  
WATERTOWN, MA 02472

PROJECT:  
**CHASE BANK SITE**  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH

PROJECT NO. 261405 DATE: 6/23/20

SCALE: 1" = 20' DWG. NAME: S-2614-05-EC

DRAFTED BY: AJR CHECKED BY: JPS

PREPARED BY:



**ALLEN & MAJOR ASSOCIATES, INC.**  
civil & structural engineering • land surveying  
environmental consulting • landscape architecture  
www.allenmajor.com

400 HARVEY ROAD  
MANCHESTER, NH 03103  
TEL: (603) 627-5500  
FAX: (603) 627-5501

WOBURN, MA • LAKEVILLE, MA • MANCHESTER, NH

THIS DRAWING HAS BEEN PREPARED IN ELECTRONIC FORMAT. CLIENT/CUSTOMER'S REPRESENTATIVE OR CONSULTANT MAY BE PROVIDED COPIES OF DRAWINGS AND SPECIFICATIONS ON MAGNETIC MEDIA FOR HIS/HER INFORMATION AND USE FOR SPECIFIC APPLICATION TO THIS PROJECT. DUE TO THE POTENTIAL THAT THE MAGNETIC INFORMATION MAY BE MODIFIED UNINTENTIONALLY OR OTHERWISE, ALLEN & MAJOR ASSOCIATES, INC. MAY REMOVE ALL INDICATION OF THE DOCUMENT'S AUTHORSHIP ON THE MAGNETIC MEDIA. PRINTED REPRESENTATIONS OF THE DRAWINGS AND SPECIFICATIONS ISSUED SHALL BE THE ONLY RECORD COPIES OF ALLEN & MAJOR ASSOCIATES, INC.'S WORK PRODUCT.

DRAWING TITLE: EXISTING CONDITIONS SHEET No. 1

**UTILITY STATEMENT**  
THE UTILITIES SHOWN HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION AND EXISTING DRAWINGS. ALLEN & MAJOR ASSOCIATES, INC. (A&M) MAKES NO GUARANTEE THAT THE UTILITIES SHOWN HEREON COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. A&M FURTHER DOES NOT WARRANT THAT THE UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED. A&M HAS NOT PHYSICALLY LOCATED THE UNDERGROUND UTILITIES.

| BENCHMARK SUMMARY |                                  |       |
|-------------------|----------------------------------|-------|
| TBM #             | DESCRIPTION                      | ELEV. |
| 1                 | CHISEL SQUARE ON LIGHT POLE BASE | 63.93 |
| 2                 | NAIL SET IN UP# 177 71           | 68.94 |

**LOCUS REFERENCES**

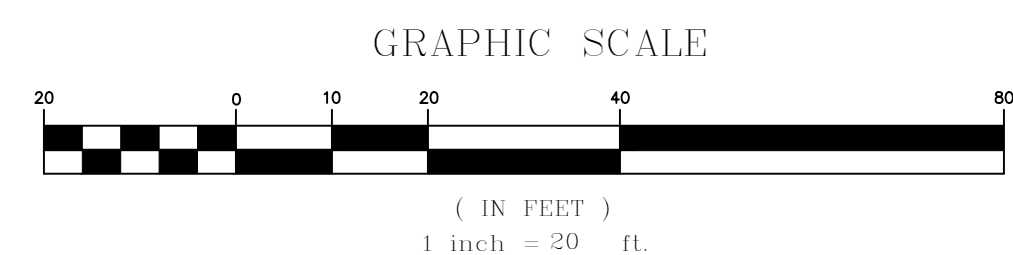
- CITY OF PORTSMOUTH ASSESSORS MAP 238, LOT 17
- R.C.R.D. BOOK 4452, PAGE 881
- PLAN ENTITLED, "LOT LINE RELOCATION PLAN FOR DSP SHOPPING CENTER, LLC., ENDICOTT HOTEL COMPANY & RICHARD P. FUSEGNI WOODBURY AVENUE AND DURGIN LANE COUNTY OF ROCKINGHAM, CITY OF PORTSMOUTH, NH." SCALE 1"=50', PREPARED BY MILLETTE SPRAGUE & COLWELL, INC. DATED JULY 24, 2003, AND ON FILE AT THE R.C.R.D. AS PLAN NO. 32458.

**PLAN REFERENCES**

- PLAN ENTITLED, "SIGNAL MAINTENANCE EASEMENT FOR PROPERTY AT 1574 WOODBURY AVENUE PORTSMOUTH, ROCKINGHAM COUNTY, NEW HAMPSHIRE, OWNED BY RICHARD P. FUSEGNI", 1"=20', DATED FEBRUARY 16, 2017, NORTH EASTERLY SURVEYING, INC., AND ON FILE AT THE R.C.R.D. AS PLAN NO. 40044.

**NOTES**

- NORTH ARROW IS BASED ON NEW HAMPSHIRE GRID COORDINATE SYSTEM (NAD 83)
- BOOK/PAGE AND PLAN REFERENCES ARE TAKEN FROM ROCKINGHAM COUNTY REGISTRY OF DEEDS IN BRENTWOOD, NH
- VERTICAL DATUM IS NAVD 88.
- CONTOUR INTERVAL IS ONE FOOT (1').







# City of Portsmouth, New Hampshire

## Site Plan Application Checklist

This site plan application checklist is a tool designed to assist the applicant in the planning process and for preparing the application for Planning Board review. A pre-application conference with a member of the planning department is strongly encouraged as additional project information may be required depending on the size and scope. The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan review regulations for full details.

**Applicant Responsibilities (Section 2.5.2):** Applicable fees are due upon application submittal along with required attachments. The application shall be complete as submitted and provide adequate information for evaluation of the proposed site development. Waiver requests must be submitted in writing with appropriate justification.

Name of Owner/Applicant: RICHARD FUSEGNI Date Submitted: 7/1/2020

Phone Number: \_\_\_\_\_ E-mail: \_\_\_\_\_

Site Address: 1574 WOODBURY AVENUE Map: 238 Lot: 17

Zoning District: G1 Lot area: 43,673 sq. ft.

| Application Requirements            |   |   |                  |
|-------------------------------------|---|---|------------------|
| <input checked="" type="checkbox"/> | Required Items for Submittal  | Item Location<br>(e.g. Page or Plan Sheet/Note #) | Waiver Requested |
| <input checked="" type="checkbox"/> | Fully executed and signed Application form.<br>(2.5.2.3)  |   | N/A              |
| <input checked="" type="checkbox"/> | All application documents, plans, supporting documentation and other materials provided in digital Portable Document Format (PDF).<br>(2.5.2.8) |   | N/A              |

| Site Plan Review Application Required Information |   |  |                  |
|---|---|--|------------------|
| <input checked="" type="checkbox"/>               | Required Items for Submittal  | Item Location<br>(e.g. Page/line or Plan Sheet/Note #) | Waiver Requested |
| <input type="checkbox"/>                          | Statement that lists and describes "green" building components and systems.<br>(2.5.3.1A)   | N/A  |                  |
| <input checked="" type="checkbox"/>               | Gross floor area and dimensions of all buildings and statement of uses and floor area for each floor.<br>(2.5.3.1B)                             | C-2  | N/A              |
| <input checked="" type="checkbox"/>               | Tax map and lot number, and current zoning of all parcels under Site Plan Review.<br>(2.5.3.1C)   | C-2, SURVEY  | N/A              |
| <input checked="" type="checkbox"/>               | Owner's name, address, telephone number, and signature. Name, address, and telephone number of applicant if different from owner.<br>(2.5.3.1D) | CV-1   | N/A              |

**Site Plan Review Application Required Information**

| <input checked="" type="checkbox"/> | Required Items for Submittal  | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|---|---|---------------------|
| <input checked="" type="checkbox"/> | Names and addresses (including Tax Map and Lot number and zoning districts) of all direct abutting property owners (including properties located across abutting streets) and holders of existing conservation, preservation or agricultural preservation restrictions affecting the subject property.<br><b>(2.5.3.1E)</b> | SURVEY PLAN   | N/A                 |
| <input checked="" type="checkbox"/> | Names, addresses and telephone numbers of all professionals involved in the site plan design.<br><b>(2.5.3.1F)</b>  | CV-1  | N/A                 |
| <input checked="" type="checkbox"/> | List of reference plans.<br><b>(2.5.3.1G)</b>   | CV-1  | N/A                 |
| <input checked="" type="checkbox"/> | List of names and contact information of all public or private utilities servicing the site.<br><b>(2.5.3.1H)</b>   | CV-1  | N/A                 |

**Site Plan Specifications**

| <input checked="" type="checkbox"/> | Required Items for Submittal  | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|---|---|---------------------|
| <input checked="" type="checkbox"/> | Full size plans shall not be larger than 22 inches by 34 inches with match lines as required, unless approved by the Planning Director. Submittals shall be a minimum of 11 inches by 17 inches as specified by Planning Dept. staff. <b>(2.5.4.1A)</b> | Required on all plan sheets                               | N/A                 |
| <input checked="" type="checkbox"/> | Scale: Not less than 1 inch = 60 feet and a graphic bar scale shall be included on all plans.<br><b>(2.5.4.1B)</b>  | Required on all plan sheets                               | N/A                 |
| <input type="checkbox"/>            | GIS data should be referenced to the coordinate system New Hampshire State Plane, NAD83 (1996), with units in feet.<br><b>(2.5.4.1C)</b>  | N/A   | N/A                 |
| <input checked="" type="checkbox"/> | Plans shall be drawn to scale.<br><b>(2.5.4.1D)</b>   | Required on all plan sheets                               | N/A                 |
| <input checked="" type="checkbox"/> | Plans shall be prepared and stamped by a NH licensed civil engineer.<br><b>(2.5.4.1D)</b>   |   | N/A                 |
| <input type="checkbox"/>            | Wetlands shall be delineated by a NH certified wetlands scientist and so stamped. <b>(2.5.4.1E)</b>   | N/A   | N/A                 |
| <input checked="" type="checkbox"/> | Title (name of development project), north point, scale, legend.<br><b>(2.5.4.2A)</b>   | SITE PLAN DRWGS.  | N/A                 |
| <input checked="" type="checkbox"/> | Date plans first submitted, date and explanation of revisions.<br><b>(2.5.4.2B)</b>   | CV-1  | N/A                 |
| <input checked="" type="checkbox"/> | Individual plan sheet title that clearly describes the information that is displayed.<br><b>(2.5.4.2C)</b>  | Required on all plan sheets                               | N/A                 |
| <input checked="" type="checkbox"/> | Source and date of data displayed on the plan.<br><b>(2.5.4.2D)</b>   |   | N/A                 |

**Site Plan Specifications**

| <input checked="" type="checkbox"/> | Required Items for Submittal   | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|--|---|---------------------|
| <input checked="" type="checkbox"/> | A note shall be provided on the Site Plan stating: "All conditions on this Plan shall remain in effect in perpetuity pursuant to the requirements of the Site Plan Review Regulations."<br><b>(2.5.4.2E)</b>   | CV-1  | N/A                 |
| <input type="checkbox"/>            | Plan sheets submitted for recording shall include the following notes:<br>a. "This Site Plan shall be recorded in the Rockingham County Registry of Deeds."<br>b. "All improvements shown on this Site Plan shall be constructed and maintained in accordance with the Plan by the property owner and all future property owners. No changes shall be made to this Site Plan without the express approval of the Portsmouth Planning Director."<br><b>(2.13.3)</b>   | CV-1  | N/A                 |
| <input type="checkbox"/>            | Plan sheets showing landscaping and screening shall also include the following additional notes:<br>a. "The property owner and all future property owners shall be responsible for the maintenance, repair and replacement of all required screening and landscape materials."<br>b. "All required plant materials shall be tended and maintained in a healthy growing condition, replaced when necessary, and kept free of refuse and debris. All required fences and walls shall be maintained in good repair."<br>c. "The property owner shall be responsible to remove and replace dead or diseased plant materials immediately with the same type, size and quantity of plant materials as originally installed, unless alternative plantings are requested, justified and approved by the Planning Board or Planning Director."<br><b>(2.13.4)</b> | CV-1  | N/A                 |

**Site Plan Specifications – Required Exhibits and Data**

| <input checked="" type="checkbox"/> | Required Items for Submittal  | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|---|---|---------------------|
|                                     | <b>1. Existing Conditions: (2.5.4.3A)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. Surveyed plan of site showing existing natural and built features;   | SURVEY PLAN   |                     |
| <input checked="" type="checkbox"/> | b. Zoning boundaries;   | C-2   |                     |
| <input checked="" type="checkbox"/> | c. Dimensional Regulations;   | C-2   |                     |
| <input checked="" type="checkbox"/> | d. Wetland delineation, wetland function and value assessment;  | N/A   |                     |
| <input checked="" type="checkbox"/> | e. SFHA, 100-year flood elevation line and BFE data.  | N/A   |                     |
|                                     | <b>2. Buildings and Structures: (2.5.4.3B)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Plan view: Use, size, dimensions, footings, overhangs, 1st fl. elevation;  | SITE PLANS  |                     |
| <input checked="" type="checkbox"/> | b. Elevations: Height, massing, placement, materials, lighting, façade treatments;  | ELEVATION DRAWINGS  |                     |
| <input checked="" type="checkbox"/> | c. Total Floor Area;  | SITE PLAN C-2   |                     |
| <input checked="" type="checkbox"/> | d. Number of Usable Floors;   | C-2   |                     |
| <input checked="" type="checkbox"/> | e. Gross floor area by floor and use.   | C-2   |                     |
|                                     | <b>3. Access and Circulation: (2.5.4.3C)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Location/width of access ways within site;   | C-2   |                     |
| <input checked="" type="checkbox"/> | b. Location of curbing, right of ways, edge of pavement and sidewalks;  | C-2   |                     |
| <input checked="" type="checkbox"/> | c. Location, type, size and design of traffic signing (pavement markings);  | C-2   |                     |
| <input checked="" type="checkbox"/> | d. Names/layout of existing abutting streets;   | C-2, SURVEY PLAN  |                     |
| <input checked="" type="checkbox"/> | e. Driveway curb cuts for abutting prop. and public roads;  | C-2   |                     |
| <input checked="" type="checkbox"/> | f. If subdivision; Names of all roads, right of way lines and easements noted;  | N/A   |                     |
| <input checked="" type="checkbox"/> | g. AASHTO truck turning templates, description of minimum vehicle allowed being a WB-50 (unless otherwise approved by TAC). | N/A   |                     |
|                                     | <b>4. Parking and Loading: (2.5.4.3D)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. Location of off street parking/loading areas, landscaped areas/buffers;  | C-2   |                     |
| <input checked="" type="checkbox"/> | b. Parking Calculations (# required and the # provided).  | C-2   |                     |
|                                     | <b>5. Water Infrastructure: (2.5.4.3E)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Size, type and location of water mains, shut-offs, hydrants & Engineering data;  | C-2   |                     |
| <input checked="" type="checkbox"/> | b. Location of wells and monitoring wells (include protective radii).   | N/A   |                     |
|                                     | <b>6. Sewer Infrastructure: (2.5.4.3F)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Size, type and location of sanitary sewage facilities & Engineering data.  | C-2   |                     |
|                                     | <b>7. Utilities: (2.5.4.3G)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. The size, type and location of all above & below ground utilities;   | C-2   |                     |
| <input checked="" type="checkbox"/> | b. Size type and location of generator pads, transformers and other fixtures.   | N/A   |                     |

**Site Plan Specifications – Required Exhibits and Data**

| <input checked="" type="checkbox"/> | Required Items for Submittal   | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|--|---|---------------------|
| <input type="checkbox"/>            | <b>8. Solid Waste Facilities: (2.5.4.3H)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. The size, type and location of solid waste facilities.  | C-2   |                     |
| <input type="checkbox"/>            | <b>9. Storm water Management: (2.5.4.3I)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. The location, elevation and layout of all storm-water drainage.   | C-2, SURVEY PLAN  |                     |
| <input type="checkbox"/>            | <b>10. Outdoor Lighting: (2.5.4.3J)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Type and placement of all lighting (exterior of building, parking lot and any other areas of the site) and; | C-2, C-7  |                     |
| <input checked="" type="checkbox"/> | b. photometric plan.   |   |                     |
| <input checked="" type="checkbox"/> | <b>11. Indicate where dark sky friendly lighting measures have been implemented. (10.1)</b>                    | C-7   |                     |
| <input type="checkbox"/>            | <b>12. Landscaping: (2.5.4.3K)</b>   |   |                     |
| <input checked="" type="checkbox"/> | a. Identify all undisturbed area, existing vegetation and that which is to be retained;                        | C-2, SITE PLANS   |                     |
| <input checked="" type="checkbox"/> | b. Location of any irrigation system and water source.   | C-2   |                     |
| <input type="checkbox"/>            | <b>13. Contours and Elevation: (2.5.4.3L)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Existing/Proposed contours (2 foot minimum) and finished grade elevations.                                  | C-3   |                     |
| <input type="checkbox"/>            | <b>14. Open Space: (2.5.4.3M)</b>  |   |                     |
| <input checked="" type="checkbox"/> | a. Type, extent and location of all existing/proposed open space.  | C-2   |                     |
| <input checked="" type="checkbox"/> | <b>15. All easements, deed restrictions and non-public rights of ways. (2.5.4.3N)</b>                          | C-2, SURVEY   |                     |
| <input checked="" type="checkbox"/> | <b>16. Location of snow storage areas and/or off-site snow removal. (2.5.4.3O)</b>                             | TO BE ADDED TO C-2  |                     |
| <input checked="" type="checkbox"/> | <b>17. Character/Civic District (All following information shall be included): (2.5.4.3Q)</b>                  | C-2   |                     |
| <input type="checkbox"/>            | a. Applicable Building Height (10.5A21.20 & 10.5A43.30);   |   |                     |
| <input type="checkbox"/>            | b. Applicable Special Requirements (10.5A21.30);   |   |                     |
| <input type="checkbox"/>            | c. Proposed building form/type (10.5A43);  |   |                     |
| <input type="checkbox"/>            | d. Proposed community space (10.5A46).   |   |                     |

**Other Required Information**

| <input checked="" type="checkbox"/> | Required Items for Submittal   | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|--|---|---------------------|
| <input checked="" type="checkbox"/> | Traffic Impact Study or Trip Generation Report, as required.<br><i>(Four (4) hardcopies of the full study/report and Six (6) summaries to be submitted with the Site Plan Application) (3.2.1-2)</i> | TRIP GENERATION<br>SUMMARY                                |                     |
| <input checked="" type="checkbox"/> | Indicate where Low Impact Development Design practices have been incorporated. (7.1)   | C-2   |                     |
| <input checked="" type="checkbox"/> | Indicate whether the proposed development is located in a wellhead protection or aquifer protection area. Such determination shall be approved by the Director of the Dept. of Public Works. (7.3.1) | N/D   |                     |
| <input checked="" type="checkbox"/> | Indicate where measures to minimize impervious surfaces have been implemented. (7.4.3)   | C-2   |                     |
| <input checked="" type="checkbox"/> | Calculation of the maximum effective impervious surface as a percentage of the site. (7.4.3.2)   | C-2   |                     |
| <input checked="" type="checkbox"/> | Stormwater Management and Erosion Control Plan.<br><i>(Four (4) hardcopies of the full plan/report and Six (6) summaries to be submitted with the Site Plan Application) (7.4.4.1)</i>               | C17-1   |                     |

**Final Site Plan Approval Required Information**

| <input checked="" type="checkbox"/> | Required Items for Submittal   | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|--|---|---------------------|
| <input type="checkbox"/>            | All local approvals, permits, easements and licenses required, including but not limited to:<br>a. Waivers;<br>b. Driveway permits;<br>c. Special exceptions;<br>d. Variances granted;<br>e. Easements;<br>f. Licenses.<br>(2.5.3.2A)  |   |                     |
| <input type="checkbox"/>            | Exhibits, data, reports or studies that may have been required as part of the approval process, including but not limited to:<br>a. Calculations relating to stormwater runoff;<br>b. Information on composition and quantity of water demand and wastewater generated;<br>c. Information on air, water or land pollutants to be discharged, including standards, quantity, treatment and/or controls;<br>d. Estimates of traffic generation and counts pre- and post-construction;<br>e. Estimates of noise generation;<br>f. A Stormwater Management and Erosion Control Plan;<br>g. Endangered species and archaeological / historical studies;<br>h. Wetland and water body (coastal and inland) delineations;<br>i. Environmental impact studies.<br>(2.5.3.2B) |   |                     |

**Final Site Plan Approval Required Information**

| <input checked="" type="checkbox"/> | Required Items for Submittal   | Item Location<br>(e.g. Page/line or<br>Plan Sheet/Note #) | Waiver<br>Requested |
|-------------------------------------|--|---|---------------------|
| <input type="checkbox"/>            | A document from each of the required private utility service providers indicating approval of the proposed site plan and indicating an ability to provide all required private utilities to the site.<br><b>(2.5.3.2D)</b> |   |                     |
| <input type="checkbox"/>            | A list of any required state and federal permit applications required for the project and the status of same.<br><b>(2.5.3.2E)</b>   |   |                     |

Applicant's Signature: Alvin de Don Date: 8/4/2020



  
**master locators**  
 800.495.4248  
 info@masterlocators.com  
 www.masterlocators.com

**Site Name and Location:**  
 1574 Woodbury Ave  
 Portsmouth NH 03801

- Assumptions & Clarifications**
- UNLESS OTHERWISE NOTED UNDERGROUND UTILITY DATA IS CONSIDERED QUALITY LEVEL B (QLB) AS DEFINED IN ASCE 38-02: STANDARD GUIDELINE FOR THE COLLECTION AND DEPICTION OF EXISTING SUBSURFACE UTILITY DATA AND IS INTENDED TO SHOW THE APPROXIMATE HORIZONTAL LOCATIONS OF EXISTING UNDERGROUND UTILITIES AS MARKED BY MASTER LOCATORS DURING A GEOPHYSICAL INVESTIGATION PERFORMED WITHIN THE OUTLINED SCOPE OF WORK.
  - ALL UTILITY LOCATIONS SHOWN ON THIS PLAN ARE FOR REFERENCE ONLY. THIS PLAN SHOULD NOT BE USED FOR CONSTRUCTION OR DESIGN PURPOSES AND MASTER LOCATORS IS NOT RESPONSIBLE FOR DAMAGE TO UTILITIES RESULTING FROM ANY CONSTRUCTION WORK BASED ON THESE PLAN.
  - NO BOUNDARY OR PROPERTY SURVEY WORK WAS CONDUCTED IN THE DEVELOPMENT OF THIS PLAN. THE PLAN IS NOT DRAWN TO SCALE.
  - ANY DEPTH INFORMATION PROVIDED IS CONSIDERED APPROXIMATE AND IS NOT GUARANTEED UNLESS LABELED AS QUALITY LEVEL A (QLA) DATA.

**Legend & Color Codes**

|                      |                       |                       |
|----------------------|-----------------------|-----------------------|
| Electric<br><b>E</b> | Unknown<br><b>UNK</b> | Water<br><b>W</b>     |
| Comms.<br><b>C</b>   | Storm<br><b>ST</b>    | Sanitary<br><b>SS</b> |
|                      | Gas<br><b>G</b>       |                       |

|                         |                |                   |                    |               |
|-------------------------|----------------|-------------------|--------------------|---------------|
| Man Hole                | Valve          | Continuation Mark | End of Information | Scope of Work |
| Bollard                 | Utility Pole   | Clean Out         | Storm Drain        | Roof Drain    |
| Floor Drain             | Test Hole      | Fire Hydrant      | Irrigation Box     |               |
| Site Light              | Transformer    | Electric Box      | Phone Pedestal     |               |
| Large (>12") Storm Line | Flow Direction | Buried Vault      | Witness Post       |               |

**Revisions:**

| Date: | Description: | By: |
|-------|--------------|-----|
|       |              |     |
|       |              |     |

ML Job #: \_\_\_\_\_  
 Date: 06/11/2020  
 Technician(s): David Castro  
 Client Name: Core-States Group





# Summary of Underground Utility Locating

---

Prepared For: Chase Bank - Core States

Prepared By:  
Peter Kessinger  
Peter.Kessinger@gprsinc.com  
Senior Project Manager-New England  
603.247.6532  
June 11, 2020

June 11, 2020

Chase Bank - Core States

**Attn:** Kevin Furao

**Site:** 1574 Woodbury Ave, Portsmouth, NH

We appreciate the opportunity to provide this report for our work completed on June 11, 2020.

## **PURPOSE**

The purpose of the project was to search for underground utilities within the project boundaries provided by the client. The scope of work consisted of 1 location measuring approximately 1-acre. The client marked the desired locations prior to our scanning and our markings were then placed onto the surface using marking paint.

## **EQUIPMENT**

- **Underground Scanning GPR Antenna.** The antenna with frequencies ranging from 250 MHz-450 MHz is mounted in a stroller frame which rolls over the surface. The surface needs to be reasonably smooth and unobstructed in order to obtain readable scans. Obstructions such as curbs, landscaping, and vegetation will limit the feasibility of GPR. The data is displayed on a screen and marked in the field in real time. The total depth achieved can be as much as 8' or more with this antenna but can vary widely depending on the types of materials being scanned through. Some soil types such as clay may limit maximum depths to 3' or less. As depth increases, targets must be larger in order to be detected and non-metallic targets can be especially difficult to locate. Depths provided should always be treated as estimates as their accuracy can be affected by multiple factors. For more information, please visit: [Link](#)
- **Electromagnetic Pipe Locator.** The EM locator can passively detect the electromagnetic fields from live AC power or from radio signals travelling along some conductive utilities. It can also be used in conjunction with a transmitter to connect directly to accessible, metallic pipes or tracer wires. A current is sent through the pipe or tracer wire at a specific frequency and the resulting EM field can then be detected by the receiver. A utility's ability to be located depends on a variety of factors including access to the utility, conductivity, grounding, interference from other fields, and many others. Depths provided should always be treated as estimates as their accuracy can be affected by multiple factors. For more information, please visit: [Link](#)
- **GPS.** This handheld GPS unit offers accuracy down to 4 inches; however, the accuracy will depend on the satellite environment and obstructions and should not be considered to be survey-grade. Features can be collected as points, lines, or areas and then exported into Google Earth or overlaid on a CAD drawing. For more information, please visit: [Link](#)

## **PROCESS**

The process typically begins with using the EM pipe locator to locate pipes or utilities throughout the scan area. First, the transmitter is used to connect to and trace any visible risers, tracer wires, or accessible, conductive utilities provided that there is an exposed, metallic surface. The areas are then swept with the receiver to detect live power or radio frequency signals. Locations and depths are painted or flagged on the surface. Depths cannot always be provided depending on the location method and can be prone to error.

Initial GPR scans were then collected in order to evaluate the data and calibrate the equipment. Based on these findings, a scanning strategy is formed, typically consisting of scanning the entire area in a grid with 2'x2' scan spacing in order to locate any potential utilities that were not found with the pipe locator. The GPR data is viewed in real time and anomalies in the data are located and marked on the surface along with their depths using spray paint, pin flags, etc.

## **LIMITATIONS**









Please keep in mind that there are limitations to any subsurface investigation. The equipment may not achieve maximum effectiveness due to soil conditions, above ground obstructions, reinforced concrete, and a variety of other factors. No subsurface investigation or equipment can provide a complete image of what lies below. Our results should always be used in conjunction with as many methods as possible including consulting existing plans and drawings, exploratory excavation or potholing, visual inspection of above-ground features, and utilization of services such as One Call/811. Depths are dependent on the dielectric of the materials being scanned so depth accuracy can vary throughout a site. Relevant scan examples were saved and will be provided in this report.

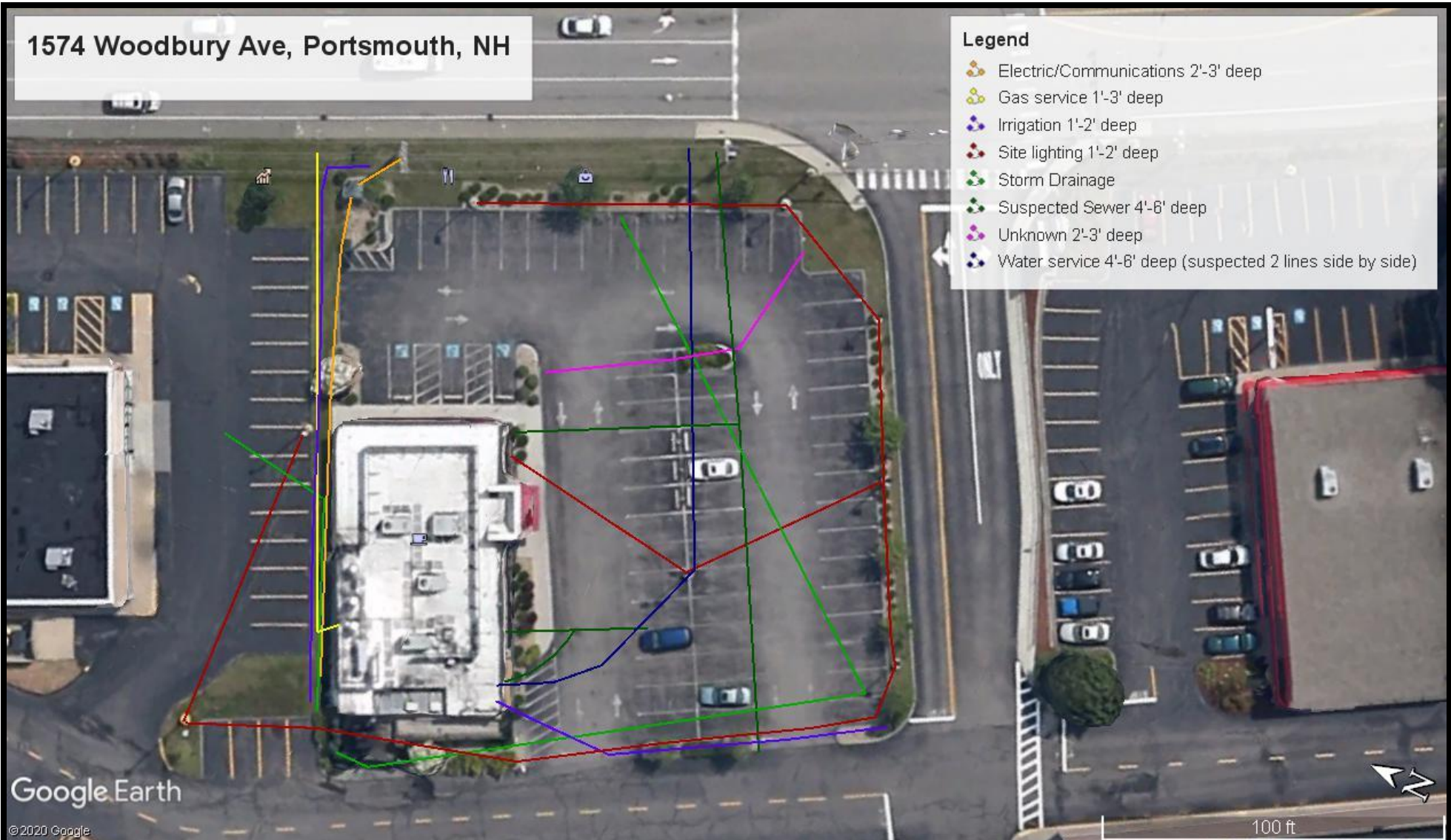
## **FINDINGS**

The subsurface conditions at the time of the scanning allowed for maximum GPR depth penetration of 3'-4' in most areas. Multiple utilities were able to be located such as water, gas, electric, communications, etc. using either the GPR or EM pipe locator. The following pages will provide further explanation of the findings.

1574 Woodbury Ave, Portsmouth, NH









**Legend**

-  Electric/Communications 2'-3' deep
-  Gas service 1'-3' deep
-  Irrigation 1'-2' deep
-  Site lighting 1'-2' deep
-  Storm Drainage
-  Suspected Sewer 4'-6' deep
-  Unknown 2'-3' deep
-  Water service 4'-6' deep (suspected 2 lines side by side)



Google Earth

© 2020 Google

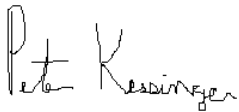
|  |  |  |  |   |   |                                   |   |
|--|--|--|--|---|---|-----------------------------------|---|
| Prepared for: Kevin Furoo<br>Prepared By: Peter Kessinger<br>Date of Scanning: 6/11/2020 | <b>Terms and Conditions</b><br>GPRS does not provide land survey or civil engineering data collection or documentation. This is provided as a reference map of the field markings and is not survey-grade. | <b>LEGEND</b>  |  |   |   | 1574 Woodbury Ave, Portsmouth, NH | Prepared by:<br> |
|  |  |  ELECTRIC |  SANITARY |  WATER |  STORM |                                   |   |
|  |  |  COMM     |  UNKNOWN  |   |   |                                   |   |
|  |  |  GAS      |  |   |   |                                   |   |

## CLOSING

GPRS, Inc. has been in business since 2001, specializing in underground storage tank location, concrete scanning, utility locating, and shallow void detection for projects throughout the United States. I encourage you to visit our website ([www.gprsinc.com](http://www.gprsinc.com)) and contact any of the numerous references listed.

GPRS appreciates the opportunity to offer our services, and we look forward to continuing to work with you on future projects. Please feel free to contact us for additional information or with any questions you may have regarding this report.

Signed,



Peter Kessinger  
Senior Project Manager—New England



Direct: 603.247.6532

[Peter.Kessinger@gprsinc.com](mailto:Peter.Kessinger@gprsinc.com)

[www.gprsinc.com](http://www.gprsinc.com)

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**STORMWATER MANAGEMENT & SOIL EROSION CONTROL REPORT**

**FOR**

**JP MORGAN CHASE BANK – PORTSMOUTH**

**LOT 17, ASSESSOR MAP 238**

**1574 WOODBURY AVENUE**

**CITY OF PORTSMOUTH**

**ROCKINGHAM COUNTY, NEW HAMPSHIRE**

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PREPARED BY:

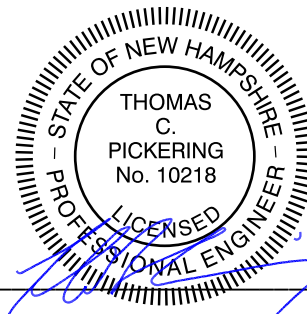
CORE STATES GROUP

9 Galen Street, Suite 117

Watertown, Massachusetts 02472

857-500-4702

July 1st, 2020



Thomas Pickering, P.E.

NH License No. 10218

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- A. Overall Site Plan
- B. Web Soil Survey Map
- C. Hydraulic Calculations Report
- D. Drainage Area Maps
- E. Contech CDS Unit Specifications

## I. INTRODUCTION

The proposed Chase Bank is a commercial re-development project within the G1, Gateway Corridor District, located on Woodbury Avenue between Durgin Lane and Arthur Brady Drive part of the overall development known as “Durgin Square” situated in the City of Portsmouth, New Hampshire. The overall site (**See Appendix “A”**) consists of a total tract area of 1.002 acres or 43,673 square-feet, as reference from Assessors Map 238 Lot 17. The proposed re-development of this property will include the demolition of approximately 4,600-square foot existing restaurant building, foundations, landscaping, pavement, concrete and associated utilities. The re-development proposes the construction of a new single-story Chase Bank with one (1) remote drive-thru ATM and bypass lane. The proposed redevelopment will disturb approximately 0.830 acres (36,110 sf) of the subject property. The design intent for stormwater management is to meet City of Portsmouth stormwater requirements under Ordinance Section-7.4 and in conjunction with New Hampshire Stormwater Manual.

The site is located solely within a commercialized area in the City of Portsmouth, New Hampshire. No streams or waterways are located on the subject site or adjoining the property. The subject property is occupying a parcel lot that is part of an overall shopping center. The intent of this report is to show the proposed stormwater analysis and conditions for the Chase Bank development. The majority of the site’s stormwater discharges to three (3) points of analyses that discharge directly to study points located as shown on the attached drainage area maps (**See Appendix “D”**).

## II. METHODOLOGY

Run-off has been generated under both pre- and post-development conditions in accordance with “Technical Release No. 55-Urban Hydrology for Small Watersheds” and City of Portsmouth stormwater requirements under Ordinance Section-7.4.

Runoff curve numbers (CN’s) have been established by use of the hydrologic soil groups associated with the soils found in the Rockingham County Soil Survey. A composite soil survey map (**See Appendix “B”**) is provided for review of the general soil characteristics. See the chart below of CN number breakdown for existing and proposed.

|                                    | <u>CN</u> |
|------------------------------------|-----------|
| Grass/Lawn Coverage (Soil Group B) | 69        |
| Impervious Coverage                | 98        |

The existing soil stratum found within the project area is comprised of one soil type, Chatfield-Jollis-Canton (140B) as identified from mapping available from the Natural Resource Conservation Service Web Soil Survey and found in **Appendix “B”**.



The proposed time of concentration is based on overland and sheet flow. The time of concentration for the drainage areas has been set to a minimum value of 6 minutes (0.10 hours), per the TR-55 Manual, for the purpose of the calculations.

Hydrographs were generated using “Hydraflow Hydrographs Extension for AutoCAD” by Autodesk, Inc. This program is based upon the Soil Conservation Service methodology for tabular hydrographs using the Type III storm event as detailed in” Technical Release No. 55- Urban Hydrology for Small Watersheds.” The 24-hour rainfall for the four (4) respective storms studied have been gathered from the National Oceanic and Atmospheric Administration rainfall data:

| <u>Storm Event</u> | <u>24 Hour Rainfall</u> |
|--------------------|-------------------------|
| 2-yr*              | 3.32”                   |
| 10-yr*             | 5.33”                   |
| 25-yr*             | 6.59”                   |
| 50-yr*             | 7.51”                   |

\*Based on Rainfall Event over a 24-hour period.

### III. HYDROLOGIC ANALYSIS

#### A. Existing Drainage Area Conditions

As shown on the Existing Drainage Area Map (**See Appendix “D”**), the existing project is comprised of three (3) drainage areas, 0.834 acres, which all discharge into three (3) study points. Study Point 1 is identified as the majority of the onsite flow that flows to the existing stormwater conveyance system. Existing Drainage Area 1, which is primarily located on the north and west part of the site, drains to Study Point-1. Study Point 2 which collects Existing Drainage Area 2 is identified by the flow that bypasses the existing stormwater conveyance system and flows to overall shopping center. The last study point, Study Point 3, is identified as the overland flow that discharges onto the public Right-of-Way and does not flow into the shopping center system. The hydraulic calculations for the existing drainage areas for each study point can be found in **Appendix “C”**.

#### B. Proposed Drainage Area Conditions

The proposed drainage conditions are design to replicate the existing drainage conditions while meeting the City of Portsmouth’s stormwater regulations. As shown on the Proposed Drainage Area Map (**See Appendix “D”**). As it is existing, the proposed drainage area map comprises of three drainages areas which discharge to three separate study points. The hydraulic calculations for the proposed drainage areas for each study point can be found in **Appendix “C”**. The description of each drainage area is located below.

## IV. STORMWATER MANAGEMENT REQUIREMENTS

### A. Best Management Practices (Section 7.4.2)

Per the City of Portsmouth Stormwater Management Regulations as described in Section 7.4 of Site Plan Review regulations all developments under site plan review regardless of limit of disturbance shall meet, as applicable, the 23 requirements for Best Management Practices. Below you will find how the development meets the applicable management practices.

- **Section 7.4.2.1-3**

The Best Management Practices for sections 1-3 are not applicable to the project and therefore do not need to be met.

- **Section 7.4.2.4**

“Snow storage areas shall be located such that no direct discharges to receiving waters are possible from the storage site. Runoff from snow storage areas shall enter treatment areas to remove suspended solids and other contaminants before being discharged to receiving waters or preferably be allowed to infiltrate into the groundwater.”

The proposed project is a redevelopment and near no streams or waterways, therefore would not have any direct discharge into receiving waters. Additionally, as part of the redevelopment the project is proposing water quality system at the most downstream part of the existing conveyance system to treat site runoff. Further details on the proposed water quality system of the project can be found below.

- **Section 7.4.2.5**

“Every effort shall be made to retain stormwater on the site using the natural or existing flow patterns of the site.”

The proposed project is a redevelopment where a majority of the existing drainage patterns are contained on the on-site storm conveyance system and do not discharge to adjacent properties. As part of the proposed redevelopment the existing drainage patterns will be maintained.

- **Section 7.4.2.6-7**

The Best Management Practices for sections 6-7 are not applicable to the project and therefore do not need to be met. Since the overall site is being reduced in impervious coverage and on-site water quality system is being proposed, the need for infiltration practices are not required.

- **Section 7.4.2.8**

“Measure shall be taken to control the post-development peak rate of runoff so that it does not exceed pre-development runoff for the 2-, 10-, 25-, 50-year, 24 hour storm event.

In order to meet this standard, a pre- and post- development comparison of the 2-, 10-, 25-, 50- year storm events for each study point can be found in **Appendix “D”**. The calculations show that at any point the post-development peak discharge rate does not exceed the pre-development peak discharge rate for any storm event. A summary of the runoff quantities can be found on the table below:

| Summary of Existing and Proposed Runoff Quantity |             |            |            |
|--|-------------|------------|------------|
| Area   | Storm Event | Existing   | Proposed   |
|  |             | Flow (cfs) | Flow (cfs) |
| Study Point - 1                                  | 2-year      | 2.131      | 1.858      |
|  | 10-year     | 3.640      | 3.428      |
|  | 25-year     | 4.575      | 4.408      |
|  | 50-year     | 5.253      | 5.120      |
| Study Point - 2                                  | 2-year      | 0.055      | 0.019      |
|  | 10-year     | 0.123      | 0.054      |
|  | 25-year     | 0.167      | 0.079      |
|  | 50-year     | 0.200      | 0.099      |
| Study Point -3                                   | 2-year      | 0.042      | 0.041      |
|  | 10-year     | 0.108      | 0.099      |
|  | 25-year     | 0.155      | 0.138      |
|  | 50-year     | 0.189      | 0.167      |
| TOTAL LOT RUNOFF                                 | 2-year      | 2.227      | 1.918      |
|  | 10-year     | 3.871      | 3.581      |
|  | 25-year     | 4.897      | 4.626      |
|  | 50-year     | 5.643      | 5.385      |

Therefore, described above and through the hydraulic analysis of the pre-development and post-development conditions of the site, the proposed development meets the criteria for stormwater quantity as defined in Section 7.4.2.8 in the City of Portsmouth Ordinance.

- **Section 7.4.2.9**

“The applicant shall demonstrate that on- and off-site downstream channel or system capacity is sufficient to carry the stormwater run-off volume flow without adverse effects, such as flooding and erosion of stream banks and shoreland areas.

As previously stated, the overall combined site runoff of the post-development drainage is being reduced from the pre-development conditions. Therefore, it is determined that no adverse effects of the downstream bank for the proposed project.

- **Section 7.4.2.10**

The Best Management Practices for section 10 are not applicable to the project and therefore do not need to be met.

- **Section 7.4.2.11**

“For a storm event of ½ inch or less, the applicant shall demonstrate that stormwater management practices will remove contaminants from the stormwater runoff that leaves the site. The use of oil and grit traps in manholes, on-site vegetated waterways, and vegetated buffer strips along waterways and drainage swales, and the reduction in use of deicing salts and fertilizers may be required by the Planning Board.”

Per the New Hampshire Stormwater Manual and the City of Portsmouth, a water quality device was added at the most downstream pipe conveyance system to meet the requirements. The proposed water quality device meets the Standards depicted in the New Hampshire Stormwater Manual which requires 80% TSS Removal rate of the “first wash” water quality storm event.

The New Hampshire Stormwater Manual defines the “first wash” water quality storm as the first 1” rainfall of any given storm event. As mentioned previously a CDS 2015-4 unit by Contech has been provided to meet these standards, a CDS Removal Rate calculated for this project has been provided in **Appendix “E”**. The CDS unit uses indirect screening technique to remove suspended solids, fine sands and larger particles. Additionally, the unit has an internal weir/bypass system to only provide solids removal for the water quality storm event and not inhibit the flow of the other storm events. A design summary of the CDS Unit can be found in **Appendix “E”**.

- **Section 7.4.2.12**

The Best Management Practices for section 12 are not applicable to the project and therefore do not need to be met.

- **Section 7.4.2.13**

“The design of the on-site stormwater drainage systems shall not increase or impede existing flows.”

As previously stated, the overall combined site runoff of the post-development drainage is being reduced from the pre-development conditions. Therefore, it is determined that no adverse effects to the existing flows.

- **Section 7.4.2.14-19**

The Best Management Practices for sections 14-19 are met through the proposed project. An extensive landscape and soil erosion plans are proposed to maintain integrity of downstream drainage systems in and the proposed development shall be stabilized as per these requirements.

- **Section 7.4.2.20-23**

The Best Management Practices for section 20-23 are not applicable to the project and therefore do not need to be met.

## **B. Groundwater Recharge**

Per the New Hampshire Stormwater Manual the proposed development groundwater recharge must meet the pre-development groundwater recharge. Since the overall impervious coverage of the site is being reduced the annual post development groundwater recharge will naturally exceed what is existing. It is determined, that no additionally BMP techniques are required to meet the groundwater recharge volume requirement.

## V. SOIL EROSION AND SEDIMENT CONTROL

### A. Overview

The Soil Erosion and Sediment Control Measures for this project include adequately installed perimeter silt fencing, temporary and permanent seeding and mulching, inlet protection, and the installation of temporary stone tracking pads at the project site entrance. A Phase I and Phase II plan has been provided in the drawings set for your reference. All provisions are to be in accordance with the “New Hampshire Stormwater Manual Volume 3”.

The soil erosion and sediment control plan will minimize the downstream erosion hazard by controlling runoff at its source, minimizing runoff from disturbed areas and de-concentrating storm water runoff. The objectives of the erosion control plan will be achieved through the management of storm water runoff during construction.

### B. Temporary Erosion and Sediment Control Measures

The temporary soil erosion and sediment control measures will include, but not limited to, silt fences, diversion ditches, stabilization of the construction entrance, sediment traps and basins, storm drain inlet protection, hydro-seeding and dust control. Detailed descriptions of each of the measures that will be employed on the project have been included in the following paragraphs:

- **Silt Fences** - Silt fences consist of standard strength filter fabric with wire mesh reinforcement (or extra strength synthetic filter fabric) secured to supporting posts and entrenched at the base. Filter fabric requirements and installation design criteria will be in accordance with the requirements in the “New Hampshire Stormwater Manual Volume 1-3”. Silt fences will be installed on the down slope side of work areas, as close to the disturbed areas as possible. Sediment will be removed from behind silt fences when sediment has accumulated to one-third of the original height of the fence.
- **Dust Control** - Dust Control shall be accomplished through the use of vegetative cover, mulch, spray adhesive, sprinkling or barriers. Water will be applied by sprinkler or water truck as necessary during grading operations to minimize sediment transport and maintain acceptable air quality conditions. Repetitive treatments will be done as needed until grades are paved or stabilized with vegetation.
- **Stabilized Construction Entrance** - A ramp of crushed stone extending a minimum distance of 50 feet will be installed at each point of ingress and egress from the site. The purpose of the device is to minimize the potential of tracking mud from the site onto public rights-of-way or adjoining properties. The entrance shall be maintained in a condition, which will prevent tracking, or flowing of sediment onto public rights-of-way, all sediment spilled, dropped, washed or tracked onto public rights-of-way must be removed immediately. When washing is required, it shall be done on an area stabilized with stone and which drains into an approved sediment trapping device.

- **Soil Stockpiling** - Topsoil and earth material shall be stockpiled for reuse at the location shown on the Erosion Control Plans. All stockpiles shall be protected using a perimeter dike of silt fence or straw bale sediment barriers to prevent sediment runoff. This applies to all stockpiles remaining in place for more than two weeks. Stockpile side slopes shall not exceed 2 horizontal to 1 vertical (2:1). Temporary seeding or covering of stockpiles shall be completed within two weeks of formation.

## VI. CONCLUSION

The implementation of the City of Portsmouth standards for stormwater management design have been presented and achieved through the proposed stormwater management analysis and design. Using stormwater conveyance systems, the runoff throughout the developed site has been designed to meet the necessary requirements. Based upon this analysis, the proposed storm water management system will benefit the existing downstream conveyance system by providing a reduction in impervious area and reducing peak flow stormwater rates to them.

**APPENDIX A**

OVERALL SITE PLAN



**GENERAL NOTES:**

THIS DRAWING REFERENCES: 1574 WOODBURY AVENUE PORTSMOUTH, NH TOPOGRAPHIC PLAN OF LAND PREPARED BY ALLEN & MAJOR ASSOCIATES, INC. 400 HARVEY ROAD MANCHESTER, NH 03103 CONTACT: JAMES P. SMITH NH LLS TEL: (603) 627-5500 DATED: 06/23/2020

PROPERTY OWNER: RICHARD P. FUSEGNI 201 KEARSARGE BLVD PORTSMOUTH, NH 03801 CONTACT: SCOTT MITCHELL (603) 475-377

APPLICANT: J.P. MORGAN CHASE BANK 1450 BRICKELL AVENUE 3RD FLOOR MIAMI, FL 33131 CONTACT: CHRIS FOIT TEL: (786) 473-1769

1) SITE ADDRESS: 1574 WOODBURY AVENUE PORTSMOUTH, NH 03801 COUNTY OF ROCKINGHAM

2) ZONING DATA: ZONED: G-1 GATEWAY CORRIDOR DISTRICT EXISTING USE: RESTAURANT - RUBY TUESDAYS (PERMITTED) PROPOSED USE: BANK (PERMITTED BY RIGHT) DRIVE THROUGH (CONDITIONAL USE PERMIT REQUIRED)

| \$10.5B34.60 (SMALL COMMERCIAL BUILDING) | REQUIRED       | EXISTING            | PROPOSED            |
|--|----------------|---------------------|---------------------|
| MIN. LOT AREA, SF:                       | N/A            | 43,673 S.F.         | NO CHANGE           |
| MIN. LOT FRONTAGE, FT:                   | 50 FT.         | 204.32 FT.          | NO CHANGE           |
| FRONT YARD SETBACK, FT:                  | 0 FT. - 20 FT. | 87.7 FT.            | 20 FT.              |
| MIN. SIDE YARD SETBACK, FT:              | 10 FT.         | 9.7 FT.             | 19.94 FT.           |
| MIN. REAR YARD SETBACK, FT:              | 15 FT.         | 16.7 FT.            | 93.6 FT.            |
| MAX. HEIGHT, FT:                         | 40 FT.         | ± 20 FT.            | 21.5 FT.            |
| MAX. HEIGHT, STORIES:                    | 3              | 1                   | 1                   |
| MIN. STREET FACADE HEIGHT:               | 18 FT.         | ± 20 FT.            | 21.5 FT.            |
| MIN. OPEN SPACE COVERAGE:                | 10%            | 17.79% (7,770 S.F.) | 18.18% (7,942 S.F.) |
| MAX. BUILDING COVERAGE:                  | 70%            | 10.53%              | 7.55%               |
| MAX. BUILDING FOOTPRINT:                 | 10,000 S.F.    | 4,600 S.F.          | 3,325 S.F.          |
| MIN. STREET FACING FACADE GLAZING:       | 50%            | "-"                 | 52%                 |

3) PARKING REQUIREMENTS: \$10.1112.30 OFF-STREET PARKING REQUIREMENTS PER THE CITY OF PORTSMOUTH ZONING ORDINANCE: FOR PROFESSIONAL, BUSINESS AND FINANCIAL SERVICES: 1 SPACE PER 350 SQUARE FEET OF GROSS FLOOR GROSS AREA GROSS FLOOR GROSS AREA = 3,325 S.F. (MAIN BUILDING) CALCULATION: 1 SPACE X (3,325 S.F. / 350 S.F.) REQUIRED = 10 SPACES EXISTING PARKING SPACES: 61 SPACES (INCLUDING 3 ACCESSIBLE SPACES) PROPOSED PARKING SPACES: 31 SPACES (INCLUDING 2 ACCESSIBLE SPACES)

PARKING DIMENSIONS EXISTING: VARIES REQUIRED: 8.5' X 19' PROPOSED: 9' X 19'

4) ALL EXISTING FEATURES ARE TO REMAIN UNLESS OTHERWISE NOTED.

5) ALL PAVEMENT MARKINGS SHALL BE LONG LIFE EPOXY.

6) PRIOR TO STARTING CONSTRUCTION, THE CONTRACTOR SHALL BE RESPONSIBLE TO MAKE SURE THAT ALL REQUIRED PERMITS AND APPROVALS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED AND THOROUGHLY REVIEWED ALL PLANS AND OTHER DOCUMENTS BY ALL OF THE PERMITTING AUTHORITIES.

7) ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS AND THE REQUIREMENTS AND STANDARDS OF THE LOCAL GOVERNING AUTHORITY.

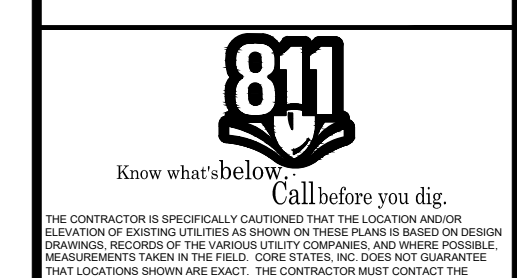
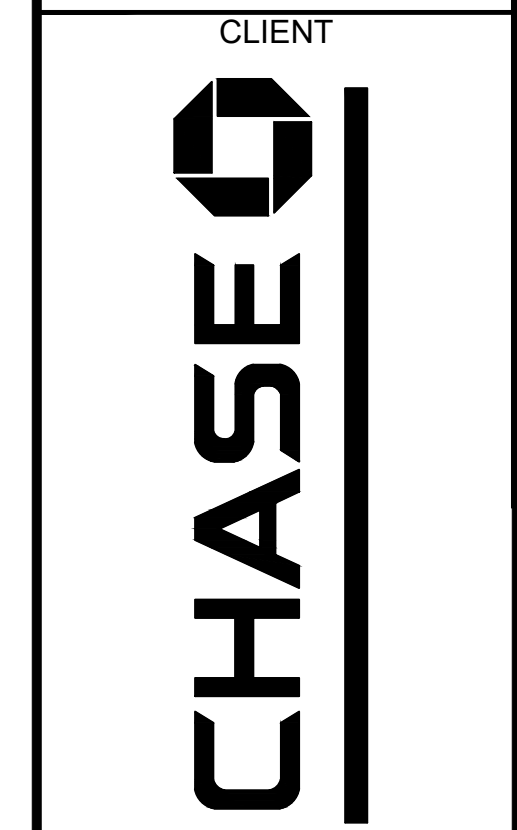
8) ALL DIMENSIONS SHOWN ON THE PLANS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CONTRACTOR SHALL NOTIFY ENGINEER IF ANY DISCREPANCIES EXIST PRIOR TO PROCEEDING WITH CONSTRUCTION FOR NECESSARY PLAN CHANGES. NO EXTRA COMPENSATION SHALL BE PAID TO THE CONTRACTOR FOR WORK HAVING TO BE REDONE DUE TO DIMENSIONS OR GRADES SHOWN INCORRECTLY ON THESE PLANS IF SUCH NOTIFICATION HAS NOT BEEN GIVEN.

9) SOLID WASTE TO BE DISPOSED OF BY CONTRACTOR IN ACCORDANCE WITH ALL LOCAL, STATE AND FEDERAL REGULATIONS.

10) ALL EXCAVATED UNSUITABLE MATERIAL MUST BE TRANSPORTED TO AN APPROVED DISPOSAL LOCATION. CONTRACTOR IS RESPONSIBLE FOR ALL SHORING REQUIRED DURING EXCAVATION AND SHALL BE PERFORMED IN ACCORDANCE WITH CURRENT OSHA STANDARDS, AS WELL AS ADDITIONAL PROVISIONS TO ASSURE STABILITY OF CONTIGUOUS STRUCTURES, AS FIELD CONDITIONS DICTATE.



DOCUMENTS PREPARED BY CORE STATES, INC. INCLUDING THIS DOCUMENT ARE TO BE USED ONLY FOR THE SPECIFIC PROJECT AND SPECIFIC USE FOR WHICH THEY WERE INTENDED. ANY EXTENSION OF USE TO ANY OTHER PROJECTS, BY OWNER OR BY ANY OTHER PARTY, WITHOUT THE EXPRESSED WRITTEN CONSENT OF CORE STATES, INC. IS DONE UNLAWFULLY AND AT THE USER'S OWN RISK. IT IS USED IN A WAY OTHER THAN THAT SPECIFICALLY INTENDED. USER WILL HOLD CORE STATES, INC. HARMLESS FROM ALL CLAIMS AND LOSSES.



REVISIONS

| REV | DATE | COMMENT | BY |
|-----|------|---------|----|
|     |      |         |    |
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DOCUMENT

SITE PLAN APPROVAL FOR CHASE BANK

SITE LOCATION  
1574 WOODBURY AVENUE, PORTSMOUTH, NH 03801

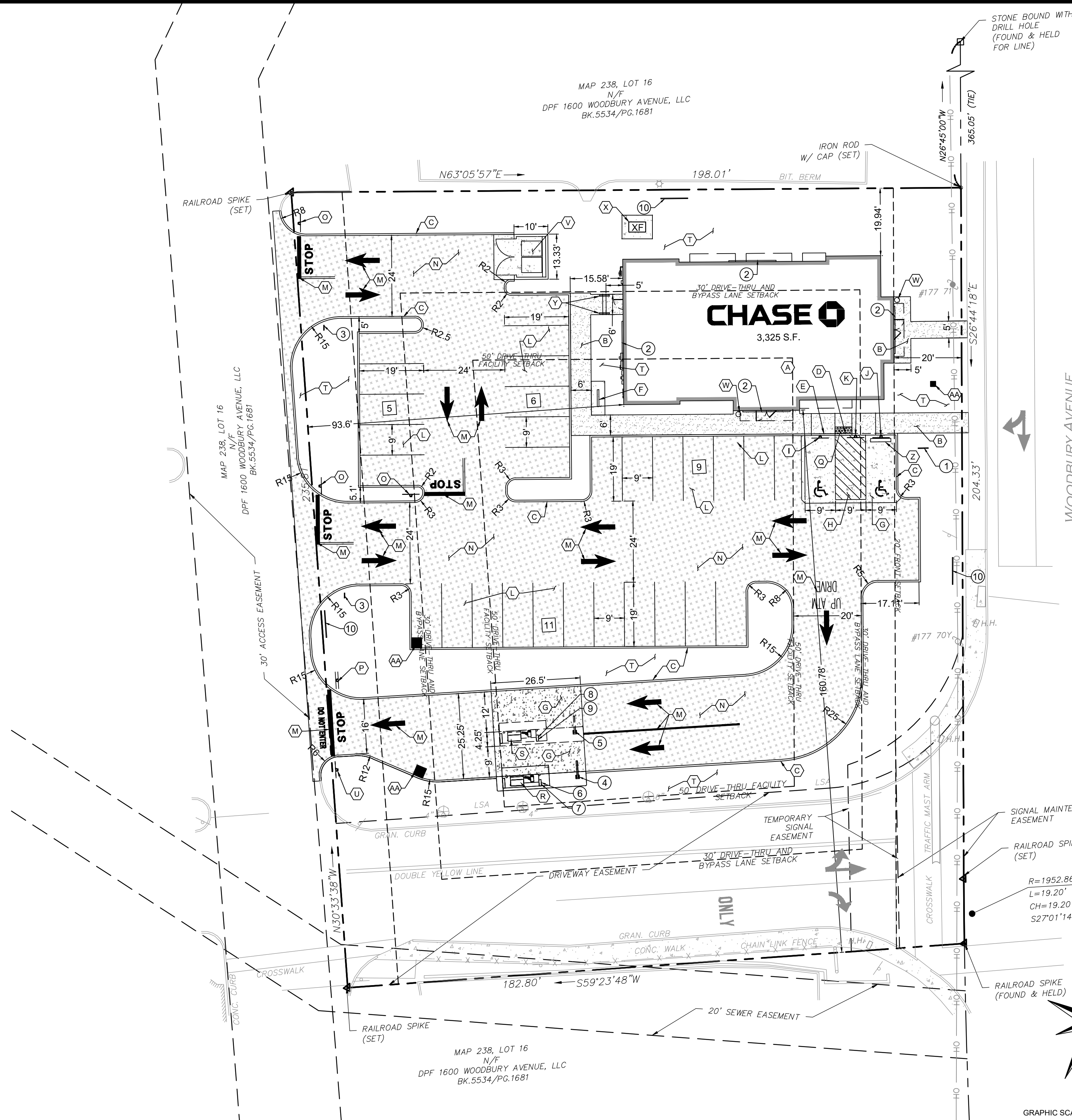
ENGINEER SEAL

SHEET TITLE

SITE PLAN

JOB #: JPM27086  
DATE: 07/01/2020  
SCALE: AS NOTED  
DRAWN BY: MAL  
CHECKED BY: KGF

SHEET NO.  
**C-2**



SIGNAGE TABLE (SIGN DISTRICT 5)

| PROVISION                      | REQUIRED   | PROPOSED  | COMMENT  |
|--------------------------------|--|---|--|
| <b>TEMPORARY SIGNS</b>         |  |   |  |
| MAXIMUM SIGN AREA              | 64 SF  | 60 SF   | COMPLIANT                                      |
| MAXIMUM SIGN HEIGHT            | 12 FEET  | <12 FT (MOUNTED ON CONSTRUCTION FENCE)  | COMPLIANT                                      |
| <b>FREESTANDING SIGN</b>       |  |   |  |
| MAXIMUM ALLOWED PER LOT        | 1  | 1 MONUMENT SIGN<br>1 ATM SIGN<br>1 FUTURE ATM SIGN  | COMPLIANT<br>VARIANCE<br>VARIANCE              |
| MAXIMUM SIGN AREA              | 100 SF   | 56.2 SF<br>9.5 SF<br>9.5 SF   | COMPLIANT                                      |
| MAXIMUM SETBACK FRONT LOT LINE | 10 FT  | 10 FT   | COMPLIANT                                      |
| <b>WALL SIGN</b>               |  |   |  |
| MAXIMUM ALLOWED                | ONE PER STREET FRONTAGE AND/OR AT MAIN ENTRANCE                        | 1 EAST ELEVATION (WOODBURY AVE)<br>1 SOUTH ELEVATION<br>1 NORTH ELEVATION<br>1 WEST ELEVATION | COMPLIANT<br>COMPLIANT<br>VARIANCE<br>VARIANCE |
| MAXIMUM SIGN AREA              | 100 SF   | 36.9 SF   | COMPLIANT                                      |
| <b>TOTAL AGGREGATE SIGNS</b>   |  |   |  |
| MAXIMUM AGGREGATE SIGN AREA    | 1.5 SF PER LINEAR FEET OF BUILDING FRONTAGE = 1.5 X 42.5 LF = 63.75 SF | 147.6 SF  | VARIANCE                                       |

SITE PLAN  
SCALE 1" = 20'

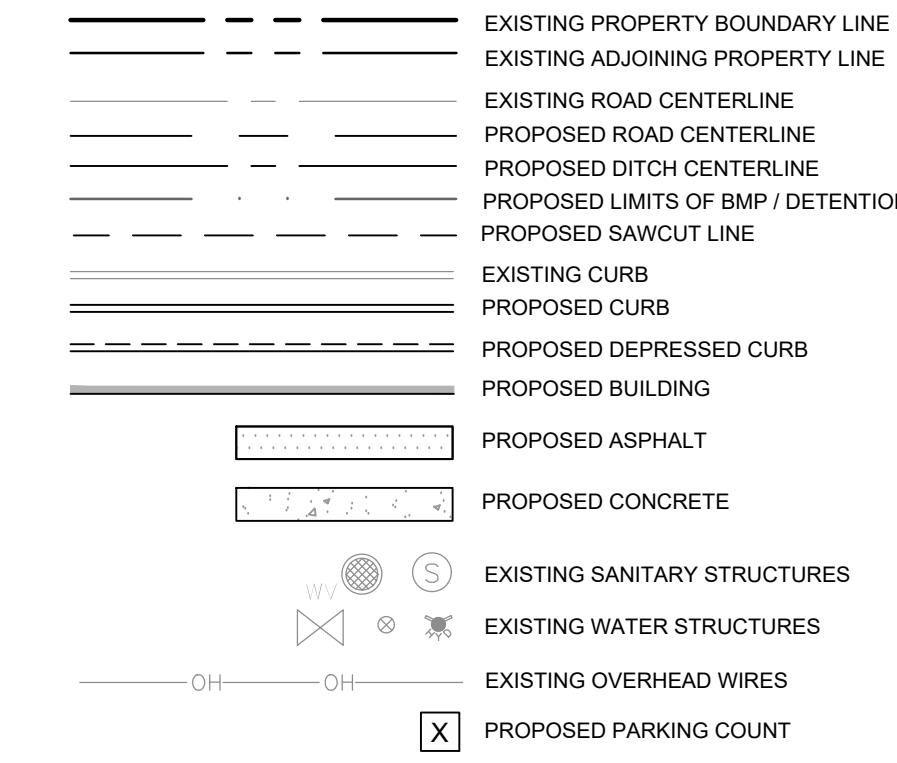
**SITE KEY NOTES:**

**SIGN KEY NOTES:**

- PROPOSED CHASE PROTOTYPICAL PYLON SIGN (56.2 S.F.).
- PROPOSED WHITE W/BLUE OCTAGON ILLUMINATED CHANNEL "CHASE" LOGO SIGN WALL SIGN (36.9 S.F.).
- PROPOSED DOUBLE FACED NON-ILLUMINATED DIRECTIONAL SIGN (2.3 S.F.).
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR.
- PROPOSED CLEARANCE SIGN AND HEADACHE BAR FOR FUTURE DRIVE-UP ATM.
- PROPOSED SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- PROPOSED SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- FUTURE SIGNATURE DRIVE-UP "CHASE" LOGO LETTERS (5.6 S.F.).
- FUTURE SIGNATURE DRIVE-UP OCTAGON (3.9 S.F.).
- TEMPORARY "COMING SOON" SIGN MOUNTED ON CONSTRUCTION FENCE (60S.F.).

- PROPOSED 3,325 S.F. JPMORGAN CHASE BANK BUILDING. REFER TO ARCHITECTURAL PLANS FOR DETAILS.
- PROPOSED CONCRETE SIDEWALK. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED CONCRETE CURB. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED DEPRESSED CONCRETE CURB. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED TRANSITION CURB SECTION. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED BIKE RACK ON CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED CONCRETE PAD. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED 9' X 19' ACCESSIBLE PARKING SPACE AND AISLE WITH SYMBOLS OF ACCESSIBILITY. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED VAN ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED ACCESSIBLE PARKING SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED NO PARKING ANYTIME SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED 9' X 19' STANDARD PARKING SPACE. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED SITE MARKINGS. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED ASPHALT PAVEMENT. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED STOP SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED STOP & DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED DETECTABLE WARNING SURFACE. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED DRIVE-UP SIGNATURE ATM CANOPY. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED "FUTURE" DRIVE-UP SIGNATURE ATM CANOPY. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED LANDSCAPE AREA. REFER TO LANDSCAPE PLAN FOR DETAILS.
- PROPOSED DO NOT ENTER SIGN. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED TRASH ENCLOSURE.
- PROPOSED TRASH BIN. REFER TO ARCHITECT PLANS FOR DETAIL.
- RELOCATED ELECTRIC TRANSFORMER AND CONCRETE PAD. REFER TO LIGHTING PLANS FOR DETAIL.
- PROPOSED HANDRAIL. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED WHEEL STOP. REFER TO CONSTRUCTION DETAILS SHEET.
- PROPOSED STORM DRAIN INLET. REFER TO CONSTRUCTION DETAILS SHEET.

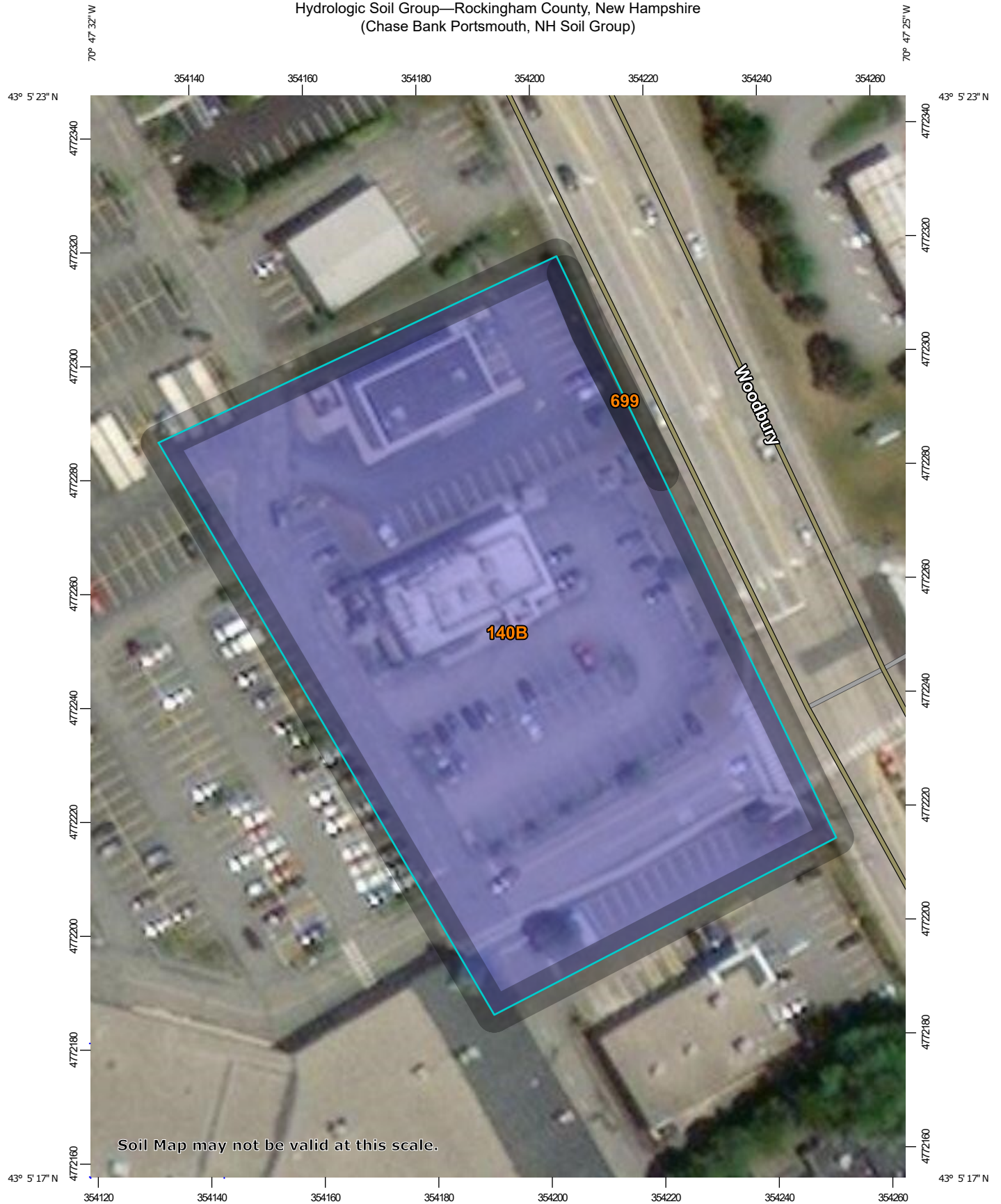
**SITE LEGEND**



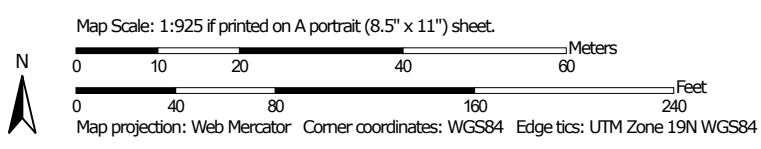
**APPENDIX B**

WEB SOIL SURVEY MAP

Hydrologic Soil Group—Rockingham County, New Hampshire  
(Chase Bank Portsmouth, NH Soil Group)



Soil Map may not be valid at this scale.



## MAP LEGEND

### Area of Interest (AOI)









 Area of Interest (AOI)

### Soils

#### Soil Rating Polygons





 A  
 A/D  
 B  
 B/D  
 C  
 C/D  
 D  
 Not rated or not available

#### Soil Rating Lines


 A  
 A/D  
 B  
 B/D  
 C  
 C/D  
 D  
 Not rated or not available

#### Soil Rating Points






 A  
 A/D  
 B  
 B/D

 C  
 C/D  
 D  
 Not rated or not available

### Water Features

 Streams and Canals

### Transportation

 Rails  
 Interstate Highways  
 US Routes  
 Major Roads  
 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rockingham County, New Hampshire  
 Survey Area Data: Version 21, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Sep 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

| Map unit symbol                    | Map unit name   | Rating | Acres in AOI | Percent of AOI |
|------------------------------------|---|--------|--------------|----------------|
| 140B                               | Chatfield-Hollis-Canton complex, 0 to 8 percent slopes, rocky | B      | 2.1          | 99.8%          |
| 699                                | Urban land  |        | 0.0          | 0.2%           |
| <b>Totals for Area of Interest</b> |   |        | <b>2.1</b>   | <b>100.0%</b>  |

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### Rating Options

*Aggregation Method:* Dominant Condition

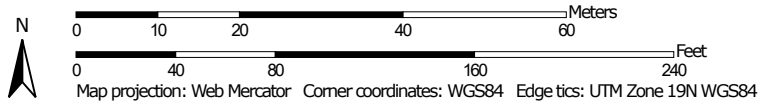
*Component Percent Cutoff: None Specified*

*Tie-break Rule: Higher*

Soil Map—Rockingham County, New Hampshire  
(Chase Bank Portsmouth, NH Soils Map)



Map Scale: 1:925 if printed on A portrait (8.5" x 11") sheet.




## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

### Water Features



Streams and Canals

### Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

### Background



Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

**Warning:** Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rockingham County, New Hampshire

Survey Area Data: Version 21, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Sep 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Map Unit Legend

| Map Unit Symbol                    | Map Unit Name   | Acres in AOI | Percent of AOI |
|------------------------------------|---|--------------|----------------|
| 140B                               | Chatfield-Hollis-Canton complex, 0 to 8 percent slopes, rocky | 2.1          | 99.8%          |
| 699                                | Urban land  | 0.0          | 0.2%           |
| <b>Totals for Area of Interest</b> |   | <b>2.1</b>   | <b>100.0%</b>  |

**APPENDIX C**

HYDRAULIC CALCULATIONS REPORT

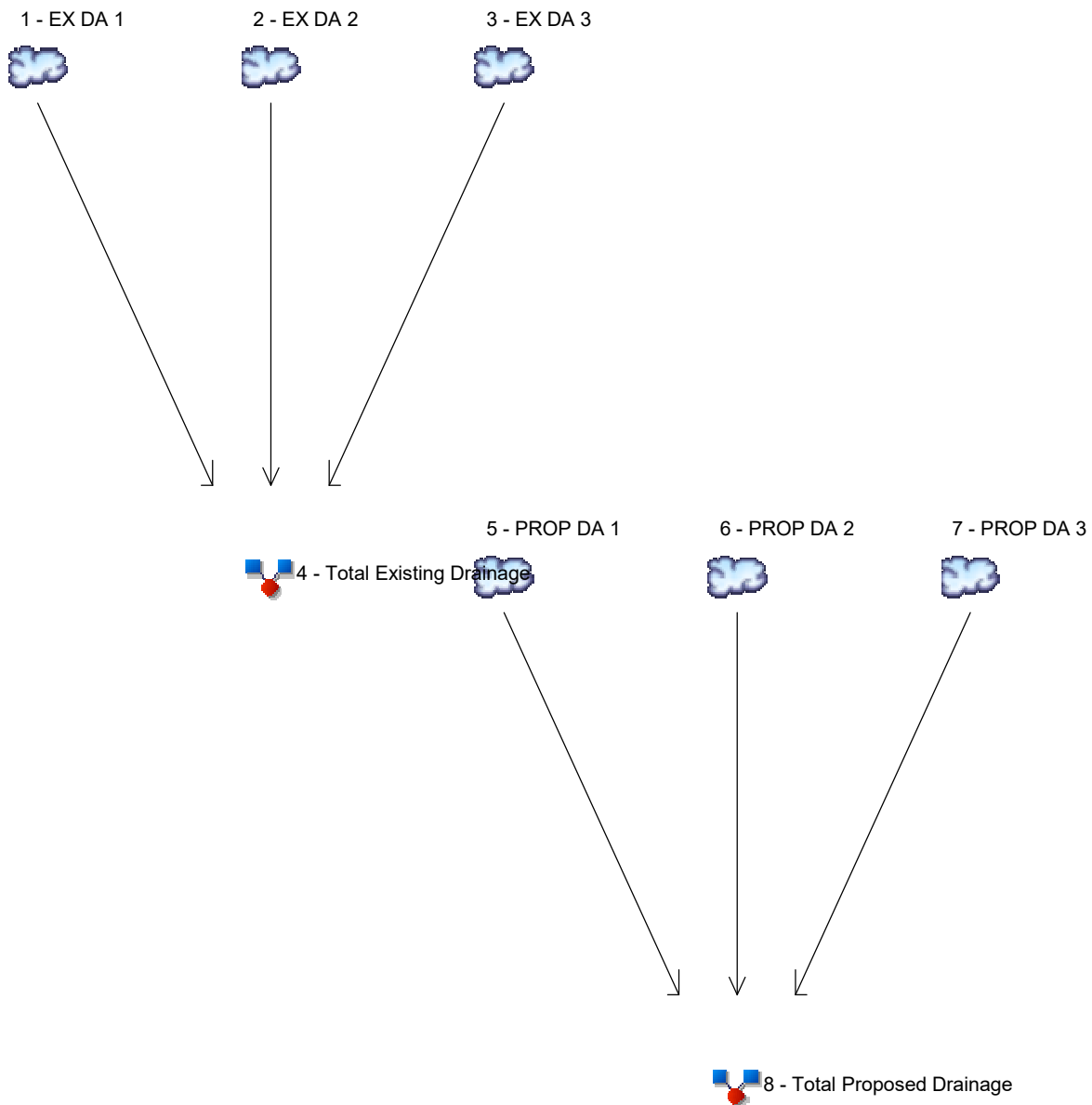
|   |           |
|---|-----------|
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|   |           |
|---|-----------|
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# Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020



## Legend

| Hyd. | Origin     | Description             |
|------|------------|-------------------------|
| 1    | SCS Runoff | EX DA 1                 |
| 2    | SCS Runoff | EX DA 2                 |
| 3    | SCS Runoff | EX DA 3                 |
| 4    | Combine    | Total Existing Drainage |
| 5    | SCS Runoff | PROP DA 1               |
| 6    | SCS Runoff | PROP DA 2               |
| 7    | SCS Runoff | PROP DA 3               |
| 8    | Combine    | Total Proposed Drainage |

# Hydrograph Return Period Recap

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

| Hyd. No. | Hydrograph type (origin) | Inflow hyd(s) | Peak Outflow (cfs) |       |       |       |       |       |       |        | Hydrograph Description  |
|----------|--------------------------|---------------|--------------------|-------|-------|-------|-------|-------|-------|--------|-------------------------|
|          |                          |               | 1-yr               | 2-yr  | 3-yr  | 5-yr  | 10-yr | 25-yr | 50-yr | 100-yr |                         |
| 1        | SCS Runoff               | -----         | -----              | 2.131 | ----- | ----- | 3.640 | 4.575 | 5.253 | -----  | EX DA 1                 |
| 2        | SCS Runoff               | -----         | -----              | 0.055 | ----- | ----- | 0.123 | 0.167 | 0.200 | -----  | EX DA 2                 |
| 3        | SCS Runoff               | -----         | -----              | 0.042 | ----- | ----- | 0.108 | 0.155 | 0.189 | -----  | EX DA 3                 |
| 4        | Combine                  | 1, 2, 3       | -----              | 2.227 | ----- | ----- | 3.871 | 4.897 | 5.643 | -----  | Total Existing Drainage |
| 5        | SCS Runoff               | -----         | -----              | 1.858 | ----- | ----- | 3.428 | 4.408 | 5.120 | -----  | PROP DA 1               |
| 6        | SCS Runoff               | -----         | -----              | 0.019 | ----- | ----- | 0.054 | 0.079 | 0.099 | -----  | PROP DA 2               |
| 7        | SCS Runoff               | -----         | -----              | 0.041 | ----- | ----- | 0.099 | 0.138 | 0.167 | -----  | PROP DA 3               |
| 8        | Combine                  | 5, 6, 7       | -----              | 1.918 | ----- | ----- | 3.581 | 4.626 | 5.385 | -----  | Total Proposed Drainage |

# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

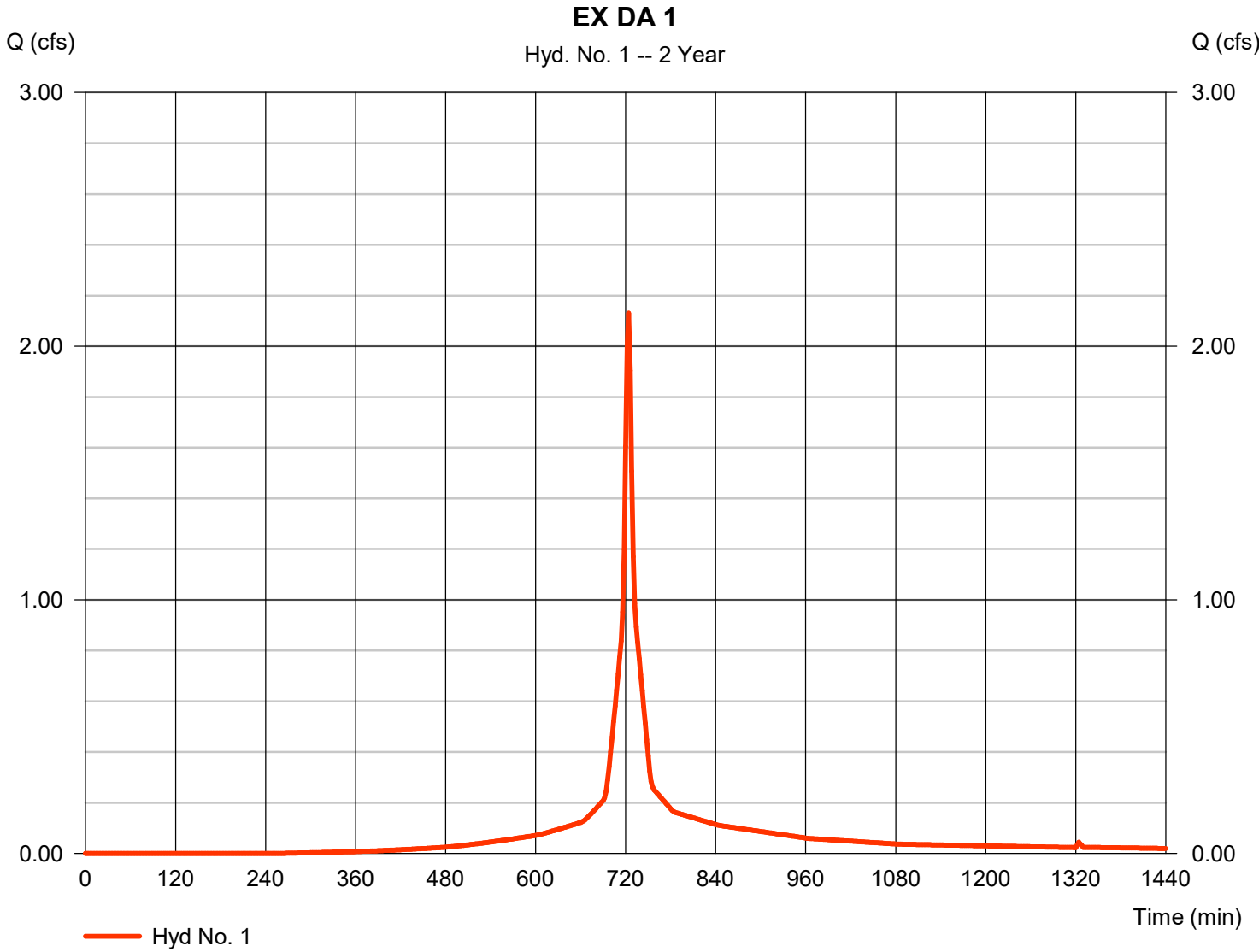
| Hyd. No.                         | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft)    | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description  |
|----------------------------------|--------------------------|-----------------|---------------------|--------------------|-----------------------|---------------|------------------------|-------------------------|-------------------------|
| 1                                | SCS Runoff               | 2.131           | 2                   | 724                | 6,624                 | -----         | -----                  | -----                   | EX DA 1                 |
| 2                                | SCS Runoff               | 0.055           | 2                   | 724                | 167                   | -----         | -----                  | -----                   | EX DA 2                 |
| 3                                | SCS Runoff               | 0.042           | 2                   | 724                | 133                   | -----         | -----                  | -----                   | EX DA 3                 |
| 4                                | Combine                  | 2.227           | 2                   | 724                | 6,925                 | 1, 2, 3       | -----                  | -----                   | Total Existing Drainage |
| 5                                | SCS Runoff               | 1.858           | 2                   | 724                | 5,588                 | -----         | -----                  | -----                   | PROP DA 1               |
| 6                                | SCS Runoff               | 0.019           | 2                   | 724                | 63                    | -----         | -----                  | -----                   | PROP DA 2               |
| 7                                | SCS Runoff               | 0.041           | 2                   | 724                | 128                   | -----         | -----                  | -----                   | PROP DA 3               |
| 8                                | Combine                  | 1.918           | 2                   | 724                | 5,780                 | 5, 6, 7       | -----                  | -----                   | Total Proposed Drainage |
| 2020.06.28 Hydraulics Design.gpw |                          |                 |                     |                    | Return Period: 2 Year |               |                        | Tuesday, 06 / 30 / 2020 |                         |

# Hydrograph Report

## Hyd. No. 1

EX DA 1

|                 |              |                    |              |
|-----------------|--------------|--------------------|--------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 2.131 cfs  |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min    |
| Time interval   | = 2 min      | Hyd. volume        | = 6,624 cuft |
| Drainage area   | = 0.760 ac   | Curve number       | = 93         |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft       |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min   |
| Total precip.   | = 3.32 in    | Distribution       | = Type III   |
| Storm duration  | = 24 hrs     | Shape factor       | = 484        |





# Hydrograph Report

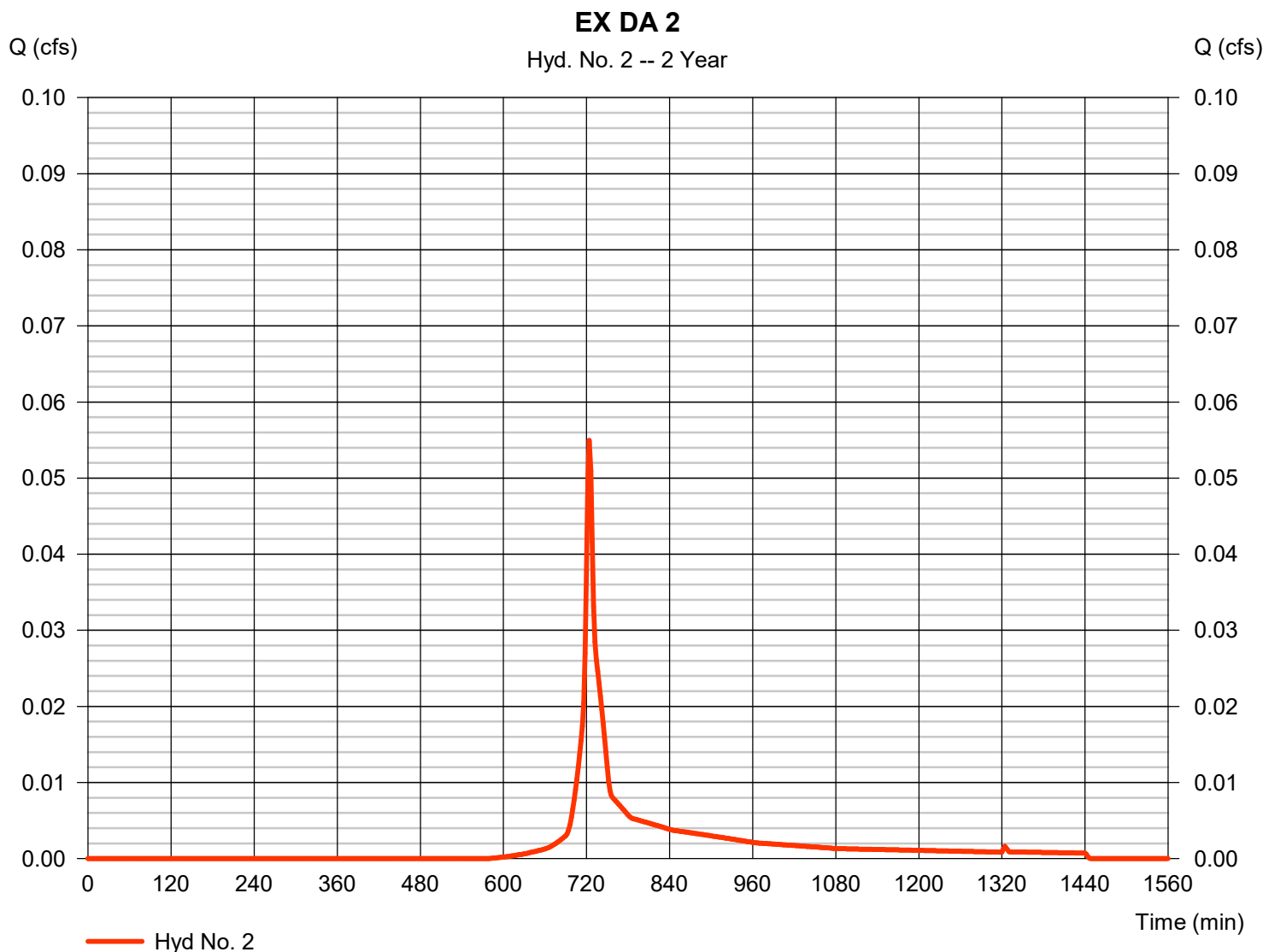
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 2

EX DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.055 cfs |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 167 cuft  |
| Drainage area   | = 0.036 ac   | Curve number       | = 78        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 3.32 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

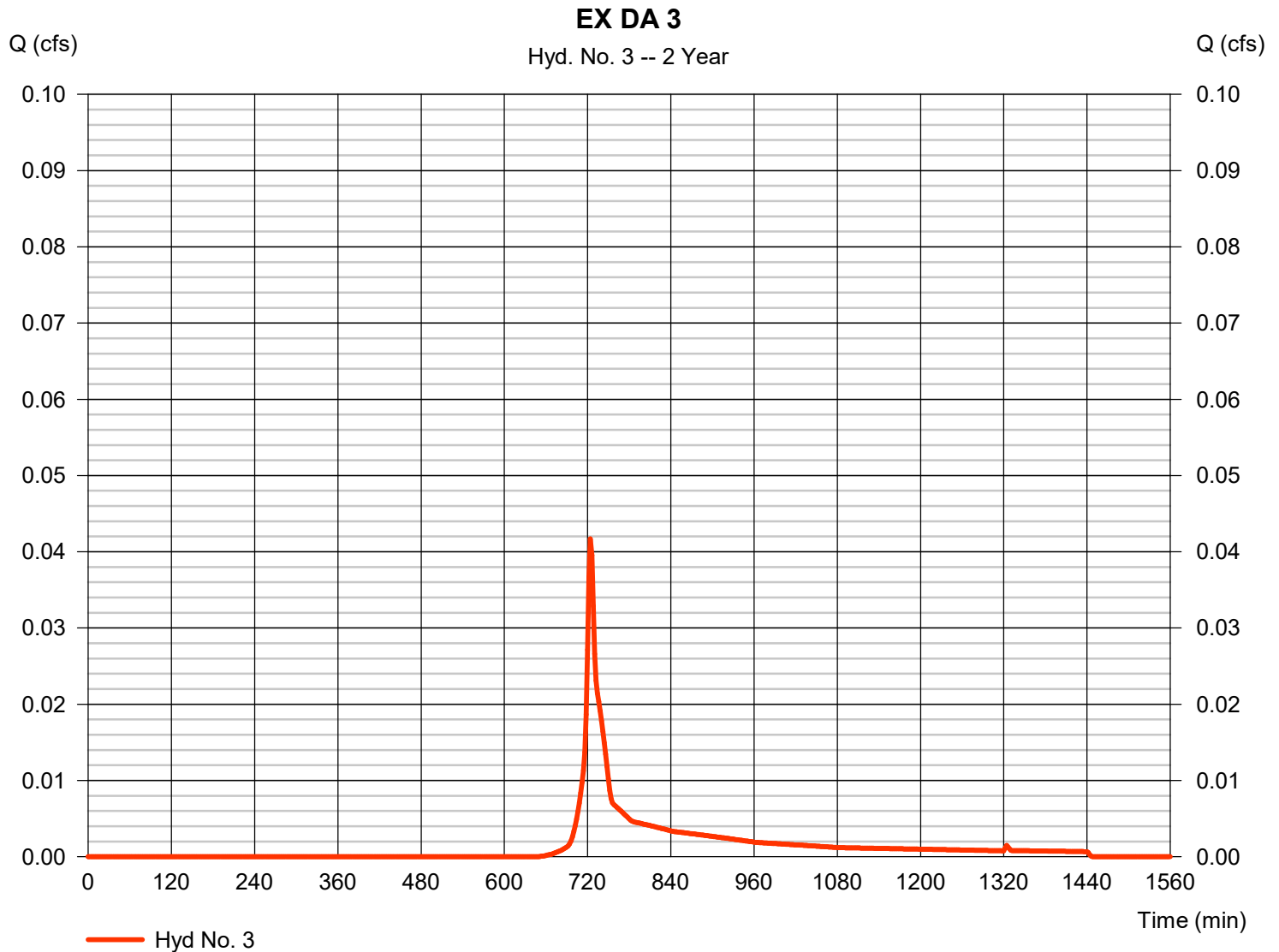
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 3

EX DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.042 cfs |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 133 cuft  |
| Drainage area   | = 0.039 ac   | Curve number       | = 72        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 3.32 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

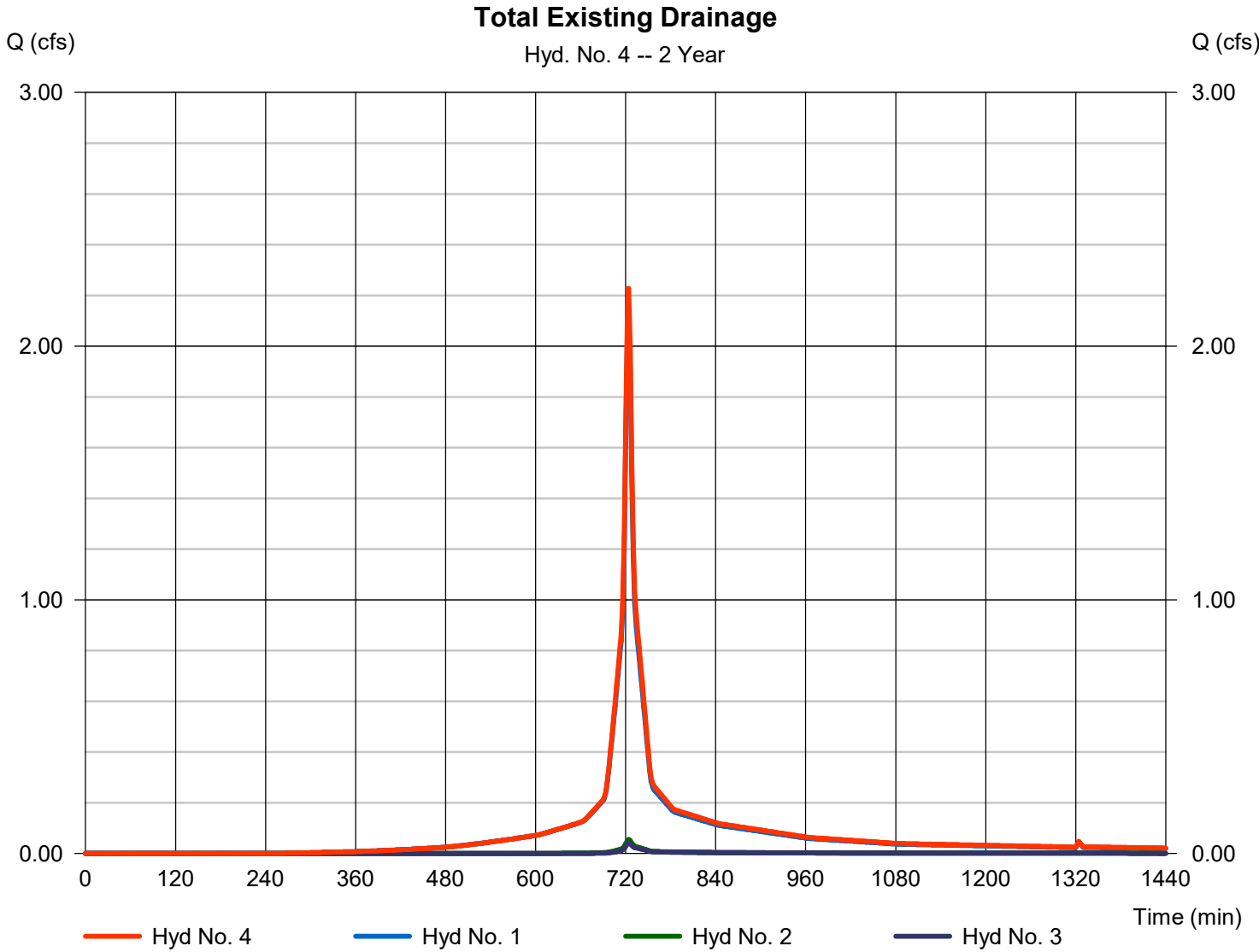
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Tuesday, 06 / 30 / 2020

## Hyd. No. 4

### Total Existing Drainage

|                 |           |                      |              |
|-----------------|-----------|----------------------|--------------|
| Hydrograph type | = Combine | Peak discharge       | = 2.227 cfs  |
| Storm frequency | = 2 yrs   | Time to peak         | = 724 min    |
| Time interval   | = 2 min   | Hyd. volume          | = 6,925 cuft |
| Inflow hyds.    | = 1, 2, 3 | Contrib. drain. area | = 0.835 ac   |

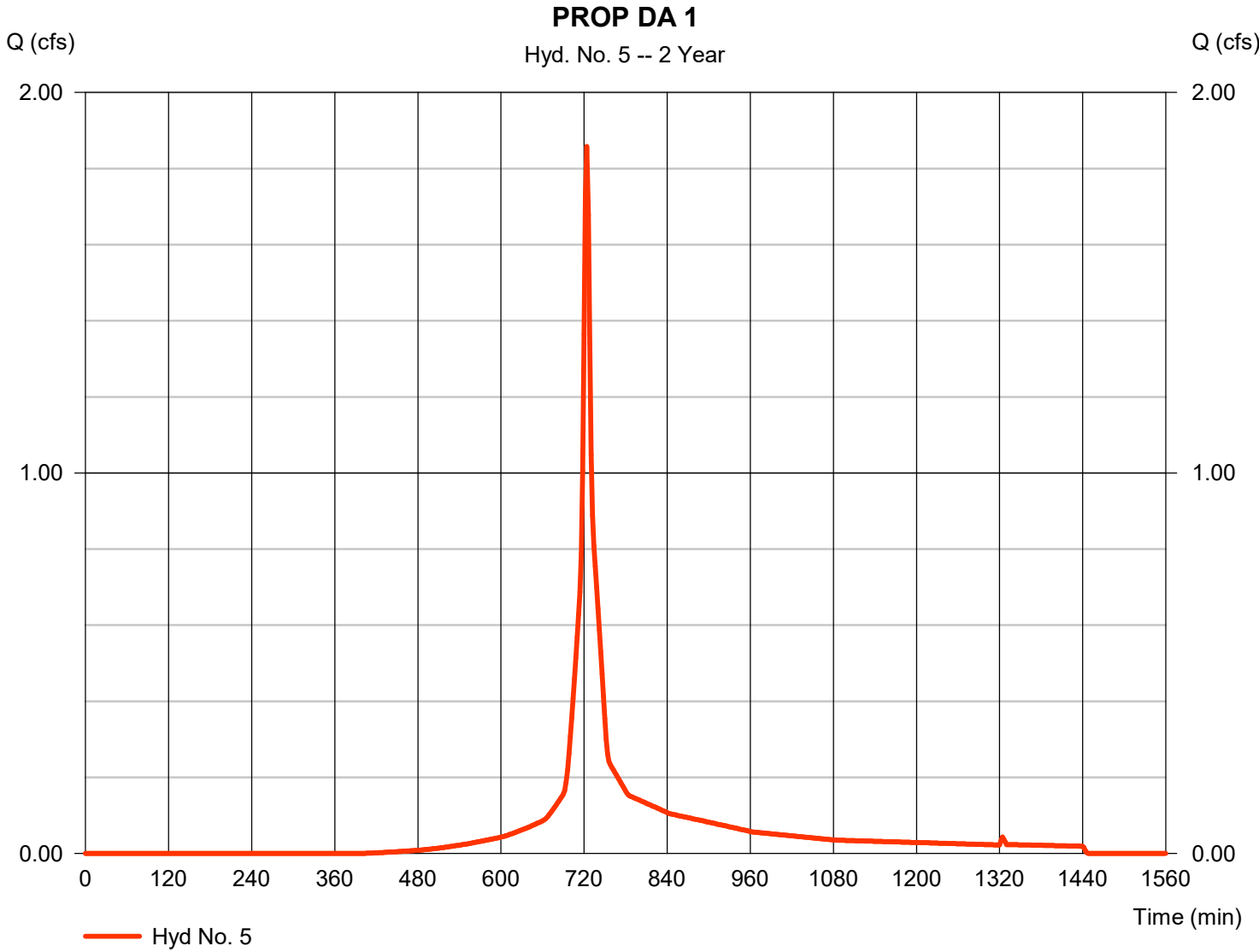


# Hydrograph Report

## Hyd. No. 5

PROP DA 1

|                 |              |                    |              |
|-----------------|--------------|--------------------|--------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 1.858 cfs  |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min    |
| Time interval   | = 2 min      | Hyd. volume        | = 5,588 cuft |
| Drainage area   | = 0.780 ac   | Curve number       | = 88         |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft       |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min   |
| Total precip.   | = 3.32 in    | Distribution       | = Type III   |
| Storm duration  | = 24 hrs     | Shape factor       | = 484        |

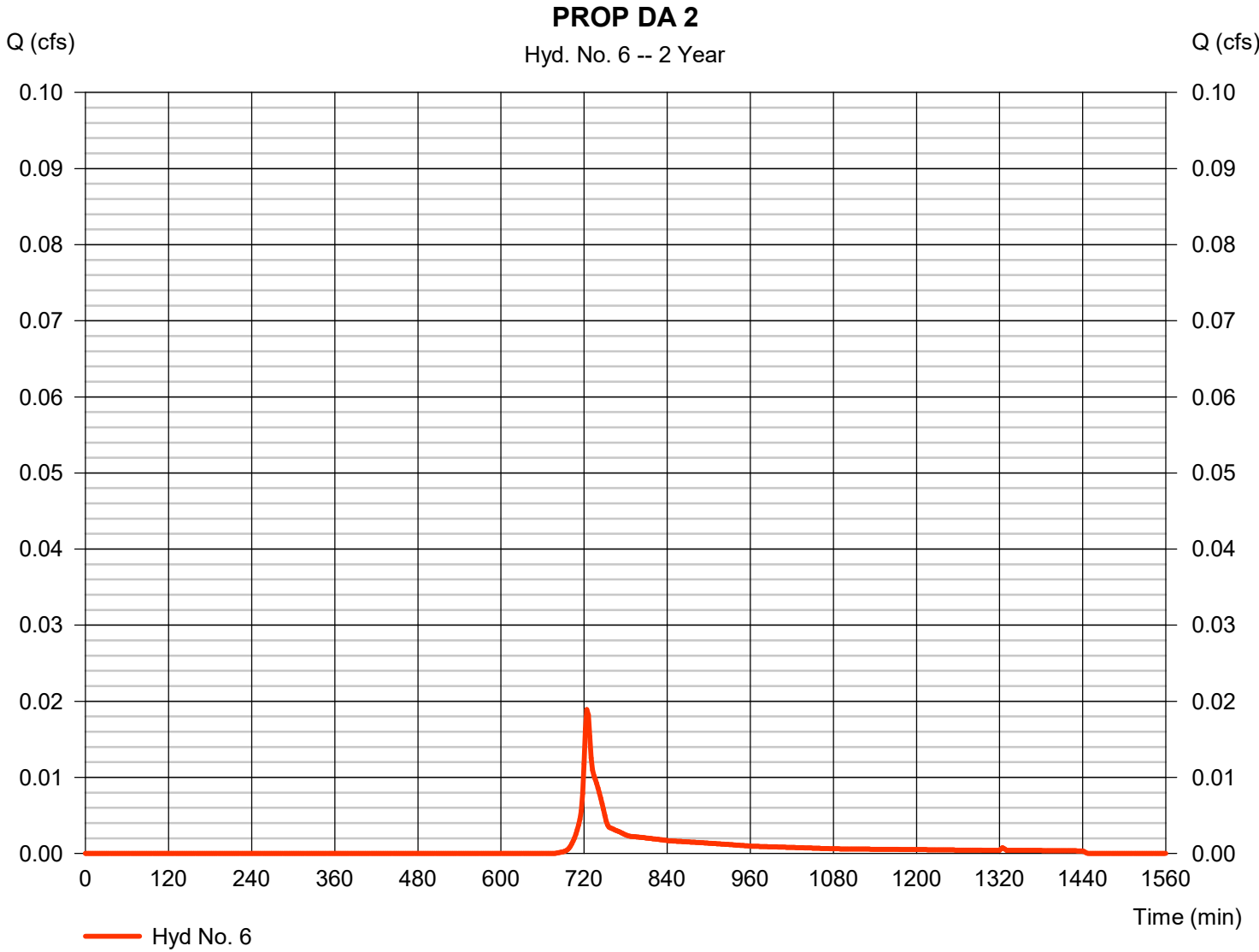


# Hydrograph Report

## Hyd. No. 6

PROP DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.019 cfs |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 63 cuft   |
| Drainage area   | = 0.022 ac   | Curve number       | = 69        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 3.32 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

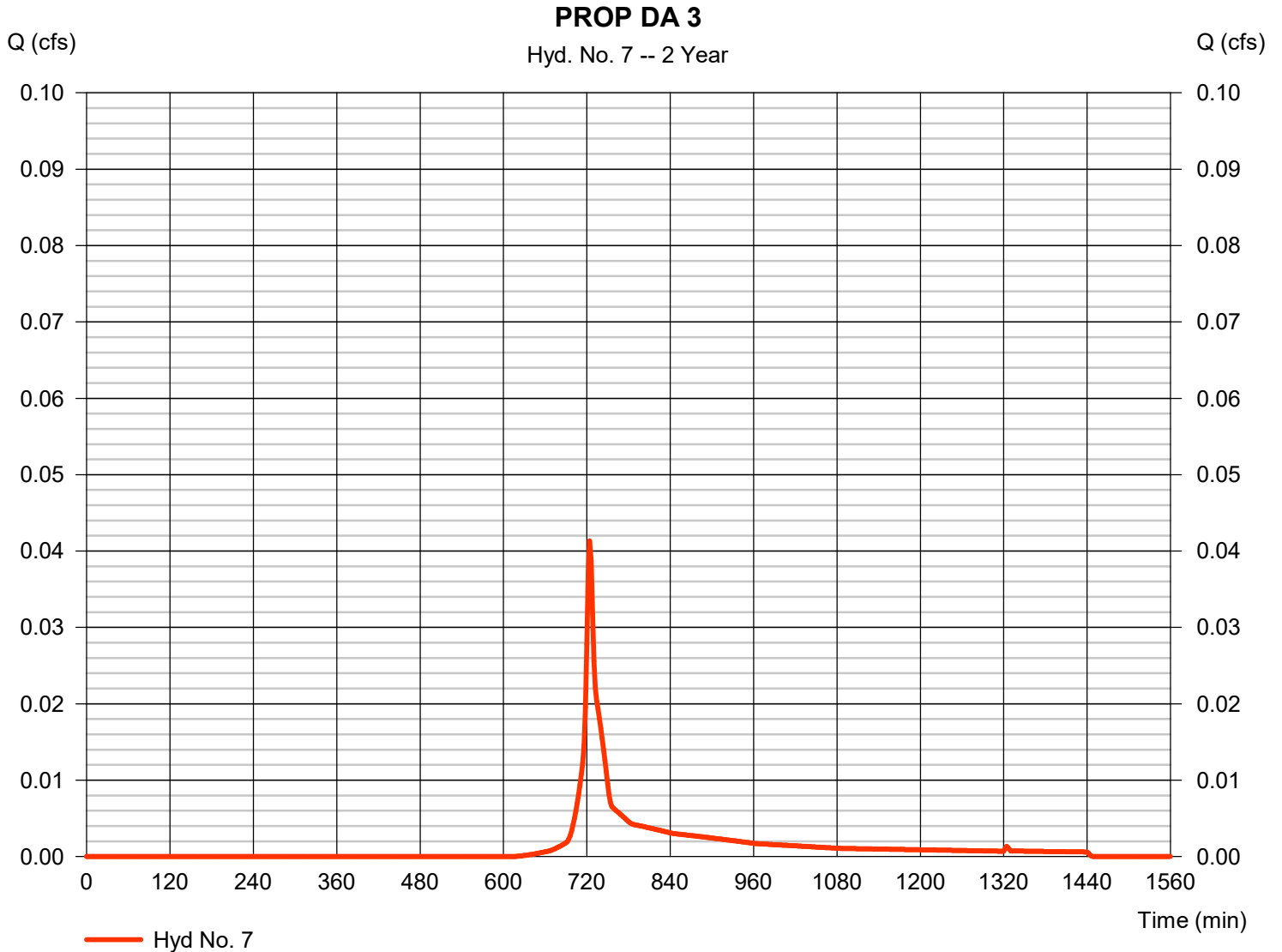
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Tuesday, 06 / 30 / 2020

## Hyd. No. 7

PROP DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.041 cfs |
| Storm frequency | = 2 yrs      | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 128 cuft  |
| Drainage area   | = 0.032 ac   | Curve number       | = 75        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 3.32 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

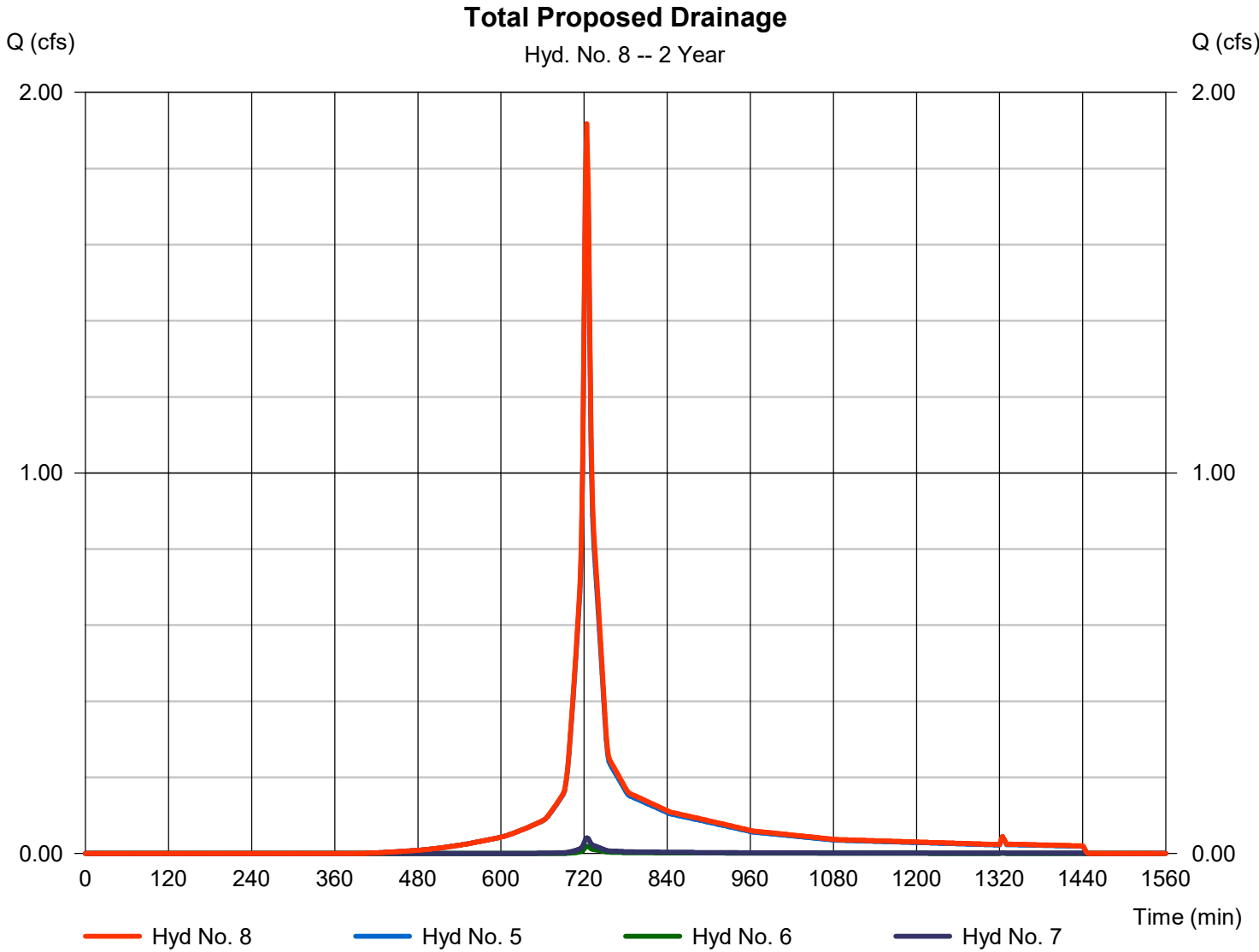
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Tuesday, 06 / 30 / 2020

## Hyd. No. 8

Total Proposed Drainage

|                 |           |                      |              |
|-----------------|-----------|----------------------|--------------|
| Hydrograph type | = Combine | Peak discharge       | = 1.918 cfs  |
| Storm frequency | = 2 yrs   | Time to peak         | = 724 min    |
| Time interval   | = 2 min   | Hyd. volume          | = 5,780 cuft |
| Inflow hyds.    | = 5, 6, 7 | Contrib. drain. area | = 0.834 ac   |



# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

| Hyd. No.                         | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft)     | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description  |
|----------------------------------|--------------------------|-----------------|---------------------|--------------------|------------------------|---------------|------------------------|-------------------------|-------------------------|
| 1                                | SCS Runoff               | 3.640           | 2                   | 724                | 11,696                 | -----         | -----                  | -----                   | EX DA 1                 |
| 2                                | SCS Runoff               | 0.123           | 2                   | 724                | 367                    | -----         | -----                  | -----                   | EX DA 2                 |
| 3                                | SCS Runoff               | 0.108           | 2                   | 724                | 326                    | -----         | -----                  | -----                   | EX DA 3                 |
| 4                                | Combine                  | 3.871           | 2                   | 724                | 12,389                 | 1, 2, 3       | -----                  | -----                   | Total Existing Drainage |
| 5                                | SCS Runoff               | 3.428           | 2                   | 724                | 10,573                 | -----         | -----                  | -----                   | PROP DA 1               |
| 6                                | SCS Runoff               | 0.054           | 2                   | 724                | 165                    | -----         | -----                  | -----                   | PROP DA 2               |
| 7                                | SCS Runoff               | 0.099           | 2                   | 724                | 296                    | -----         | -----                  | -----                   | PROP DA 3               |
| 8                                | Combine                  | 3.581           | 2                   | 724                | 11,034                 | 5, 6, 7       | -----                  | -----                   | Total Proposed Drainage |
| 2020.06.28 Hydraulics Design.gpw |                          |                 |                     |                    | Return Period: 10 Year |               |                        | Tuesday, 06 / 30 / 2020 |                         |



# Hydrograph Report

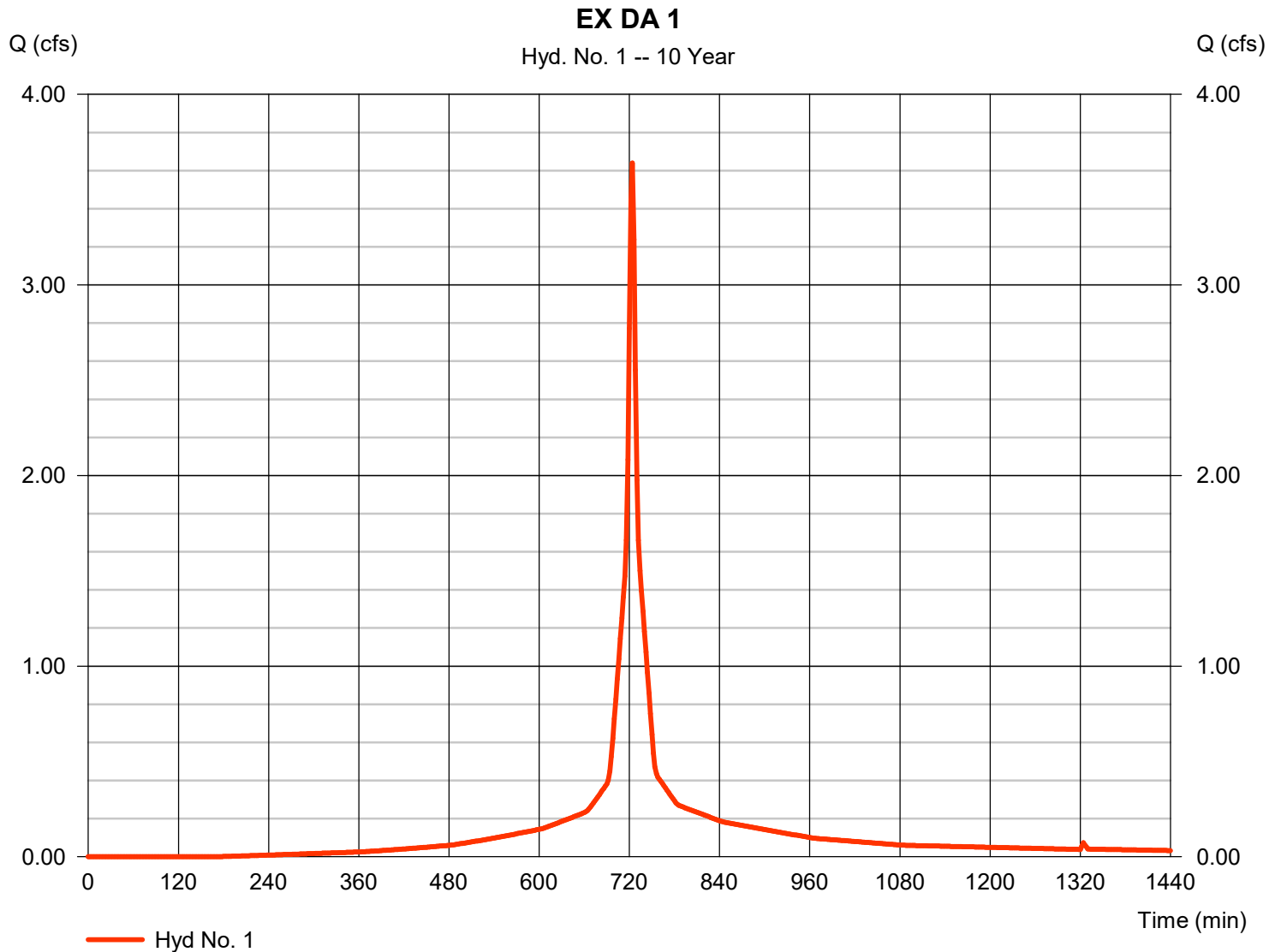
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Tuesday, 06 / 30 / 2020

## Hyd. No. 1

EX DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 3.640 cfs   |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 11,696 cuft |
| Drainage area   | = 0.760 ac   | Curve number       | = 93          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 5.33 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

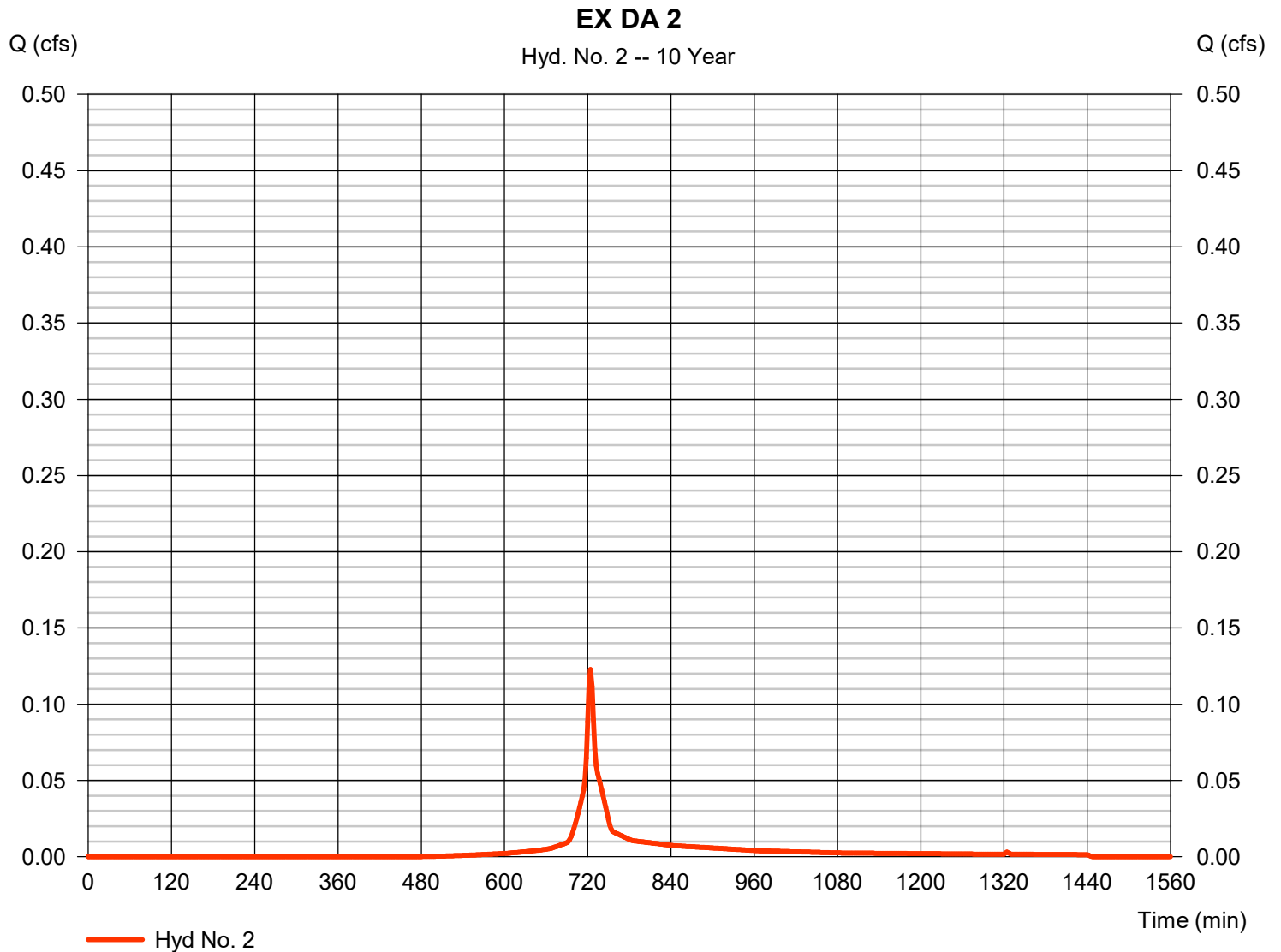
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 2

EX DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.123 cfs |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 367 cuft  |
| Drainage area   | = 0.036 ac   | Curve number       | = 78        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 5.33 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

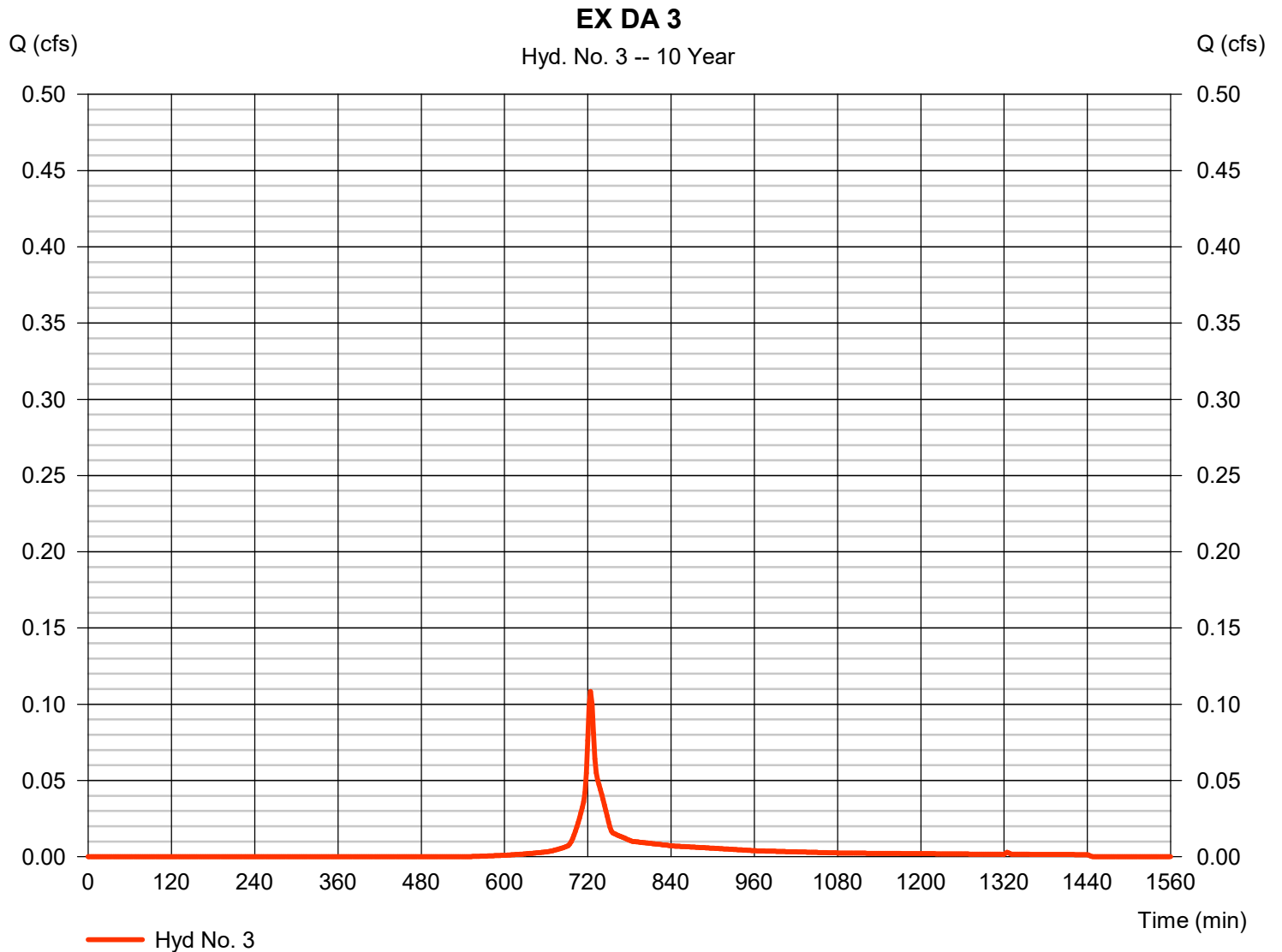
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 3

EX DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.108 cfs |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 326 cuft  |
| Drainage area   | = 0.039 ac   | Curve number       | = 72        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 5.33 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

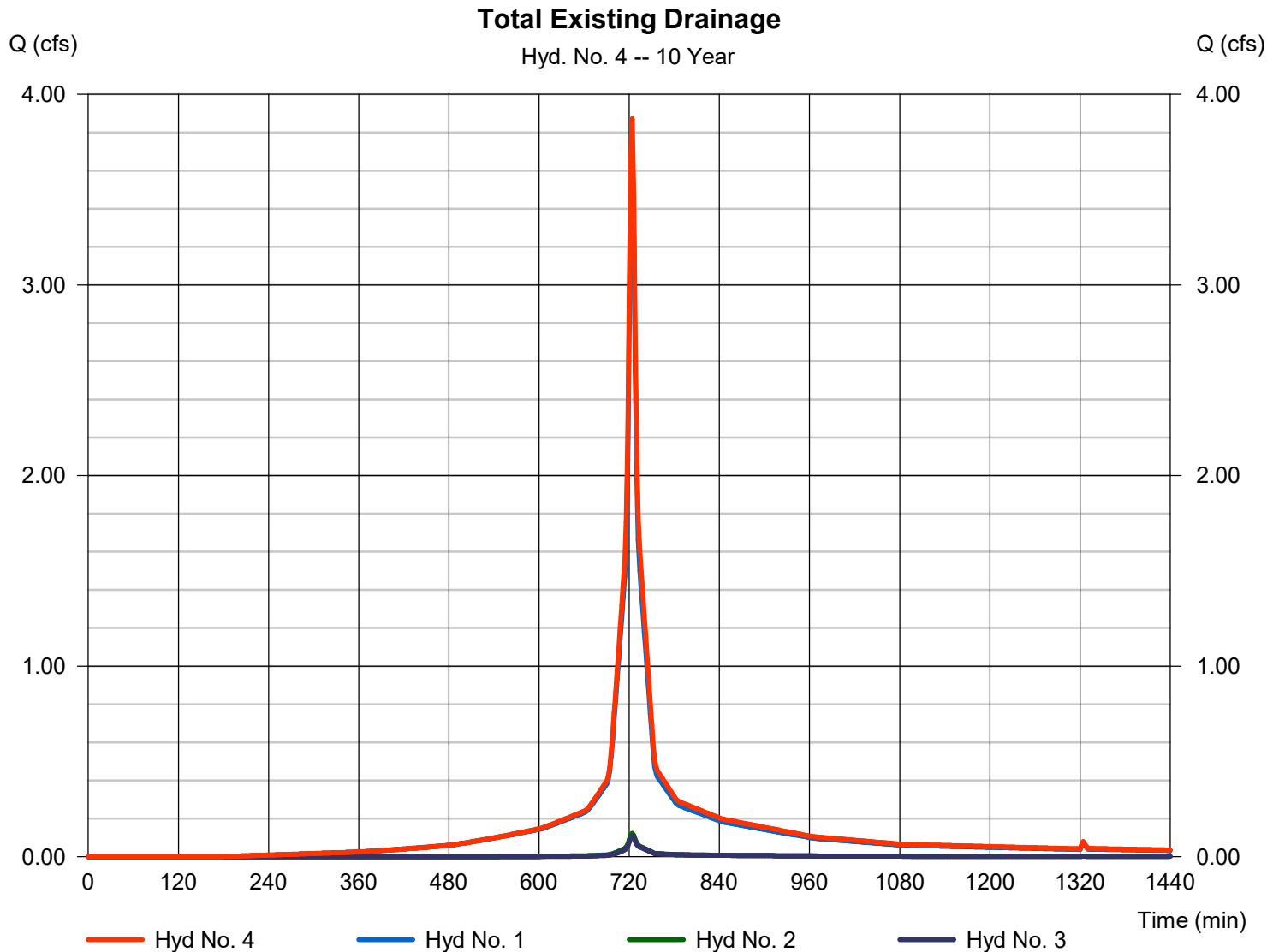
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 4

### Total Existing Drainage

|                 |           |                      |               |
|-----------------|-----------|----------------------|---------------|
| Hydrograph type | = Combine | Peak discharge       | = 3.871 cfs   |
| Storm frequency | = 10 yrs  | Time to peak         | = 724 min     |
| Time interval   | = 2 min   | Hyd. volume          | = 12,389 cuft |
| Inflow hyds.    | = 1, 2, 3 | Contrib. drain. area | = 0.835 ac    |

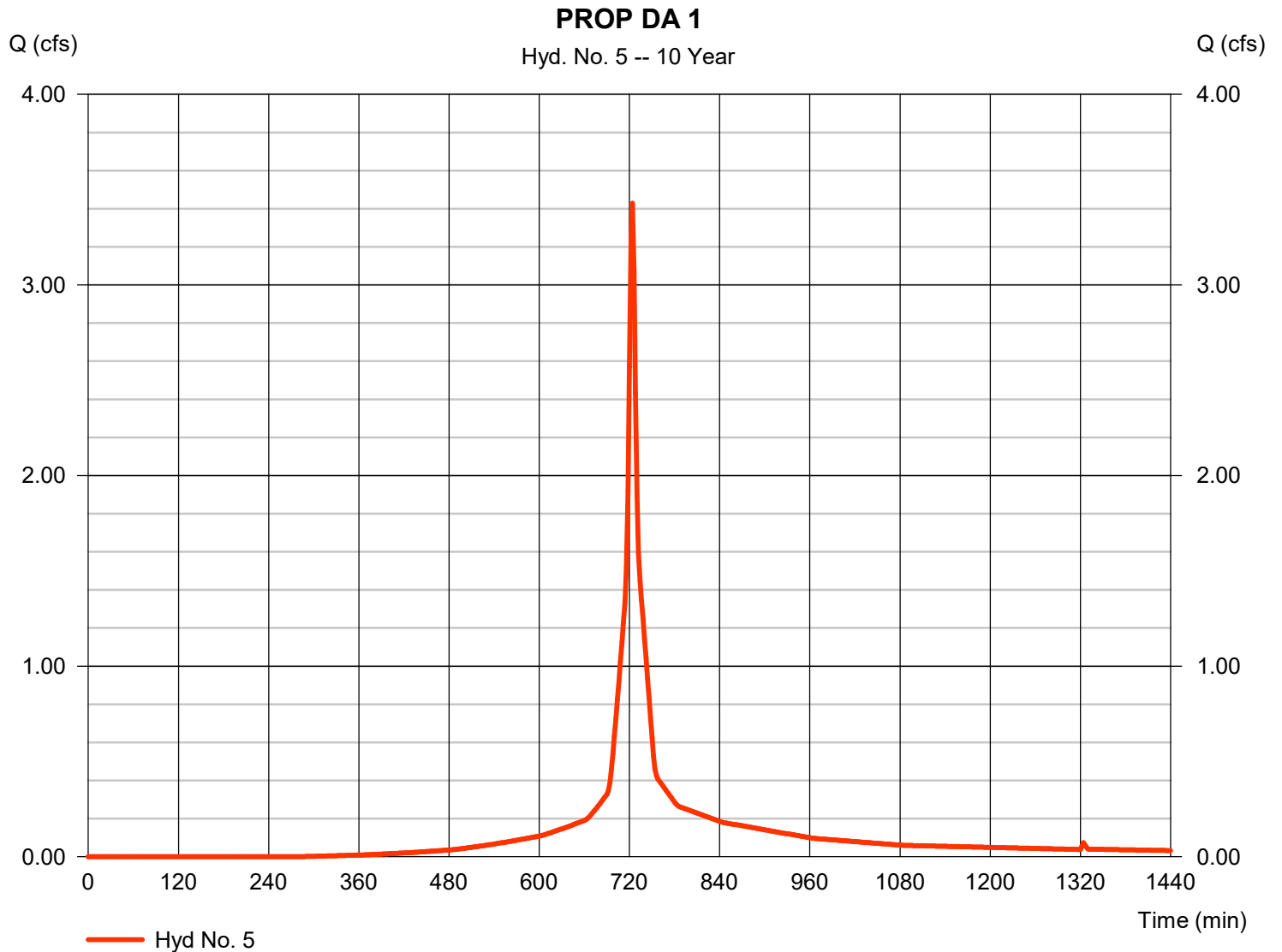


# Hydrograph Report

## Hyd. No. 5

PROP DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 3.428 cfs   |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 10,573 cuft |
| Drainage area   | = 0.780 ac   | Curve number       | = 88          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 5.33 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

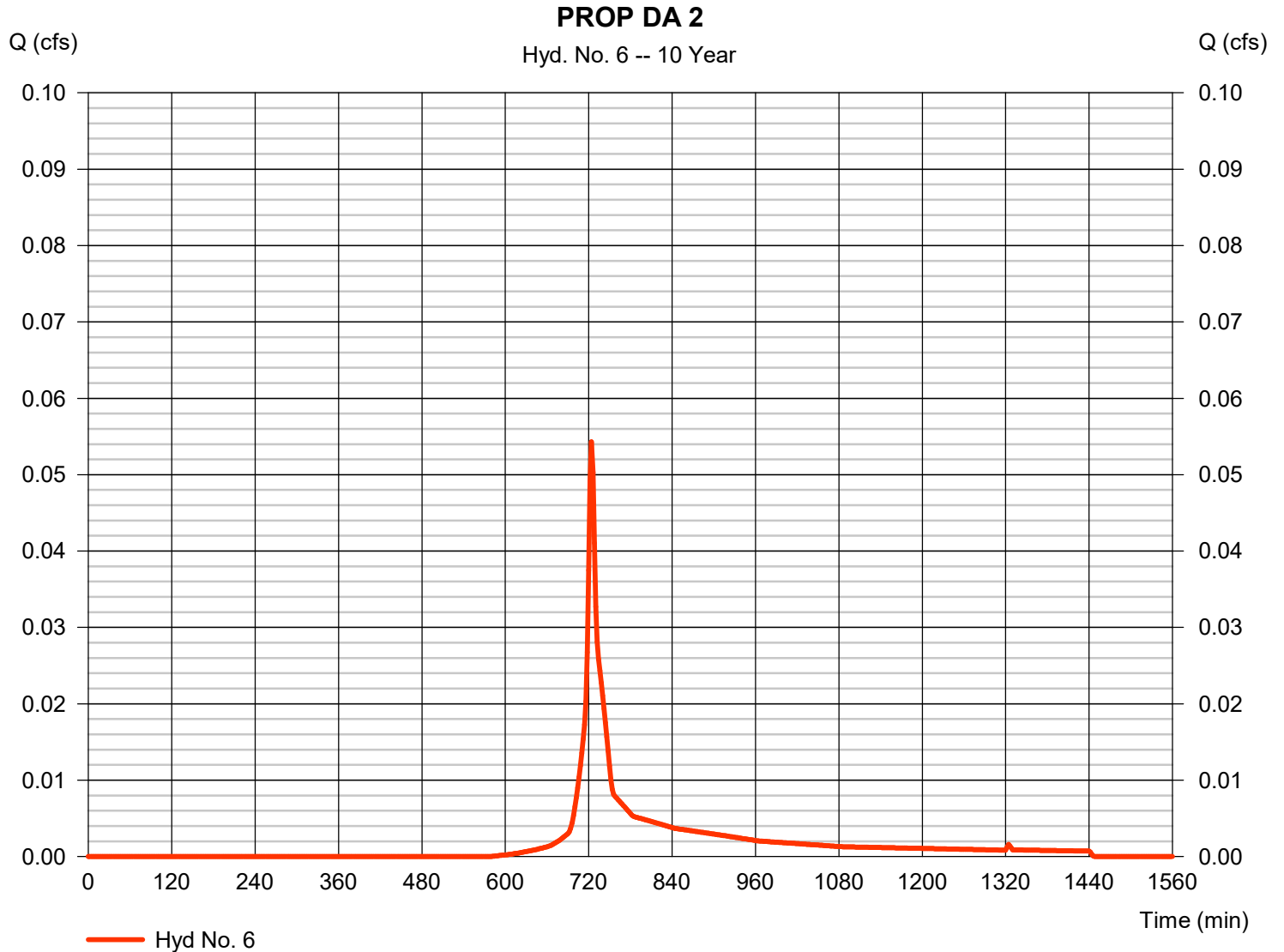
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 6

PROP DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.054 cfs |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 165 cuft  |
| Drainage area   | = 0.022 ac   | Curve number       | = 69        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 5.33 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

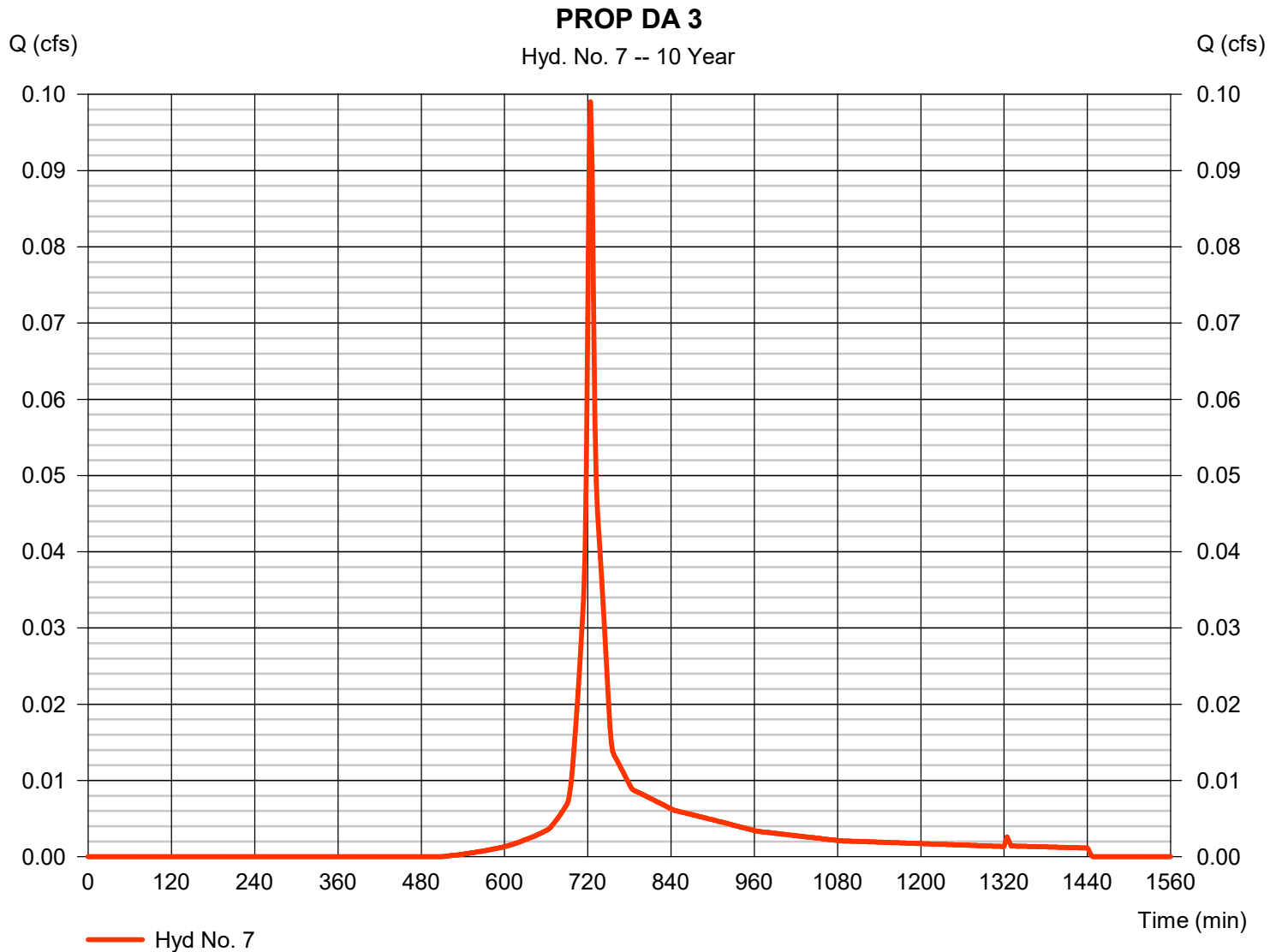
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 7

PROP DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.099 cfs |
| Storm frequency | = 10 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 296 cuft  |
| Drainage area   | = 0.032 ac   | Curve number       | = 75        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 5.33 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

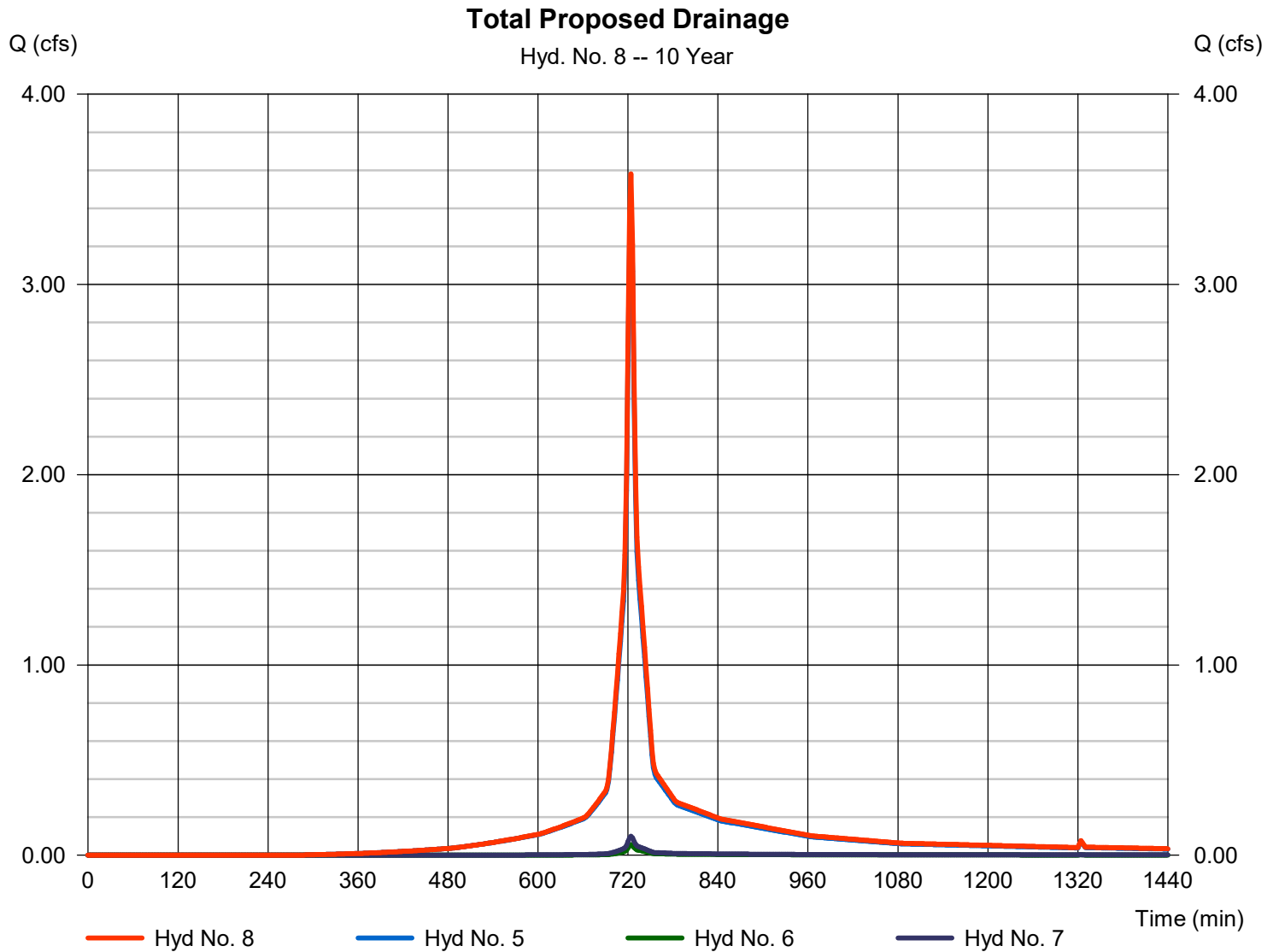
Tuesday, 06 / 30 / 2020

## Hyd. No. 8

Total Proposed Drainage

Hydrograph type = Combine  
Storm frequency = 10 yrs  
Time interval = 2 min  
Inflow hyds. = 5, 6, 7

Peak discharge = 3.581 cfs  
Time to peak = 724 min  
Hyd. volume = 11,034 cuft  
Contrib. drain. area = 0.834 ac





# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

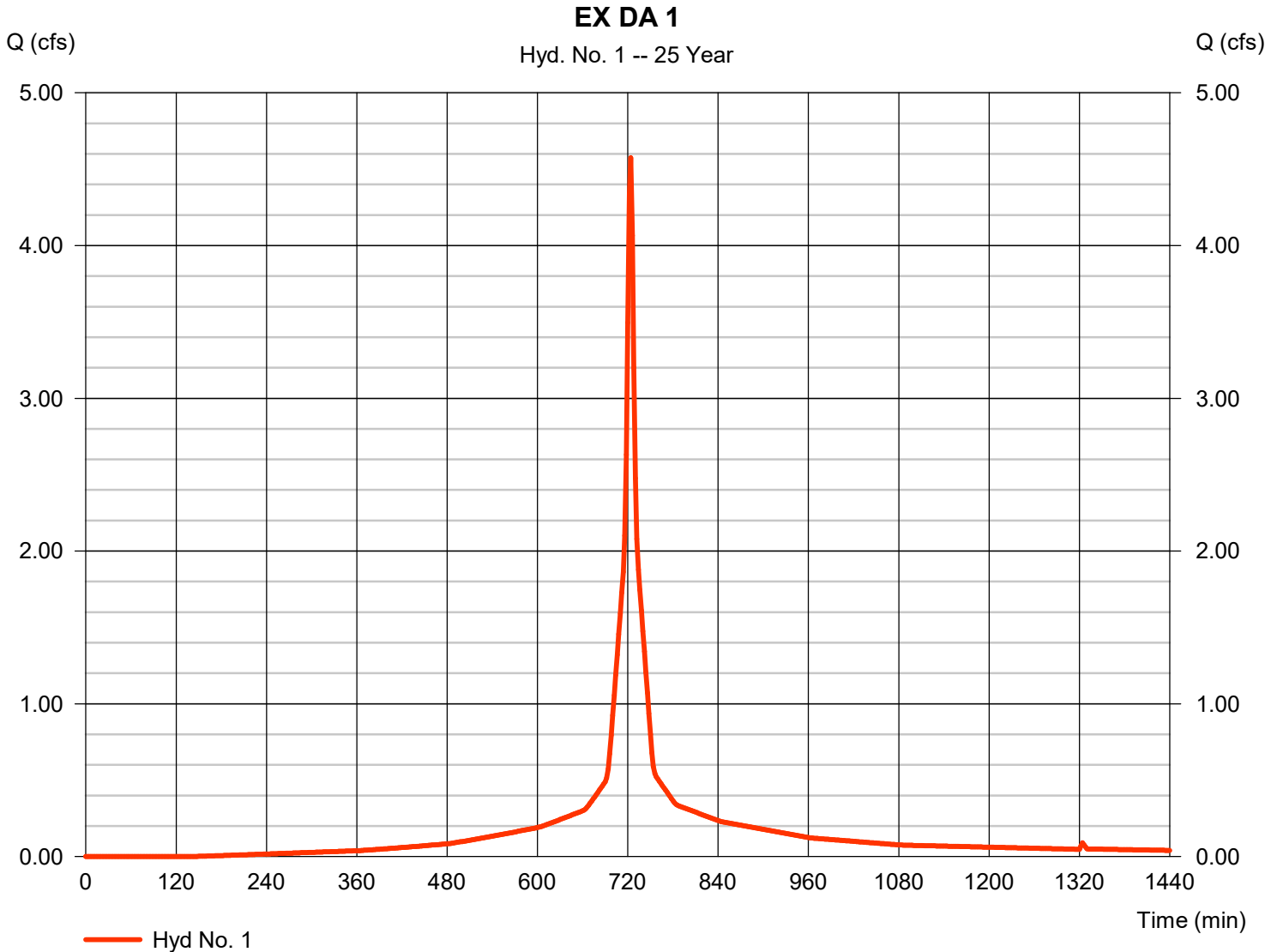
| Hyd. No.                         | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft)     | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description  |
|----------------------------------|--------------------------|-----------------|---------------------|--------------------|------------------------|---------------|------------------------|-------------------------|-------------------------|
| 1                                | SCS Runoff               | 4.575           | 2                   | 724                | 14,912                 | -----         | -----                  | -----                   | EX DA 1                 |
| 2                                | SCS Runoff               | 0.167           | 2                   | 724                | 503                    | -----         | -----                  | -----                   | EX DA 2                 |
| 3                                | SCS Runoff               | 0.155           | 2                   | 724                | 462                    | -----         | -----                  | -----                   | EX DA 3                 |
| 4                                | Combine                  | 4.897           | 2                   | 724                | 15,877                 | 1, 2, 3       | -----                  | -----                   | Total Existing Drainage |
| 5                                | SCS Runoff               | 4.408           | 2                   | 724                | 13,792                 | -----         | -----                  | -----                   | PROP DA 1               |
| 6                                | SCS Runoff               | 0.079           | 2                   | 724                | 238                    | -----         | -----                  | -----                   | PROP DA 2               |
| 7                                | SCS Runoff               | 0.138           | 2                   | 724                | 413                    | -----         | -----                  | -----                   | PROP DA 3               |
| 8                                | Combine                  | 4.626           | 2                   | 724                | 14,443                 | 5, 6, 7       | -----                  | -----                   | Total Proposed Drainage |
| 2020.06.28 Hydraulics Design.gpw |                          |                 |                     |                    | Return Period: 25 Year |               |                        | Tuesday, 06 / 30 / 2020 |                         |

# Hydrograph Report

## Hyd. No. 1

EX DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 4.575 cfs   |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 14,912 cuft |
| Drainage area   | = 0.760 ac   | Curve number       | = 93          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 6.59 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

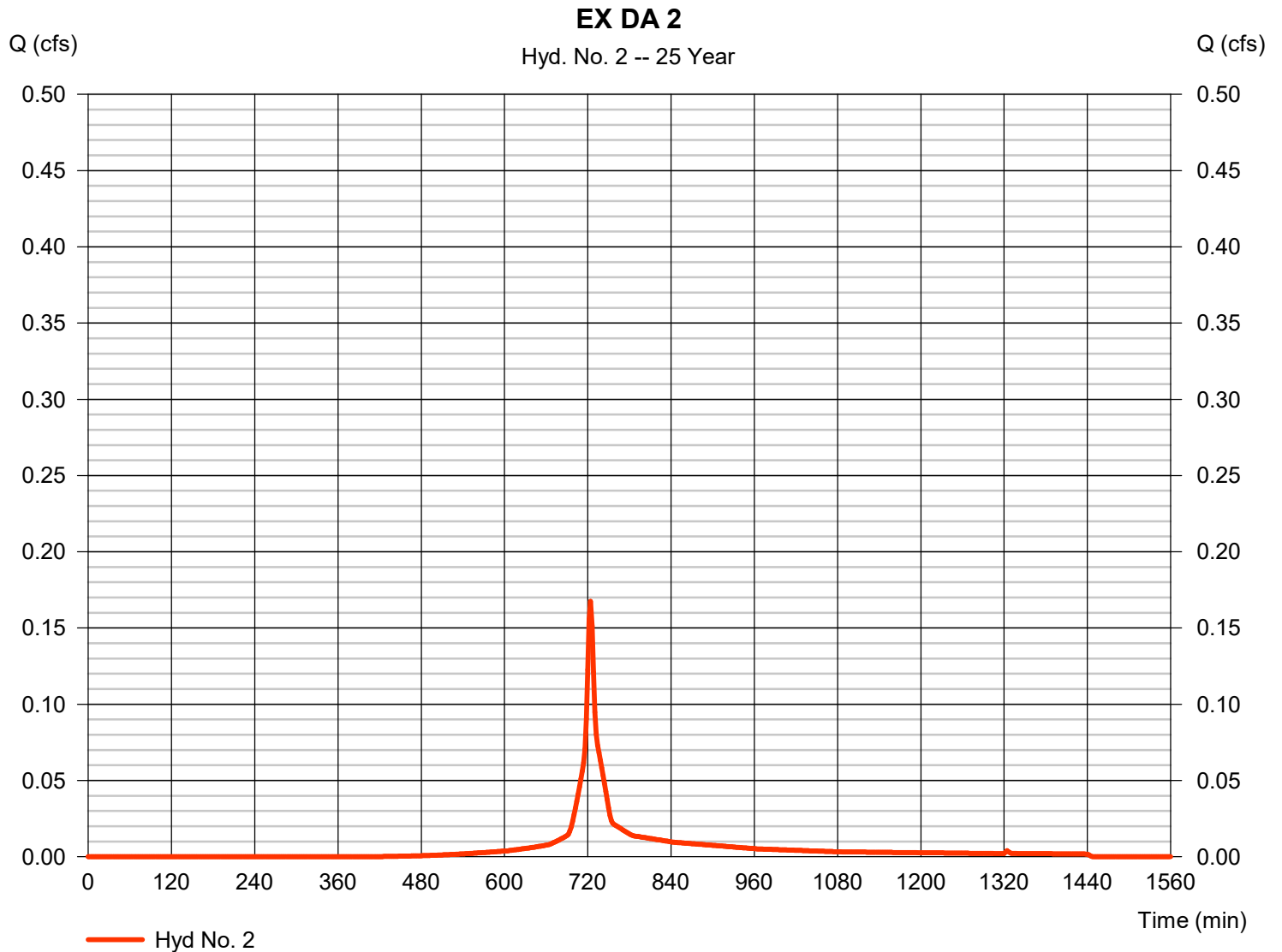
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 2

EX DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.167 cfs |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 503 cuft  |
| Drainage area   | = 0.036 ac   | Curve number       | = 78        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 6.59 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

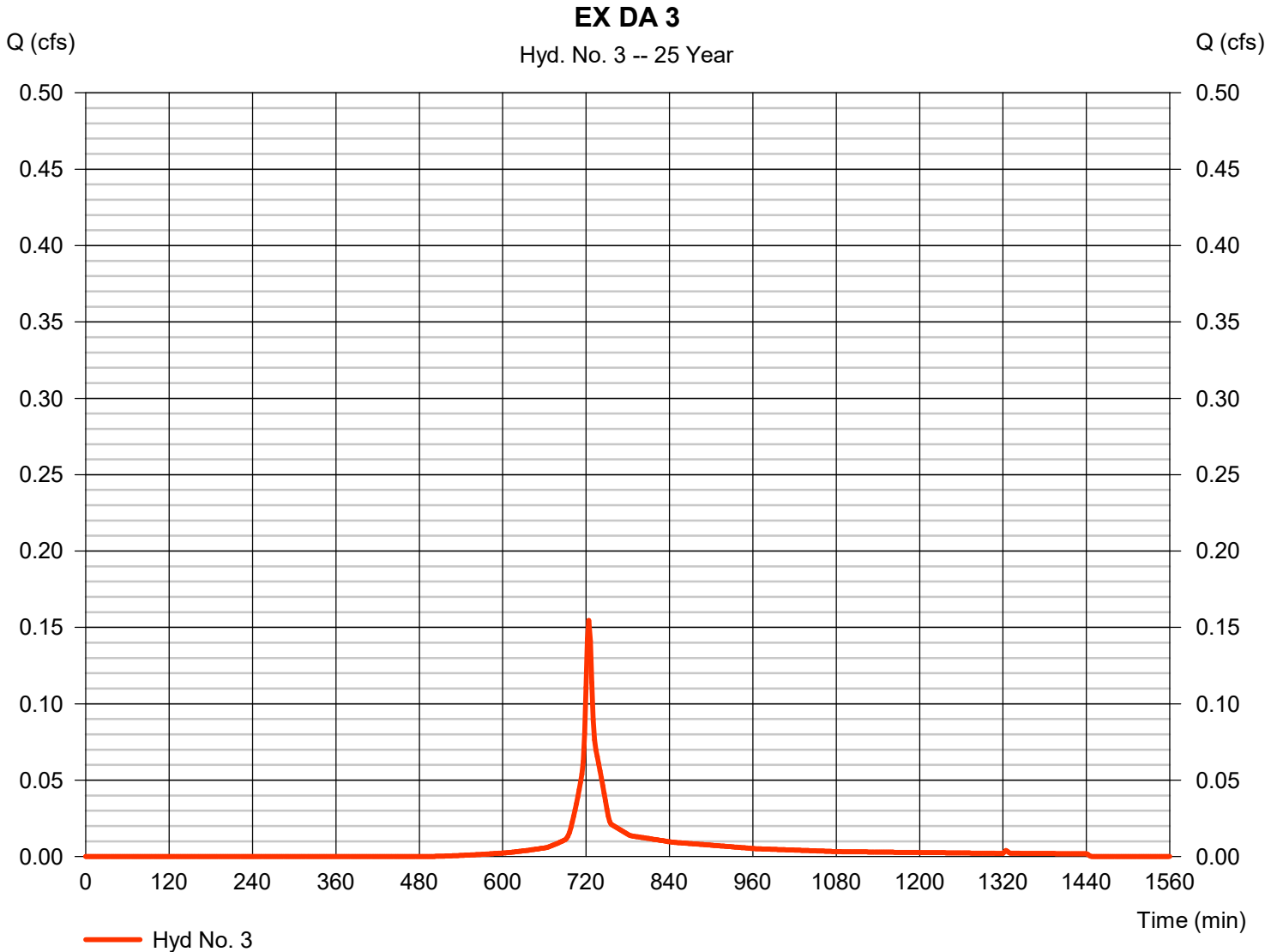
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 3

EX DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.155 cfs |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 462 cuft  |
| Drainage area   | = 0.039 ac   | Curve number       | = 72        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 6.59 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

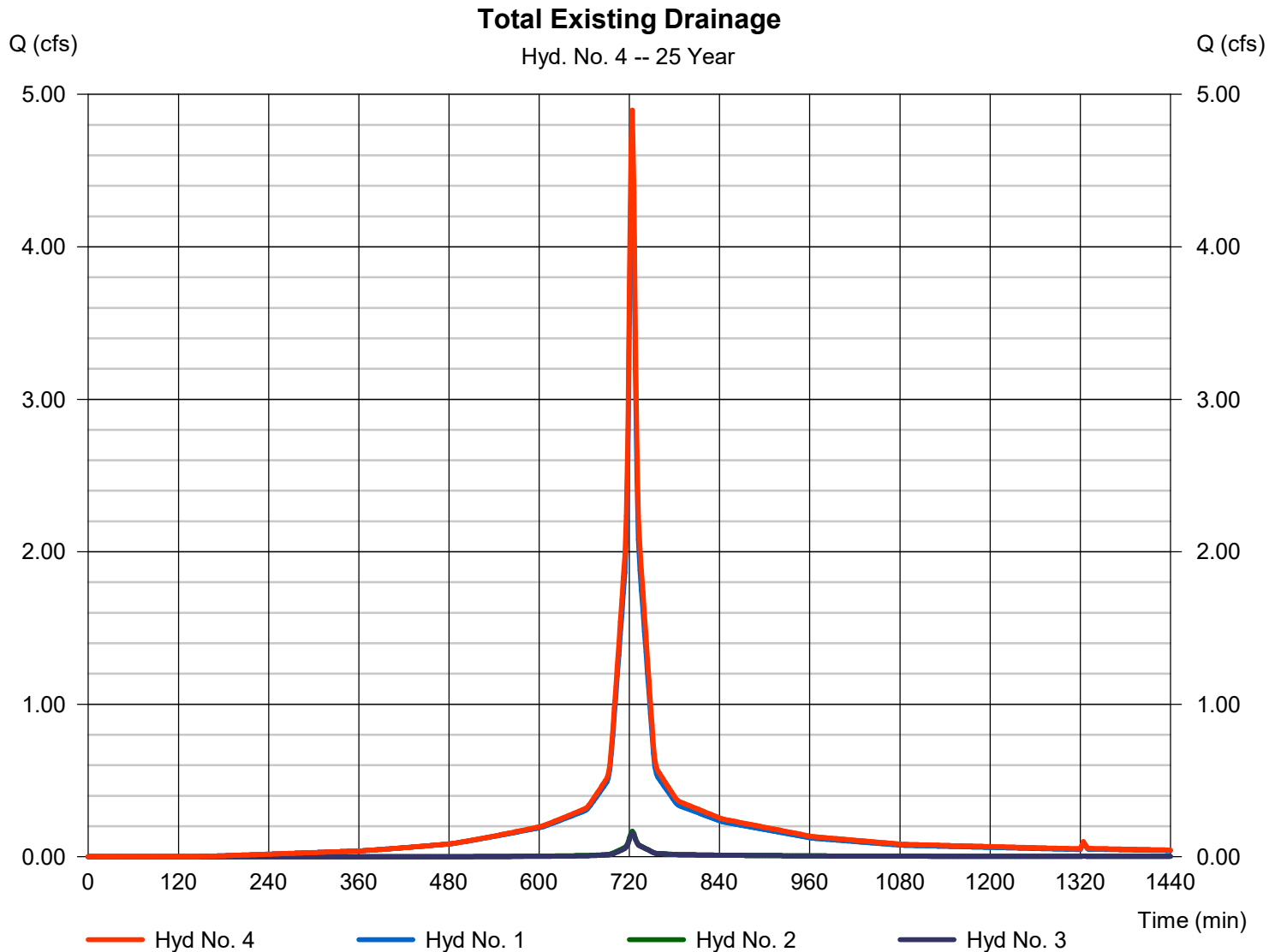
Tuesday, 06 / 30 / 2020

## Hyd. No. 4

### Total Existing Drainage

Hydrograph type = Combine  
Storm frequency = 25 yrs  
Time interval = 2 min  
Inflow hyds. = 1, 2, 3

Peak discharge = 4.897 cfs  
Time to peak = 724 min  
Hyd. volume = 15,877 cuft  
Contrib. drain. area = 0.835 ac



# Hydrograph Report

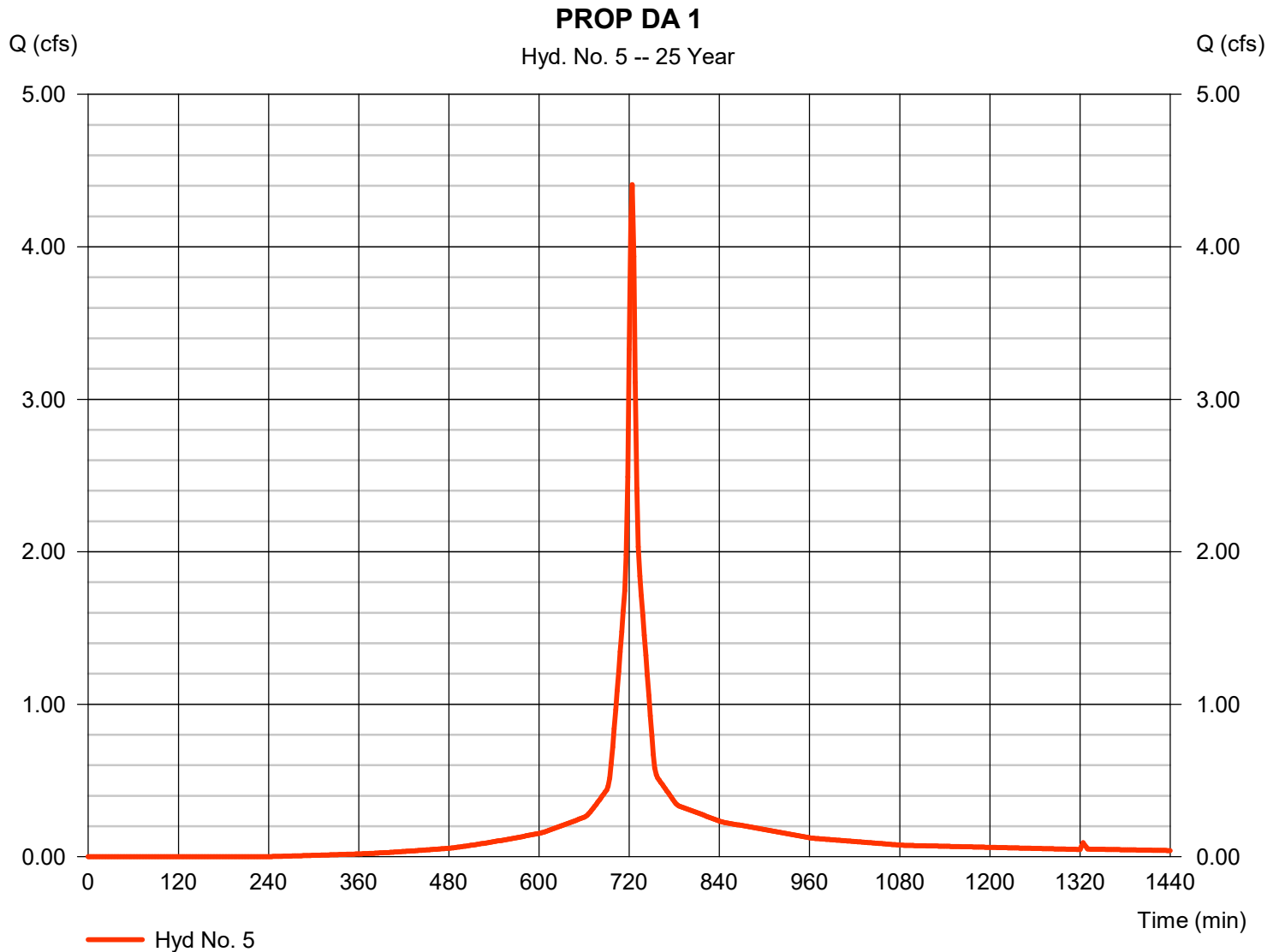
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 5

PROP DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 4.408 cfs   |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 13,792 cuft |
| Drainage area   | = 0.780 ac   | Curve number       | = 88          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 6.59 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

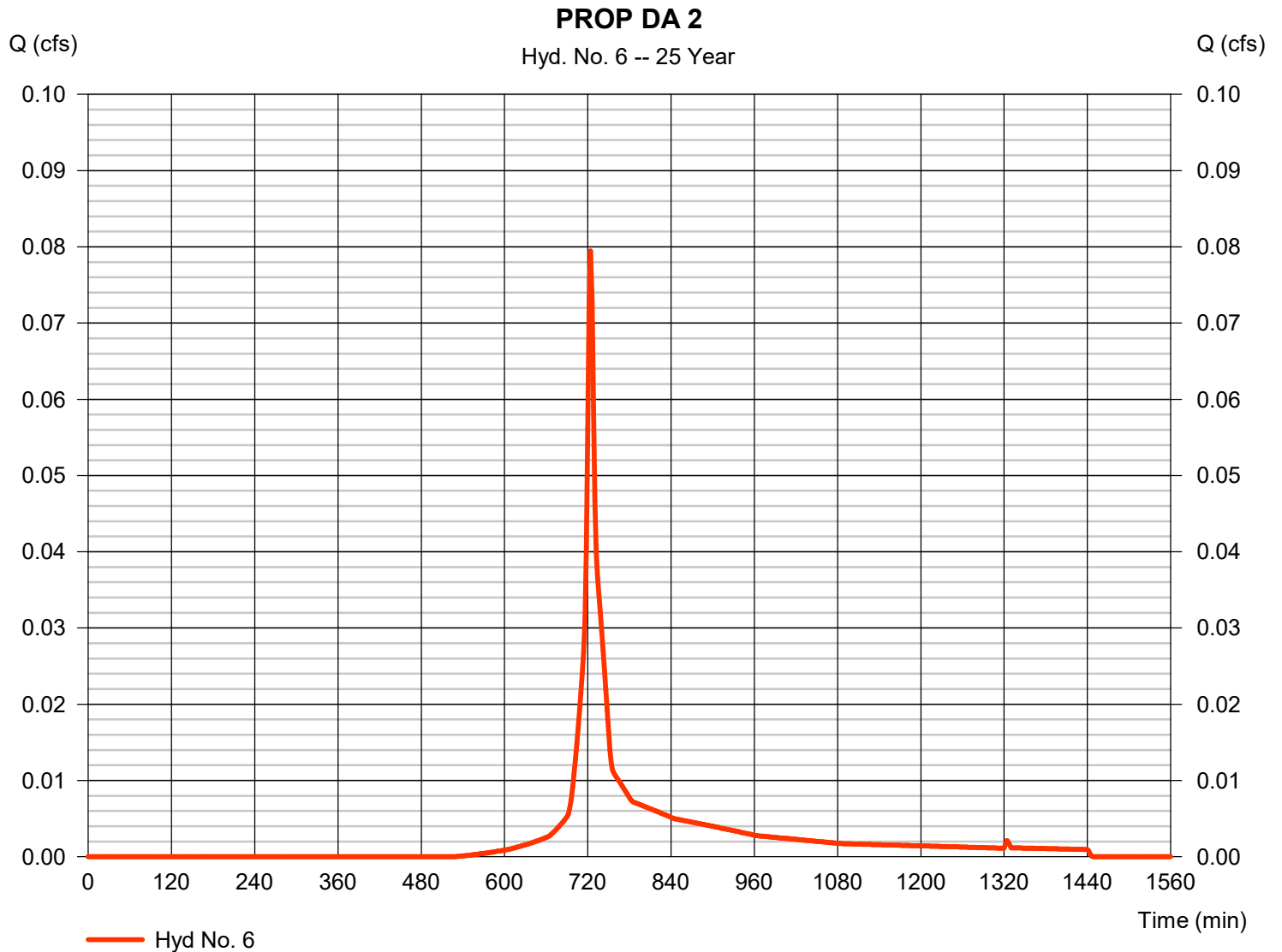
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 6

PROP DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.079 cfs |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 238 cuft  |
| Drainage area   | = 0.022 ac   | Curve number       | = 69        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 6.59 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

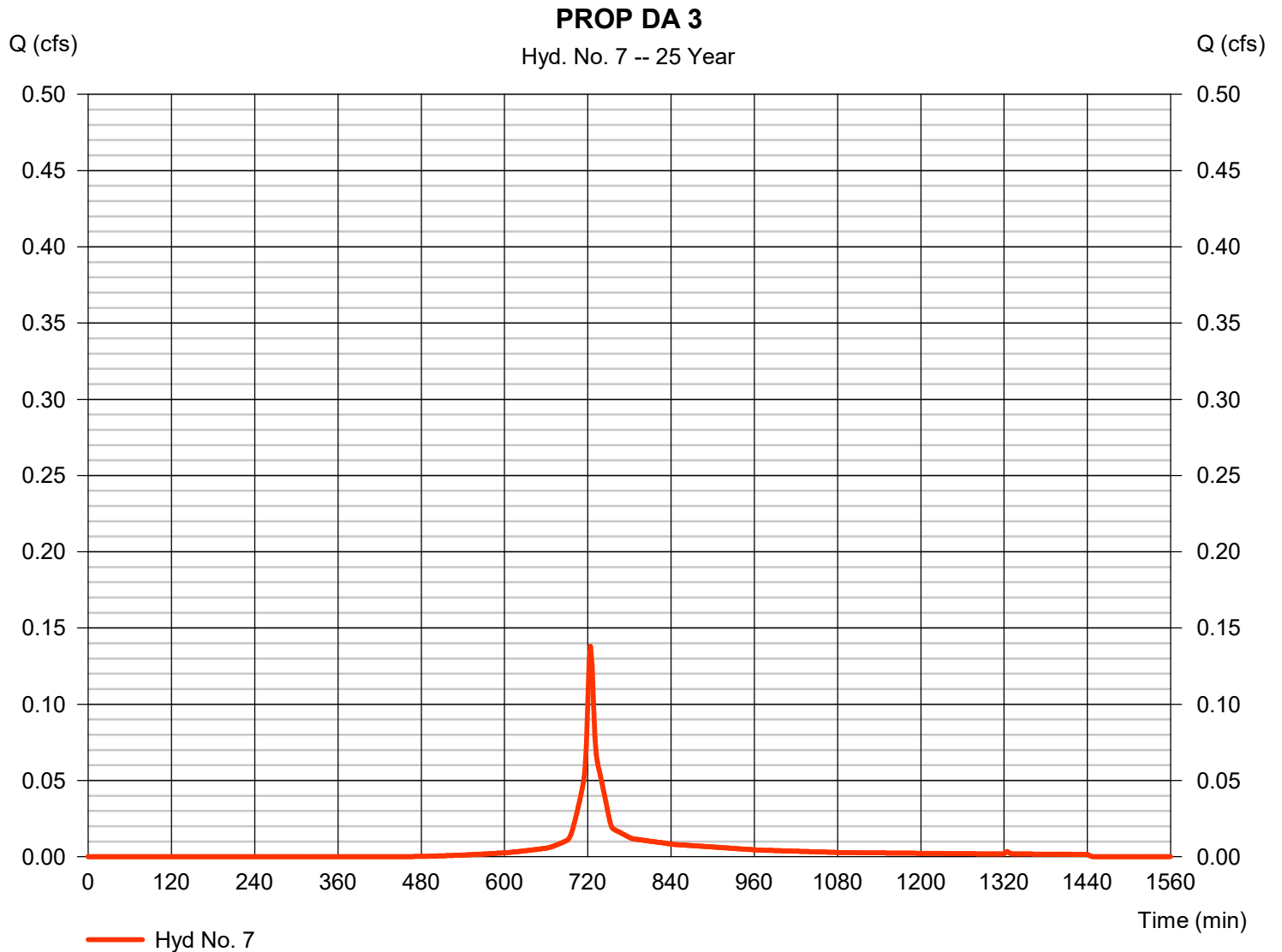
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 7

PROP DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.138 cfs |
| Storm frequency | = 25 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 413 cuft  |
| Drainage area   | = 0.032 ac   | Curve number       | = 75        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 6.59 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |





# Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

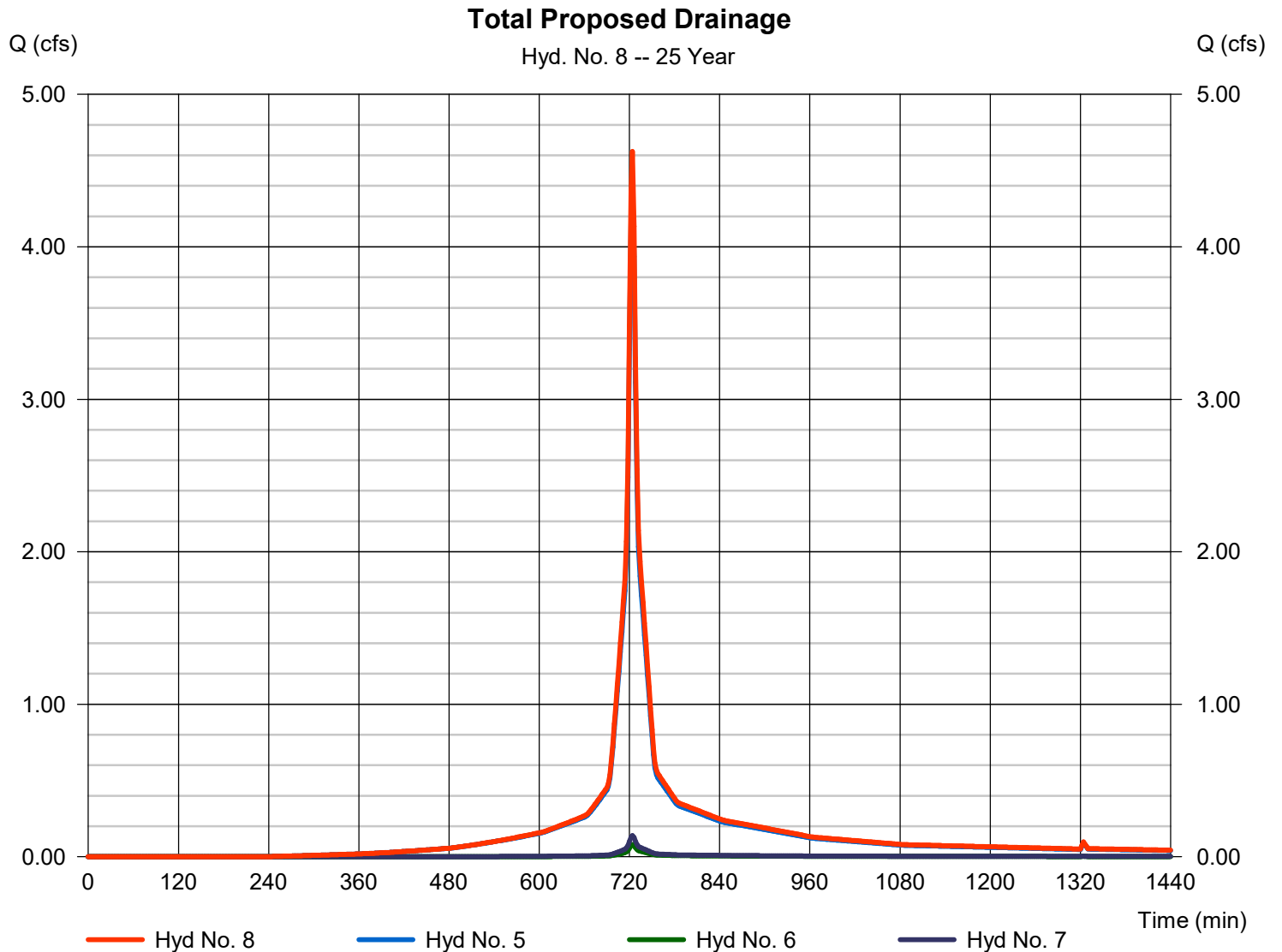
Tuesday, 06 / 30 / 2020

## Hyd. No. 8

Total Proposed Drainage

Hydrograph type = Combine  
Storm frequency = 25 yrs  
Time interval = 2 min  
Inflow hyds. = 5, 6, 7

Peak discharge = 4.626 cfs  
Time to peak = 724 min  
Hyd. volume = 14,443 cuft  
Contrib. drain. area = 0.834 ac



# Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

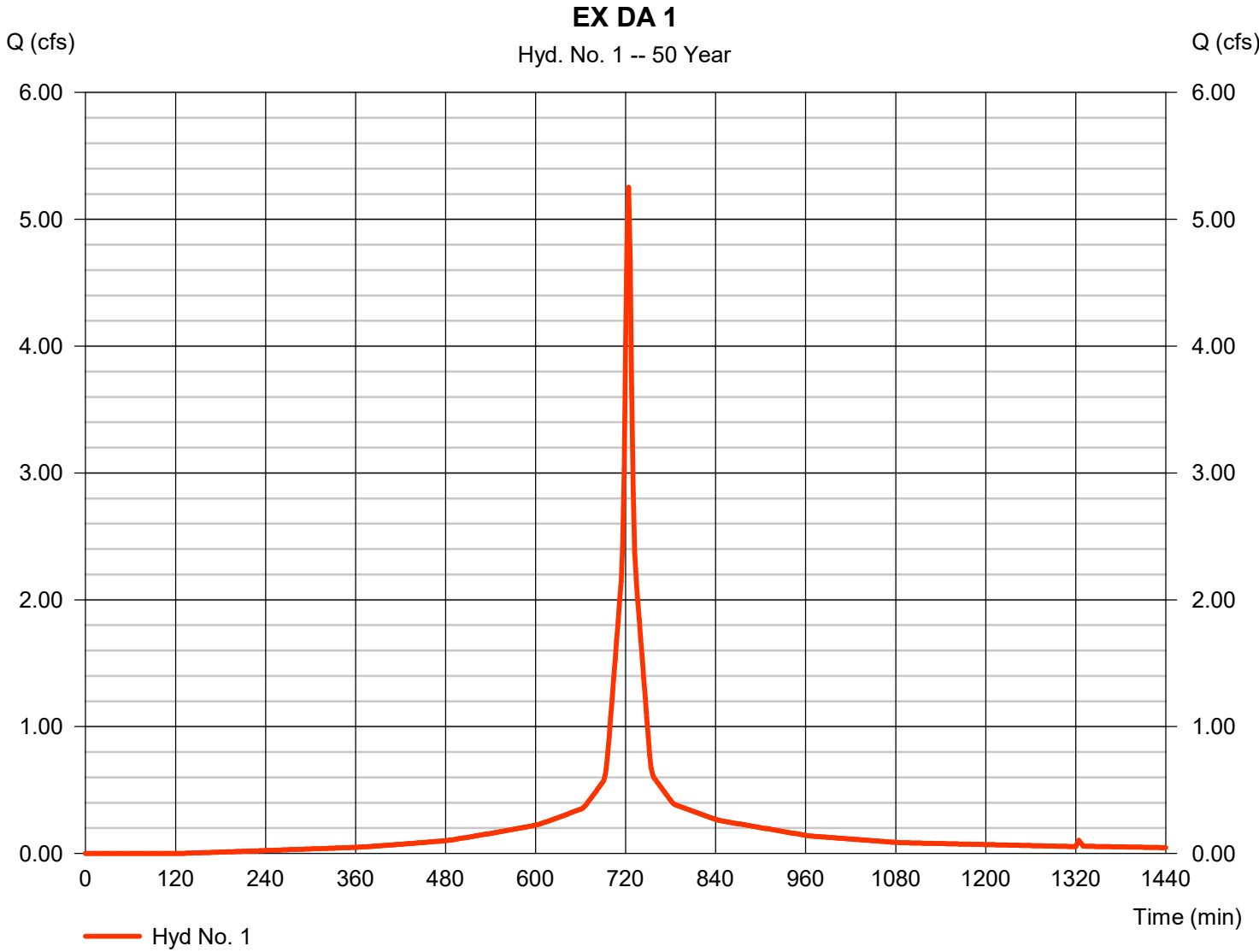
| Hyd. No.                         | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to Peak (min) | Hyd. volume (cuft)     | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph Description  |
|----------------------------------|--------------------------|-----------------|---------------------|--------------------|------------------------|---------------|------------------------|-------------------------|-------------------------|
| 1                                | SCS Runoff               | 5.253           | 2                   | 724                | 17,268                 | -----         | -----                  | -----                   | EX DA 1                 |
| 2                                | SCS Runoff               | 0.200           | 2                   | 724                | 605                    | -----         | -----                  | -----                   | EX DA 2                 |
| 3                                | SCS Runoff               | 0.189           | 2                   | 724                | 566                    | -----         | -----                  | -----                   | EX DA 3                 |
| 4                                | Combine                  | 5.643           | 2                   | 724                | 18,440                 | 1, 2, 3       | -----                  | -----                   | Total Existing Drainage |
| 5                                | SCS Runoff               | 5.120           | 2                   | 724                | 16,165                 | -----         | -----                  | -----                   | PROP DA 1               |
| 6                                | SCS Runoff               | 0.099           | 2                   | 724                | 295                    | -----         | -----                  | -----                   | PROP DA 2               |
| 7                                | SCS Runoff               | 0.167           | 2                   | 724                | 501                    | -----         | -----                  | -----                   | PROP DA 3               |
| 8                                | Combine                  | 5.385           | 2                   | 724                | 16,961                 | 5, 6, 7       | -----                  | -----                   | Total Proposed Drainage |
| 2020.06.28 Hydraulics Design.gpw |                          |                 |                     |                    | Return Period: 50 Year |               |                        | Tuesday, 06 / 30 / 2020 |                         |

# Hydrograph Report

## Hyd. No. 1

EX DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 5.253 cfs   |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 17,268 cuft |
| Drainage area   | = 0.760 ac   | Curve number       | = 93          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 7.51 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

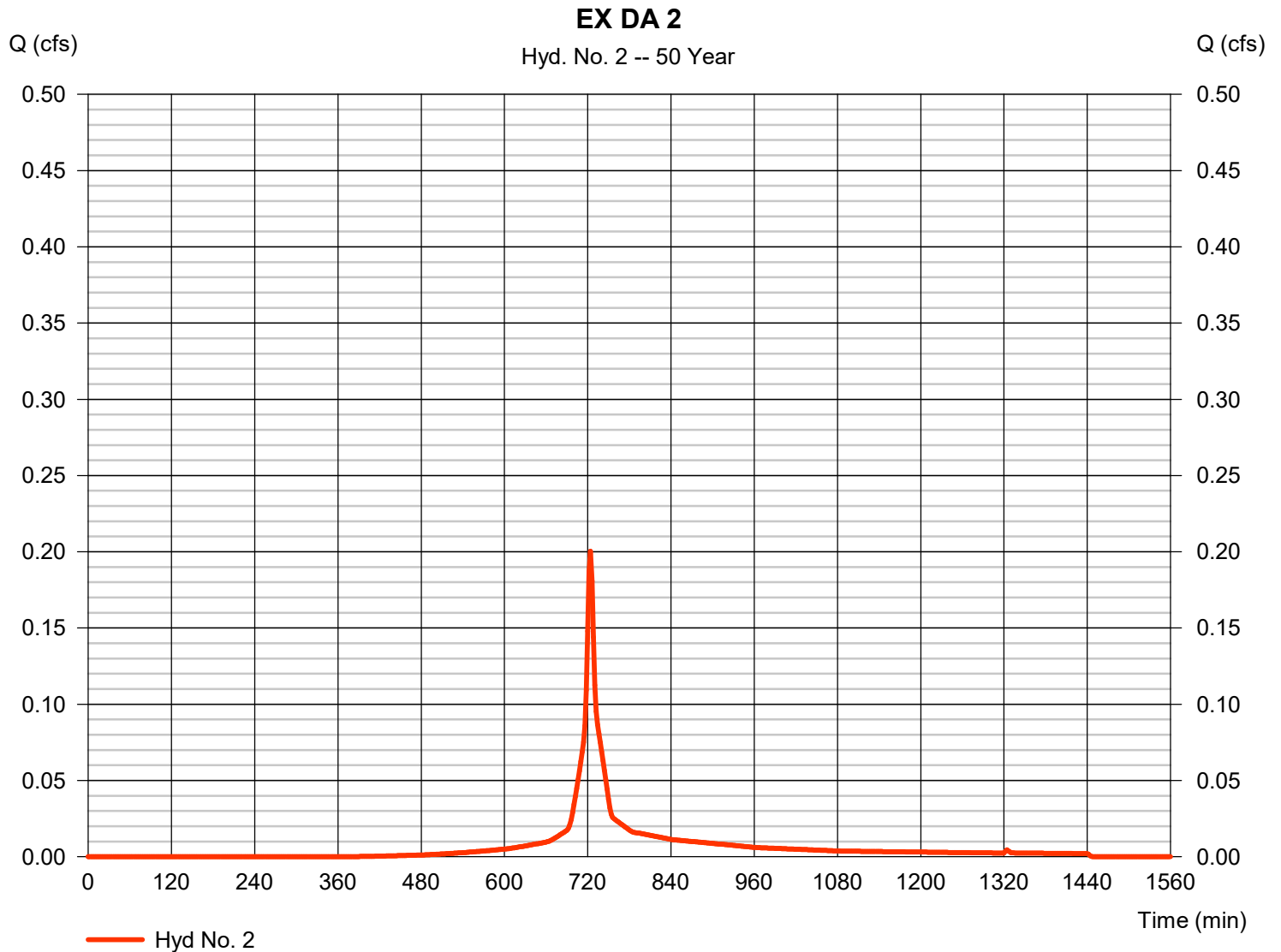
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 2

EX DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.200 cfs |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 605 cuft  |
| Drainage area   | = 0.036 ac   | Curve number       | = 78        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 7.51 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

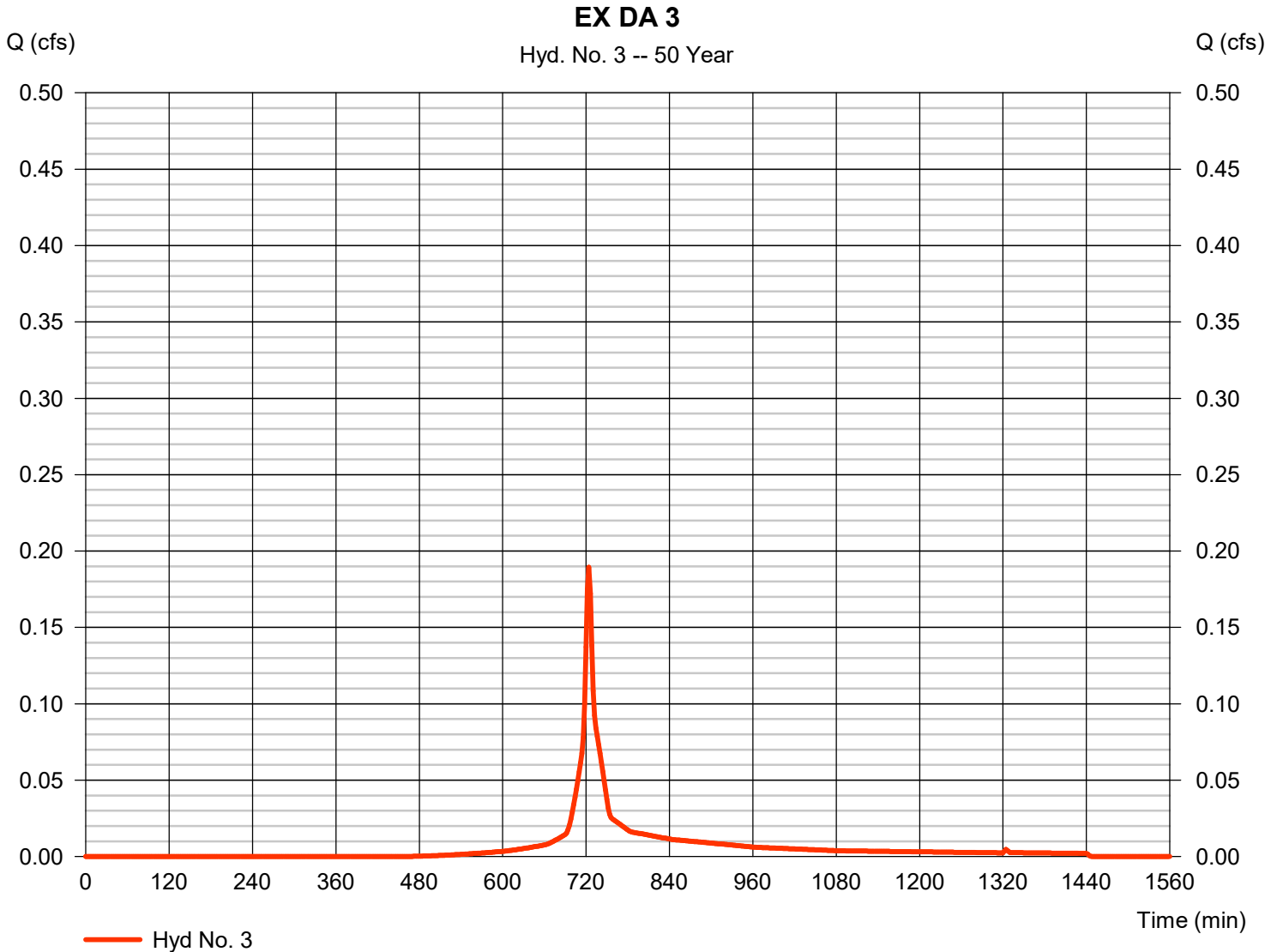
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 3

EX DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.189 cfs |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 566 cuft  |
| Drainage area   | = 0.039 ac   | Curve number       | = 72        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 7.51 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

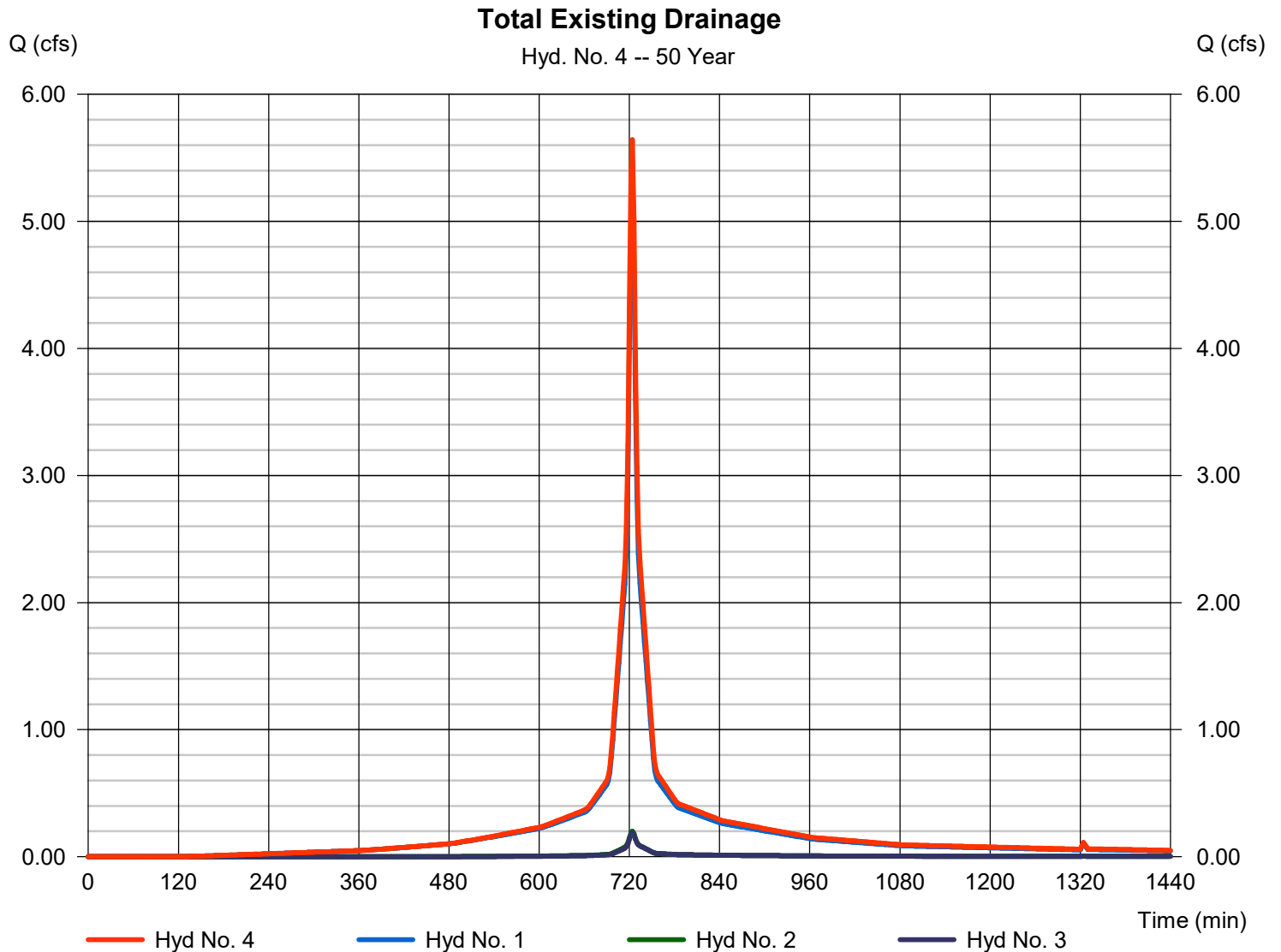
Tuesday, 06 / 30 / 2020

## Hyd. No. 4

### Total Existing Drainage

Hydrograph type = Combine  
Storm frequency = 50 yrs  
Time interval = 2 min  
Inflow hyds. = 1, 2, 3

Peak discharge = 5.643 cfs  
Time to peak = 724 min  
Hyd. volume = 18,440 cuft  
Contrib. drain. area = 0.835 ac



# Hydrograph Report

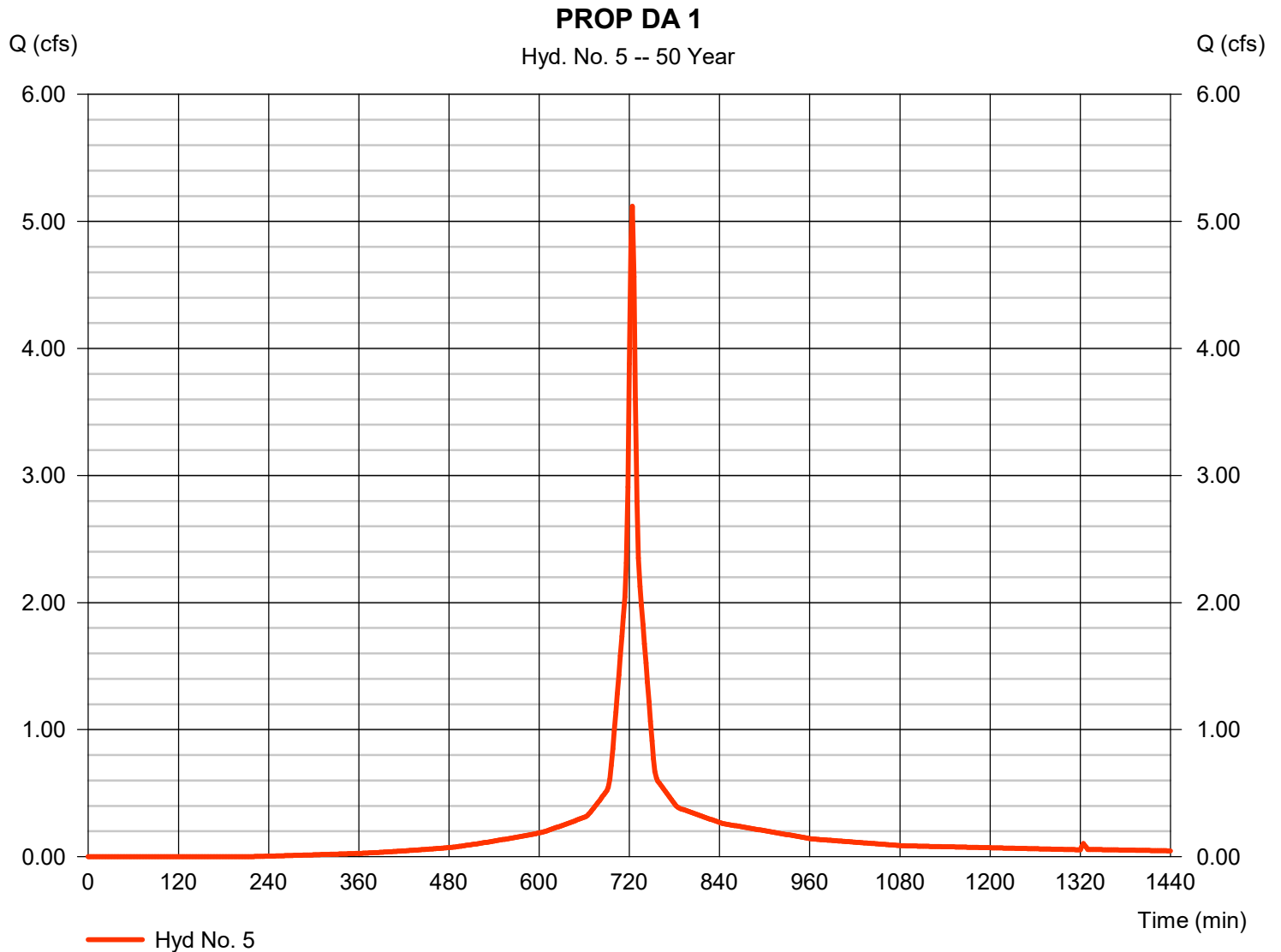
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 5

PROP DA 1

|                 |              |                    |               |
|-----------------|--------------|--------------------|---------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 5.120 cfs   |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min     |
| Time interval   | = 2 min      | Hyd. volume        | = 16,165 cuft |
| Drainage area   | = 0.780 ac   | Curve number       | = 88          |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft        |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min    |
| Total precip.   | = 7.51 in    | Distribution       | = Type III    |
| Storm duration  | = 24 hrs     | Shape factor       | = 484         |



# Hydrograph Report

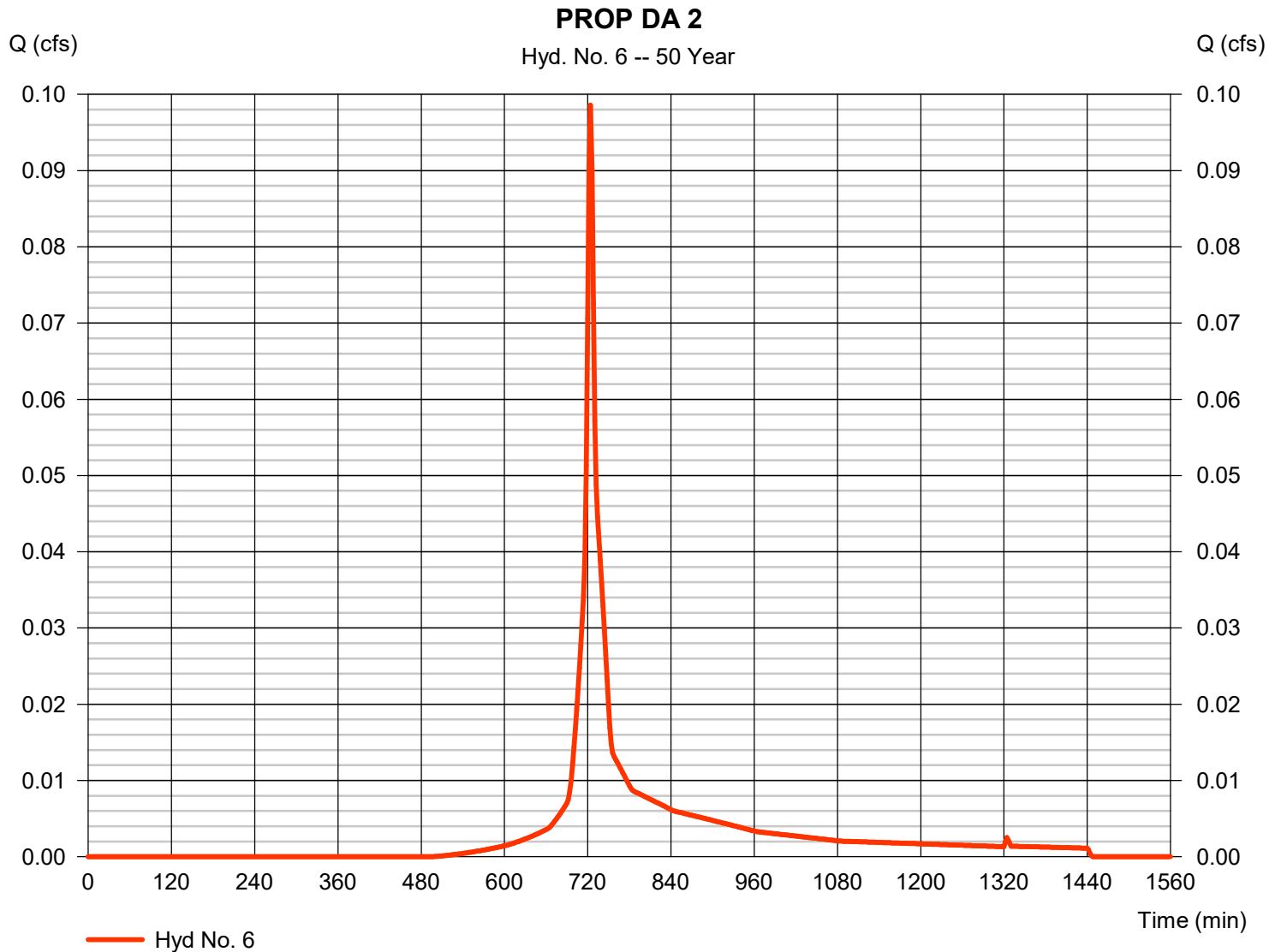
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 6

PROP DA 2

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.099 cfs |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 295 cuft  |
| Drainage area   | = 0.022 ac   | Curve number       | = 69        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 7.51 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |





# Hydrograph Report

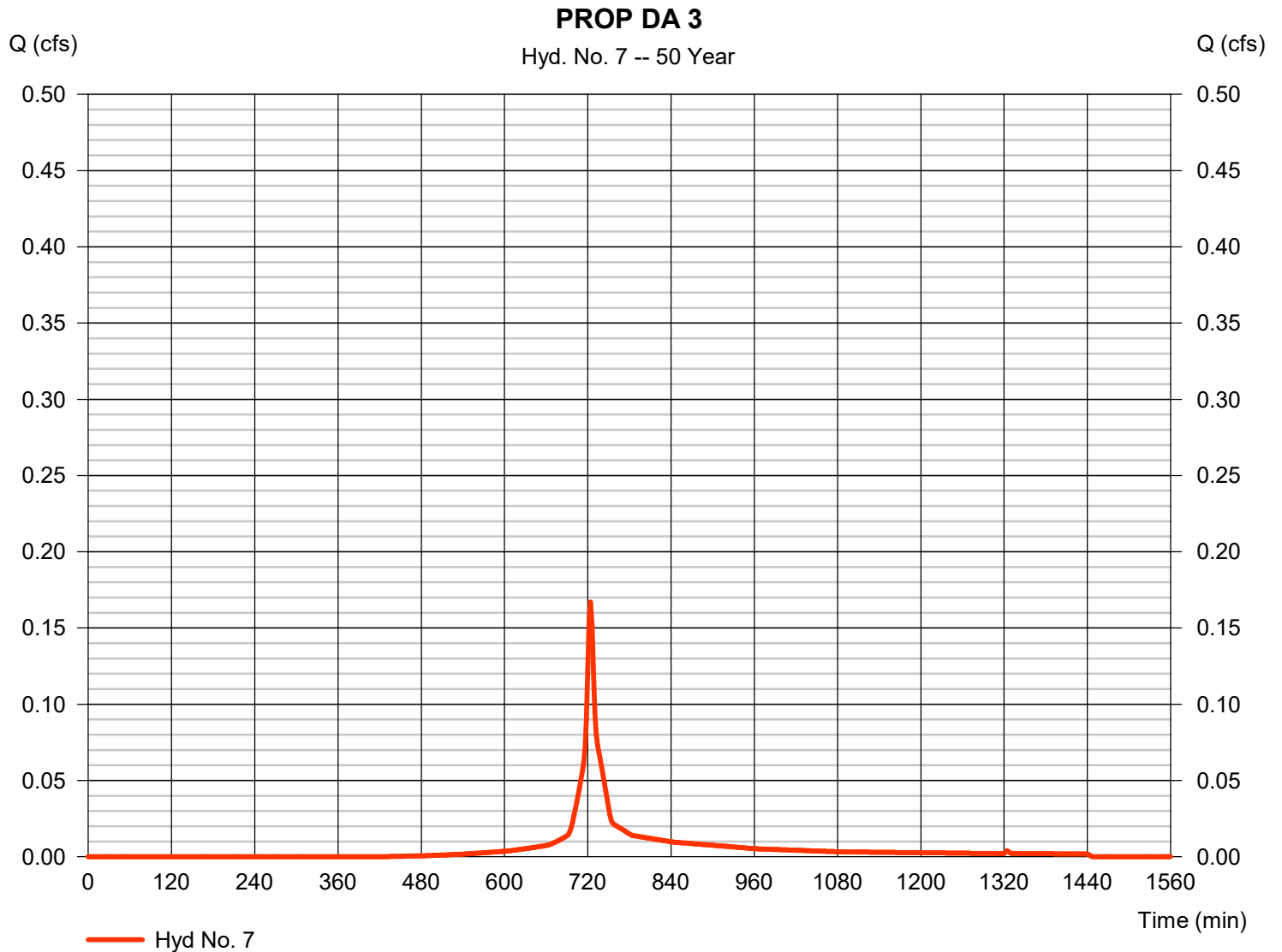
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 7

PROP DA 3

|                 |              |                    |             |
|-----------------|--------------|--------------------|-------------|
| Hydrograph type | = SCS Runoff | Peak discharge     | = 0.167 cfs |
| Storm frequency | = 50 yrs     | Time to peak       | = 724 min   |
| Time interval   | = 2 min      | Hyd. volume        | = 501 cuft  |
| Drainage area   | = 0.032 ac   | Curve number       | = 75        |
| Basin Slope     | = 0.0 %      | Hydraulic length   | = 0 ft      |
| Tc method       | = User       | Time of conc. (Tc) | = 6.00 min  |
| Total precip.   | = 7.51 in    | Distribution       | = Type III  |
| Storm duration  | = 24 hrs     | Shape factor       | = 484       |



# Hydrograph Report

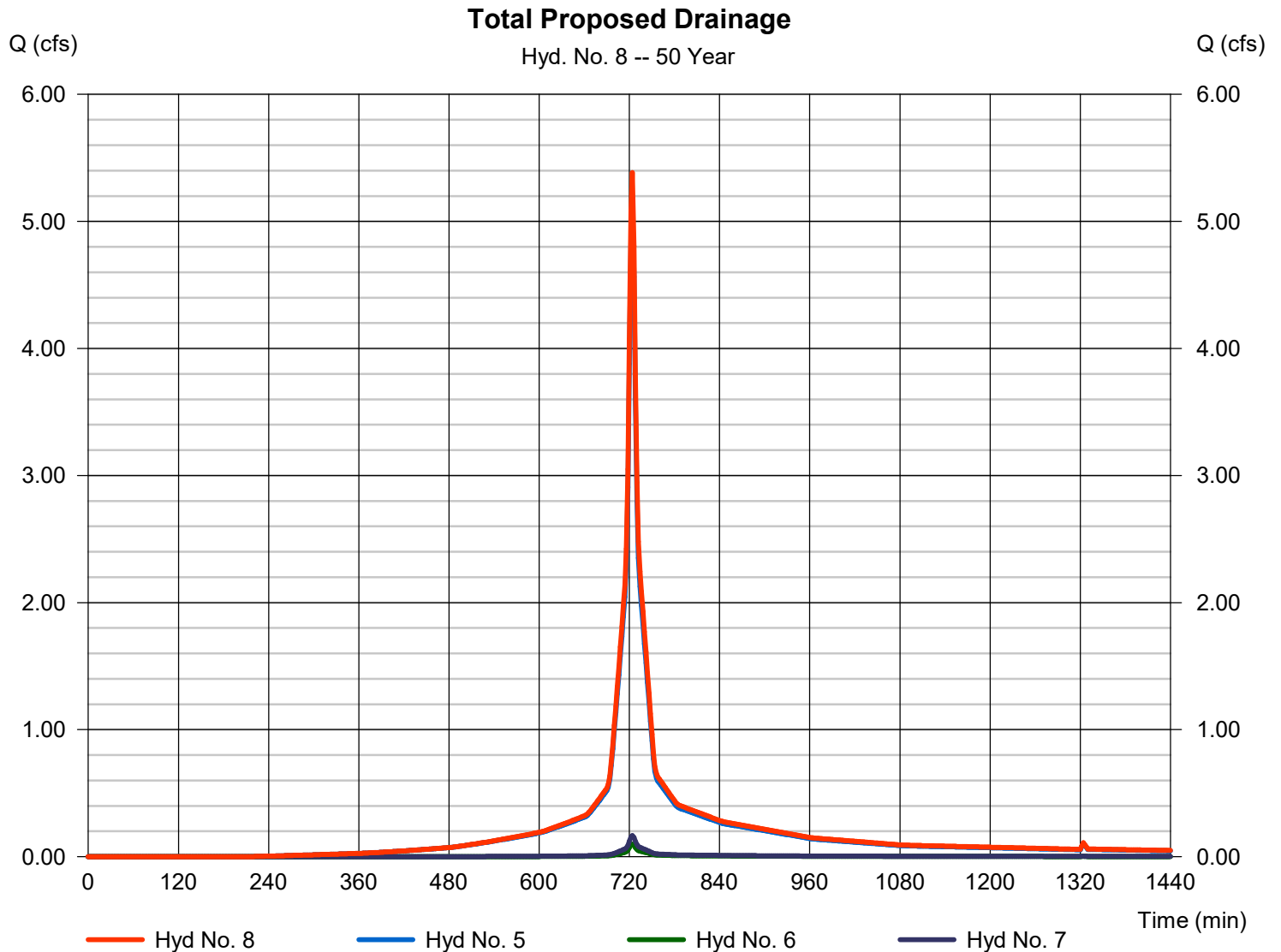
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Tuesday, 06 / 30 / 2020

## Hyd. No. 8

Total Proposed Drainage

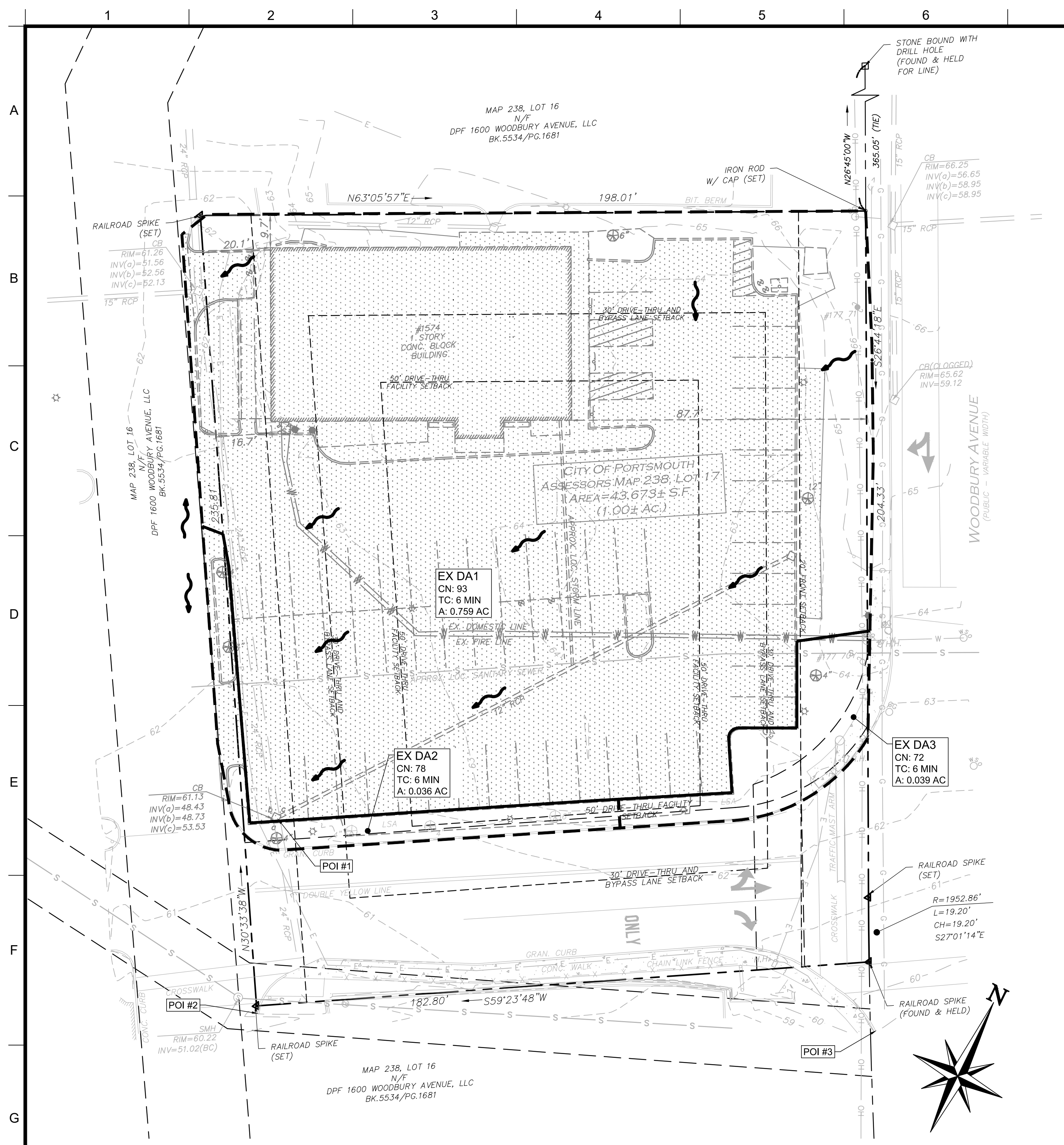
|                 |           |                      |               |
|-----------------|-----------|----------------------|---------------|
| Hydrograph type | = Combine | Peak discharge       | = 5.385 cfs   |
| Storm frequency | = 50 yrs  | Time to peak         | = 724 min     |
| Time interval   | = 2 min   | Hyd. volume          | = 16,961 cuft |
| Inflow hyds.    | = 5, 6, 7 | Contrib. drain. area | = 0.834 ac    |



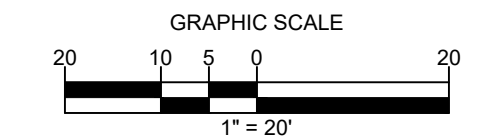


**APPENDIX D**

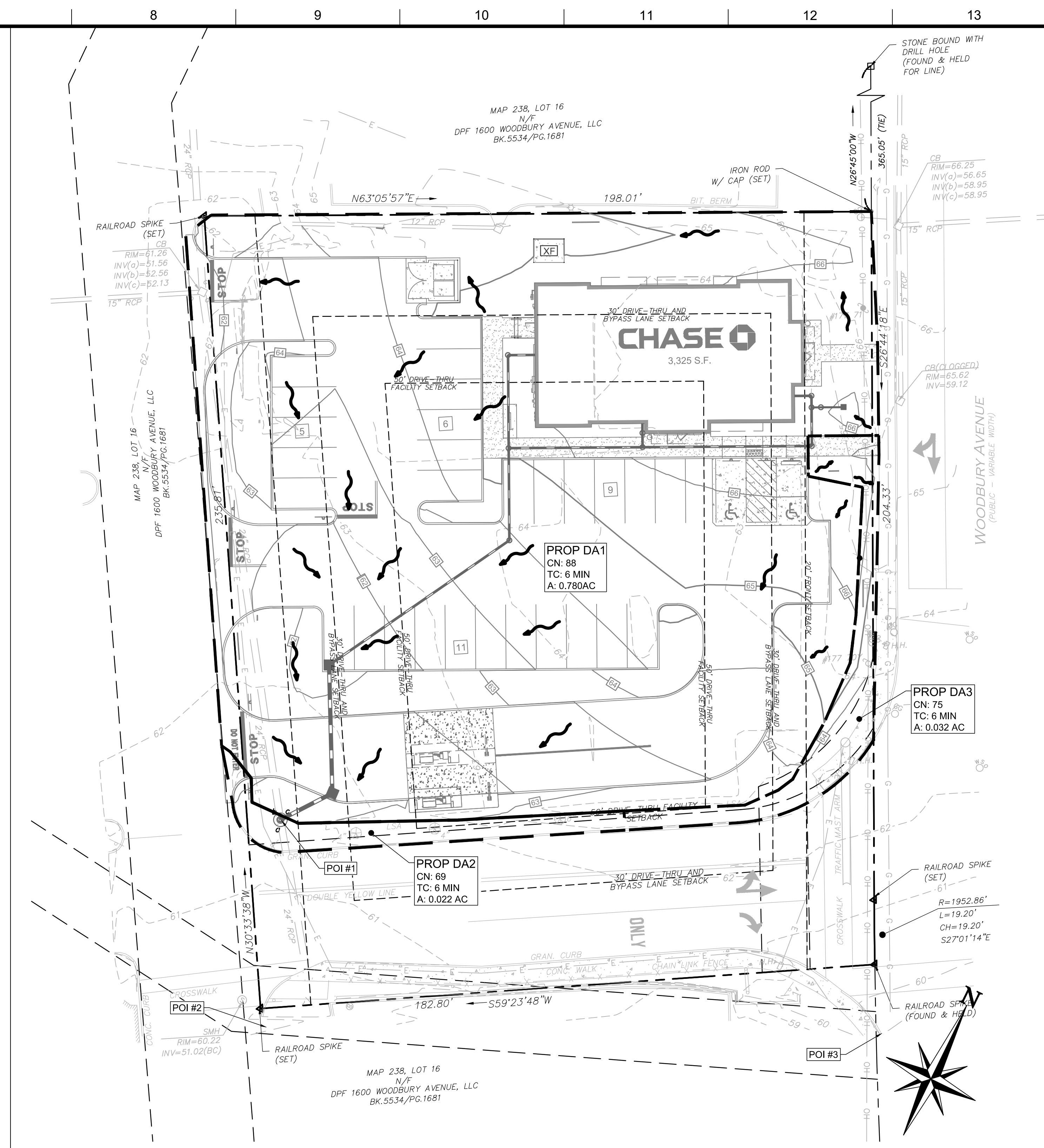
DRAINAGE AREA MAPS



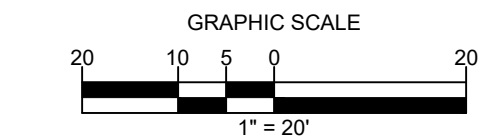
**PRE-DEVELOPMENT DRAINAGE AREA MAP**  
SCALE 1" = 20'



|  |
|--|
| <b>EX-DA 1</b>   |
| IMPERVIOUS (CN 98) = 27,143 SQUARE FEET<br>PERVIOUS (CN 69) = 5,919 SQUARE FEET<br>TOTAL (CN 93) = 33,062 SQUARE FEET (0.759 AC) |
| <b>EX-DA 2 (BYPASS)</b>  |
| IMPERVIOUS (CN 98) = 495 SQUARE FEET<br>PERVIOUS (CN 69) = 1,064 SQUARE FEET<br>TOTAL (CN 78) = 1,559 SQUARE FEET (0.036 AC)     |
| <b>EX-DA 3 (BYPASS PROW)</b>   |
| IMPERVIOUS (CN 98) = 171 SQUARE FEET<br>PERVIOUS (CN 69) = 1,515 SQUARE FEET<br>TOTAL (CN 72) = 1,686 SQUARE FEET (0.039 AC)     |



**POST-DEVELOPMENT DRAINAGE AREA MAP**  
SCALE 1" = 20'




|   |
|---|
| <b>PROP-DA 1</b>  |
| IMPERVIOUS (CN 98) = 22,846 SQUARE FEET<br>PERVIOUS (CN 69) = 11,114 SQUARE FEET<br>TOTAL (CN 88) = 33,960 SQUARE FEET (0.780 AC) |
| <b>PROP-DA 2 (BYPASS)</b>   |
| IMPERVIOUS (CN 98) = 0 SQUARE FEET<br>PERVIOUS (CN 69) = 919 SQUARE FEET<br>TOTAL (CN 69) = 919 SQUARE FEET (0.022 AC)            |
| <b>PROP-DA 3 (BYPASS PROW)</b>  |
| IMPERVIOUS (CN 98) = 297 SQUARE FEET<br>PERVIOUS (CN 69) = 1,097 SQUARE FEET<br>TOTAL (CN 75) = 1,394 SQUARE FEET (0.032 AC)      |

**GENERAL NOTES:**

- THIS PROJECT REFERENCES A SURVEY PREPARED BY:  
EXISTING CONDITIONS SURVEY  
CHASE BANK SITE  
1574 WOODBURY AVENUE  
PORTSMOUTH, NH  
ALLEN & MAJOR ASSOCIATES, INC.  
DATED 8/28/19

**ALERT TO CONTRACTOR:**

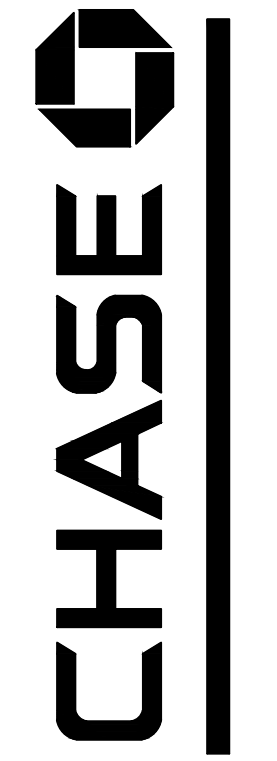
PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM ENGINEER AND THE OWNER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. ENGINEER AND OWNER SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.



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DALLAS, TX 75220  
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CLIENT



811

Know what's below. Call before you dig.

THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND DEPTH OF EXISTING UTILITIES SHOWN ON THESE PLANS ARE BASED ON RECORD DRAWINGS AND FIELD SURVEY. CORE STATES, INC. DOES NOT GUARANTEE THE LOCATION OR DEPTH OF ANY UTILITIES. THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF ALL UTILITIES PRIOR TO CONSTRUCTION.

| REVISIONS |      |         |    |
|-----------|------|---------|----|
| REV       | DATE | COMMENT | BY |
|           |      |         |    |
|           |      |         |    |
|           |      |         |    |
|           |      |         |    |
|           |      |         |    |

DOCUMENT

**SITE PLAN APPROVAL FOR CHASE BANK**

SITE LOCATION  
1574 WOODBURY AVENUE,  
PORTSMOUTH, NH  
03801

ENGINEER SEAL

SHEET TITLE  
**DRAINAGE AREA MAP**

|             |            |
|-------------|------------|
| JOB #:      | JPM 27086  |
| DATE:       | 07/01/2020 |
| SCALE:      | AS NOTED   |
| DRAWN BY:   | MAL        |
| CHECKED BY: | KGF        |

SHEET NO.  
**C-6**

**APPENDIX E**

CDS UNIT SPECIFICATIONS

## Chase Bank: WQ

Portsmouth, NH

### Information Provided:

- Total Contributing Drainage Area = 33,960 sf (0.780 Acres)
- Impervious cover = 22,846 sf (0.524 Acres)
- Design Storm = 1.00" Rainfall
- $T_c = 6$  minutes
- Unit Peak Discharge,  $q_u = 650$  cfs/mi<sup>2</sup>/in

### CDS Information:

The CDS technology features a patented non-blocking, indirect screening technique developed to treat stormwater runoff. The unit is highly effective in the capture of suspended solids, fine sands and larger particles. Because of its non-blocking screening capacity, the CDS unit is un-matched in its ability to capture and retain gross pollutants such as trash and debris.

### Design Summary:

A CDS 2015-4 was selected for this site. The CDS 2015-4 was sized to treat the 1" first flush of 0.519 cfs, and to remove greater than 80% TSS on an annual basis.

### **Water Quality Volume (WQV)**

|               |   |
|---------------|---|
| 0.78 ac       | A = Area draining to the practice                                     |
| 0.52 ac       | $A_I$ = Impervious area draining to the practice                      |
| 0.67 decimal  | I = percent impervious area draining to the practice, in decimal form |
| 0.66 unitless | $R_v$ = Runoff coefficient = $0.05 + (0.9 \times I)$                  |
| 0.51 ac-in    | $WQV = 1'' \times R_v \times A$                                       |
| 1,855 cf      | WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")                       |

### **Water Quality Flow (WQF)**

|                               |  |
|-------------------------------|--|
| 1 inches                      | P = amount of rainfall. For WQF in NH, P = 1".   |
| 0.66 inches                   | Q = water quality depth. $Q = WQV/A$   |
| 96 unitless                   | CN = unit peak discharge curve number. $CN = 1000 / (10 + 5P + 10Q - 10 * [Q^2 + 1.25 * Q * P]^{0.5})$                               |
| 0.4 inches                    | S = potential maximum retention. $S = (1000/CN) - 10$  |
| 0.076 inches                  | $I_a$ = initial abstraction. $I_a = 0.2S$  |
| 6.0 minutes                   | $T_c$ = Time of Concentration  |
| 650.0 cfs/mi <sup>2</sup> /in | $q_u$ is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III   |
| 0.519 cfs                     | $WQF = q_u \times WQV$ . Conversion: to convert "cfs/mi <sup>2</sup> /in * ac-in" to "cfs" multiply by $1 \text{mi}^2/640 \text{ac}$ |

Fig. 1 – BMP Worksheet for WQF

**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION  
BASED ON THE RATIONAL RAINFALL METHOD**

**Chase Bank  
Portsmouth, NH**

Area **0.78 ac**  
 Weighted C **0.88**  
 $t_c$  **6 min**  
 CDS Model **2015-4**

Unit Site Designation **WQ**  
 Rainfall Station # **104**

CDS Treatment Capacity **0.7 cfs**

| <u>Rainfall Intensity<sup>1</sup></u><br>(in/hr) | <u>Percent Rainfall Volume<sup>1</sup></u> | <u>Cumulative Rainfall Volume</u> | <u>Total Flowrate</u><br>(cfs) | <u>Treated Flowrate</u><br>(cfs) | <u>Incremental Removal (%)</u> |
|--|--|-----------------------------------|--------------------------------|----------------------------------|--------------------------------|
| 0.02   | 13.0%                                      | 13.0%                             | 0.01                           | 0.01                             | 12.5                           |
| 0.04   | 12.2%                                      | 25.2%                             | 0.03                           | 0.03                             | 11.7                           |
| 0.06   | 11.2%                                      | 36.4%                             | 0.04                           | 0.04                             | 10.7                           |
| 0.08   | 10.0%                                      | 46.4%                             | 0.05                           | 0.05                             | 9.4                            |
| 0.10   | 8.2%                                       | 54.6%                             | 0.07                           | 0.07                             | 7.7                            |
| 0.12   | 5.8%                                       | 60.4%                             | 0.08                           | 0.08                             | 5.4                            |
| 0.14   | 6.5%                                       | 66.9%                             | 0.10                           | 0.10                             | 6.0                            |
| 0.16   | 4.6%                                       | 71.5%                             | 0.11                           | 0.11                             | 4.3                            |
| 0.18   | 3.7%                                       | 75.2%                             | 0.12                           | 0.12                             | 3.3                            |
| 0.20   | 3.3%                                       | 78.5%                             | 0.14                           | 0.14                             | 3.0                            |
| 0.25   | 6.7%                                       | 85.2%                             | 0.17                           | 0.17                             | 5.9                            |
| 0.30   | 3.7%                                       | 88.9%                             | 0.21                           | 0.21                             | 3.2                            |
| 0.35   | 2.4%                                       | 91.3%                             | 0.24                           | 0.24                             | 2.1                            |
| 0.40   | 1.8%                                       | 93.1%                             | 0.27                           | 0.27                             | 1.5                            |
| 0.45   | 1.9%                                       | 95.0%                             | 0.31                           | 0.31                             | 1.6                            |
| 0.50   | 1.1%                                       | 96.1%                             | 0.34                           | 0.34                             | 0.9                            |
| 0.75   | 2.6%                                       | 98.7%                             | 0.51                           | 0.51                             | 1.9                            |
| 1.00   | 0.9%                                       | 99.6%                             | 0.69                           | 0.69                             | 0.6                            |
| 1.50   | 0.4%                                       | 100.0%                            | 1.03                           | 0.70                             | 0.2                            |
| 2.00   | 0.0%                                       | 100.0%                            | 1.37                           | 0.70                             | 0.0                            |
| 0.00   | 0.0%                                       | 100.0%                            | 0.00                           | 0.00                             | 0.0                            |
|  |  |                                   |                                |                                  | 91.9                           |

Removal Efficiency Adjustment<sup>2</sup> = 6.5%

Predicted % Annual Rainfall Treated = 93.4%

**Predicted Net Annual Load Removal Efficiency = 85.4%**

1 - Based on 10 years of hourly precipitation data from NCDC 1683, Concord WSO Airport, Merrimack County, NH

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

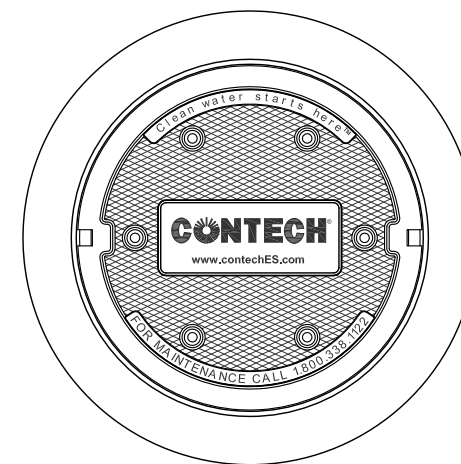
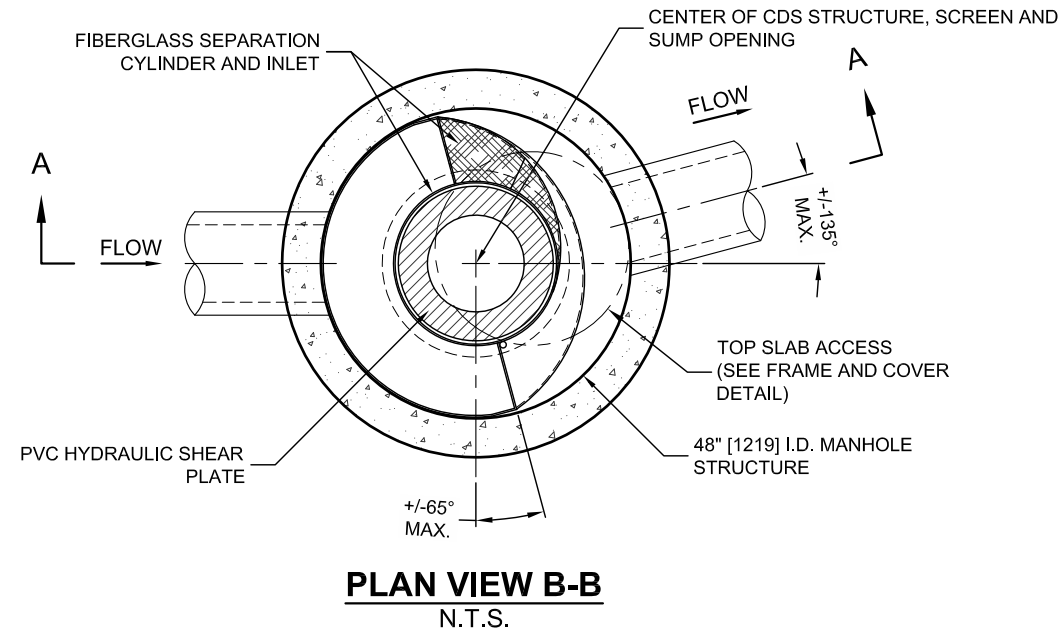


## CDS2015-4-C DESIGN NOTES

THE STANDARD CDS2015-4-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

### CONFIGURATION DESCRIPTION

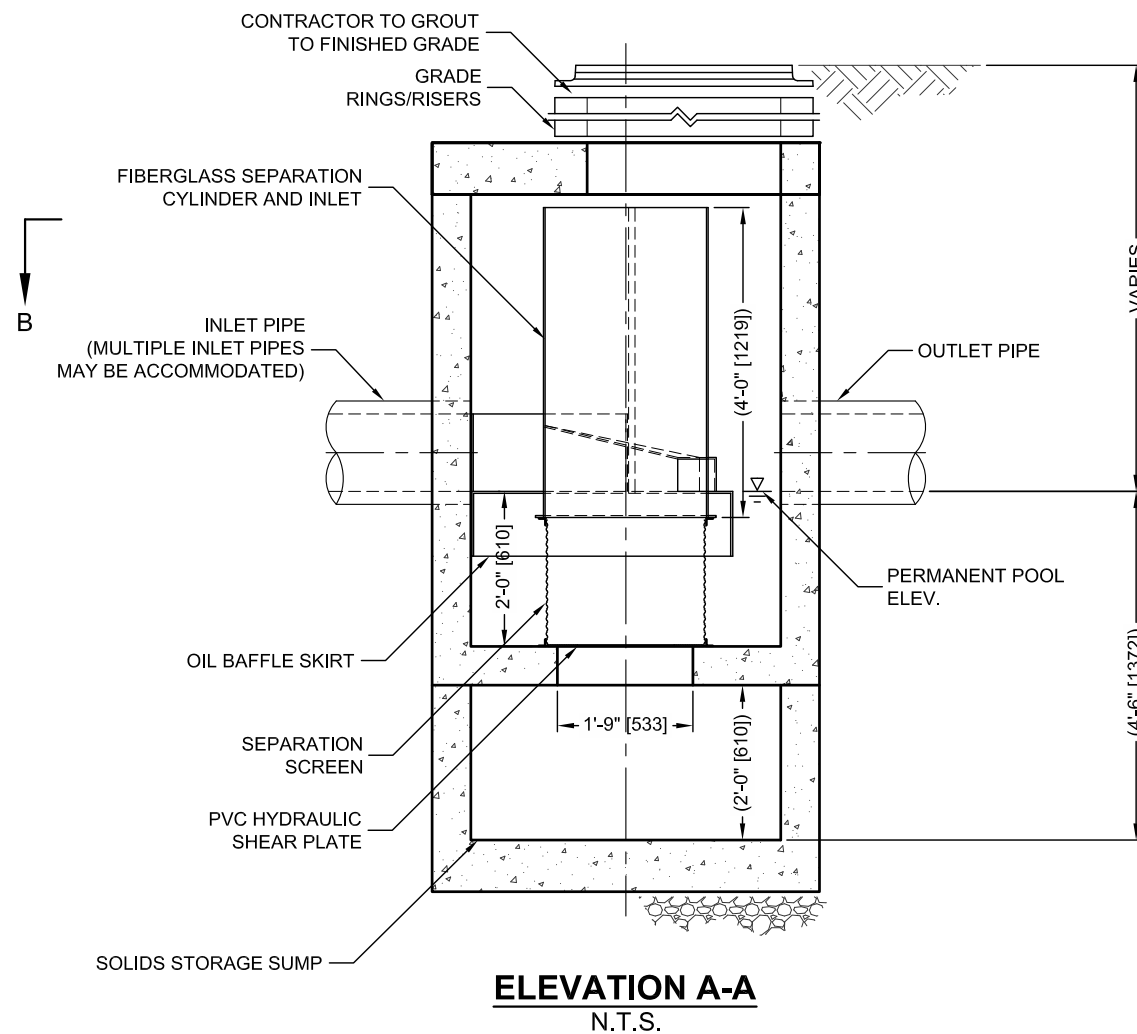
- GRATED INLET ONLY (NO INLET PIPE)
- GRATED INLET WITH INLET PIPE OR PIPES
- CURB INLET ONLY (NO INLET PIPE)
- CURB INLET WITH INLET PIPE OR PIPES
- SEPARATE OIL BAFFLE (SINGLE INLET PIPE REQUIRED FOR THIS CONFIGURATION)
- SEDIMENT WEIR FOR NJDEP / NJCAT CONFORMING UNITS



**FRAME AND COVER**  
(DIAMETER VARIES)  
N.T.S.

### SITE SPECIFIC DATA REQUIREMENTS

|                                      |       |          |          |   |
|--------------------------------------|-------|----------|----------|---|
| STRUCTURE ID                         |       |          |          |   |
| WATER QUALITY FLOW RATE (CFS OR L/s) |       |          |          | * |
| PEAK FLOW RATE (CFS OR L/s)          |       |          |          | * |
| RETURN PERIOD OF PEAK FLOW (YRS)     |       |          |          | * |
| SCREEN APERTURE (2400 OR 4700)       |       |          |          | * |
| PIPE DATA:                           | I.E.  | MATERIAL | DIAMETER |   |
| INLET PIPE 1                         | *     | *        | *        |   |
| INLET PIPE 2                         | *     | *        | *        |   |
| OUTLET PIPE                          | *     | *        | *        |   |
| RIM ELEVATION                        |       |          |          | * |
| ANTI-FLOTATION BALLAST               | WIDTH | HEIGHT   |          |   |
|                                      | *     | *        |          |   |
| NOTES/SPECIAL REQUIREMENTS:          |       |          |          |   |
| * PER ENGINEER OF RECORD             |       |          |          |   |



**ELEVATION A-A**  
N.T.S.

### GENERAL NOTES

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
2. DIMENSIONS MARKED WITH ( ) ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. [www.contechES.com](http://www.contechES.com)
4. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
5. STRUCTURE SHALL MEET AASHTO HS20 AND CASTINGS SHALL MEET HS20 (AASHTO M 306) LOAD RATING, ASSUMING GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.
6. PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.

### INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE (LIFTING CLUTCHES PROVIDED).
- C. CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
- E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

**CONTECH**  
ENGINEERED SOLUTIONS LLC

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CDS2015-4-C  
INLINE CDS  
STANDARD DETAIL



THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: 6,788,040; 6,841,720; 6,911,585; 6,981,762. RELATED FOREIGN PATENTS, OR OTHER PATENTS PENDING.