Breakfast Hill Water Main Extension



Preliminary Design and

Basis of Design Report

March 29, 2019

Portsmouth/Greenland, NH

Drinking Water Groundwater Trust Fund Project#: DWGT-08

Submitted by:



25 Vaughan Mall Portsmouth, New Hampshire 99 North State Street Concord, New Hampshire

2303.00

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1. Introduction

The Town of Greenland (TOG) and the City of Portsmouth (COP) have agreed to cooperatively advance the preliminary design phase of a water main extension from the existing Portsmouth system to the Breakfast Hill area of Greenland. The work is being advanced by the City of Portsmouth based on a request of the Town of Greenland with the primary goal of mitigating the risks associated with the presence of certain contaminants in groundwater around the Coakley Landfill groundwater management zone. The municipal system would provide the potable water instead of the individual private wells. This work was complete in conjunction with a Master Plan Supplement dated March 22, 2019.

Preliminary design drawings for the water main extension to the Breakfast Hill service area and conceptual drawings for recommended distribution improvements to the existing Portsmouth Water System have been developed (Appendix A) in accordance with the Work Scope Authorization approved by NHDES on March 14, 2018 (Project #DWGT-08). The purpose of this report is to serve as a basis of design for the following improvements:

- Transmission Water Main (Breakfast Hill Road)
- Neighborhood Water Mains to the Breakfast Hill service area
- Greenland Pressure Zone Water Storage Tank
- Greenland Well Improvements
- Pressure Reducing Valve/Meter Pit Ocean Road
- Booster Pump Station Pease C&J Bus Station Site
- Potential Pease Interconnect

This report has been completed in conjunction with a Master Plan Supplement prepared by Underwood Engineers on March 22, 2019.



2. Distribution Improvements Background

Three (3) different scenarios were evaluated for providing Portsmouth municipal water to the Breakfast Hill service area. The scenarios are as follows:

- 1. Create a loop from Post Road to Lafayette Road.
- 2. Create a stand-alone pressure zone in Greenland.
- 3. Connect Greenland to the Pease pressure zone.

A complete evaluation of the three (3) alternatives has been completed and is provided in the *Master Plan Supplement* report, dated March 22, 2019, by Underwood Engineers to supplement this basis of design. Scenario No. 2 was found to be the best option of those considered to meet the goals mentioned above. This scenario includes the following:

- 9,000 feet of 12" transmission main extension from Post Road to Lafayette Road.
- 28,000 feet of 8" water main to serve the neighborhoods.
- 500,000-gallon, 91-foot elevated water storage tank.
- Upgrade to the Greenland Well Pump.
- Pressure Reducing Valve on Ocean Road to allow water flow from proposed Greenland pressure zone into Portsmouth (main pressure zone).
- New booster pump station (and PRV) near Grafton Road/Greenland Road intersection Alternative location at Ocean Road.
- Pease interconnection for emergency on Grafton Road (normally off). The Greenland pressure zone will match the Pease pressure zone.
- Future/potential loop along Lafayette Road.

This preliminary design report considers the basis of design for Scenario No. 2.: creating a standalone pressure zone for the portion of the water system in Greenland (including the existing system in Greenland, the proposed Breakfast Hill service area, and a small part of the system that extends into Newington from Greenland).



3. Field Investigations

3.1. Survey

UE subcontracted Doucet Survey, Inc. (DSI) to complete the following work in the Breakfast Hill service area:

Digital orthophotography data from NH Granit (photos compiled from 2010 and 2015) was compiled to generate aerial base plans for the Breakfast Hill service area. This includes Breakfast Hill Road, the neighborhood streets, and Lafayette Road. Lafayette Road was included for future consideration of a loop to Lafayette Road.

Topographic survey was completed by DSI in 2018 for the bridge deck to create 1-foot topographic contours of the bridge top. Additionally, DSI located other features such as structures and observable utilities. DSI used the Leica C10 Scanner to locate I-95 pavement, guardrails, bridge abutments, bridge girders, piers, underside bridge deck, and sleeves.

DSI also field located the following features to supplement base maps:

- Existing drainage including headwalls, catch basins, drain manholes, drain pipes, culverts, inverts, etc.
- Falls Way bridge and concrete open-bottom/box culverts in neighborhood and wetlands beneath
- Existing water system near Post Road (valves, hydrants, etc.)

Due to the limited survey as part of this work, it is recommended to complete detailed field survey for final design.

3.2. Subsurface Investigations

Preliminary field investigations including soil borings and ledge probes were completed by S.W. Cole Explorations, LLC (Bangor, ME) in the Breakfast Hill service area. Approximately 3-days of subsurface work was completed to log 62 ledge probes and 8 soil borings throughout the project area. Subsurface exploration information is provided in Appendix B.

From available subsurface information, rock removal will be required on parts of Breakfast Hill Road to install the water main, specifically the section adjacent to Breakfast Hill Golf Club (STA 40+00 to STA 61+00) and the section from Berry Farm Lane to Lafayette Road (STA 78+00 to 89+00). Borings and probes to date indicate depth to top of ledge ranges 2.7 feet to 11.4 feet in these sections. Rock removal will also be required on sections of Maple Drive, Berry Farm Lane, Falls Way, Ridgecrest Drive, and Pinewood Circle. Borings and probes to date indicate depth to top of rock ranges from 3.0 feet to 9.2 feet in these sections. The amount of total



rock removal required for the service area is estimated to be 2,500 CY assuming 5.5 feet of required pipe cover and a trench width of 5 feet. This should be confirmed during final design.

Note that subsurface investigations were not completed at the tank, booster pump station, PRV vault, or Pease interconnect sites and will be needed to advance the design.

3.3. Hazardous Materials Review

A NHDES OneStop Data Mapper review was completed to evaluate the potential for encountering contaminated soil or groundwater in the project area during construction. The following sites within the project area were found on the OneStop Data Mapper:

3.3.1. <u>Palmer Residence</u> (Remediation Site)

- Site number: 201211063
- Address: 19 Maple Drive Greenland, NH 03840
- Project type: OPUF (leaking residential heating oil tank)
- Based on information available on the OneStop Data Mapper, this site is a "closed" site and there are no known sources of risk and no ambient groundwater quality standard violations. Workload priority is considered to be "low priority".

3.3.2. <u>Breakfast Hill Crossing (Plaza)</u> (Remediation Site)

- Site number: 199503025
- Address: 611 Breakfast Hill Road Greenland, NH 03840
- Project type: UIC (underground injection control; discharges of benign wastewaters not requiring a discharge permit or request to cease a discharge)
- Based on information available on the OneStop Data Mapper, this site is considered to have a low workload priority. However, potential risks for this site have not yet been defined.

3.3.3. <u>A Hair Ahead</u> (Local Potential Contamination Source)

- Site number: PCS00204
- Address: 611 Breakfast Hill Road Greenland, NH 03840
- Project type: CLN (dry cleaners; laundromats; beauty salons; and car washed)

The Coakley Landfill superfund site is adjacent to the project area. The New Hampshire Department of Environmental Services and The United States Environmental Protection Agency



have identified 1,4-Dioxane, Dissolved Arsenic, and Dissolved Manganese as "contaminants of concern".

Due to the potential issues, special soils and groundwater handling may be required during construction.



4. Proposed Water Main

This section describes preliminary design considerations and the basis of design for the Breakfast Hill Road water transmission main extension of the existing Portsmouth Water System as well as the water main extension into the adjacent neighborhoods. Preliminary design drawings for the extension are included in Appendix A.

4.1. Transmission Main Description/Alignment

The proposed transmission main extension to the Breakfast Hill service area will begin at the corner of Post Road and Breakfast Hill Road (NHDOT owned road) where there is an existing 12" stub on the north side of the road. The proposed water main will extend east on the north side of Breakfast Hill Road and cross the I-95 bridge.

The proposed main will continue to run east on the north side of Breakfast Hill Road until the intersection with Lafayette Road where the water main will be capped for potential future extension of the system. Note that directional drilling is assumed to be required to cross the existing, paved over, railroad tracks and the three (3) 24" RCP culverts east of the Breakfast Hill Golf Club (STA 61+75 to STA 63+00). Additional details for proposed directional drilling methods can be found in Section 4.3.5.



Figure 1. Three (3) 24" RCP culverts on Breakfast Hill Road.

Also note that the transmission main is proposed to connect to the existing water system on Seavey Way that intersects Breakfast Hill road twice. Two tees are proposed to tie into both intersections of Seavey Way and Breakfast Hill Road (STA 66+50 and STA 74+50).



The majority of the preliminary alignment for the water main is located on the northern shoulder of Breakfast Hill Road. The basis for a north alignment is that there is more shoulder available for placement of a water main and that keeping the main on the north side avoids any road crossings. Based on preliminary discussions with NHDOT, this general alignment would be acceptable. However, there is potential for the main to be constructed off the pavement within the R.O.W., but a topographic/R.O.W. survey would be required to provide the level of detail required to assure if there is adequate space. Alignments will be adjusted in final design phases of work.

4.2. I-95 Bridge Crossing

The I-95 bridge was designed in 1971 by Howard, Needles, Tammen, and Bergendoff Consulting Engineers (HNTB). As-built plans for the bridge were provided by NHDOT to aid with the design of the bridge crossing. Per the plans provided, the original intent was to accomadate an insulated 12" cast iron pipe between the bridge girders/cross frames of the bridge and install the pipe through abutment penetrations on each side of the bridge. On each end, the pipe was proposed to be installed through the penetration and extend past the 20' approach slab for potential future hookup to a water system. However, the 12" pipe was never installed beneath the bridge deck and it is assumed that a 12" pipe was never installed in the approach slab to the bridge either.

The proposed bridge crossing method for this project includes installing a 24" DI sleeve that will start at the bridge abutment penetration and extend past the approach slab to the bridge. The sleeve is provided so that the approach slab will not have to be disturbed in the event of any future maintenance. The proposed 12" water main, with polyurethane insulation and a high-density polyethylene (HDPE) casing (total OD = 17.92") will be installed within the 24" sleeve with casing spacers to stabilize the pipe. The 12" main will then be installed through the existing 20" abutment penetration. Link-Seal modular seals are proposed to stabilize the pipe within the penetration.



Figure 2. Abutment penetration at east abutment of I-95 bridge.



Once through the penetrations, the watermain (with insulation and HDPE casing) is proposed to be carried from the existing bridge girder/cross frames and secured by proposed utility support rollers. An evaluation was completed by Tirey & Associates, P.C. to confirm that the bridge has the structural capacity to support the load of the proposed water main. Information on the proposed water main weight and the structural evaluation completed by Tirey & Associates is provided in Appendix C. Also proposed is one (1) 12" self-restrained expansion/contraction joint to be constructed between the back wall of the west abutments and the first hanger. Bridge details are provided on Sheet 31 of Appendix A. Technical information sheets for bridge crossing appurtenances including specified 12" ductile iron pipe, polyurethane insulation, HDPE casing, expansion/contraction joint, casing spacers for 24" DI sleeve, Link-Seal modular seals, and pipe support rolls are provided in Appendix D.

4.3. Neighborhood Water Main Description/Alignment

It is proposed to tee off from the proposed 12-inch water main to provide water service to each of the neighborhoods off of Breakfast Hill Road. The neighborhoods include the following streets:

- Maple Drive
- Maple Drive Extension
- Sunnyside Drive
- Coombs Farm Road
- Windsor Green Road
- Stone Meadow Way

- September Drive
- October Drive
- Falls Way
- Skyview Drive
- Ridgecrest Drive
- Balsamic Circle
- Pinewood Circle

- Red Oak Drive
- Berry Farm Lane

Preliminary drawings show alignments for the streets above. The preliminary alignment places the water main near the edge of pavement. However, it is anticipated that the alignment can be modified to decrease impacts to the roadway once a topographic/R.O.W. survey is complete.

It is anticipated that directional drilling is needed in the Falls Way neighborhood to cross a 100ft long bridge as you enter the neighborhood and six (6) precast concrete box culverts throughout the neighborhood.





Figure 3. 100-ft long bridge on Falls Way.



Figure 4. Example Falls Way neighborhood precast concrete box culvert.



4.4. Basis of Design

The basis of design for the transmission water main on Breakfast Hill Road is summarized as follows:

4.4.1. <u>Water Main</u>

- Diameter: 8-inch and 12-inch, mechanical joint (MJ) piping
- Material: Ductile Iron, Class 52
 - Corrosion control provisions to be provided (either polyethylene pipe-wrap or Zinc coated pipe)
- Minimum cover: 5.5 feet
- Valves: 8-inch/12-inch MJ gate valves
- Valve open direction: right, per City of Portsmouth standards

4.4.2. <u>Hydrants</u>

- Provided for fire protection and water quality flushing purposes
- Type: Kennedy K-81A Guardian, per City of Portsmouth standards
- Valve open direction: right, per City of Portsmouth standards
- Spacing: approximately 600-ft apart (to be reviewed by the City)

4.4.3. <u>Water Services</u>

- Domestic Services: 1" copper
 - Duplexes will be provided two (2) water services
- Commercial Domestic Services: sized to allow ≤ 1-ft of head loss
 - Breakfast Hill Golf Club: 4" ductile iron
 - Garden of Eves Greenhouses: 4" ductile iron
 - Rolling Green Nursery: 4" ductile iron
 - 611 Breakfast Hill Road: 2" copper
 - Bethany Church: 2" copper
- Commercial Fire Protection Services: 4" ductile iron (or as required/requested by by property owner)



- Curb stop and box to be provided for all services
- Preliminary drawings do not include work required to connect to existing water plumbing
- Note: this project provides service to the ROW for each improved property. Property owners will need to construct the service from the ROW to their foundation. Additionally, a meter, backflow preventer, and expansion tank will be needed.

4.4.4. <u>I-95 Bridge Crossing</u>

- 24" DI sleeve installed from abutment penetration (20") to beyond existing approach slab on each side of the bridge
 - Sized based on OD of pipe plus insulation and HDPE jacket (17.92")
 - Casing spacers to stabilize pipe within 24" DI sleeve
 - 8" wide stainless-steel band w/ ribbed flanges
- Link-Seal modular seals to stabilize pipe within 20" abutment penetration
- Pipe: 12" DI TR Flex (OD = 13.2")
 - Self-restrained push-on joint pipe
 - ASTM C150 Type II cement mortar lining
 - Maximum 5^o deflection angle
 - 4 self-restraining locking segments
- Insulation: foamed in place closed cell polyurethane insulation
 - Thickness: 2.16"
- Casing: HDPE jacket
 - Thickness: 0.2"
- Expansion/contraction joint
 - Model: 200 EX-TEND Series No. 212MO by EBAA Iron, Inc.
 - Nominal pipe size: 12"
 - Expansion (linear): 4"
 - Expansion determined by change in length due to thermal compression/expansion



- 4" expansion provides a temperature delta of 185 °F
- Pipe Supports
 - Cast iron roll and sockets; steel rod roll
 - Model: Grinnell Pipe Role Fig. 171 or Fig. 177
 - Pipe roll to be provided above and below proposed water main
 - Steel pipe hanger rod

4.4.5. <u>Directional Drilling</u>

- Location: stream crossing east of Breakfast Hill Golf Club
 - 8-ft clearance between proposed drilling and existing 24" RCP culverts, per DBU Construction, Inc. field review recommendation
- Location: Falls Way neighborhood
 - 100' bridge on Falls Way
 - 10-ft of clearance required between proposed drilling and existing wetlands, per DBU Construction, Inc. field review recommendation
 - Concrete box/open-bottom culverts within Falls Way neighborhood
 - 5-ft of clearance required between proposed drilling and existing wetlands, per DBU Construction, Inc. field review recommendation
- Diameter: 8inch/12-inch
- Material: HDPE
- Typical min. space required for drill rig equipment: 75' x 100'
- Entry angle at drill rig: 8°-20°
- Drill rig entry point is approx. 10-ft back from entry pit for every foot of depth required at entry pit.
- Gate valves to be provided on both ends of directionally drilled HDPE sections.

4.5. Wetlands Permitting

A wetlands evaluation was not completed in this phase of the design. Required wetlands permitting, if any, should be coordinated with the NHDES Wetlands Bureau for directional drilling operations on Breakfast Hill Road and the Falls Way neighborhood. No other wetland impacts are anticipated.



4.6. NHDOT Coordination and Construction Standards

UE met with Lucas Miller of NHDOT District 6 and Jason Tremblay of NHDOT Bridge Design on July 9th, 2018 to discuss the water main extension into the Breakfast Hill service area, specifically construction on Breakfast Hill Road and the I-95 bridge crossing. NHDOT provided the following feedback for project requirements:

4.6.1. <u>General</u>

- Long-form excavation permit with 1 set of full-size plans and 1 set of half-size plans
- Environmental checklist
- Bond amount (to be determined after plan review)
- Signature from the Contractor and Municipality
- Separate permits for work in separate municipalities
- DOT requires both physical and digital submittals
- Design shall conform with 2017 NHDOT UAM requirements

4.6.2. <u>Bridge Work</u>

- Bridge work will be permitted under excavation permit
- LAROW access for construction will require a Use and Occupancy agreement (after final design)
- Structural calcs to support that the existing bridge has adequate capacity to carry the load of a new 12" main.
- Submit final design drawings to NHDOT to be incorporated into bridge record drawings
- A sleeve that extends past the bridge approach slab is required if there is not an existing sleeve through the existing abutment penetration. Note that it is anticipated that the existing sleeve shown on the record drawings was not constructed and does not exist.
- NHDOT suggested that the best plan is likely to cut the approach slab, remove a section, then pour a new slab and tie into the existing after the pipe is placed.



4.7. Final Design Considerations

- Complete topographic survey for project areas including wetlands
- Complete ROW survey at tank site, PRV site, and booster pump station site
- Confirm water main alignment based on existing utility locations and R.O.W. availability
- Complete Geotech report
- Confirm locations of proposed stubbed services based on existing plumbing/well location
- Confirm property owners that will connect
- Consider corrosion control pipe installation methods/pipe specifications
 - Ex: polyethylene pipe-wrap (V-bio or standard) or Zinc coated pipe
- Confirm construction methods for trench repair on Breakfast Hill Road with NHDOT District 6, trench repair with overlay assumed.
- Further evaluate 24" sleeve design in approach slab to 20" abutment penetration
- Confirm bridge crossing methods/approach slab repair with NHDOT Bridge Design
- Coordinate with NHDES Wetlands Bureau any permitting requirements for crossing the stream east of the Breakfast Hill Golf Club



5. Greenland Pressure Zone Water Storage Tank

Per Master Plan update by UE, a **0.5 MG elevated storage tank is recommended**. This size tank will also provide improved water age conditions compared to a larger tank. The recommended tank type is a composite elevated tank like the existing Pease and Spinney Road tanks (Figure 5) The recommended site for the tank is the Town of Greenland owned land between the Bethany Church and the former Rye landfill. A conceptual tank site plan and details are provided in Appendix A on Sheets 28 and 32, respectively.



Figure 5. Pease composite elevated tank (left) and Spinney Road composite elevated tank (right).

5.1. Basis of Design

Major components and features of the tank are provided below:

- Tank type: composite elevated tank
- Tank material: steel w/ steel-lined concrete bottom
- Tank capacity: 500,000-gal
- Tank diameter: 50-ft
- Pedestal diameter: 30-ft
- Head range: 35-ft
 - Tank levels can be controlled using Greenland Well/proposed BPS to control water age
- Reinforced concrete pedestal support shaft
- Tank color: TBD



- Other appurtenances include: overflow pipe w/ concrete splash pad, vent, roof hatch, roof handrail, access tube hatch, interior pedestal ladder w/ OSHA compliant safety climbing device and rest seats at 50-ft maximum spacing, walkway w/ handrail under tank, tank access ladder, and inside tank ladder
- Usable space for miscellaneous storage in pedestal base w/ option for multiple storage floors
- One (1) overhead door to pedestal
- One (1) personnel door to pedestal
- Tank color: TBD in final design phase

5.2. Final Design Considerations

- Confirm recommended tank site
- Complete topographic survey at proposed site
- Complete subsurface investigations and geotechnical evaluation at proposed site
- Confirm pedestal storage options
- Confirm tank color



6. Greenland Well Pump Improvements

The Greenland Well is located near the Mike Maloney Rec Center off of Post Road. The pump (replaced in 2017 as part of another project) pumps to the main pressure zone's HGL of 171-ft. In order to pump to the proposed Greenland pressure zone (HGL = 230-ft), the existing pump would need to be replaced with a larger pump with a TDH 59-ft larger than the existing pump (230 ft – 171 ft = 59 ft).

6.1. Existing Conditions

- Pump type: vertical turbine, 30 HP
- Pump capacity = 450 gpm (Greenland Well limited @ 450 gpm per City of Portsmouth Water Department)
- TDH = 195-ft (per existing pump submittal)

6.2. Proposed Conditions

- Pump type: vertical turbine, 40-45 HP (estimated)
- Pump capacity = 450 gpm (Greenland Well limited @ 450 gpm per City of Portsmouth Water Department)
- TDH = 254-ft (increase of 59-ft)
- Pump horsepower may potentially be reduced if 10-in. main has low C factor and is replaced.

Product information and data sheets are provided in Appendix E for a recommended replacement pump from Sulzer Ltd.

6.3. Final Design Considerations

- Confirm any other improvements that should be considered.
 - Example: evaluate replacing existing 10" main from the Greenland Well to Post Road with a 12" main for hydraulic purposes replacement not included currently.



7. Pressure Reducing Valve/Metering Vault – Ocean Road

As mentioned above, the water system in Greenland is currently part of the main pressure zone. It is proposed to install pressure reducing valves (PRVs) to isolate the main pressure zone and the proposed Greenland pressure zone. PRVs allow water to move from high pressure to low pressure in the event of a water emergency in the lower pressure zone. Water currently enters Greenland from the main pressure zone in two (2) locations. The Greenland service area is fed water from distribution piping on Ocean Road and from piping on Route 33 near Grafton Road. PRVs are recommended at both these locations. The proposed PRV for the piping near Route 33/Grafton Road is proposed to be installed as part of a proposed booster pumping station (BPS). More information on the proposed BPS is provided in Section 9.

The Ocean Road PRV is proposed to be installed in a vault and, per request by the City of Portsmouth, is to include metering capabilities to track flow between pressure zones. For preliminary purposed, the vault site is proposed to be in proximity to the City of Portsmouth/Town of Greenland border. The preliminary site is on the north side of Ocean Road just west of the Portsmouth/Greenland border. Preliminary evaluations show adequate space for the construction of the vault, but grading impacts should be anticipated to flatten the site. A conceptual site plan is provided in Appendix A on Sheet 28. The vault is proposed to be constructed in line with the existing 10" cast iron water main that is off the north side of the road.

The proposed vault will allow water to pass in both directions; Greenland pressure zone to main pressure zone and vice versa. There are two different sized pipes branches, 6-in. and 10-in. within the proposed vault to transmit water in each direction. Water flowing from Greenland to the main pressure zone will flow through 6-in. piping including the PRV and a 6-in. meter. This is the primary function of the vault. However, when pressure drops to less than 25 psi (to be confirmed in final design phase) a 10-in. check valve will open and allow flow from the main pressure zone to the Greenland pressure zone. This water passes through the 10-in piping and a 10-in. meter. This direction of flow will likely only occur during emergency demand events such as fire flows or tank cleanings. Note that when water is flowing into Greenland, water will flow through the 10-in. and the 6-in. piping.

7.1. Basis of Design

Major components and features of the proposed vault as follows:

- Precast concrete vault dimensions: 20' L x 8' W x 9' H
 - Sized to provide adequate straight length pipe for metering purposes and adequate interior height (7-ft) for personnel access, operation, and maintenance
- 10" meter to measure flow from main pressure zone to Greenland pressure zone



- 6" meter to measure flow from Greenland pressure zone to main pressure zone
 - Water flows through 6" meter in reverse direction; meter will measure net flow
- 10" check valve
- 8" pressure reducing valve
 - Upstream pressure set point: 81 psi
 - Downstream pressure set point: 55 psi
 - Pressure set points based on Google Earth elevation of 52-ft
- 30" vault roof hatch/access cover
- Sump pit and sump pump
- PVC vent pipe w/ insect screen
- 5 kW unit heater
- Portable electric dehumidifier w/ drain to sump

7.2. Final Design Considerations

- Confirm location and land acquisition/easements.
- Complete topographic survey at proposed site
- Complete subsurface investigations at proposed site
- Confirm final valve sizing with vendors.
- Confirm controls/SCADA requested



8. Booster Pumping Station – Pease C&J Bus Station

A booster pumping station (BPS) is required to provide water from Portsmouth's main system which provides the two (2) sources necessary. The Greenland Well is the other source.

The booster pumping station will pump water from the main pressure zone HGL (171-ft) to the Greenland high pressure zone HGL (230-ft). Together with the Greenland Well, the BPS maintains levels in the proposed storage tank on Breakfast Hill Road.

It is proposed to install an above-grade factory built pumping system by SyncroFlo or equivalent. The station will house booster pumps (two for redundancy), controls, and metering equipment. The proposed BPS would also include PRVs to allow flow from the Greenland pressure zone to the main pressure zone. An enclosed generator is also proposed outside the BPS for emergency backup power.

There are several potential locations for the BPS all of which are adjacent to the Pease C&J bus station. One possible site is shown on Sheet 28 of the drawings.

Although a BPS on Ocean Road is also possible, preliminary evaluations show that this alternative location increased max water age in the majority of the existing Greenland system. There is also more land availability at the C&J site so that is where conceptual site plans are provided for the BPS. Additional evaluation would be needed if a BPS on Ocean Road were to be pursued.

8.1. Basis of Design

The proposed BPS is sized to provide the Greenland pressure zone's maximum day demand (MDD) of 500 gpm and to pump to the proposed Greenland pressure zone HGL (230-ft). We have assumed 13 psi (30-ft) of headloss through the system bringing the total dynamic head (TDH) of the pumps to 89-ft (a pressure differential of 39 psi). Detailed technical information on the proposed BPS is provided in Appendix F. Major BPS components and features include the following:

- Two (2) 20 hp frame mounted end suction pumps each rated at 500 gpm @ 89-ft
 - Pumps rated at 89-ft to include BPS head losses
 - Pumps to operate in duty/standby mode; one pumps to fill proposed tank on Breakfast Hill Road while the other is in standby
- Parallel PRVs to allow flow and reduce pressure in opposite direction
- Flow meter for pump control and metering purposes
- System controls for all logic and motor control with SCADA interface



- Low inlet pressure controllable shut-down
 - Will assist with fire flows in main pressure zone
- Skid approximate minimum footprint: 6' x 6'
- Building: steel self-framed building with exterior brick façade and gable roof
- Preliminary building footprint: 16' x 16'
 - Extra room provided for storage
- Exterior generator w/ weather/sound enclosure (diesel or propane TBD)

8.2. Final Design Considerations

- Confirm pumping station location.
- Complete topographic survey at proposed site
- Complete subsurface investigations at proposed site
- Proceed with correspondence with NHDOT for any permits, requirements, and land acquisition needed.
- Obtain land acquisition/easements needed.
- Confirm location of existing water main in respect to drainage swale.
- Confirm site layout.
- Confirm desired building construction/architecture (brick facade and gable roof recommended).
- Confirm final valve sizing with vendors.
- Booster pumps and piping layout: confirm preferences with City to facilitate operation and maintenance. Pump style options may include horizontal centrifugal or vertical turbine pumps.
- Confirm pumping layout. Current recommendation is two (2) 500 gpm pumps.
- Evaluate impacts on pump station location and inlet pressures.



9. Potential Pease Interconnect

As mentioned above, the Greenland pressure zone is proposed to match the existing Pease pressure zone at an HGL of 230-ft. The two pressure zones will normally operate separately, but it is proposed to provide a manual connection for emergency purposes. The proposed connection point is based off where the Pease pressure zone comes closest to the proposed Greenland pressure zone. This location is Grafton Road. There is an existing 8" cast iron main on the north end of Grafton Road. There is an existing 12" cast iron main on the opposite end of Grafton Road, approximately 1,600-ft away that is proposed to be part of the Greenland pressure zone. It is proposed to install a 12" water maim to connect the two existing pipes. Valves will be provided so that there is not normally flow between the two systems. A conceptual site plan for the interconnect is provided in Appendix A on Sheet 28.

9.1. Basis of Design

The following improvements are recommended:

- 1,700 linear feet of total pipe
 - Includes pipe installed on Grafton Road and to tie into existing water mains
- Diameter: 12-inch
- Material: Ductile Iron, Class 52
- Minimum cover: 5.5 feet
- Valves: 12-inch MJ gate valves & 8-inch MJ gate valves (normally closed)
- Valve open direction: right, per City of Portsmouth standards

9.2. Final Design Considerations

- Complete topographic survey at proposed site
- Complete subsurface investigations at proposed site
- Coordinate with Pease Development Authority for any required permits/requirements/administrative steps to connect the pressure zones



APPENDIX A

PRELIMINARY DESIGN DRAWINGS

(PROVIDED UNDER SEPARATE COVER)

APPENDIX B

SUBSURFACE EXPLORATION INFO



Data shown on this map is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this map.



Data shown on this map is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this map.





Data shown on this map is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this map.

Exploration Log Sheet Greenland NH

				Groundwater	
Exploration #	Street	Target Depth	Refusal Depth	Depth	Soil Type
P1	Breakfast Hill Rd	10	N/A		sand and gravel
P2	Breakfast Hill Rd	10	N/A		sand and gravel
Р3	Breakfast Hill Rd	10	N/A		sand and gravel
B1	Breakfast Hill Rd	10	N/A		fine to medium sand
P4	Breakfast Hill Rd	10	N/A		fine sand
P5	Breakfast Hill Rd	10	N/A		fine to coarse sand
P6	Breakfast Hill Rd	10	N/A		fine to coarse sand
P7	Breakfast Hill Rd	10	N/A	5'	silty sand and gravel
B2	Breakfast Hill Rd	10	11.4'		fine to coarse sand and gravel
P8	Breakfast Hill Rd	10	5.2'		sand and gravel
Р9	Breakfast Hill Rd	10	5.7'		sand and gravel
P10	Breakfast Hill Rd	10	4.9'		sand and gravel
B3	Breakfast Hill Rd	15	3.5'		sand and gravel
P11	Breakfast Hill Rd	15	N/A		fine sand, trace of gravel
P12	Breakfast Hill Rd	10	6.0'		sand and gravel
P13	Breakfast Hill Rd	10	N/A		sand and gravel
P14	Breakfast Hill Rd	10	7.9'		sand and gravel
P15	Breakfast Hill Rd	10	3.3'		sand and gravel
P16	Breakfast Hill Rd	10	2.7'		sand and gravel

				Groundwater	
Exploration #	Street	Target Depth	Refusal Depth	Depth	Soil Type
P17	Maple Dr Ext	10	N/A		fine sand
P18	Maple Dr Ext	10	N/A		fine to coarse sand and gravel
P19	Maple Dr Ext	10	N/A		sand and gravel
P20	Maple Dr Ext	10	N/A		fine sand and gravel
P21	Maple Dr Ext	10	N/A		fine sand with silt
P22	Maple Dr Ext	10	3.0'		sand and gravel
P23	Maple Dr Ext	10	N/A		fine sand
P24	Maple Dr Ext	10	N/A		sand and gravel
P25	Sunnyside Dr	10	N/A		fine sand
B4	Sunnyside Dr	10	10.3'		fine to coarse sand and gravel
P26	Sunnyside Dr	10	N/A		fine to coarse sand and gravel
P27	Sunnyside Dr	10	N/A		fine to coarse sand and gravel
P28	Coombs Farm Rd	10	N/A		silty fine sand
P29	Coombs Farm Rd	10	N/A		silty fine sand
P30	Winsor Green Rd	10	N/A		silty fine sand
P31	Winsor Green Rd	10	N/A		silty fine sand
P32	Winsor Green Rd	10	N/A		fine to coarse sand and gravel
B5	Stone Meadow Wy	10	11.0'		fine to coarse sand and gravel
P33	Stone Meadow Wy	10	N/A		fine to coarse sand and gravel
P34	Berry Farm Wy	10	5.9'		sand and gravel
P35	Berry Farm Wy	10	6.0'		sand and gravel

				Groundwater	
Exploration #	Street	Target Depth	Refusal Depth	Depth	Soil Type
P36	October Dr	10	N/A		sand and gravel
P37	October Dr	10	N/A		sand and gravel
P38	September Dr	10	N/A		sand and gravel
P39	September Dr	10	N/A		sand and gravel
P40	September Dr	10	N/A		sand and gravel
B6	September Dr	10	N/A	7'	fine to coarse sand and gravel
P41	September Dr	10	N/A	7'	fine to coarse sand and gravel
P42	September Dr	10	N/A		sand and gravel
P43	Falls Way	10	N/A		sand and gravel
P44	Falls Way	25	18.0'		sand and gravel / gray silt
B7	Falls Way	25	5.0'		sand and gravel
P45	Falls Way	10	3.0'		sand and gravel
P46	Falls Way	10	4.0'		sand and gravel
P47	Falls Way	10	4.5'		sand and gravel
P48	Skyview Dr	25	14.5'	7'	sand and gravel
P49	Skyview Dr	10	N/A		sand and gravel
P50	Skyview Dr	10	6.0'		sand and gravel
P51	Skyview Dr	10	N/A		sand and gravel
P52	Skyview Dr	10	N/A		stiff clay/sand and gravel
B8	Skyview Dr	25	18.0'	6'	stiff clay/sand and gravel
P53	Ridgecrest Dr	10	4.5'		sand and gravel

				Groundwater	
Exploration #	Street	Target Depth	Refusal Depth	Depth	Soil Type
P54	Ridgecrest Dr	25	9.2'		sand and gravel
P55	Ridgecrest Dr	25	6.6'		sand and gravel
P56	Ridgecrest Dr	10	N/A		sand and gravel
P57	Ridgecrest Dr	10	6.5'		sand and gravel
P58	Ridgecrest Dr	10	3.4'		sand and gravel
P59	Ridgecrest Dr	10		Electric was not	t marked / did not drill
P60	Ridgecrest Dr	25	7.4'		sand and gravel
P61	Ridgecrest Dr	10	N/A		sand and gravel
P62	Balsam Cir	25	22.2'	8.5'	sand and gravel
P63	Balsam Cir	10		Electric was not	t marked / did not drill
P64	Pinewood Cir	25	6.0'		sand and gravel

SWCOLE EXPLORATIONS, LLC

BORING LOG

BORING NO .:	B-1
SHEET:	1 OF 1
PROJECT NO .:	18-16
DATE START:	6/25/2018
DATE FINISH:	6/25/2018
ELEVATION:	N/A

WATER LEVEL INFORMATION

TROULOT.	11001					
LOCATION:	BREAK	(FAST HILL R	OAD, GREEN	ILAND, NH		
CLIENT:	UNDEF	RWOOD ENGI	INEERS		DRILLER:	JEFF LEE
					-	
		TYPE	SIZE I.D.	HAMMER WT	. HAMMER FALL	
CASING:		HSA	2 1/4"			
SAMPLER:		SS	1 3/8"	140 LB	30"	

CORE BARREL:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT FINE TO COARSE SAND WITH SILT AND GRAVEL 1D 24" 16" 2.0' 11 13 14 12 2.0' FINE TO MEDIUM SAND 2D 24" 22" 7.0' 14 15 20 22 3D 24" 22" 12.0' 21 21 27 35 12.0' -----BOTTOM OF EXPLORATION AT 12.0' SOIL CLASSIFIED BY: SAMPLES: REMARKS: D = SPLIT SPOON C = 2" SHELBY TUBE DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE Х S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-1

EXPLORATIONS, LLC		SWCOL EXPLORATIONS.	E
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BORING LOG

LEE

BORING NO .:	B-2
SHEET:	1 OF 1
PROJECT NO .:	18-16
DATE START:	6/25/2018
DATE FINISH:	6/25/2018
ELEVATION:	N/A

WATER LEVEL INFORMATION WET AT 7.5'

PROJECT:	PROPOSED WATERLINE						
LOCATION:	BREAKFAST HILL ROAD, GREENLAND, NH						
CLIENT:	UNDE	RWOOD ENGI	DRILLER:	JEFF			
					_		
		TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL		
CASING:		HSA	2 1/4"				
SAMPLER:		SS	1 3/8"	140 LB	30"		

CORE BARREL:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT 1D 24" 12" 2.0' 4 6 12 12 FINE TO COARSE SAND AND GRAVEL FINE TO COARSE SAND AND GRAVEL 2D 24" 16" 7.0' 20 28 32 46 3D 17" 10" 11.4' 11 22 50/5" 11.4' FINE SAND AND SILT _____ _ _ _ _ BOTTOM OF EXPLORATION AT 11.4' SOIL CLASSIFIED BY: SAMPLES: REMARKS: D = SPLIT SPOON C = 2" SHELBY TUBE DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE Х S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-2

S.W.COLE
PROPOSED WATERI INE

BORING LOG

BORING NO .:	B-3
SHEET:	1 OF 1
PROJECT NO .:	18-16
DATE START:	6/25/2018
DATE FINISH:	6/25/2018
ELEVATION:	N/A

WATER LEVEL INFORMATION

PROJECT:	PROPO	PROPOSED WATERLINE					
LOCATION:	BREAK	BREAKFAST HILL ROAD, GREENLAND, NH					
CLIENT:	UNDEF	RWOOD ENGI	DRILLER:	JEFF LEE			
		TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL		
CASING:		HSA	2 1/4"				
SAMPLER:	-	SS 1 3/8" 140 LB 30"					

SAMPLER: CORE BARREL:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT 1D 24" 12" 2.0' 16 20 24 31 SAND AND GRAVEL 3.5' AUGER REFUSAL AT 3.5' SOIL CLASSIFIED BY: REMARKS: SAMPLES: D = SPLIT SPOON C = 2" SHELBY TUBE Х DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-3

	S.W.COLE
PROJECT:	PROPOSED WATERLINE

SS

BORING LOG

BORING NO .:	B-4
SHEET:	1 OF 1
PROJECT NO .:	18-16
DATE START:	6/26/2018
DATE FINISH:	6/26/2018
ELEVATION:	N/A

TROOLOT.	THOI OULD WITTEN						
LOCATION:	SUNNYSIDE DRIVE,	GREENLAN	D, NH				
CLIENT:	UNDERWOOD ENG	INEERS	DRILLER:	JEFF LEE			
	TYPE	SIZE I.D.	HAMMER WT. HAMMER FALL				
CASING:	HSA	2 1/4"					

140 LB

30"

1 3/8"

WATER LEVEL INFORMATION

SAMPLER: CORE BARREL:

CASING BLOWS		SAN	IPLE	DEDTH	SAMF	PLER BL	_OWS P	/S PER 6" DE		STRATA & TEST DATA
PER FOOT	NO.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24		
	1D	24"	12"	2.0'	6	8	7	9		
										FINE TO COARSE SAND AND GRAVEL WITH SILT
	2D	12"	10"	6.0'	40	50				
	3D	4"	4"	10.3'	50/4"				10.3'	
										BOTTOM OF EXPLORATION AT 10.3'
				SOIL C	LASSII	FIED B	Y:		REMAR	KS:
D = SPL C = 2" S	HELB	TUBE		Х		LLER -				STRATIFICATION LINES REPRESENT THE
S = 3" S U = 3.5"						L TECH				APPROXIMATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. BORING NO.: B-4

	EXPLORATIONS,	E
PROJECT	: PROPOSED WATERLINE	

BORING LOG

LEE

B-5
1 OF 1
18-16
6/26/2018
6/26/2018
N/A

WATER LEVEL INFORMATION

LOCATION:	STONE MEADOW WAY, GREENLAND, NH					
CLIENT:	UNDER	RWOOD ENGI	DRILLER:	JEFF		
		TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL	
CASING:		HSA	2 1/4"			
SAMPLER:		SS	1 3/8"	140 LB	30"	

SAMPLER: CORE BARREL:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT 1D 24" 12" 2.0' 8 10 10 9 FINE TO COARSE SAND AND GRAVEL WITH SILT 2D 24" 22" 7.0' 11 21 24 31 3D 12" 10" 11.0' 15 50 11.0' -----BOTTOM OF EXPLORATION AT 11.0' SOIL CLASSIFIED BY: SAMPLES: REMARKS: D = SPLIT SPOON C = 2" SHELBY TUBE DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE Х S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-5

	SWCOLE EXPLORATIONS, LLC
PROJECT:	PROPOSED WATERLINE

BORING LOG

B-6
1 OF 1
18-16
6/27/2018
6/27/2018
N/A

PROJECT:	PROPOSED WATERLINE										
LOCATION:	SEPTE	MBER DRIVE	, GREENLAN	ID, NH							
CLIENT:	UNDE	RWOOD ENGI	JEFF LEE								
					-						
CASING:		HSA	2 1/4"								
SAMPLER:		SS	1 3/8"	140 LB	30"						

WATER LEVEL INFORMATION WATER AT 7'

SAMPLER: CORE BARREL:

TA
EL WITH SILT
AT 12.0'
DRING NO.: B-6

S.W.COL	E
EXPLORATIONS,	LLC

PROPOSED WATERLINE

BORING LOG

BORING NO .:	B-7
SHEET:	1 OF 1
PROJECT NO .:	18-16
DATE START:	7/12/2018
DATE FINISH:	7/12/2018
ELEVATION:	N/A

WATER LEVEL INFORMATION

LOCATION:	FALLS WAY, G				
CLIENT:	UNDERWOOD	ENGINEERS	DRILLER:	JEFF LEE	
	TYP	E SIZE I.D.	HAMMER WT	. HAMMER FALL	
CASING:	HS	A 2 1/4"			
SAMPLER:	SS	1 3/8"	140 LB	30"	

CORE BARREL:

PROJECT:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT 1D 24" 12" 2.0' 10 14 10 12 SAND AND GRAVEL 5.0' AUGER REFUSAL AT 5.0' SOIL CLASSIFIED BY: REMARKS: SAMPLES: D = SPLIT SPOON C = 2" SHELBY TUBE DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE Х S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-7

S.W.COL	E
EXPLORATIONS,	LLC

BORING LOG

BORING NO .:	B-8
SHEET:	1 OF 1
PROJECT NO.:	18-16
DATE START:	7/12/2018
DATE FINISH:	7/12/2018
ELEVATION:	N/A

WATER LEVEL INFORMATION WATER AT 6'

PROJECT:	PROPO	OSED WATER	LINE			
LOCATION:	SKYVI	EW DRIVE, G	REENLAND,	NH		
CLIENT:	UNDEF	RWOOD ENGI	DRILLER:	JEFF LEE		
					-	
		TYPE	SIZE I.D.	HAMMER WT.	HAMMER FALL	
CASING:	_	HSA	2 1/4"			
SAMPLER:	-	SS	1 3/8"	140 LB	30"	

CORE BARREL:

CASING SAMPLE SAMPLER BLOWS PER 6" BLOWS DEPTH **STRATA & TEST DATA** DEPTH PER NO. PEN. REC. 0-6 6-12 12-18 18-24 FOOT @ BOT 1D 24" 13" 2.0' 21 22 21 16 FINE TO COARSE SAND AND GRAVEL OLIVE STIFF SILTY CLAY 2D 24" 24" 7.0' 7 9 14 18 3D 24" 12" 12.0' 7 7 7 9 GRAY SILTY CLAY 4D 18" 18" 16.5' 20 38 FINE SAND AND GRAVEL WITH SILT 50 18.0' AUGER REFUSAL AT 18.0' SOIL CLASSIFIED BY: SAMPLES: REMARKS: D = SPLIT SPOON C = 2" SHELBY TUBE DRILLER - VISUALLY STRATIFICATION LINES REPRESENT THE Х S = 3" SHELBY TUBE SOIL TECH. - VISUALLY APPROXIMATE BOUNDARY BETWEEN SOIL TYPES U = 3.5" SHELBY TUBE LABORATORY TEST AND THE TRANSITION MAY BE GRADUAL. BORING NO .: B-8

BORING NO.: P-44-B SHEET: 1 OF 1 PROJECT NO.: 18-16 DATE START: 7/12/2018 DATE FINISH: 7/12/2018 ELEVATION: N/A

PROJECT:	PROPOSED WATERLINE										
LOCATION:	FALLS	ALLS WAY, GREENLAND, NH									
CLIENT:	UNDEF	WOOD ENG	INEERS	DRILLER:	JEFF LEE						
		TYPE	SIZE I.D.	HAMMER WT	. HAMMER FALL						
CASING:	_	HSA	2 1/4"								
SAMPLER:	-	SS	1 3/8"	140 LB	30"						

WATER LEVEL INFORMATION WATER AT 3'

SAMPLER: CORE BARREL:

CASING BLOWS		SAN	IPLE	DEDT	SAM	PLER BI	LOWS P	ER 6"	DEPTH STRATA & TEST DATA			
PER FOOT	NO.	PEN.	REC.	DEPTH @ BOT	0-6	6-12	12-18	18-24				
	1D	24"	14"	7.0'	15	14	11	10		FINE TO COARSE SAND AND GRAVEL		
	20	24"	20"	12.0'	0	9	6	7				
	2D	24	20	12.0	9	9	6	1		GRAY FINE SAND AND SILT		
	3D	24"	18"	17.0'	4	6	8	8		GRAY FINE SAND AND SILT WITH TRACE OF CLAY		
	00	27	10	17.0	-	Ŭ	0	0	18.0'			
										AUGER REFUSAL AT 18.0'		
SAMPLES: SOIL CLASSIFIED BY:							v.					
SAMPLI D = SPL		DON		SUILC	LASSI	LIED R	r:		REMAR	ND:		
C = 2" S				Х			VISUAL			STRATIFICATION LINES REPRESENT THE		
S = 3" S							I VISU			APPROXIMATE BOUNDARY BETWEEN SOIL TYPES		
U = 3.5"	SHEL	SY IUB	E		LAB	ORATO	ORY TE	51		AND THE TRANSITION MAY BE GRADUAL. BORING NO.: P-44-B		

APPENDIX C

I-95 BRIDGE – STRUCTURAL EVALUATION

TIREY & ASSOCIATES, P.C.

CONSULTING STRUCTURAL ENGINEERS

January 7, 2019

Note: See UE files for computations.

Mr. Tim K. Noble Underwood Engineers, Inc. 25 Vaughan Mall, Suite 1 Portsmouth, NH 03801

RE: Breakfast Hill Rd. I-95 Overpass Bridge Evaluation, Greenland, NH

Dear Tim:

At your request, I have reviewed the existing Breakfast Hill Rd. bridge structure which is an overpass of I-95 in Greenland, NH, for the addition of a new 12" nominal diameter forced main pipe beneath the bridge deck, utilizing existing framing which was originally installed to support a 12" insulated water main pipe which was never installed. As part of my review, I made a visit with you to the bridge site on December 6, 2018, to check the constructed conditions against the original design drawings, as well as make observations of the condition of the beams that will be supporting the new pipe, relative to corrosion of the beams.

You have provided us with original design drawings of the bridge, weight information for both the proposed new and originally specified pipe, and pipe insulation systems.

The originally specified 12" nominal pipe with 4" thick insulation and an aluminum jacket is estimated to weigh 103 pounds per linear foot (plf). The proposed new insulated 12" nominal pipe with an HDPE jacket is estimated to weigh 110 plf. The new pipe is proposed to be a 12" ductile iron TR Flex Pipe by US Pipe, with 2" of insulation on the pipe and an HDPE protective jacket. For purposes of evaluating the existing W6x15.5 pipe support beams that are spaced at 5'-0" o/c, we assumed that the new pipe would be located at the same centerline as the originally specified pipe location. In addition to the 12" water main pipe, the W6 beams also support two $3\frac{1}{2}$ " diameter conduits with electrical wires in them. The weight of each conduit with wires was estimated to be 15 plf. These conduits are in place under the bridge deck and are located as shown on the original design drawings.

The analysis of the existing W6x15.5 beam was performed using a yield strength of 36 ksi, typical for the 1971 time period when this bridge was designed. With a fully unbraced top flange, our analysis shows that the W6 beam has forces that are less than 10% of its capacity. The condition of the beams show light surface pitting and very light corrosion, but no corrosion plating formed. Even reducing the capacity of the existing beams by 10% for these light conditions will still leave plenty of capacity in the beams for the slightly higher new pipe loads. If the new pipe is shifted slightly towards the centerline of the bridge deck, there will still be plenty of capacity in the W6 beams to carry the new pipe load.

The two bridge girders that support the beams supporting the pipe and conduit were checked for the very slight increase in dead load from the new pipe, insulation and HDPE jacket. A slab analysis was first performed to determine the distribution of the slab dead loads to each girder, accounting for the

P.O. Box 639, 105 West Main St., Littleton, NH 03561 Tel: 603-444-6211 e-mail: <u>tirey@tireyandassociates.com</u> web: www.tireyandassociates.com

Mr. Tim K. Noble RE: Breakfast Hill Rd. I-95 Overpass Bridge Evaluation, Greenland, NH

continuity of the slab and the cantilevers at each slab edge. Each girder, G1 and G2 referencing the original design drawings, was modeled and dead loads applied to each girder. Our analysis looked at the potential increase in dead load forces only from the original design to the design with the new, slightly heavier insulated pipe. Loads from guardrails, raised concrete walkways, pipe roller supports, pipe support beams, lateral bracing and cross frames for the girders were included in the dead loads applied to the two girders.

Girder G1 was found to have an increase of 0.2% in dead load forces and deflection with the new loads. This increase is insignificant and will be even smaller when dead plus live load cases are considered. Girder G2 was found to have an increase of 0.28% in forces and dead load deflection with the new loads. This increase is insignificant and will be even less when dead plus live load cases are considered. Therefore, based on the insignificant change in force and deflections in these two girders, it is my opinion that the existing girders have sufficient capacity as is to accept the addition of the new pipe as described above.

Please call if you have any questions or we can be of further assistance.

Truly Yours,

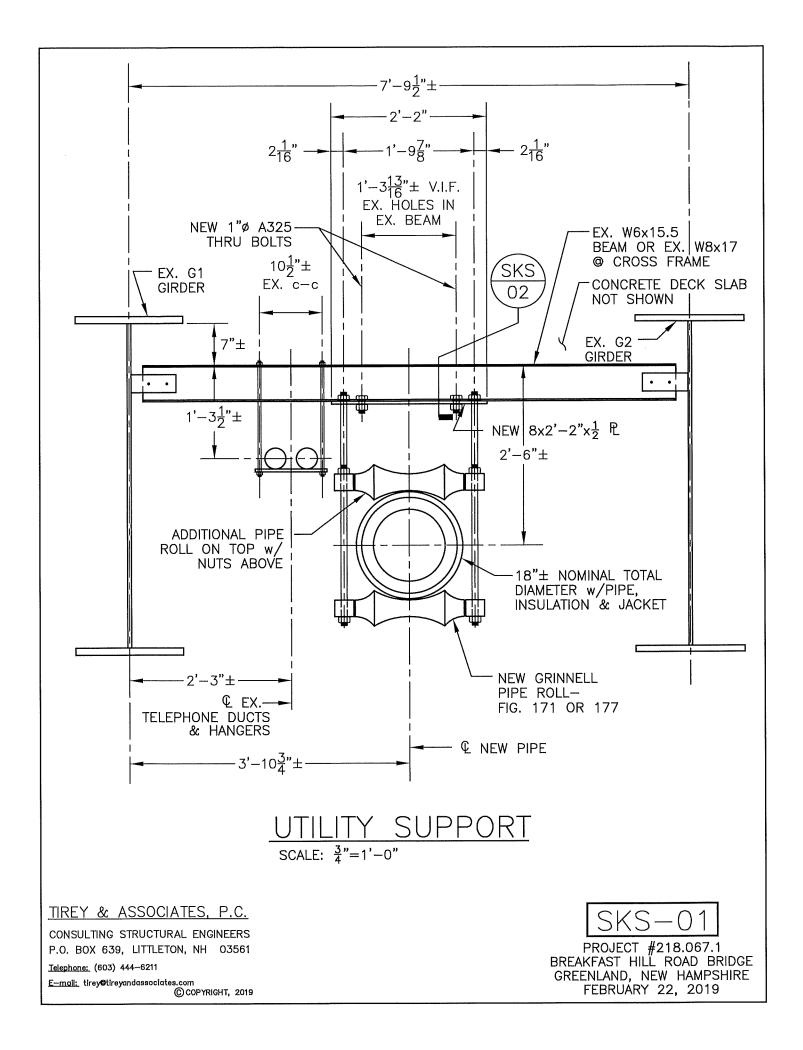
July 1 G

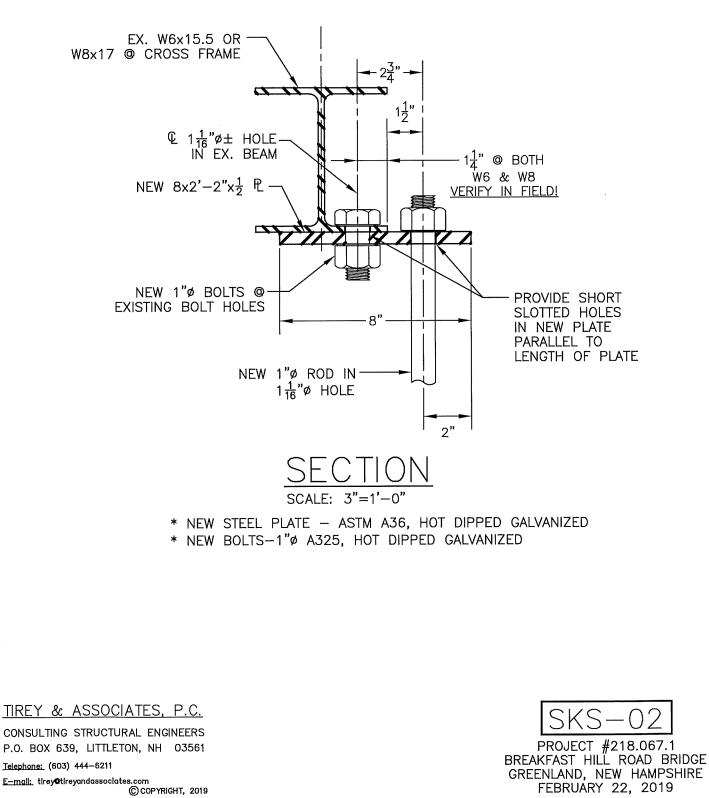
Jeffrey L. Tirey, P.E.

jlt

Enclosure







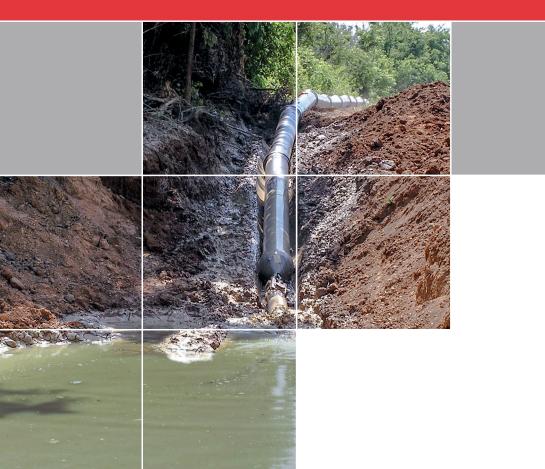
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APPENDIX D

BRIDGE CROSSING DESIGN INFORMATION



TR Flex® Pipe & Fittings DUCTILE IRON



PIPE <

FABRICATION RESTRAINED JOINTS

FITTINGS <

GASKETS COATINGS & LININGS



ANSI/AWWA STANDARDS

ANSI/AWWA C151/A21.51 Ductile-Iron Pipe, Centrifugally Cast, for Water. Asphaltic outside coating and inside lining are in accordance with ANSI/AWWA C151/A21.51.

ANSI/AWWA C153/A21.53 Ductile-Iron Compact Fittings For Water Service or ANSI/ AWWA C110/A21.10 Ductile-Iron and Gray-Iron Fittings, 3 in. Through 48 in., For Water. TR FLEX® Fittings conform to applicable requirements of ANSI/AWWA C153/A21.53 or ANSI/AWWA C110/A21.10.

ANSI/AWWA C104/A21.4 Cement-Mortar Lining for Ductile-Iron Pipe and Fittings for Water.

Cement lining, in accordance with ANSI/AWWA C104 or special linings and/or coatings can be furnished for specific conditions.

ASTM A746-03 Standard Specification for Ductile Iron Gravity Sewer Pipe. ASTM A716-08 Standard Specification for Ductile Culvert Pipe. ASTM A536 Standard Specification for Ductile Iron Castings. ASTM C150 Standard Specification for Portland Cement

TR FLEX PIPE AND FITTINGS PROVIDE FLEXIBLE RESTRAINED PUSH-ON JOINTS FOR 4" THROUGH 36" DUCTILE IRON PIPE AND FITTINGS.

The TR Flex Restrained Joint has a working pressure rating equivalent to the working pressure rating of the parent pipe, with a maximum restrained joint working pressure rating of 350 psi except that 30"–36" have a 100 psi reduction in joint rating for a given parent pipe pressure class e.g. 30–36" TR Flex Restrained Joints are rated for a maximum 250 psi with Pressure Class 350 parent pipe. Conversely, a 24" PC 200 pipe will have a restrained joint rating of 200 psi.

For fire protection service, 4" through 12" pipe and fittings are listed by Underwriters Laboratories for 350 psi and are approved by Factory Mutual for 250 psi.

TR Flex Pipe and Fittings utilize the conventional Tyton Gasket for 4" through 36" sizes.

Ductile Iron locking segments, inserted through a slot (or slots) in the bell face, provide a positive axial lock between the bell interior surface and a retainer weldment on the spigot end of the pipe.

The standard cement-mortar lining supplied in pipe is ASTM C150 Type II. The lining may be supplied in either single or double thickness. Sulfate resistant Type V cement is available on special order.

*For higher pressure ratings, consult your U.S. Pipe Sales Representative.

For a suggested design procedure for the restraint of thrust forces in pressurized, buried Ductile Iron piping systems, the design engineer should refer to the current DIPRA publication "Thrust Restraint Design for Ductile Iron Pipe" or the U.S. Pipe Brochure "The Use and Application of Restrained Joints for Ductile Iron Pipelines."

NOTE: If specifiers or users believe that corrosive soils will be encountered where our products are to be installed, please refer to ANSI/AWWA C105/A21.5 "Polyethylene Encasement for Ductile Iron Pipe Systems" for proper external protection procedures.

For certain wastewater applications a ceramic epoxy lining is available—see PROTECTO 401™ Ceramic Epoxy Lined Pipe and Fittings Brochure.

NOTE: Each of the following is a nationally recognized standards organization: American National Standards Institute (ANSI), American Water Works Association (AWWA), American Society for Testing and Materials (ASTM), Underwriters Laboratories (UL), National Fire Protection Association (NFPA), National Sanitation Foundation (NSF), Factory Mutual (FM)

When joint restraint for a 4" through 36" push-on joint pipe installation is required and indicated in the project plans and specifications, restrained push-on joint pipe and fittings utilizing Ductile Iron components shall be provided.

Restrained joint pipe shall be Ductile Iron manufactured in accordance with the requirements of ANSI/AWWA C151/A21.51. Push-on joints for such pipe shall be in accordance with ANSI/AWWA C111/A21.11 "Rubber-Gasket Joints for Ductile-Iron Pipe and Fittings." Pipe thickness shall be designed in accordance with ANSI/AWWA C150/A21.50 "Thickness Design of Ductile-Iron Pressure Pipe," and shall be based on laying conditions and internal pressures as stated in the project plans and specifications.

Restrained joint fittings and the restraining components shall be Ductile Iron in accordance with applicable requirements of ANSI/AWWA C110/A21.10 and/or C153/A21.53 with the exception of the manufacturer's proprietary design dimensions. Push-on joints for such fittings shall be in accordance with ANSI/AWWA C111/A21.11.

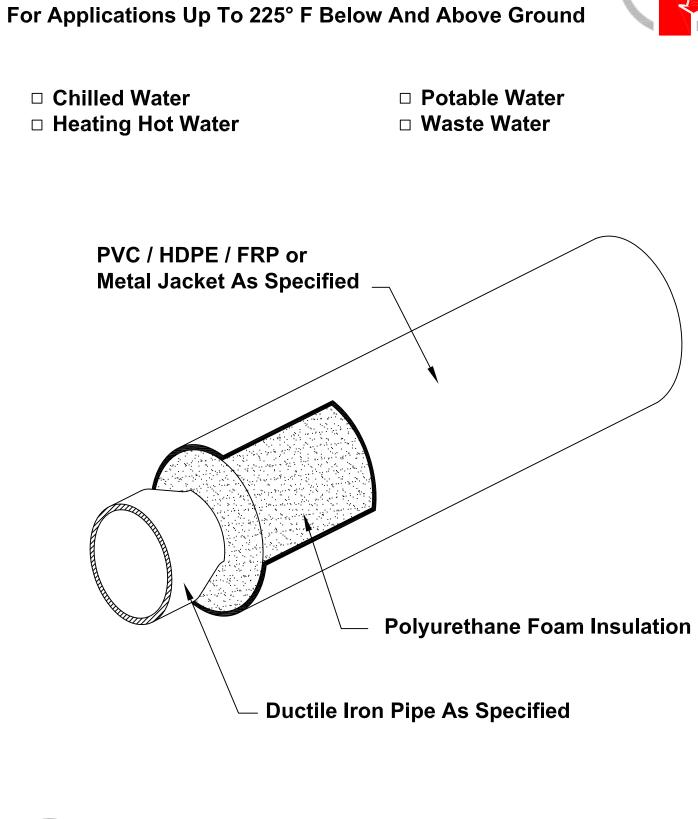
Restrained joint pipe and fittings shall be U.S. Pipe's TR Flex Pipe and Fittings or approved equal. Restraint of field cut pipe shall be provided with U.S. Pipe's TR Flex Gripper Ring, TR Flex Pipe field weldments or approved equal.

Cement mortar lining and seal coating for pipe and fittings, where applicable, shall be in accordance with ANSI/AWWA C104/A21.4. Asphaltic outside coating shall be in accordance with ANSI/AWWA C151/A21.51 for pipe and ANSI/AWWA C110/A21.10 or ANSI/AWWA C153/A21.53 for fittings.

Restrained push-on joints for pipe and fittings shall be designed for a water working pressure of 350 psi for sizes 4" through 24" and 250 psi for sizes 30" through 36".

TRICON DUCTILE IRON PIPE SYSTEM



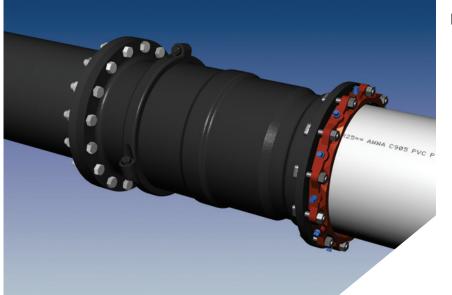




P.O. Box 361, Canastota, New York 13032 Tel: 315.697.8787 Fax: 315.697.8788



EX-TEND® Expansion Joint for Water and Waste Water Pipelines



▲ Series 216C0 EX-TEND, 16 inch combination mechanical joint by flanged expansion joint. (DIP by PVC)
 ▼ Series 216C0 EX-TEND, 16 inch combination mechanical joint by flanged expansion joint.



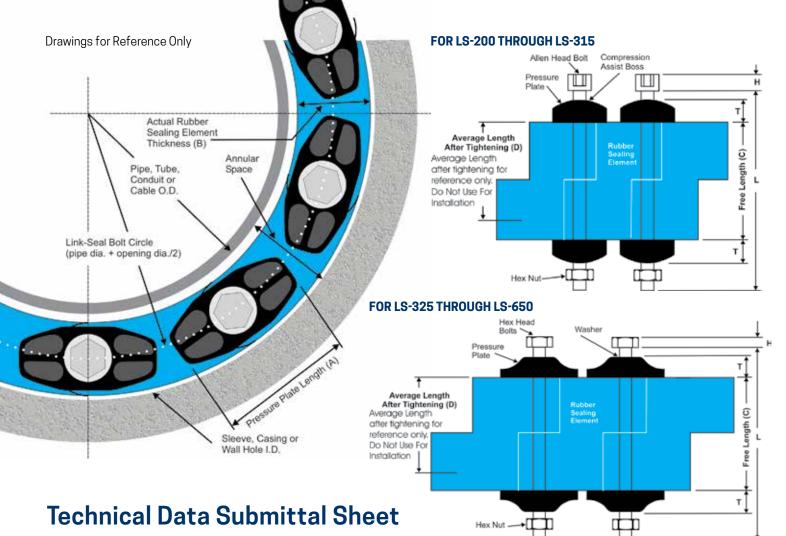
Sample Specification

Expansion joints shall be installed in the locations indicated on the drawings and shall be manufactured of ductile iron conforming to the material properties of ANSI/AWWA C153/A21.53. All expansion joints shall be capable of expanding or contracting to the amounts shown on the drawings, or indicated in the specifications, but in no case shall there be less than 4" total axial movement. Separation beyond the maximum extension of the expansion joint shall be prevented without the use of external tie rods. Each expansion joint shall be pressure tested against its own restraint to a minimum of 350 psi (250 psi 24 inch and greater). MEGALUG joint restraint shall be provided with each mechanical joint connection. All internal surfaces (wetted parts) shall be lined with a minimum of 15 mils of fusion bonded epoxy conforming to the applicable requirements of ANSI/AWWA C213. Exterior surfaces shall be coated with a minimum of 6 mils of fusion bonded epoxy conforming to the applicable requirements of ANSI/AWWA C116/A21.16. Sealing gaskets shall be constructed of EPDM. The coating shall meet ANSI/NSF-61. All expansion joints shall be EX-TEND 200, as manufactured by EBAA Iron, Inc., or approved equal.

Features and Applications:

- Sizes 3 inch through 36 inch
- For Ductile Iron, Steel, PVC or HDPE pipe
- 3 inch through 20 inch rated at 350 PSI 24 inch and greater rated at 250 PSI
- Constructed of ASTM A536 Ductile Iron
- No Periodic Maintenance is Required
- Each unit tested to rated working pressure prior to shipment
- Self restrained at full expansion without the use of external tie bars
- Due to the design of the seals, no periodic maintenance is required
- Seals conform to the applicable requirements of ANSI/AWWA C111/A21.11
- End connections: Flanged Joint; 3 inch through 36 inch Mechanical Joint; 3 inch through 24 inch Combination of the two Joints available
- Flange outlets conform to the dimensional requirements of ANSI/AWWA C110/A21.10 (class 150) with the addition of an O-ring gasket which is provided to ensure a watertight seal
- Mechanical Joint end connections conform to the dimensional requirements of either ANSI/ AWWA C111/A21.11 or ANSI/AWWA C153/ A21.53 depending on size
- All "wetted" parts are coated with a NSF61 approved fusion bonded epoxy
- Insertion of additional sleeves for increased expansion capacity can be done at the factory or in the field as the need occurs

For use on water or wastewater pipelines subject to hydrostatic pressure and tested in accordance with either AWWA C600, C605, or ASTM D2774.



Supplement 11/06/2018

* DIMENSIONAL DATA FOR MODELS C, L, O, S-316, S61, LS-316 & OS-316

Rubber Sealing Elements				Pres Pla	sure tes		Bolts				
LINK- SEAL [®] Model No.	Actual Thinckness (B)	Free Length (C)	Avg. Length After Tighening (D)	(A)	(T)	Allen Head Hex Across Flats	(H)	Thread Size	(L)	for 10 Link Sections (lbs)	Min. Required Seating Width
LS-200-*	0.48"	1.75"	1.38"	1.06"	0.31"	4mm Allen (0.157")	4.95mm (0.195")	M5-0.8	65mm (2.559")	0.70	2.25"
LS-275-*	0.61"	1.75"	1.38"	0.97"	0.31"	4mm Allen (0.157")	4.95mm (0.195")	M5-0.8	65mm (2.559")	0.75	2.25"
LS-300-*	0.69"	2.37"	1.87"	1.56"	0.44"	6mm Allen (0.236")	7.87mm (0.310")	M8-1.25	90mm (3.543")	2.15	3.00"
LS-315-*	0.81"	2.37"	1.87"	1.44"	0.44"	6mm Allen (0.236")	7.87mm (0.310")	M8-1.25	90mm (3.543")	2.30	3.00"
LS-325-*	0.88"	2.63"	2.00"	3.13"	1.00"	13mm (0.511")	5.30mm (0.215")	M8-1.25	110mm (4.33")	5.50	4.00"
LS-340-*	1.00"	2.70"	2.25"	1.48"	0.66"	13mm (0.511")	5.30mm (0.215")	M8-1.25	110mm (4.33")	3.30	4.00"
LS-360-*	1.24"	2.70"	2.25"	2.05"	0.77"	13mm (0.511")	5.30mm (0.215")	M8-1.25	110mm (4.33")	5.10	4.00"
LS-400-*	1.38"	3.50"	2.75"	3.50"	1.06"	17mm (0.669")	6.40mm (0.250")	M10-1.5	130mm (5.118")	12.00	5.00"
LS-410-*	1.43"	3.37"	2.87"	2.52"	0.88"	17mm (0.669")	6.40mm (0.250")	M10-1.5	130mm (5.118")	8.20	5.00"
LS-425-*	1.06"	3.00"	2.25"	3.50"	1.19"	17mm (0.669")	6.40mm (0.250")	M10-1.5	130mm (5.118")	10.00	5.00"
LS-475-*	1.56"	3.38"	2.63"	2.63"	0.88"	17mm (0.669")	6.40mm (0.250")	M10-1.5	130mm (5.118")	10.00	5.00"
LS-500-*	2.25"	3.75"	2.75"	3.63"	1.06"	19mm (0.748")	7.50mm (0.300")	M12-1.75	140mm (5.511")	22.50	5.00"
LS-525-*	2.06"	3.75"	2.87"	3.63"	1.06"	19mm (0.748")	7.50mm (0.300")	M12-1.75	140mm (5.511")	21.00	5.00"
LS-575-*	1.81"	3.75"	3.00"	3.00"	1.00"	19mm (0.748")	7.50mm (0.300")	M12-1.75	140mm (5.511")	15.50	5.00"
LS-615-*	3.09"	4.00"	3.00"	6.00"	1.90"	24mm (0.944")	10.57mm (0.416")	5/8-11	185mm (7.280")	60.60	6.00"
LS-650-*	2.71"	3.98"	3.00"	3.96"	1.19"	19mm (0.748")	7.50mm (0.300")	M12-1.75	140mm (5.511")	26.10	6.00"

GPT an EnPro Industries company

SING SPACERS 5-INSULATORS DB ADVANCE PRODUCTS & SYSTEMS.LLC

ISO 9001:2015 CERTIFIED COMPANY - FM537405

019



Casing Spacers with ApogecAero® Rollers -see page 7 Casing Spacers with

1121039 81130

Casing Spacers with field changeable risers/runners -see page 5

CASING INSULATOR SPECIFICATION

PART#: SSIM-1826-CR-XV

BAND

8" WIDE 14 GA. (.074") 304 STAINLESS STEEL 2-B FINISH RIBBED FLANGES

LINER

PVC LINER (0.096") HARDNESS - DUROMETER "A" 80 +/- 5 OPERATING TEMP: 140 F DIELECTRIC STRENGTH - 60,000 V

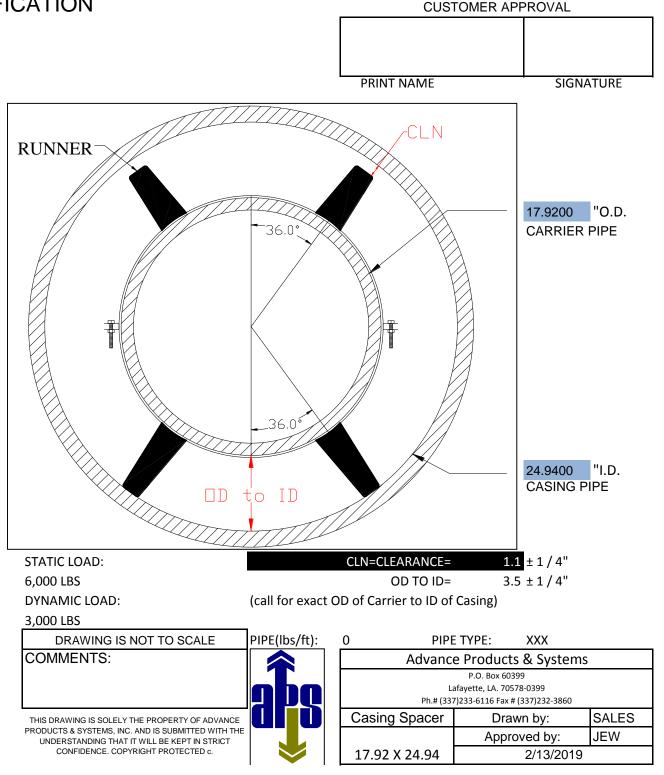
RUNNER

2" WIDE X 2" HIGH MOLDED POLYMER-MOD 2 TOP

2" WIDE X 3 1/2" HIGH MOLDED POLYMER-MOD 2 BOTTOM

ASSEMBLY HARDWARE

3 SETS PER FLANGE 1/4" - 20 X 2" STAINLESS STEEL BOLTS 1/4" STAINLESS STEEL WASHERS 1/4" STAINLESS STEEL NUTS



PIPE ROLLS & SADDLES



Project: Address:

Contractor:

Engineer:

Notes 1: Notes 2:

Submittal Date:

Size Range: 1" through 30" Material: Cast iron roll and sockets, steel roll rod

Finish: Plain, Zinc Plated (Hot-Dip Galvanized optional) or Resilient Coated **Service:** For suspension of pipe from two rods where longitudinal expansion and contraction may occur. **Approvals:** Complies with Federal Specification A-A-1192A (Type 41), WW-H-171-E (Type 42),

ANSI/MSS SP-69 and MSS SP-58 (Type 41).

Adjustment: Adjustable socket permits vertical adjustment at the roll. **Maximum Temperature:** 400° F at roller, 300° F at resilient coated roller. **How to size:**

- (1) If the roll is to support non-insulated pipe, select the size directly from nominal pipe size (column 1) in table below.
- (2) If used with pipe covering protection saddle, see Figure 160 to Figure 166A for size of pipe roll.

Features:

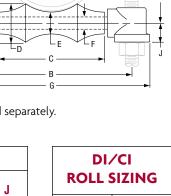
- Provides for vertical adjustment; nut at bottom of hanger rod fits into the socket preventing loosening or turning due to vibration.
- Pipe roll is designed for two point surface contact with pipe or saddle.
- Features: Advantages of pipe rollers with a protective resilient coated covering.
- Non conductive pipe rollers prevent the passing of current from pipeline to structure.
- Corrosion resistant for protection against severe weather conditions, moderate corrosive conditions such as marine atmospheres and weather resistant to ultra-violet radiation.
- Low coefficient of friction between pipe and resilient coated pipe roller.

Ordering:

- Specify pipe roll size.
- Order should include figure number, name and finish in all cases. Hanger rods and nuts to be ordered separately.
- Be certain to order oversized rolls when insulation and protection saddles makes this necessary.

	FIG. 171: DIMENSIONS (IN) • LOADS (LBS) • WEIGHT (LBS)											
Pipe Size	Max O.D. Covering	Rod Size A	Max Load	Weight	G	В	C	D	E	F	н	J
1	2			0.45	4 ¹ / ₈	3	1 ½	1			1 ¹ / ₁₆	
1¼	2 ¹ / ₂	3/8	600	0.48	4 ¹ / ₂	3 ¾	11/8	1 ¹ ⁄16	3/4	3/8	11/4	9⁄16
1 ½	2 ³ ⁄ ₄	78	000	0.51	4 ³ ⁄ ₄	35⁄/8	2 1⁄/8	11/8	74	78	13%	716
2	3 ¹ /4			0.57	5¼	41⁄8	25⁄%	1 ³ ⁄16			15%	
2 ¹ / ₂	33⁄4		660	1.00	61⁄4	41/8	31/8	13%	7/8		1 ¹⁵ ⁄16	11/16
3	4 ¹ / ₂	1/2	700	1.10	61/8	5½	3 ³ ⁄4	17/16	78	1/2	2 ¹ /4	716
3 ½	5			1.40	7 ½	6½	4 ¹ / ₄	15%	1	/2	2 ⁹ ⁄16	3/4
4	5 ¹ / ₂	5/8	750	1.70	8 ¹ / ₄	61/8	4 ³ ⁄ ₄	13⁄4			2 ¹³ /16	-
5	7	78		2.60	9 ¹¹ / ₁₆	8 ¹ / ₁₆	5 ¹³ ⁄16	2	1½	5⁄8	3 ⁷ / ₁₆	7/8
6	8 ¹ / ₄	3/4	1,070	4.50	11 ⁷ ⁄16	9 %16	61/8	2 ⁵ ⁄16	11/4	3⁄4	4	1
8	101/2	74	1,350	7.20	14 ¹ / ₁₆	11 ¹⁵ ⁄16	81/8	2 ¹³ /16	11/2	7/8	5½	1 1/8
10	12 ³ ⁄4	7/8	1,730	9.50	16 ³ ⁄16	14 ¹ / ₁₆	11	3 3⁄8	13⁄4	-78	6 ³ / ₈	178
12	14 ³ ⁄4	/8	2,400	15.90	17 ¹⁵ ⁄16	15 ¹³ ⁄16	12 ¹ /2	31/8	2	1	7 ⁷ /16	1 ¹ /4
14	16 ¹ ⁄4		3,130	24.30	201//8	17¾	14¼	45/8	2 ¹ / ₂	1 ½	8 ³ / ₈	13/8
16	18	1	3,970	31.90	22 ¹ /8	19¾	16¼	5	25/8		9 ⁷ / ₁₆	1 ½
18	201/4		4,200	35.50	24 ¹ / ₂	21 ⁷ / ₈	18¼	5 ⁷ ⁄16	2 ³ / ₄	11/4	101/2	172
20	22 ¹ / ₂	1 ¹ ⁄ ₄	4,550	47.00	27 ¹ / ₄	24¼	20 ¹ /4	6	3		115%	15%
24	26 ¹ / ₂	1 ½	6,160	76.30	32 ¹ / ₈	28 ⁷ / ₈	24 ¹ /4	7 ³ ⁄16	35⁄8	1 ¹ / ₂	14	1 ³ ⁄4
30	32 ¹ / ₂	172	7,290	129.90	39	35½	30 ¹ ⁄ ₄	8 ¹⁵ / ₁₆	4 ¹ / ₂	1 ³ ⁄4	17 ⁷ /16	2 ⁷ / ₁₆

PROJECT INFORMATION



DI/CI

Pipe Size

3

4

6

8 10

12

14 16

18

20

24 30

APPROVAL STAMP

Approved

Remarks:

Approved as noted

Not approved

Fig. 171

Roller

Size

4

5

6 8

10

14 16

18

20

24 30

No Recom.



æ



Single Pipe Roll

PIPE ROLLS & SADDLES

Fig. 177

Notes 2:

Submittal Date:

PH-8.16

Project: Address:

Contractor:

Engineer:

Notes 1:

Size Range: 1" through 30"

Material: Cast iron roll and sockets; steel roll rod, continuous thread rods and hex nuts Finish: 🗋 Plain, 🗋 Zinc Plated (Hot-Dip Galvanized optional) or 🗋 Resilient Coated Service: For support of pipe where longitudinal movement due to expansion and contraction will occur and where vertical adjustment up to 6" may be necessary. Maximum Temperature: 400° F at roller, 300° F at Resilent coated roller.

Approvals: Complies with Federal Specification A-A-1192A (Type 41), WW-H-171-E (Type 42), ANSI/MSS SP-69 and MSS SP-58 (Type 41).

Installation: Normally used directly above steel beams, brackets angles, etc. Features: Advantages of pipe rollers with a protective resilient coated covering.

- Non conductive pipe rollers prevent the passing of current from pipeline to structure. •
- Corrosion resistant for protection against severe weather conditions, moderate corrosive conditions such as marine atmospheres and weather resistant to ultra-violet radiation.
- Low coefficient of friction between pipe and resilient coated pipe roller.

PROJECT INFORMATION

How to size:

- (1) If roll is to support bare pipe, select the size directly from nominal pipe size (see below).
- (2) If used with pipe covering protection saddle, see Figure 160 to Figure 166A for size of pipe roll.
- (3) If roll is to support covered pipe, the O.D. of the covering should not be greater than the O.D. of the pipe for which the roll was designed.

Ordering: Specify size of roll, figure number and name. Be certain to order oversized rolls when insulation and protection saddles makes this necessary.

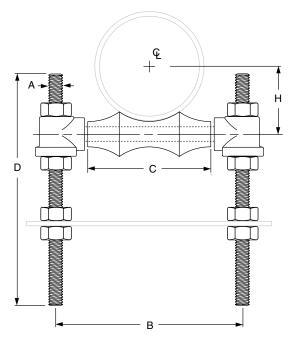
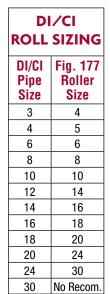


FIG. 177: DIMENSIONS (IN) • WEIGHT (LBS)									
Pipe Size	Weight	A	В	C	D	Н			
1	1.1		3	11/2		1 ¹ / ₁₆			
1¼	1.2	3/8	33/8	11 %]	11/4			
1½	1.2	78	35/8	2 ¹ / ₈]	1¾			
2	1.3		4 ¹ / ₈	25⁄8]	15%			
2 ¹ / ₂	2.3	1/2 5%	47⁄8	3 ¹ / ₈	12	1 ¹⁵ ⁄16			
3	2.4		5 ¹ /2	3 ³ ⁄4		2 ¹ /4			
3 ½	2.7		6 ¹ / ₈	4 ¹ / ₄		2 %16			
4	3.8		67⁄8	4 ³ ⁄ ₄		2 ¹³ /16			
5	4.7	78	8 ¹ / ₁₆	5 ¹³ ⁄16		37/16			
6	7.6	3/4	9 ⁹ ⁄16	67⁄8		4			
8	11.0	74	11 ¹⁵ ⁄16	87⁄8		51/%			
10	13.7	7/8	14 ¹ / ₁₆	11		6 3%			
12	19.4	78	15 ¹³ /16	12½		7 ⁷ /16			
14	31.2		17¾	14¼		8 3%			
16	42.5	1	19 ³ ⁄4	16¼		9 ⁷ / ₁₆			
18	46.6		21 %	18¼	18	10½			
20	66.2	1 ¹ ⁄4	241/4	201⁄4		11%			
24	102.5	1 ½	287/8	24 ¹ / ₄	24	14			
30	186.8	1 /2	35 ¹ /2	30¼	24	17 ⁷ /16			



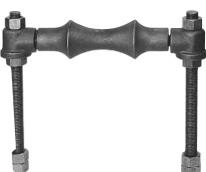
APPROVAL STAMP

Approved

Remarks:

Approved as noted

Not approved





Adjustable Pipe Roll Support

APPENDIX E

GREENLAND WELL PUMP REPLACEMENT INFORMATION

Existing Pump Information

Shop Drawing Transmittal						Weston & Sampson					
Instruction for Preparing Transmittal No action will be taken on any item unless accompanied by this form. TRANSMITTAL NOS. to be consecutive (1, 2, 3, etc.). Each resubmittal of same item shall use same number with suffix letter SPEC. SECT. NO: Only one spec. section no. to each transmittal. DESCRIPTION: Complete identification of document or group of docu SOURCE: Originator of document(s) being submitted.				tter (A		details of docu SPECIAL INS distribution and	DRAWING Iment(s). TRUCTION d special h DF CONTF	REFEREN NS: Special andling rec RACTOR: \$	CE: Contract d I cases and em quests, etc. sho Signature of inc	ergencies, cl ould be entere	nanges in ed here.
			THIS SECTIO	N TC	BE COMP	LETED BY CON	NTRACTO	R			
TRAN	SM. NO.	SPEC. SECT. NO.	DATE	CO	NTRACTOF	RS JOB NO.	W&S JO	B NO.			
30		11210	6/27/2017	T-1	638						
PROJ	ECT NAME & CONTR	ACT NO.		LOO	CATION						
Green	land Well Replacem	ent		Por	tsmouth, N	ΙH					
T W O 5	ttention: CSD (<u>Davida(</u> /eston & Sampson Eng Centennial Drive eabody, MA 01960			F R O M	R O						
	1							1		B	′ W&S
ITEM NO.		DESCRIPTION			SOURCE	DRAWIN CATALO BROUCHUI	G NO.	NO. OF COPIES	CONTRACT DRAWING REF.	ACTION CODE	REVIEWED BY
1	Vertical Turbine P	ump Test		Ma	her			1			
2											
3											
4											
CONFO	THIS CERTIFIES THAT ALL ITEMS SUBMITTED HEREWITH HAVE BEEN CHECKED BY THE CONTRACTOR, ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, EXCEPT AS NOTED, AND ARE APPROVED BY THE CONTRACTOR FOR THIS PROJECT. SIGNATURE & TITLE										
			THIS SE	CTIC	N TO BE C	OMPLETED BY	′ W&S				
1. NO E 2. MAKE 3. AMEN 4. REJE 5. ACKN	ACTION CODE: 1. NO EXCEPTIONS TAKEN a. INSTALLATION SHALL PROCEED ONLY WHEN ACTION CODE IS 1 OR 2 2. MAKE CORRECTIONS NOTED b. ACTION CODED 3 SHALL BE RESUBMITTED WITHIN TIME LIMIT SET IN CONTRACT 3. AMEND AND RESUBMIT c. REVIEW DOES NOT RELIEVE CONTRACTOR FROM RESPONSIBILITY OF COMPLIANCE WITH 4. REJECTED- SEE REMARKS ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS 5. ACKNOWLEDGEMENT 6. SUBMITTAL NOT REQUIRED, RETURNED WITHOUT REVIEW										

submittal cover pg

	Submitta	l Certification	
	302B Turn Phone	Construction Auburn Road er, ME 04282 e: 207-783-6223 207-783-3970	
Project:	Greenland Well PS Replace	ment Project No:	
Date:	27-Jun-17	Engineer:	David A.
Transmittal #:	30	Submittal #:	1B
Spec. Section No:	11210	Titled:	Vertical Turbine Pum
Sub-contractor/Supplier Manufacturer:	Maher Services		Pump Test
	eeds the project specificatio No Deviations A complete list of deviation		
	General Contractor's Centration of the submittal of the s	ined the enclosed submittal(s field measurements, constru- ers and similar data, coordina issions and the work of other the best of my knowledge a is/are in full compliance with	ction ated r nd
	T. Buck Co	nstruction, Inc.	
	Travis	Saucier	
	Sig	nature	
Comments			



71 Concord Street North Reading, MA 01864 Tel: 978-664-WELL (9355) Fax: 978-664-9356 www.maherserv.com

6/23/2017

Mark McPheters T. Buck Construction 302B Auburn Road Turner, Maine 04282 Phone 207-783-6223 ext 203 Fax 207-783-3970 Cell 207-754-8999 Email: mark@tbuckcon.net

Proposal No.: MS-2911

naker@maherserv.com

4

!

RE: Greenland Well Pump Station Replacement Portsmouth, NH

Dear Mark,

Attached are the results of the factory pump test. The pump will not ship until the curve is approved.

If you need further information please contact us. ER SERVICES, INC.

Experience & Reliability " - The Third Generation of Mahars Servicing New England's Valer Acads Since (241 "



•

TESTING SERVICES

CO#: 4423323

CUSTOMER: MAHER SERVICES

PUMP TYPE: DWT-DT

BOWL MODEL: 9RCLC

PREPARED BY: Casey Kinman

DATE:

6/22/2017



4423323 6/22/2017	CO #: Test Date:	/HI 14.6-2B	nce Standard: ANS Project: 0	Accepta		HER SERVICES	Customer: MA PO #: 116
			st Conditions	mance Te	Bowl Perfor		
1770	Speed (rpm):	Required	1.00	ific Gravity:	Spec	1	Pump #:
450	pacity (gpm):	Required Ca	1.0	-	Kinematic Vis	DWT-DT	Pump Type:
195.0	lowl TDH (ft):	Required E	70	r Temp. (°F)	Wate	9RCLC	Bowl Model:
84%	fficiency (%):		4-pole 200hp	Test Driver:		5	Stages:
300psig (Q4021	ssure Trans.:	Pre	J01BF82AM	Driver S/N:		1203	Impeller Material:
0K in lb (Q4010	orque Meter: 1	Т	30.0	Power (hp):	NOL	6911	Bowl Material:
8in (Q3012	Flow Meter:		5	st Imp. Qty.:	1	6.88	1st imp. Dia. (in):
Adrian Mindieta	Tested By:			d Imp. Qty.:	21		2nd Imp. Dia. (in):
-	•	-	-	ings Durir			
140.7	6 188.6	5	4	3	2	1	POINTS
140.	188.6	198.4	222.6	247.5	262.3	285.4	Discharge Head (ft)
0.23	0.15	5.0	5.0	5.0	5.0	5.0	Gauge Elevation (ft)
0.04	0.13	0.13 0.02	0.09	0.04	0.01	0.00	Velocity Head (ft)
12.0	9.69		0.02	0.01	0.00	0.00	Friction Head (ft)
0.0	9.09	9.16	7.65	4.91	2.48	0.02	Expansion Head (ft)
0.0	0.00	0.00	0.00	0.00	0.00	0.00	Miscellaneous Head (ft)
0.0	0.0	0	0	0	0	0	Input Voltage (V)*
0.0	0.0	0.0	0.0	0.0	0.0	0.0	Input Current (amp)*
0.	0.0	0.0	0.0	0.0	0.0	0.0	Input Elec. Power (kW)*
			for reference only.	r readings are	* electrical powe		
170			nning Speed	ded at Ru	Data Reco		
179	1797	1797	1797	1798	1798	1798	Speed (rpm)
58	468	442	369	237	120	1	Capacity (gpm)
158. 27.	203.4	212.8	235.4	257.5	269.8	290.4	Bowl TDH (ft)
85.2%	26.7	26.4	25.0	20.0	16.8	15.9	Bowl Shaft Power (hp)
	90.0%	89.9%	87.8%	76.8%	48.6%	0.5%	Bowl Efficiency (%)
		l	tomer's Speed	ted to Cus	Data Conver		
177	1770	1770	1770.	1770	1770	1770	Speed (rpm)
57	460	435	363	233	118	1	Capacity (gpm)
153.	197.4	206.4	228.3	249.5	261.5	281.4	Bowl TDH (ft)
25.	25.5	25.2	23.9	19.1	16.0	15.1	Bowl Power (hp)
85.29	90.0%	89.9%	87.8%	76.8%	48.6%	0.5%	Bowl Efficiency (%)
	s	Witnes			Paquilto	0.00	
	-	, Thures	By:			Certified Test	Due
						ENGINE	By: Title:
-003 Rev: 02/02/201			Date:			June 22, 2	Date:

Acceptance Standard: ANSI/HI 14.6-2B CO #: 4423323 **Customer: MAHER SERVICES** Project: 0 Test Date: 6/22/2017 PO #: 11653 100% 350 Effic nc 90% 300 Head - - ---80% 250 70% Efficiency (%) 60% 200 Total Head (ft) 50% 150 40% 100 30% 50 20% 10% 0 500 600 200 300 400 100 0 Capacity (gpm) 35 30 25 Power (hp) 20 Hp 15 10 5 0 300 400 500 600 100 200 0 Capacity (gpm) Pump#: 1 1770 Required Speed (rpm): Bowl Model: 9RCLC 450 Required Capacity (gpm): Stages: 5 Required Bowl TDH (ft): 195 6.88 1st Imp. Dia.: Required Efficiency (%): 84% 2nd Imp. Dia.: 450 Required Flow (gpm): 201 Design Head (ft): Design Efficiency (%): 90 Witness Certified Test Results 10 By: By: ENGINEER Title: Date: 6/22/2017 Date:

GOULDS WATER TECHNOLOGY

a <mark>xylem</mark> brand

EN.F007 Rev: 02/02/2017



Customer: MA PO: 116	HER SERVICES	Acceptance	Standard: AN Project: 0	on the the	0: 4423323 9: 6/22/2017
Pump #: Pump Type: Bowl Model: Stages:	1 DWT-DT 9RCLC 5	CONDITION Specific Gravity: Shut-Off TDH (ft): Pressure Guage: 2000p	1.00 268	Required Speed (rpm Required Capacity (gpm Required Bowl TDH (f Tested B): 450

Goulds Water Technology Texas Turbine Operations a division of Xylem Inc certifies that the parts or assemblies listed below have been hydrostatically tested to the pressures and length of time indicated.

TEST READINGS									
Description	Part Number	Material	Quantity	Pressure	Time				
Top Bowl	C03254B01	6911	1	158.0 psi	10.0 Minutes				
Int Bowl:	A03902B01	0	4	158.0 psi	10.0 Minutes				

	Certified Test Results	Witness
By:	Color	Ву:
Title:	ENGIŇEER	
Date:	June 22, 2017	Date:

EN.F010 Rev: 02/02/2017

Recommended Pump Information

Scope of Supply



		Cappiy	
Customer	Premier Pump & Supply, Inc.	Sulzer Reference ID	USA.9934-XND.18.0051-P0
Project Name	UE - Greenland, NH	Inquiry Date	11/30/2018
Inquiry Number/ID		Bid Submitted Date	
Item number	Replacement Pump	Date last saved	11/30/2018 2:07 PM
Application	MUW Water Supply & Distribution - 4606	Type / Size / Stages	JTS-9LC / 7
Pump speed	1780 rpm		

Pump Total

Qty Description JTS-9LC 7 stage(s) 1 Well pump Head measured at CL discharge Product Lube TPL: 49.99 ft **Bowl Assembly** BOWL ASSEMBLY BOWL ASSEMBLY MODEL: JTS-9LC Material: Cast Iron [A48 CL. 30] w/ Coating Impeller Material: 410 Stainless Steel [A743-CA6NM] Impeller Wear Ring: Alum Bronze [B148 Alloy C95400] Stationary Wear Ring: Ni. Al. Bronze [B148 Alloy C95500] Pump Shaft Material: 12% Chrome [A582 Type 416 Cond. A] Bowl Bolting: [316SS A276-TP-316] Bolts Bowl Bearing: Bronze [B584 Alloy C87300] Suction Bell: Trimmed Bell Cast Iron [A48 CL. 30] Suction Strainer: None Bowl Coating: Catalyst Cured Epoxy:Carboline 891 Column Assembly COLUMN ASSEMBLY COLUMN ASSEMBLY: Length 44.3 ft. **Column Selection** Column Selection: Column Selected 60 In. (Qty 2 per pump) Column Selection: Column Selected 120 In. (Qty 4 per pump) Max Bearing Spacing: 120 in. Column Diameter: 6 in. Column Pipe Material: Carbon Steel [A36 and A53 Gr. B] Column Connection Type: Threaded Lineshaft Lubrication: Product Lube Lineshaft Diameter: 1.25 in. Lineshaft Bearing Material: Rubber Lineshaft Material: 12% Chrome [A582 Type 416 Cond. A] Column Coating: Catalyst Cured Epoxy:Carboline 891 **Discharge Head Assembly** DISCHARGE ASSEMBLY DISCHARGE HEAD ASSEMBLY: Fab-150# Carbon Steel [A36 and A53 Gr. B] Discharge Nozzle Diameter: 6 in. Base Type: Round Base Base Diameter: 16.5 in. Separate Motor Stand: None Discharge Head Bolting: [316SS A276-TP-316] Bolts Head Shaft Couplings: Threaded Coupling Seal Selection: John Crane Type 5610 Silicon Carb. Vs Silicon Carb. Seal Plans: None Discharge Head Coating: Catalyst Cured Epoxy:Carboline 891-Inside & Outside Diameter Sole Plate Type: Carbon Steel [A36 and A53 Gr. B] Soleplate: Soleplate 20"x20" Carbon Steel [A36 and A53 Gr. B]



Pump TotalQtyDescription

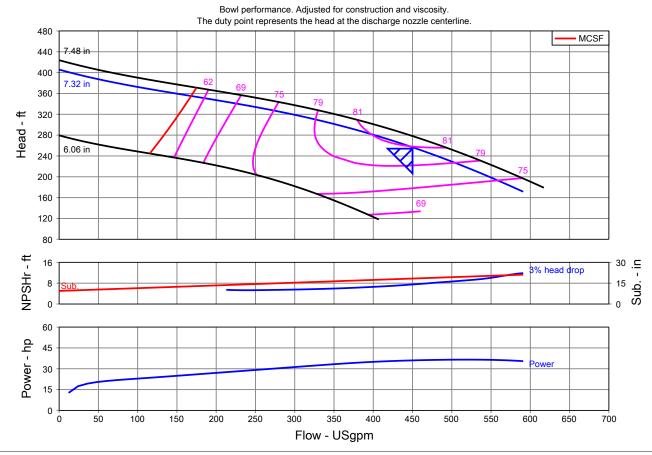
Description
Sole Plate Coatings: Catalyst Cured Epoxy:Carboline 891
Buyout Components
Driver Selection
Motor Frame: Motor Frame: L324TP16
Driver Base Diameter: 16.5 in. Base Diameter
Testing, Quality Inspections, Engineering Analysis
Product Testing
Non Witnessed Tests (Performance, Pump and Hydro)
Non Witnessed Tests (Performance, Pump and Hydro): Pump Performance Test
Transport and Packing
Transportation: Not included
Packaging: Domestic Packaging

Driver

Qty	Description
1	Buyout Components
	Driver Selection
	Motor manufacturer: VHS NEMA Motor - User Defined
	Selected Motor And Price: User Defined Motor
	Driver Manufacturer: [Error: No Description]: GE Motors
	Driver Efficiency: [Error: No Description]: Premium
	Driver Shaft Type: Hollow shaft
	Driver Power: 40 HP
	Driver Speed: : 1780 rpm
	Driver Voltage : : 460 V
	Driver Weight : : 690.0 lb
	Driver Height : : 32.58 in
	Additional Driver Data: [Error: No Description]: AEGIS Shaft Grounding Ring, Non-Reverse Ratchet & Steady Bushing



		Pump Perform	ance Datasheet	
Customer	: Premier Pump & Sup	oply, Inc.	Sulzer Reference ID	: USA.9934-XND.18.0051-P0
Inquiry Number/ID	:		Type / Size	: JTS-9LC
Item number	: Replacement Pump		Stages	:7
Service	:		Based on curve number	: JTS-845-000-61-11-10 Rev JTS-9LC
Quantity	:1		Date of Last Update	: 11/30/2018 2:07 PM
	Operating Condition	ıs	L	_iquid
Flow, rated		: 450.0 USgpm	Liquid type	: Water
Differential head / pre	ssure, rated (requested)	: 254.0 ft	Additional liquid description	:
Suction pressure, rate	ed / max	: 0.00 / 0.00 psi.g	Solids diameter, max	: 0.00 in
NPSH available, rated	b	: Ample	Solids concentration, by volume	: 0.00 %
Frequency		: 60 Hz	Temperature, rated / max	: 68.00 / 68.00 deg F
	Performance		Fluid density, rated / max	: 1.000 / 1.000 SG
Speed, rated		: 1780 rpm	Viscosity, rated	: 1.00 cP
Impeller diameter, rat	ed	: 7.32 in	Vapor pressure, rated	: 0.34 psi.a
Impeller diameter, ma	aximum	: 7.48 in	M	aterial
Impeller diameter, mit	nimum	: 6.06 in	Material selected	: I-1 CI Bowl/SS Impeller
Efficiency (bowl / pum	np)	: 80.94 / 78.91 %	Pres	sure Data
NPSH (3% head drop) / margin required	: 7.51 / 3.00 ft	Maximum casing/bowl working pres	ssure : See the Additional Data page
Submergence, minim	um required	: 18.34 in	Maximum allowable working pressu	ure : See the Additional Data page
nq (imp. eye flow) / S	(imp. eye flow)	: 45 / 163 Metric units	Maximum allowable suction pressu	re : N/A
MCSF		: 168.1 USgpm	Hydrostatic test pressure	: See the Additional Data page
Head, maximum, rate	d diameter	: 405.8 ft	Driver & Power	Data (@Max density)
Head rise to shutoff (I	oowl / pump)	: 58.08 / 59.55 %	Driver sizing specification	: Maximum power
Flow, best eff. point (bowl / pump) : 4		: 421.4 / 415.5 USgpm	Margin over specification	: 0.00 %
Flow ratio, rated / BEP (bowl / pump) : 106.78 / 108.30 %		: 106.78 / 108.30 %	Service factor	: 1.15
Diameter ratio (rated / max) : 97.89 %		Power, hydraulic	: 29.16 hp	
Head ratio (rated dia	,	: 92.26 %	Power (bowl / pump)	: 36.02 / 36.61 hp
Cq/Ch/Ce/Cn [ANSI/	HI 9.6.7-2010]	: 1.00 / 1.00 / 1.00 / 1.00	Power, maximum, rated diameter	: 37.16 hp
Selection status		: Acceptable	Minimum recommended motor ratir	ng : 40.00 hp / 29.83 kW





		um <u>p Perfor</u>	mance - Addi	tional Data			
Customer	: Premier Pump & Supp		Sulzer Reference ID		: USA.9934-XND.18.0051-P0		
Inquiry Number/ID	:		Type / Size		: JTS-9LC		
Item number	: Replacement Pump		Stages		:7		
Service	: '		Speed, rate	ed	: 1780 rpm		
Quantity : 1			Intellicode		:		
			Date of Last Update		: 11/30/2018 2:07 PM		
	Performance Data		Stage, Speed and Solids Limits				
Head, maximum diameter, rated flow : 278.2 ft			Stages, maximum : 25				
		: 87.64 ft	Stages, minimum		: 1		
Head, maximum, rated diameter		: 405.8 ft	Pump speed limit, maximum		: 3600 rpm		
Efficiency adjustment factor, total		: 0.97	Pump speed limit, minimum		: 900 rpm		
Power adjustment, total		: 0.99 hp	Curve speed limit, maximum		: 3600 rpm		
Head adjustment factor, total		: 1.00	Curve speed limit, minimum		: 200 rpm		
Flow adjustment facto	•	: 1.00	Variable speed limit, minimum		: -		
	or, efficiency only (shift	: 1.00	Solids diameter limit		: 0.50 in		
BEP)	on, onloiding only (onlic		Typical Driver Data				
Flow adjustment factor	or, end-of-curve only, total	: 1.00	Driver speed, full load		: 1780 rpm		
MCSF adjustment fac	ctor	: 1.00	Driver speed, rated load		: 1782 rpm		
Flow max adjustment factor		: 1.00	Driver efficiency, 100% load		: N/A		
NPSHR adjustment factor, total		: 1.00	Driver efficiency, 75% load		: N/A		
NPSHR slope correct	tion factor	: 1.00	Driver efficiency, 50% load		: N/A		
User applied perform	ance adjustment comment	s:		-,,_,,			
NPSH margin dictate		: 3.00 ft					
NPSH margin dictate	d by user	: 0.00 ft					
0	added to 'required' values)	: 3.00 ft					
5 (Mechanical Limits						
Torque, rated power,	rated speed	: 2.06 hp/100 rpr	n				
Torque, maximum po		: 2.09 hp/100 rpr					
		: 2.25 hp/100 rpr					
		: 2.25 hp/100 rpr					
		: 11.80 hp/100 rp					
Radial load, worst ca		:-					
Radial load, worst ca Radial load limit	se	:-					
Radial load, worst ca Radial load limit Impeller peripheral sp	se beed, rated	:- :- :-					
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp	se beed, rated beed limit	:- :- :- :-	Head (ft)	Efficiency (%)	NPSHr (ft)	Power (hp)	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe	se beed, rated beed limit erformance Data	: - : - : - Flow (USgpm)	Head (ft)	Efficiency (%)	NPSHr (ft)	Power (hp)	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet	se beed, rated beed limit erformance Data er	: - : - : - Flow (USgpm) 0.00	405.8	Efficiency (%)	NPSHr (ft)	-	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia	se beed, rated beed limit erformance Data er	: - : - : - Flow (USgpm) 0.00 0.00	405.8 423.8	-	-	- 490	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF	se beed, rated beed limit erformance Data er ameter	: - : - : - Flow (USgpm) 0.00 0.00 168.1	405.8 423.8 355.1	58.66	NPSHr (ft) - 7.26	- 490 25.69	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum	se beed, rated beed limit erformance Data er ameter diameter	: - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0	405.8 423.8 355.1 87.64	- - 58.66 63.63	-	- 490 25.69 15.65	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum	se beed, rated beed limit erformance Data er ameter diameter h diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0	405.8 423.8 355.1 87.64 278.2	- - - 58.66 63.63 81.72	- - 7.26 - -	- 490 25.69 15.65 38.67	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam	se beed, rated beed limit erformance Data er ameter diameter n diameter eter	: - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 421.4	405.8 423.8 355.1 87.64 278.2 270.8	- - 58.66 63.63 81.72 81.16	- - 7.26 - - - 6.93	- 490 25.69 15.65 38.67 35.50	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam 120% rated flow, rate	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter ed diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 421.4 540.0	405.8 423.8 355.1 87.64 278.2 270.8 204.9	- 58.66 63.63 81.72 81.16 76.55	- - 7.26 - - - 6.93 9.67	- 490 25.69 15.65 38.67 35.50 36.50	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, maximum Rated flow, maximum BEP flow, rated diam 120% rated flow, rated	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter eter eter eter ad diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 450.0 421.4 540.0 590.9	405.8 423.8 355.1 87.64 278.2 270.8 204.9 171.4	- 58.66 63.63 81.72 81.16 76.55 72.05	- - 7.26 - - - 6.93 9.67 11.86	- 490 25.69 15.65 38.67 35.50 36.50 35.48	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam 120% rated flow, rated End of curve, rated d End of curve, minimum	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter eter ed diameter iameter im diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 421.4 540.0 590.9 407.0	405.8 423.8 355.1 87.64 278.2 270.8 204.9 171.4 118.1	- 58.66 63.63 81.72 81.16 76.55 72.05 67.40	- - 7.26 - - - 6.93 9.67 11.86 8.81	- 490 25.69 15.65 38.67 35.50 36.50 35.48 18.00	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam 120% rated flow, rated End of curve, rated d End of curve, minimum End of curve, maximum	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter et diameter iameter iameter iameter um diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 450.0 421.4 540.0 590.9	405.8 423.8 355.1 87.64 278.2 270.8 204.9 171.4 118.1 179.0	- 58.66 63.63 81.72 81.16 76.55 72.05 67.40 72.67	- - 7.26 - - - 6.93 9.67 11.86	- 490 25.69 15.65 38.67 35.50 36.50 35.48 18.00 38.39	
Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam 120% rated flow, rated End of curve, rated d End of curve, minimu End of curve, maximum Maximum value, rate	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter ed diameter iameter im diameter um diameter diameter diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 421.4 540.0 590.9 407.0	405.8 423.8 355.1 87.64 278.2 270.8 204.9 171.4 118.1	- 58.66 63.63 81.72 81.16 76.55 72.05 67.40 72.67 81.16	- - 7.26 - - - 6.93 9.67 11.86 8.81	- 490 25.69 15.65 38.67 35.50 36.50 35.48 18.00 38.39 37.16	
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Radial load, worst ca Radial load limit Impeller peripheral sp Impeller peripheral sp Various Pe Shutoff, rated diamet Shutoff, maximum dia MCSF Rated flow, minimum Rated flow, maximum BEP flow, rated diam 120% rated flow, rated End of curve, rated d End of curve, maximum Maximum value, rate Maximum value, max	se beed, rated beed limit erformance Data er ameter diameter diameter eter eter et diameter iameter iameter um diameter um diameter diameter eter et diameter eter et diameter iameter um diameter et diameter et diameter et diameter um diameter et diameter	: - : - : - : - Flow (USgpm) 0.00 0.00 168.1 450.0 450.0 421.4 540.0 590.9 407.0 617.2 -	405.8 423.8 355.1 87.64 278.2 270.8 204.9 171.4 118.1 179.0 405.8 - @ Density	- 58.66 63.63 81.72 81.16 76.55 72.05 67.40 72.67 81.16 81.74 7, rated	- - 7.26 - - - - - - - - - - - - @ Densi	- 490 25.69 15.65 38.67 35.50 36.50 35.48 18.00 38.39 37.16 352 ty, max	
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Pump Performance - Additional Data

Pump Performance - Additional Data									
	Head and Power			Dimensions					
Friction loss rate, column : 1.73 %			Minimum clearance below suction bell lip/case						
Friction loss, column		: 0.77 ft		Minimum well diameter	: 10.00 in				
Friction loss, discharge head		: 0.53 ft		Suction nozzle centerline height	:-				
Friction loss, can/barrel		:-		Suction to first stage impeller centerline	: 3.94 in				
Friction loss, suction bell and strainer		: 0.00 ft		Bowl assembly length, first stage	: 13.94 in				
Friction loss, bowl/column adaptor		: 1.06 ft		Bowl assembly length, upper stage	: 8.50 in				
Friction loss, total		: 2.36 ft		Bowl assembly length, total	: 64.94 in				
Power loss, lineshaft bearings		: 0.37 hp		Suction bearing hub length	: 3.38 in				
Power loss, thrust bearing		: 0.22 hp		Strainer length	: 0.00 in				
Power loss, total		: 0.59 hp		Bowl to column adaptor length	: 0.00 in				
Во	owl vs. Pump Per	formance		Discharge head stick-down	: 0.00 in				
Head (bowl / pump)		: 256.7 ft	/ 254.3 ft	Submersible motor adaptor length	:-				
Efficiency (bowl / pump)		: 80.94 %	/ 78.91 %	Submersible motor length	:-				
Power (bowl / pump)		: 36.02 hp	o / 36.61 hp	Column length	: 44.30 ft				
NPSH required at first stage impeller eye :		: 7.51 ft		Total pump length	: 49.99 ft				
	NPSH			Can / barrel length	:-				
NPSH at bowl (available	/ required)	: Ample /	7.51 ft	Stuffing box sleeve diameter	: 1.69 in				
NPSH at low liquid level (available / require	ed) : Ample /	-	Suction bell diameter	: 9.44 in				
NPSH at suction flange (a	available / require	d) : - / -		Minimum submergence to prevent vortexing	: 18.34 in				
	Liquid Veloci	ties		Actual submergence (based on LLL)	: 597 in				
Column liquid velocity		: 5.22 ft/s		Discharge head height	: 27.00 in				
Can liquid velocity		:-		Discharge nozzle centerline height	: 8.50 in				
Suction nozzle liquid velo	ocity	:-		Min distance discharge nozzle centerline to	: 0.00				
Pressure Data	Maximum	Maximum	Hydrostatic	suction bell					
	casing/	allowable	test pressure	Lineshaft length	: 46.55 ft				
	bowl working pressure	working pressure	(psi.g)	Bowl shaft diameter	: 1.50 in				
	(psi.g)	(psi.g)		Bowl diameter, outside	: 9.44 in				
Bowl	175.6	455.0	263.0	Bowl diameter, exit	: 4.04 in				
Column	175.6	450.0	-	Column diameter, inside	: 6.07 in				
Discharge head	175.6	275.0	-	Column internal obstruction diameter	: 1.25 in				
Can/Barrel	-	-	-	Can/barrel diameter, inside	:-				
	Torque Limi	its		Can/barrel obstruction diameter					
Torque, lineshaft limit		: 7.50 hp/	100 rpm						
			•	Down Thrust					
Weight, lineshaft			5	: 202.5 lb					
Weight, bowl assembly ro	otating element			: 84.00 lb					
Thrust factor	0			: 5.60 lb/ft					
Thrust, hydraulic, rated (@ Density, rated	/ @ Density, max	x)		: 1,434.8 / 1,434.8 lbf				
Thrust, hydraulic, shutoff			,		: 2,268.1 / 2,268.1 lbf				
Thrust, hydraulic, end of				: 958.1 / 958.1 lbf					
Thrust, hydraulic, MCSF	· ·			: 1,985.0 / 1,985.0 lbf					
Thrust, bowl shaft end (@				: 0.00 / 0.00 lbf					
Thrust, shaft step (@ Su				: 49.63 / 49.63 lbf					
Thrust, stuffing box sleev	•			: -111.50 / -111.50 lbf					
Thrust, stuffing box sleev				: -178.09 / -178.09 lbf					
Thrust, stuffing box sleev			: -73.26 / -73.26 lbf						
Thrust, stuffing box sleev				: -155.67 / -155.67 lbf					
Thrust, total, rated (@ Si		•		: 1,659.4 / 1,659.4 lbf					
Thrust, total, shutoff (@					: 2,426.2 / 2,426.2 lbf				
Thrust, total, end of curve				: 1,221.0 / 1,221.0 lbf					
Thrust, total, MCSF (@ S				: 1,221.0 / 1,221.0 lbf : 2,165.5 / 2,165.5 lbf					
Momentary upthrust (@					: 2,165.5 / 2,165.5 lbf : 497.8 / 497.8 lbf				
Thrust bearing capacity	cuotion pressure,		· <u>-</u> · · · · · · · · · · · · · · · · · · ·	. 497.0 / 497.0 IUI					
index bearing capacity			 * Negative values indic	- * Negative values indicate upthrust					
				-					



Pump Performance - Additional Data Conditions Pump Type : Well Low Liquid Level : 0.00 ft Head Measured at : Discharge Nozzle Centerline Column Liquid Velocity Limit : 15.00 ft/s Pressure Strategy : Maximum Pressure, Rated Well Diameter : 0.00 in Diameter (shut off) Suction Elevation : 0.00 in NPSHA Measured at : Base of Discharge Head **Discharge Location** : Above Ground Length Strategy : Pump Length Hydrotest Pressure Strategy : 1.5 x Bowl Working Length : 50.00 ft Pressure Construction : US Standards Pumpshaft Material : PSM-416A Pump Construction Standards Discharge Head Design : JTLF 150# Pumpshaft Diameter : 1.50 in / 38.1 mm : Mechanical Seal Discharge Nozzle Size : 6 inch Shaft Seal Type Column Diameter : 6 inch Bell Design : Trimmed Bell **Column Construction** : Threaded Thrust Bearing : No Thrust Bearing Suction Strainer : None Seal Size : Single seal, 1.5 inch : Open Lineshaft Seal Chamber Pressure Estimate : 100 to 150 psi Lineshaft Lubrication Lineshaft Material : LSM-416A Liquid Viscosity : Water (any viscosity) Lineshaft Diameter : 1.25 inch

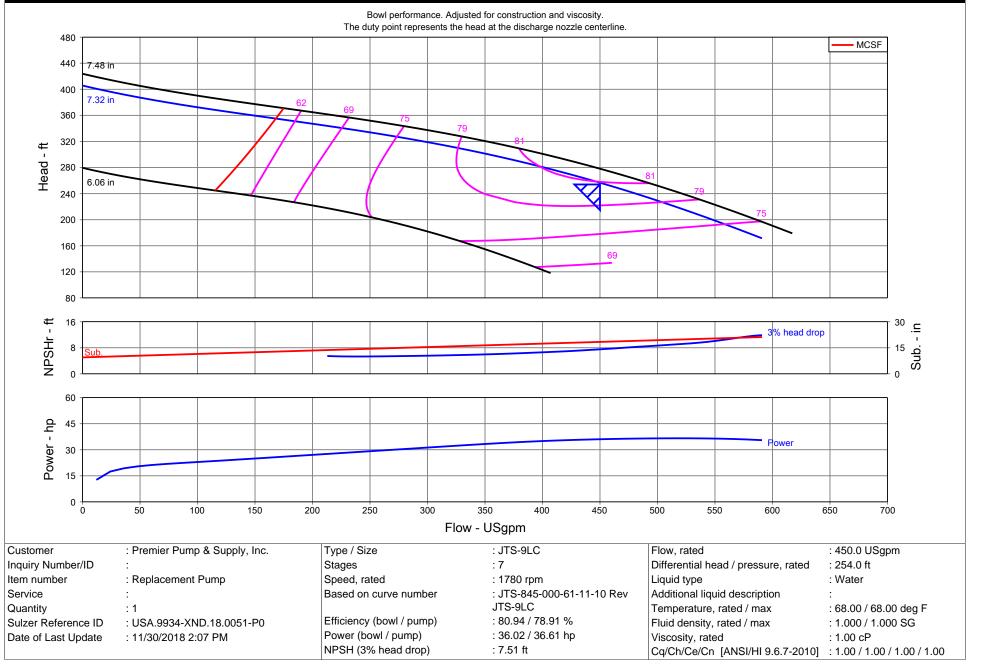


Centrifugal Pump Data Sheet

Customer:	Premier Pump & Supply, Inc.	Sulzer Reference ID:		USA.9934-XND.18.0	051-P0	
Project name:	UE - Greenland, NH	Inquiry Date:		11/30/2018		
Inquiry Number/ID:		Bid Submitted Date:		1,00,2010		
Item number:	Replacement Pump	Date of Last Update:		11/30/2018 2:01 PM		
Service:	Replacement rump	Quantity:		1		
	Conditions		Pump De	sign Data		
Liquid type:	Water	Pump Type:	r amp Bo	Well pump		
Temperature, Rated / Max:	68.00 deg F / 68.00 deg F	Product Line:		JTS		
Fluid density, rated / max:	1.000 SG / 1.000 SG	Pump Size / No. of Sta	ages:	JTS-9LC / 7		
Vapor pressure, rated:	0.34 psi.a	Rotation (viewed from	0	Counter-clockwise		
Viscosity, rated:	1.00 cP	Impeller type:	,	Single		
Consistency:	N/A	Casing mounting:		N/A		
Air Content:	N/A	Casing split:		N/A		
Discharge Flow, Rated:	450.0 USgpm	Casing Type:		N/A		
Differential Head, Rated / Actual:	254.0 ft / 257.4 ft	Nozzle	Size	Rating	Face	Position
Suction pressure, rated / max:	0.00 psi.g / 0.00 psi.g	Suction	-	N/A	N/A	N/A
NPSH Available:	Ample	Discharge	6.00 in	150#	RF	Above
	rmance	Lineshaft Diameter:		1.25 in		
Performance Curve No.(s):	-	Bearing Type, Radial:		N/A		
Pump speed:	1780 rpm	Lineshaft Bearing / Bo		Bronze [B584 Alloy	C873001	
Frequency:	60 Hz	pumps only): Bearing Type, Thrust:		None		
Fixed / Variable Speed:	Constant Speed	Bearing lubrication:		Product lubricated		
Impeller diameter, rated:	7.32 in	Baseplate type:		N/A		
Impeller diameter, maximum:	7.48 in	Daseplate type.	Mate	erials		
Impeller diameter, minimum:	6.06 in	API Material Class:	Wett	Cast Iron [A48 CL. 3	01 w/ Coat	ing
Efficiency:	80.94% / 78.91%	Barrel / Can:		N/A	oj w/ 0001	ing
NPSH (3% head drop):	7.51 ft / 0.00 ft	Case / Bowls:			01 w/ Coat	ing
Ns / Nss:	45 Metric: nq (imp. eye flow) / 163	Case / Bowls: Cast Iron [A48 CL. 30] w/ Coating Column: Carbon Steel [A36 and A53 Gr. B		-		
	Metric: nq (imp. eye flow)			Fab-150# Carbon St		
Head, maximum, rated diameter:	405.8 ft	Discharge Head:		B]		
Head rise to shutoff:	0.00 ft	Impeller:		410 Stainless Steel		-
Flow, best eff. point:	421.4 USgpm	Case / Impeller Wear	Rings:	Ni. Al. Bronze [B148	-	-
Diameter ratio (rated / max):	97.89 %	Shaft:		12% Chrome [A582	Type 416	Cond. A]
Head ratio (rated dia / max dia):	92.26 %	Diffusers:		N/A		
Viscous Coefficients (CQ / CH / CE):	1/1/1	Shaft	Sealing, Flush 8	Cooling Piping R	Plans	
Press	ure Data	Seal Size / Type:		1.25 in. / Cartridge		
Maximum Working Pressure:	175.6 psi.g	Seal Code:		5610		
Working Pressure Limit:	455.0 psi.g	Seal Manufacturer:		John Crane		
Suction Pressure Limit:	0.00 psi.g	Seal Flush Piping, Pri	mary:	N/A		
Hydrostatic Test Pressure (Suction/Discharge):	263.0 psi.g	Seal Flush Piping, See	condary:	-		
Suction pressure, rated / max:	0.00 psi.g / 0.00 psi.g	Cooling Water Piping:				
Discharge pressure, rated:	109.9 psi.g		Driver & F	ower Data		
Differential Pressure, Rated:	111.1 psi	Driver Size:		40 HP / 1780 RPM		
Equipment Weig	hts (Approximate)	Volts/ Phase / Hz:		460 V / 3 / 60 Hz		
Pump:	1,889.2 lb	Service factor:		1.15		
Driver:	690.0 lb	Power, rated: 27.302362				
Baseplate:	N/A	Power, maximum, rated diameter: 27.709307				
Total Package	2,579.2 lb	Enclosure:				
	Ac	cessories				
Driver:	- 40 HP / 1780 RPM,					
Coupling:	Threaded Coupling					
	Cc	omments				
-						

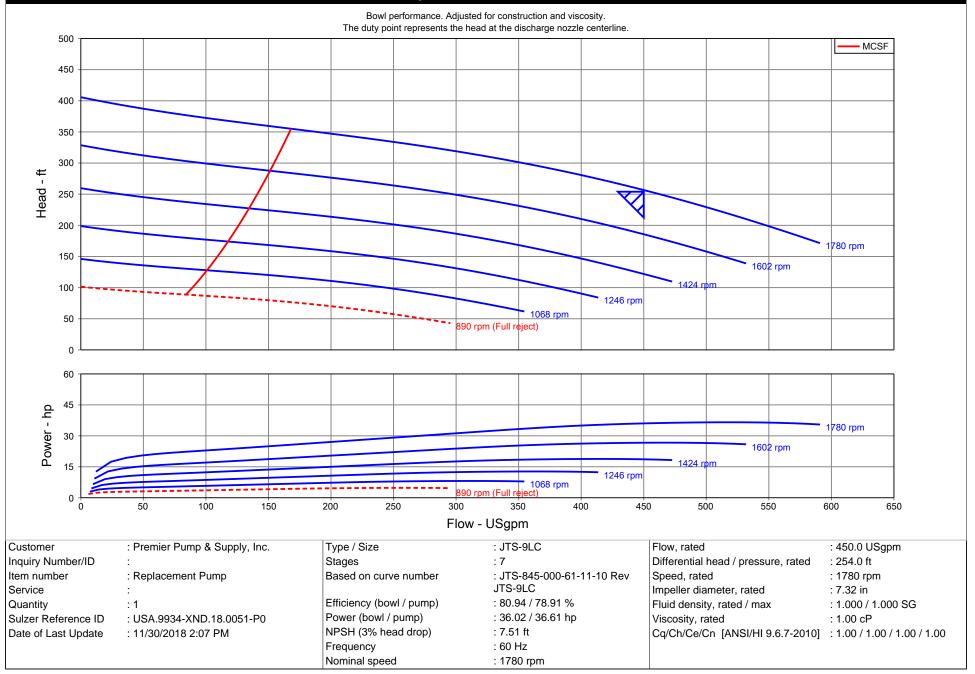
SULZER

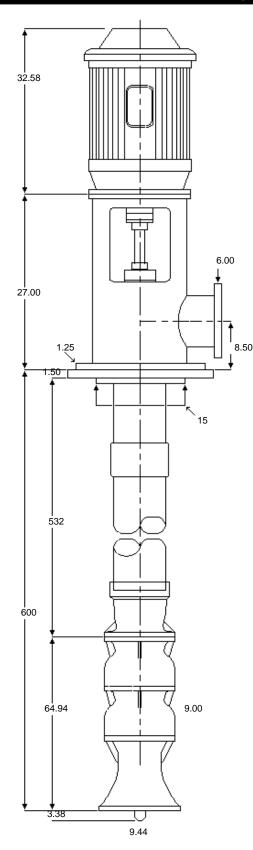
Pump Performance Curve

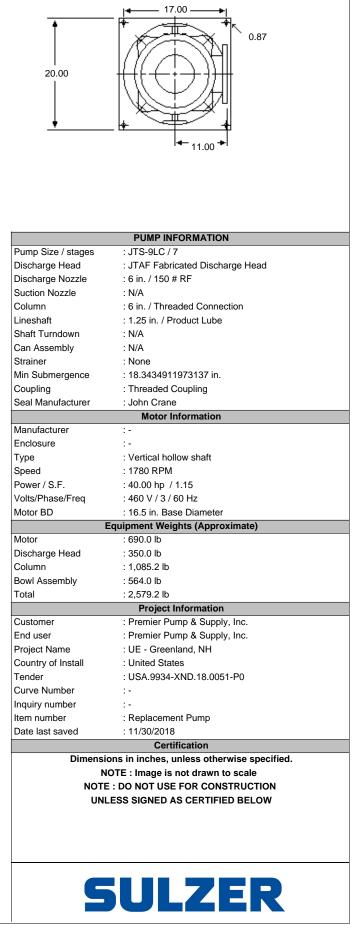


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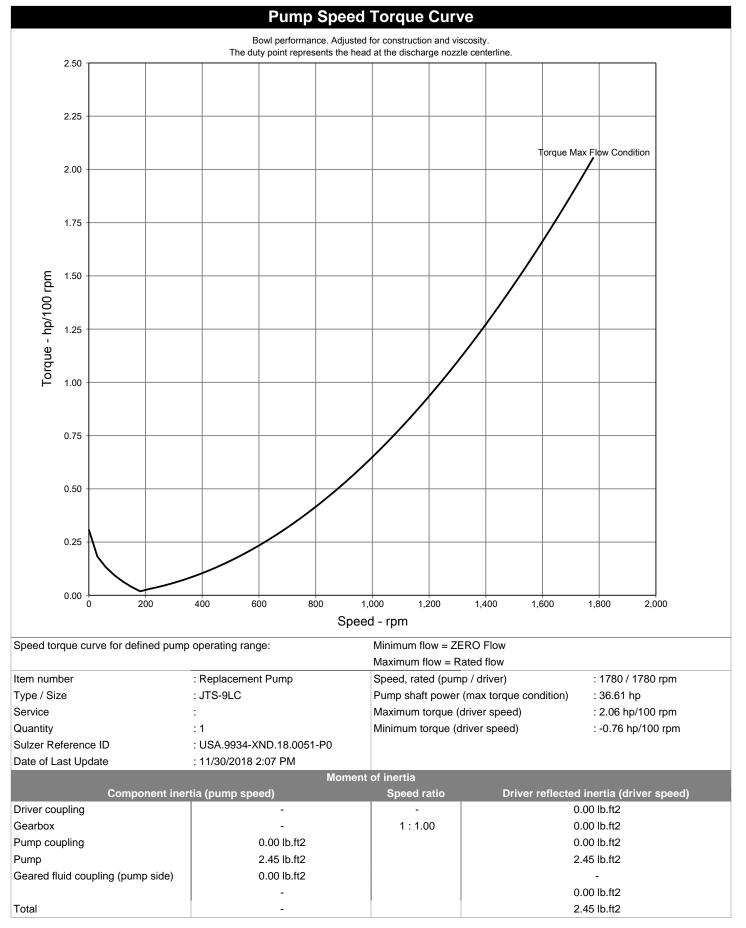
Multi-Speed Performance Curve













	Pump Speed Torque Table of Valu	les
	Table of values	
SPEED	Torque Max Flow	Torque Min Flow
rpm	hp/100 rpm	hp/100 rpm
0	0.31	-0.11
181	0.02	-0.01
362	0.09	-0.03
543	0.19	-0.07
724	0.34	-0.13
905	0.53	-0.20
1086	0.77	-0.28
1267	1.04	-0.38
1448	1.36	-0.50
1629	1.72	-0.63
1780	2.06	-0.76

GE Power Conversion

Product Information Packet

May 6, 2017

Data shown is for the current revision model #. Ensure your nameplate model # matches.

Model Number:	5KS324DAJ6021B		
Catalog Number:	V4513		
Instruction Manual:	GEK-95353		
Connection Diagram:	GEM2034E-FIG9		
Outline Drawing:	148CB32VMHKBCLA0001		

Accessory Connection Diagrams					
Bearing Thermocouple:	None	Heater:	3027JE-1C		
RTD:	None	Thermistor:	None		
Thermostat:	None	Winding Thermocouple:	None		
Bearing RTD:	None				

Table of Contents	
Specification	01
Performance Characteristics	02
Outline Drawing	03
Connection Drawing(s)	04
Spare parts	05



Marks:

MODEL NUMBER:	5KS324DAJ6021B	Estimated Weight:	690 Lbs
Outline Drawing:	148CB32VMHKBCLA0001	Time Rating:	CONT
Connection Diagram:	GEM2034E-FIG9	Enclosure:	WPI
Instruction Book:	GEK-95353	Encl Construction:	OPEN
Design Code:	32BD1241AA	Ambient Max(°C):	40
Туре:	KS	Alt Ambient Max(°C):	
Frame:	L324TP16	Insulation Class:	Н
Phases:	3	NEMA Design:	В
Poles:	4	Nominal Efficiency:	94.1 %
Output Power:	40HP 29.6KW	Guaranteed Efficiency:	93.0
RPM:	1780	3/4 Load Efficiency:	
Voltage:	230/460	KVA Code:	G
Hertz:	60	Max KVAR:	15.7
Amps - FL:	101.2/50.6	Power Factor:	79.0
Service Factor:	1.15	Bearing - DE:	6212C3
Alt Service Factor:		Bearing - ODE:	235A2523AD01

Enclosure is Weather Protected One

Stamped Nameplate Notes:

HTR LDS HE1-HE2 115V 100W ROT CCW FACING ODE LEAD/PH SEQ 1-2-3/1-2-3 INVERTER DUTY PER NEMA MG1 PART 31 ALTERNATE RATING FOR PWM CONTROL:1.0SF 40C AMBIENT VAR TORQUE RANGE 5-60 HZ UPPER BRG LUBE OIL: 2.1 QTS 0 DEG C TO 40 DEG C : ISO 32(MINERAL OR SYNTHETIC) -15 DEG C TO 0 DEG C : ISO 32 SYNTHETIC SUITABLE FOR 30 HP, 190/380V, 50 HZ WITH 92.0/46.0 AMPS AND 1480 RPM AT 1.0 SF

Additional Information:

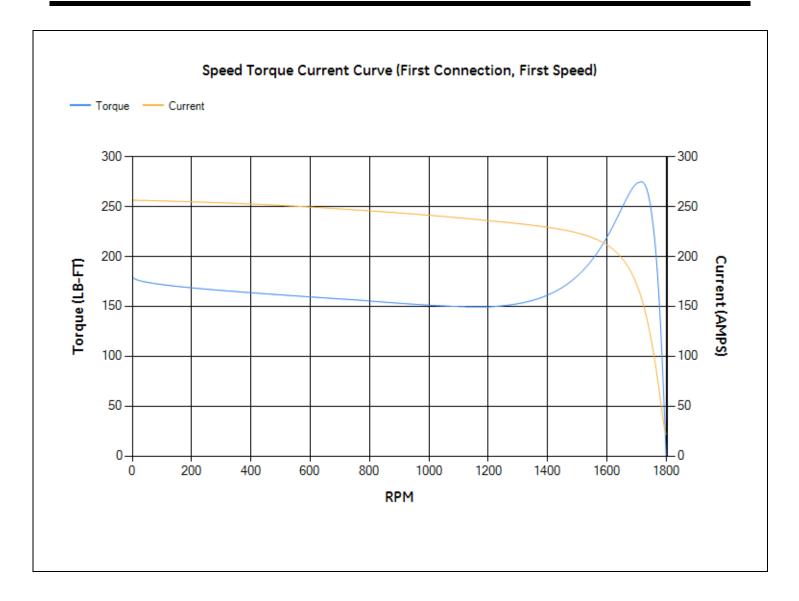
4P, VERT HOLLOW SHAFT HIGH THRUST (1D) SPECIAL BALANCE 346 CU IN - 3.00" NPT OIL RESISTANT SLEEVING ON LEADS 115V HTR LDS TO MAIN CONDUIT BOX COUPLING NOT INCLUDED IN BOM, WILL BE ORDERED SEPERATELY GROUND PAD SHAFT GROUNDING RING MOUNTED ON LOWER END BRG CAP BEARING LIFE 8760 HRS AT 6324 LB THRUST CG:13.86 IN FROM P-BASE FACE,STAT DEF:0.0025 IN RCF:3600 CPM AT C/BOX SIDE, 3870 CPM AT 90 DEG FROM C/ BOX SIDE NON-REVERSE BALL CARRIER

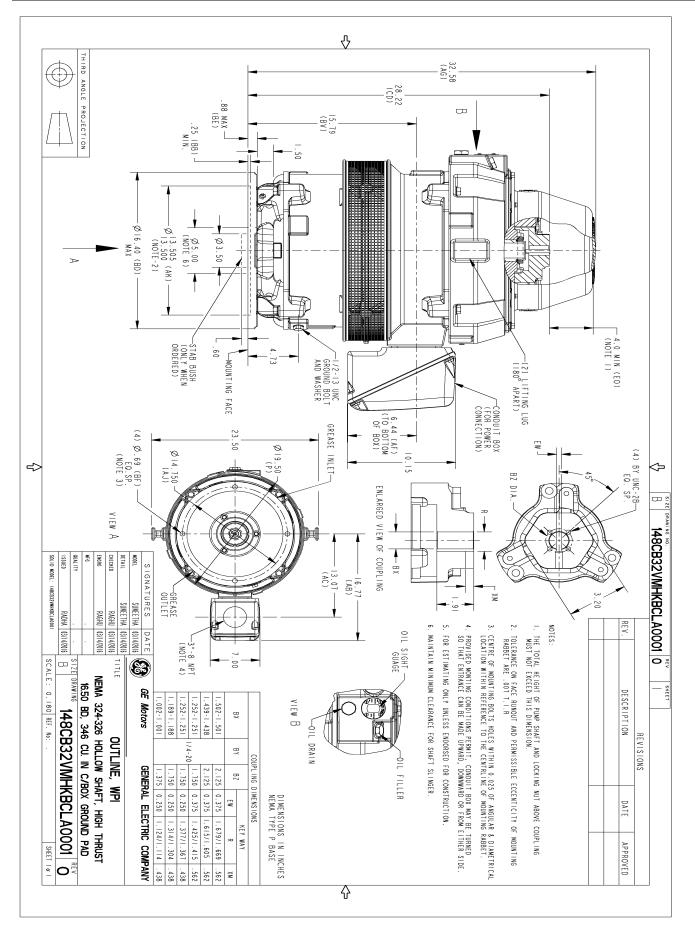


Model Number:5KS324DAJ6021B							May 6, 2017
Performance (Marks:	Characteristics		1st Winding 1st Connection				: 32BD1241AA
LOAD %	125.0	115.0	100.0	75.0	50.0	25.0	0.0
% EFF	93.13	93.41	93.93	93.92	93.29	89.79	0.00
% PF	81.26	80.69	79.23	74.3	63.68	41.7	4.06
AMPS	61.84	57.12	50.19	40.24	31.51	25	21.92
TORQ(FL)#FT AMPS(LR)	118.03 256.37		(LR)%FL START	152.25 0.33	TORG	Q(BD)%FL	232

This motor is capable of two cold or one hot start with a maximum connected load inertia of 1146 Lb-Ft Sq (48.25 Kgmeter Sq)at 100% voltage, where the load torque varies with the square of the speed. Acceleration time with maximum inertia and the above load type is 51 seconds. Safe stall time at 100% voltage is 87 seconds cold, 62 seconds hot. Rotor inertia is 6.38 Lb-Ft Sq (0.27 Kg-meter Sq).

Open Circuit A-C:	0.497	Short Circuit D-C:	0.023
Short Circuit A-C:	0.035	X/R Ratio:	8.686
Stator Slots:	48	Rotor Slots:	38





Marks:

	<u>Connection Diagram</u> GEM2034E-FIG9					
		DUA	AL VOL	TAGE]	
	NNECTION TAGE RA		2 <u>0</u> / <u>0</u> /2		GEM2034E-F1G9	
	T3	Т9 Т6		T 4	12	
	VOLTS	LI	L2	L3	TOGETHER	
2۵	LOW	TI-T6 T7	T2-T4 T8	T3-T5 T9		
۱۵	HIGH	ΤI	Τ2	Τ3	T4-T7,T5-T8, T6-T9	

Heater Connection 3027JE-1C

HEATER CONNECTION



SU	JPPLY VOLTAGE LEADS	L1	L2
HE	ATER LEADS	HE1	HE 5



	End shield Assembly					
Part Description	DE Side Part#	ODE Side Part#				
End Shield	115E7205AA1	115E7208LA1				
Bearing	235A2509BE01	235A2523AD01				
Slinger/Inproseal	235A2300FM1					

	Fan & Fan Cover Assembly
Part Description	Part#
Fan	
Fan Cover	161C1052AA1

Conduit & Accessories Box Assembly		
Part Description	Part#	
Conduit Box	149C4429AA2	

Mechanical Accessories			
Part Description	Part#		
Brake			
Tachometer			



APPENDIX F

BOOSTER PUMP STATION INFORMATION



THE MAHER CORP.

SyncroFlo Elite Proposal System

Project Name: Breakfast Hill Water Main Extension Project SyncroFlo Proposal Number: 18153 Proposal Date: December 4, 2018





DECEMBER 4, 2018

Maher Corp. 192 Pleasant Street Rockland, MA 02370

Dear Mr. Sussman,

We want to thank you for considering us for the supply of the pumps/system and look forward to supporting you on the Breakfast Hill Water Main Extension Project. Please let us know if you have any questions.

Warmest Regards,

Chris

Christopher Lott Application Engineer 832-389-1136 <u>clott@syncroflo.com</u>

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1. **PROPOSAL SECTIONS**

- 1.1. Pricing Summary and Scope
- 1.2. ISO Certificate
- 1.3. Field Service Rate Sheet
- 1.4. Standard Terms and Conditions

2. TECHNICAL PROPOSAL

- 2.1. Pump Documentation
- 2.2. Accessories Documentation
- 2.3. Example General Arrangement Drawing
- 2.4. Brochures



Breakfast Hill Water Main Extension Project

SECTION 1 COMMERCIAL PROPOSAL



Breakfast Hill Water Main Extension Project

SECTION 1.1 PRICING SUMMARY AND SCOPE



2905 Pacific Drive Norcross, GA 30071 Tel: (770) 447-4443 www.syncroflo.com



YOUR SINGLE-SOURCE PACKAGED PUMPING SYSTEMS MANUFACTURER

TO: Maher Corp. ATTN: Paul Sussman DATE: December 4, 2018 QUOTE NUMBER: 18153

QUOTED BY: C. Lott PROJECT: Breakfast Hill Water Main Ext. LOCATION: Portsmouth, NH ENGINEER: Underwood Engineers

Schedule

Submittals:	3-4 weeks after acceptance and acknowledgement of purchase order. Include this proposal as part of the purchase order for this system. One copy in pdf is offered for representative and contractor. Contractor may print as many hard copies as he requires from the pdf.
Resubmittals:	If required, resubmittals will be provided within 4 weeks after review of submittal comments, and after all questions generated by the engineer's submittal comments are received, clarified, and accepted by SyncroFlo.
Production:	Process begins on receipt of approved submittals and release to production (release for production consists of a signed and approved or accepted submittal, signed general arrangement drawing and written release from the purchaser).
Shipment:	6 weeks after receipt of all equipment necessary for manufacturing (estimated at 16 weeks from receipt of approved submittals and release to production). Shipment date may change, depending on component availability from key vendors.
Delivery:	Delivery date may be up to 10 days after shipment date, depending on carrier and distance from the factory in Norcross, GA 30071.

Scope of Equipment

Factory built pumping system. Principal components are as listed below. Station shall be UL Listed as a complete unit under UL and ETL categories. Control panel shall be UL 508 listed.

System is quoted per specification 2145 BPS Basis of Design and no addenda. SyncroFlo is **ISO 9001:2015** certified, certificate number 109240Q.



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System Particulars

- System Model Number: SFIMC2-2P500-FMES74
- System Design Flow Rate (GPM):
- System Rated Suction Pressure (PSI):
- System Rated Boost Pressure (PSI):
- Power to Be Provided (Voltage/Phase/Hertz): 460/3/60
- System Approximate Dry Weight (lbs.): 4,700
- System Approximate Footprint: 6' x 6'

Mechanical

• Qty (2) Peerless F11030A M Frame Mounted End Suction Pumps with mechanical seals, each rated at 500 GPM @ 89 ft including station losses.

500

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- Qty (2) 20 hp 1800 RPM TEFC premium efficient motors.
- Qty (2) 6" pump suction 150 PSI AWWA gates valves, and flex connectors.
- Qty (2) 6" pump discharge flex connectors, check valves and 150 PSI AWWA gates valves.
- Qty (1) 8" system inlet connection flange.
- Qty (1) 8" steel suction manifold, lined with fusion bonded epoxy.
- Qty (1) set of steel branch piping, lined with fusion bonded epoxy.
- Qty (1) 8" steel discharge manifold, lined with fusion bonded epoxy.
- Qty (1) 1/2" sample tap.
- Qty (1) 3/4" hose bibb with vacuum breaker.
- Qty (2) 1" air release valves.
- Qty (1) 8" system outlet connection flange.
- Qty (2) 4.5" glycerin filled pressure gauges with isolation valves, sub-panel mounted.
- Qty (4) Skid anchor clips and stainless steel anchor bolts.
- Station skid, suitable for grouting on site.
- System to be blasted to SSPC-SP6.
- System to be primed and painted with an ISO 121944 C3 coating system, comprised of:
 - One coat of PPG Amerlock 2 series primer, 4.0-8.0 mils DFT
 - Two coats of PPG's Amercoat 450H Acrylic Aliphatic Polyurethane, 2.0-5.0 mils DFT per coat

Controls:

• Qty (1) UL listed NEMA 12 rated dead front control panel with key lockable handle to house all logic and motor control for the pump system, including:



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- Qty (1) Secondary surge & lightning arrestor.
- Qty (1) Main disconnect circuit breaker, 25kAIC rated, with through outer door operator.
- Qty (1) Power monitor.
- Qty (1) Phase monitor.
- Qty (2) Motor circuit breakers, 25kAIC rated, with through inner door operator.
- Qty (2) Full solid state soft starters, with NEMA sized isolation contactor and bypass starter.
- Qty (1) 110 volt control disconnect circuit breaker (from 120/208/3/60 power), 25kAIC rated, with through inner door operator.
- Qty (1) 15A GFCI duplex receptacle mounted adjacent to control panel
- Qty (1) Uninterruptable Power Supply to support PLC, OIT, pressure transmitters, and RTU interface devices in the event of power failure (either utility or generator)
- Qty (1) Set of 30.5 mm operator controls including:
 - Local/Remote switch for selecting either RTU or local control installed on the inner panel door
 - Pump H-O-A switches installed on the inner panel door
 - Alarm reset & silence push button installed on the outer panel door
 - Pump run lights Green, Push to Test installed on the inner panel door
 - Alarm light Red, Push to Test installed on the inner panel door
 - Control power light White, Push to Test installed on the inner panel door
 - Alarm horn rated NEMA 1 will be installed on the outer panel door.
- Qty (2) run time meters for pumps.
- Qty (1) 24VDC power supply.
- Qty (2) Pressure transmitters, one on suction header and one on discharge header.
- Qty (1) Allen Bradley MicroLogix 1400 programmable controller with successive and 24 hour pump alternation, sequence shifting, power failure restart sequence, pressure sequencing with staging delay, alarm systems, etc.
- Qty (1) Maple HMI5070NL color touch screen OIT (800 x 480 pixels). System can be used to access registers within the Microprocessor from the front of the control panel.
- Qty (1) Set of station OIT displayed items:
 - Selection of lead pump or selection of automatic alternation
 - Current suction pressure
 - Current discharge pressure
 - Pump run times
 - Pumping system diagnostics
 - Low inlet pressure alarm display



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- Low discharge pressure alarm display
- High discharge pressure alarm display
- Phase failure alarm display
- Pump failure alarm display
- Alarm beacon, NEMA 4X rated (shipped loose for mounting on building exterior)
- Audible alarm, NEMA 4X rated (shipped loose for mounting on building exterior)
- Audible alarm with silence button on the control panel outer door
- Panel intruder alarm on the outer door
- Qty (1) Terminal strip in control panel dedicated to SCADA interface.
- Schedule 40 PVC conduit with non-metallic flexible conduit as required.

Spare Parts:

- Qty (1) Spare pump and motor
- Qty (1) Set of control panel replacement fuses
- Qty (1) Set of pilot light replacement lamps

SCADA Interface:

- Qty (2) auxiliary analog Ethernet signals including:
 - Suction Header Pressure (AO)
 - Discharge Header Pressure (AO)
- Qty (10) auxiliary Ethernet contacts including:
 - Each Pump Call (DI x 2)
 - Each Pump Running (DO x 2)
 - Each Pump Fault (DO x 2) (as determined by overload relay trip)
 - Each H-O-A in Hand (DO x 2)
 - Each H-O-A in Auto (DO x 2)
 - Each System Alarm (DO x 5):
 - Low inlet pressure
 - Low discharge pressure
 - High discharge pressure
 - Loss of power
 - Phase Failure

Building:



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- Steel self-framed building with Gable Roof, mounted on pump skid.
- 115 MPH rated building and 30 PSF rated roof.
- 4" walls, insulated to R-21.
- Qty (1) Double doors.
- Qty (1) Building power supply disconnect, 22.5 kVA transformer, load center and circuit breakers.
- Qty (1) Exhaust fan and intake louvers.
- Qty (1) Air conditioner and heater, 2 ton and 5 kW.
- Qty (4) 48" dual fluorescent fixtures.
- Qty (2) External lights at doors.
- Qty (2) Internal emergency lights
- Qty (3) GFCI Receptacles general purpose, interior
- Qty (2) GFCI Receptacles general purpose, exterior, weather protected
- Qty (1) Smoke alarm
- Qty (1) Door Switch for Intruder Alarm
- Qty (1) Dehumidifier with dedicated GFCI receptacle
- Exterior Brick Façade.
- All skid mounted with pumping system.
- Rigid conduit to be run under the skid deck to each motor and sensor

Post Production:

- Full factory functional and performance testing with X-Y plot.
- Freight to site is not included.
- Installation is not included.
- Start-up, station calibration & operator training is included, 1 trip 1 days allotted.
- Warranty is as listed below.

Notes:

- Pumps will be primed to provide sufficient NPSHA. Suction lift has not been considered.
- Controls: No harmonic testing is included. Field Harmonic analysis to be provided by others. No external operating devices are included.
- No actuating devices outside the control panel are included.



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Package Price

1) Base Bid: Please see reference email.

Commercial Items

- 1) Submittals: All drawings are generated in AutoCAD. Unless otherwise stated the submittals will be electronically transmitted and no printed copies will be supplied. The following components make up the standard submittal:
 - a. General arrangement drawings
 - b. Component & equipment cut sheets
 - c. Complete electrical diagrams
 - d. Enclosure assembly drawings
 - e. Controls diagram
 - f. Panel assembly drawings
 - g. IO&M provided 6 weeks after shipment
- 2) Shipment: 16 weeks after full approval and full "Release for Fabrication" (release for fabrication consists of a signed submittal, signed general arrangement drawing and written release from the customer) and full receipt of customer supplied equipment.
- 3) Lead times may vary based on plant capacity and component availability at the time of release.
- 4) Prices quoted are firm for thirty (90) days.
- 5) Prices do not include any: Federal, state, local or use taxes.
- 6) SyncroFlo standard terms and conditions apply.
 - a. Payment terms NET 30 days.
 - b. Progress payments are required on all orders over \$100,000.00 per the schedule below:
 - i. 20% Due with purchase order
 - ii. 20% Due with drawing approval
 - iii. 30% Upon receipt of major equipment
 - iv. 30% Due net 30 after shipment
- 7) Rigging and off loading at the site is not included and will be coordinated with the contractor at the time of shipment.
- 8) Standard Warranty: 1 Year parts warranty on complete pumping station one year from startup or 18 months from the date of shipment. A non-standard warranty has been offered.
- 9) As a company, we are always conscious of cost, and we work hard to minimize how market raw material price adjustments impact our customers. Recent governmental changes however (March 2018), regarding import tariffs on raw steel and aluminum, have created a certain level of uncertainty in the market, and as a company, we are already seeing price rises



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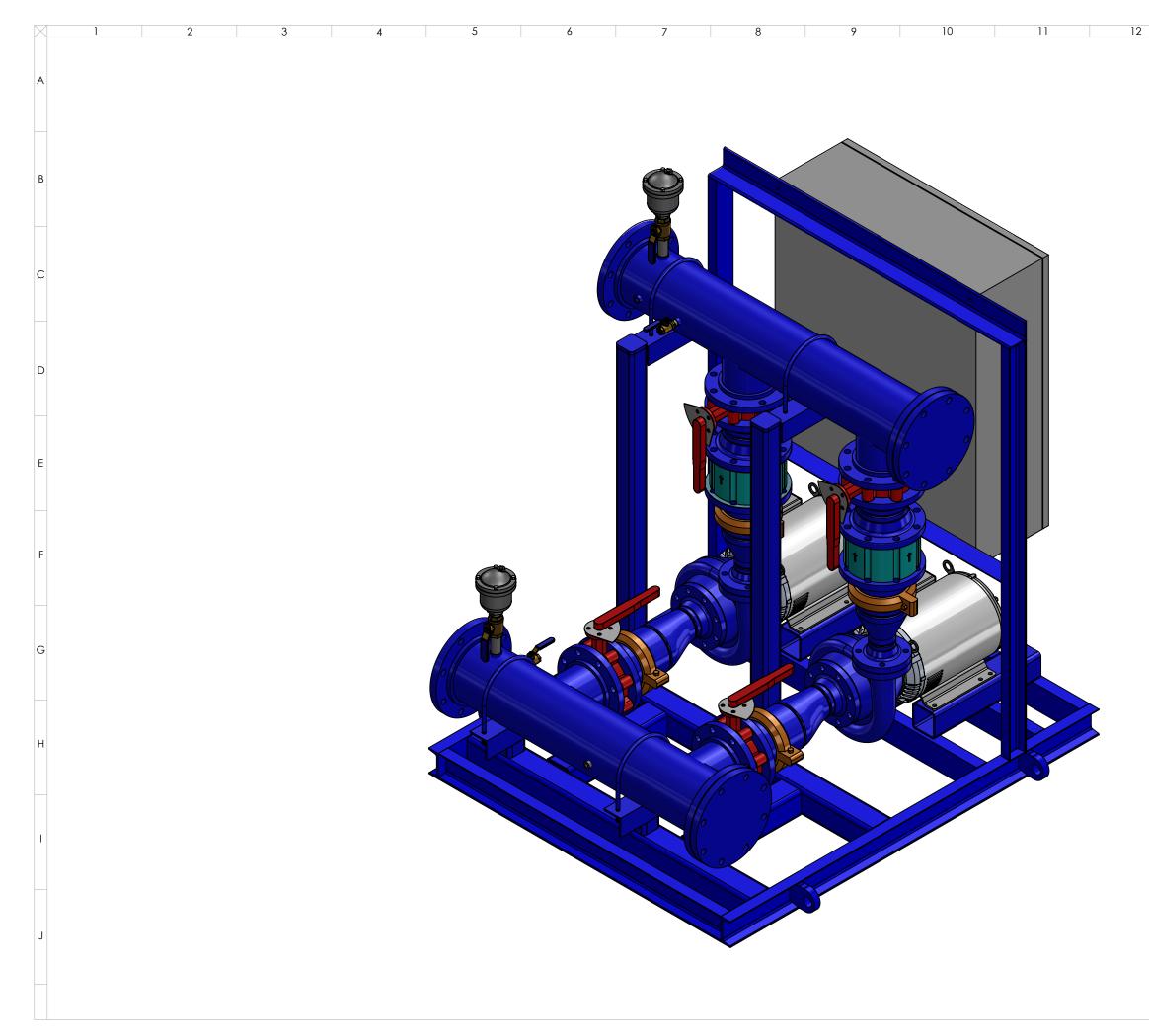
for some key components / raw material. Due to this current uncertainty, we as a company must advise, that we reserve the right to amend our package prices, if raw material costs escalate beyond typical yearly increase levels. We apologize for any inconvenience this causes, and assure you that we will do everything possible to minimize this impact.

Submitted by: Chris Lott Application Engineer <u>clott@syncroflo.com</u> <u>www.syncroflo.com</u>

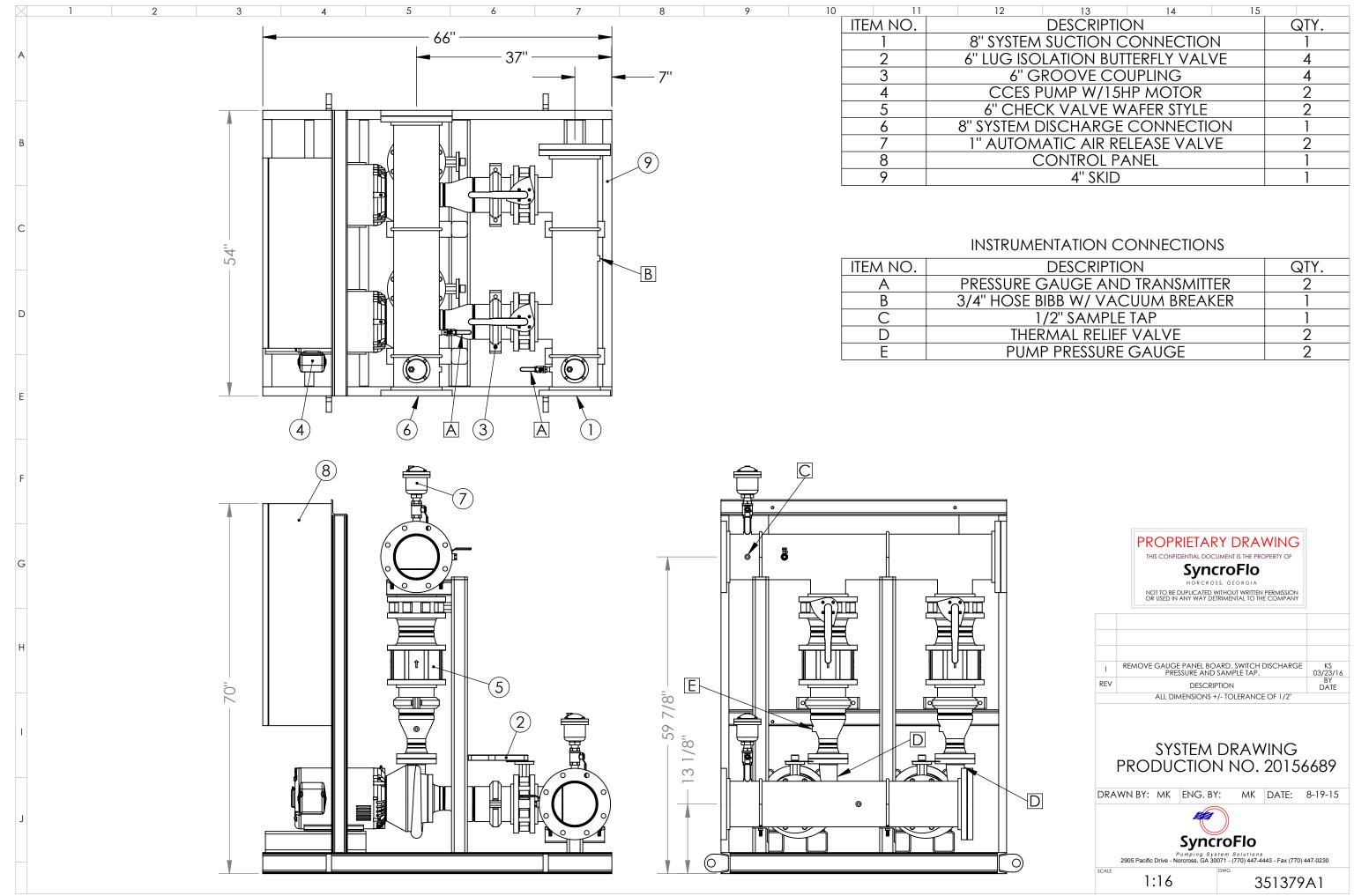


Breakfast Hill Water Main Extension Project

SECTION 2.3 EXAMPLE GENERAL ARRANGEMENT DRAWING



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			icroFlo	
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		Syn	croFlo	
	SCAL	2905 Pacific Drive - Norcross, GA	System Solutions 30071 - (770) 447-4443 - Fax (77 DWG	'0) 447-0230
	SCAL	<u>.</u>	35137	9M1



12	13	14	15	
DESCRIPTION				QTY.
SYSTEM	SUCTION C	:ONNECTIOI	N	1
JUG ISO	LATION BUT	TERFLY VALV	∕E	4
6" G	ROOVE CO	UPLING		4
CCES P	UMP W/15H	IP MOTOR		2
6'' CHEC	CK VALVE W	AFER STYLE		2
ystem d	ISCHARGE	CONNECTIO	NC	1
AUTOM	ATIC AIR RE	LEASE VALV	Έ	2
С	ONTROL PA	ANEL		1
	4" SKID			1

STRUMENTATION CONNECTIONS	
DESCRIPTION	QTY.
SSURE GAUGE AND TRANSMITTER	2
HOSE BIBB W/ VACUUM BREAKER	1
1/2'' SAMPLE TAP	1
THERMAL RELIEF VALVE	2
PUMP PRESSURE GAUGE	2









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TOTAL COMMITMENT

Since 1962, our commitment to excellence has enabled SyncroFlo to become the world's leading manufacturer of pumping stations. The SyncroFlo name has become synonymous with quality-built, cost-effective pumping systems. We pioneered pre-assembled pumping systems decades ago. Today, our installed base includes some of the largest pumping systems ever built.